

CONTRACT: ID: R-3601

NOTE: SEE SHEET 2A FOR PLAN SHEET LAYOUT AT TIME OF INVESTIGATION

CONTENTS

LINE	STATION	PLAN	PROFILE
-LMED-	42+00 TO 85+80	4-7	14-15
-LLT-	85+80 TO 127+00	7-10	16-17
-LRT-	85+80 TO 130+50	7-11	18-19
-Y-	18+10 TO 41+95	4,13	20
-Y1-	12+60 TO 14+40	4	21
-Y2-	11+00 TO 24+50	4,13	22
-Y2A-	10+00 TO 12+20	4	23
-Y3-	10+23 TO 13+50	12	24
-RPLLT-	114+80 TO 127+15	10	25
-RPLRT-	115+28 TO 122+75	10	26
-RPLRT-	160+00 TO 166+50	12	27
-RPAYLT-	23+00 TO 28+09	4	28
-RPAYRT-	23+00 TO 29+44	4	28
-RPBY-	14+00 TO 20+59	4	29
-RPBYLT-	18+00 TO 20+73	4	30
-RPBYRT-	18+00 TO 21+70	4	30
-RPCY-	22+00 TO 27+19	4	31
-RPCYLT-	25+15 TO 27+21	4	32
-RPCYRT-	25+15 TO 28+78	4	32
-RPDY-	10+00 TO 27+75	4-5	33
-RPDYLT-	25+60 TO 28+22	4	34
-RPDYRT-	25+60 TO 30+93	4	34

CROSS SECTIONS

LINE	STATION	SHEET
-LMED-	44+00	35
-LMED-	54+00	36
-LMED-	80+00	36
-LLT-	90+00	37
-LLT-	100+00	37
-LLT-	115+50	38
-LLT-	116+00	38
-LLT-	116+50	38

DRAWN BY: C.P. TURNER

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

ROADWAY
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 38868.1.1 (R-3601) F.A. PROJ. NHS-0017(68)
COUNTY BRUNSWICK AND NEW HANOVER
PROJECT DESCRIPTION US 17-74-76 FROM NC 133 /SRI472
INTERCHANGE TO US 421 /NC 133 INTERCHANGE

INVENTORY

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	38868.1.1 (R-3601)	1	38
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
38868.1.1	NHS-0017(68)	P.E.	
		RAW & UTIL.	

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

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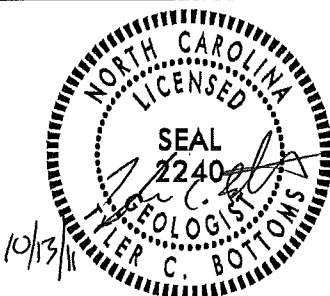
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DATE OCTOBER 2011



NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE:
VERY STIFF, GRAY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, HIGH PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)		SILT-CLAY MATERIALS (> 35% PASSING #200)		ORGANIC MATERIALS					
GROUP CLASS.	A-1	A-3	A-2	A-4	A-5	A-6	A-7	A-1, A-2	A-4, A-5	A-6, A-7
SYMBOL										
% PASSING	100	100	100	100	100	100	100	100	100	100
LIQUID LIMIT	≤ 5	≤ 10	≤ 15	≤ 20	≤ 25	≤ 30	≤ 35	≤ 40	≤ 45	≤ 50
GROUP INDEX	0	0	0	0	0	0	0	0	0	0
USUAL TYPES OF MAJOR MATERIALS	GRAVEL AND SAND	FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND	SILTY SOILS	CLAYEY SOILS	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER				
GEN. RATING AS A SUBGRADE	EXCELLENT TO GOOD			FAIR TO POOR		FAIR TO POOR		POOR		UNSATURABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TENS/F ²)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.50 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.75	2.00	0.42	0.25	0.075	0.053
BOULDER (BLDR.)						
COBBLE (COB.)						
GRAVEL (GR.)						
COARSE SAND (CSE, SD.)						
FINE SAND (F SD.)						
SILT (SL.)						
CLAY (CL.)						

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
LL - LIQUID LIMIT	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
PL - PLASTIC LIMIT	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
OM - OPTIMUM MOISTURE	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
SL - SHRINKAGE LIMIT	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

NONPLASTIC	PLASTICITY INDEX (PI)	DRY STRENGTH
LOW PLASTICITY	0-5	VERY LOW
MED. PLASTICITY	6-15	SLIGHT
HIGH PLASTICITY	16-25	MEDIUM
	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED)
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	MODERATELY COMPRESSIBLE	HIGHLY COMPRESSIBLE
LIQUID LIMIT LESS THAN 31	LIQUID LIMIT EQUAL TO 31-50	LIQUID LIMIT GREATER THAN 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY

GROUND WATER

WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
STATIC WATER LEVEL AFTER 24 HOURS
PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
SPRING OR SEEP

MISCELLANEOUS SYMBOLS

ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
SOIL SYMBOL
ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
INFERRED SOIL BOUNDARY
INFERRED ROCK LINE
ALLUVIAL SOIL BOUNDARY
DIP & DIP DIRECTION OF ROCK STRUCTURES

SPT DPT DMT VST PNT TEST BORING
AUGER BORING
CORE BORING
MONITORING WELL
PIEZOMETER INSTALLATION
SLOPE INDICATOR INSTALLATION
CONE PENETROMETER TEST
SOUNDING ROD

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)
CRYSTALLINE ROCK (CR)
NON-CRYSTALLINE ROCK (NCR)
COASTAL PLAIN SEDIMENTARY ROCK (CP)

NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH
VERY SLIGHT (V SL.)
SLIGHT (SL.)
MODERATE (MOD.)
MODERATELY SEVERE (MOD. SEV.)
SEVERE (SEV.)
VERY SEVERE (V SEV.)
COMPLETE

ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. IF TESTED, WOULD YIELD SPT REFUSAL.
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. IF TESTED, YIELDS SPT N VALUES > 100 BPF.
ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. IF TESTED, YIELDS SPT N VALUES < 100 BPF.
ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD
HARD
MODERATELY HARD
MEDIUM HARD
SOFT
VERY SOFT

CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGER NAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FEET
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	> 4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE
MODERATELY INDURATED
INDURATED
EXTREMELY INDURATED

RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
AQUIFER - A WATER BEARING FORMATION OR STRATA.
ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC.
ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.
CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.
CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.
DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.
FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.
JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.
LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.
RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.
SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
STRATA CORE RECOVERY (SCREC) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK:

ELEVATION: _____ FT.

NOTES:

APPROXIMATE LIMITS OF ORGANIC SOILS

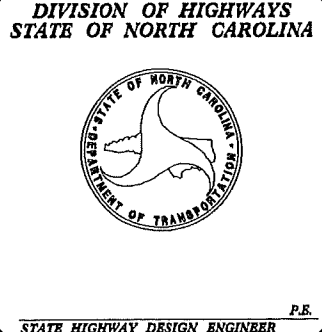
TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURES



INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

CONTRACT:





STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

BEVERLY EAVES PERDUE
GOVERNOR

EUGENE A. CONTI, JR.
SECRETARY

October 13, 2011

STATE PROJECT: 38868.1.1 (R-3601)
F.A. PROJECT: NHS-0017(68)
COUNTIES: Brunswick
DESCRIPTION: US 17-74-76 FROM NC 133/ SR 1472 Interchange to
the US 421/ NC 133 Interchange

SUBJECT: Geotechnical Inventory

Project Description

This project is located in Brunswick County on US 17-74-76 between the NC 133/ SR 1472 interchange and the US 421/ NC 133 interchange. Proposed construction consists of widening US 17-74-76, a realignment of Blackwell Road, and a diverging diamond interchange at US 17-74-76 and NC 133/ SR 1472. This geotechnical investigation was confined to the areas of proposed construction.

Fieldwork for this project was conducted during August and September of 2007 and May and June of 2011. Standard Penetration Test borings were advanced with a CME 45-B drill machine with an automatic hammer. Hand auger borings were also completed. Representative soil samples were collected for visual classification in the field and for laboratory analysis by the Materials and Tests Unit.

The following alignments were investigated. Subsurface profiles and selected cross sections of these alignments are included in this report.

<u>Line</u>	<u>Station(±)</u>
-LMED-	42+00 to 85+80
-LLT-	85+80 to 127+00
-LRT-	85+80 to 130+50
-Y-	18+10 to 41+95
-Y1-	12+60 to 14+40
-Y2-	11+00 to 24+50

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LOCATION:
CENTURY CENTER COMPLEX
ENTRANCE B-2
1020 BIRCH RIDGE DRIVE
RALEIGH NC

<u>Line</u>	<u>Station(±)</u>
-Y2A-	10+00 to 12+20
-Y3-	10+23 to 13+50
-RPLLT-	114+80 to 127+15
-RPLRT-	115+28 to 122+75
-RPLRT-	160+00 to 166+50
-RPAYLT-	23+00 to 28+09
-RPAYRT-	23+00 to 29+44
-RPBY-	14+00 to 20+59
-RPBYLT-	18+00 to 20+73
-RPBYRT-	18+00 to 21+70
-RPCY-	22+00 to 27+19
-RPCYLT-	25+15 to 27+21
-RPCYRT-	25+15 to 28+78
-RPDY-	10+00 to 27+75
-RPDYLT-	25+60 to 28+22
-RPDYRT-	25+60 to 30+93

Areas of Special Geotechnical Interest

- 1) The following sections contain cohesive soils which have the potential to cause embankment stability and/or long term settlement problems:

<u>Line</u>	<u>Station(±)</u>
-LMED-	43+75 to 44+25
-LMED-	53+75 to 54+25
-LLT-	89+30 to 90+70
-LLT-	99+30 to 100+70
-LRT-	89+25 to 90+75
-Y-	34+90 to 36+10
-Y2-	11+70 to 18+35
-Y2-	23+25 to 23+95
-Y2A-	10+00 to 10+40
-RPDY-	19+25 to 21+95

- 2) The following sections contain organic soils, which have the potential for embankment stability and/or subgrade problems during construction:

<u>Line</u>	<u>Station(±)</u>
-LLT-	99+65 to 100+70
-LLT-	113+05 to 114+10
-LLT-	115+75 to 116+25
-LRT-	99+25 to 100+70
-LRT-	111+90 to 122+60
-RPLRT-	116+85 to 121+00

- 3) The entire project was found to exhibit seasonal high ground water.

- 4) Artificial Fill was encountered along the following line and consists of sand and clay.

<u>Line</u>	<u>Station(±)</u>
-Y2-	20+60 to 24+05

Physiography and Geology

This project corridor is located within the Coastal Plain Physiographic Province. Topography along the project is nearly flat to gently sloping. Natural ground elevations ranged from 44± feet below sea level along the bed of the Brunswick River to 59± feet above sea level along the existing US 17-74-76 embankment.

Surficial soils in this area are generally classified as alluvial sediments.

Ground Water

Ground water data was collected in August and September of 2007 and May and June of 2011, during a time of normal precipitation. Ground water elevations ranged from sea level to 56± feet above sea level.

Soils

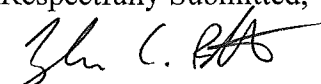
Soils within this project area have been divided into four categories: roadway embankment, artificial fill, and alluvial, and formational.

Roadway Embankment soils were encountered along the existing US 17-74-76, NC 133, and associated intersecting roads. These soils are comprised of 1± to 15 or more feet of very loose to medium dense sand (A-2-4, A-3, A-1-b).

Soils classified as artificial fill are comprised of 10 or more feet of loose to medium dense sand (A-2-4) and soft sandy clay (A-7-6). This artificial fill was encountered near the proposed interchange of -Y- and -Y2-.

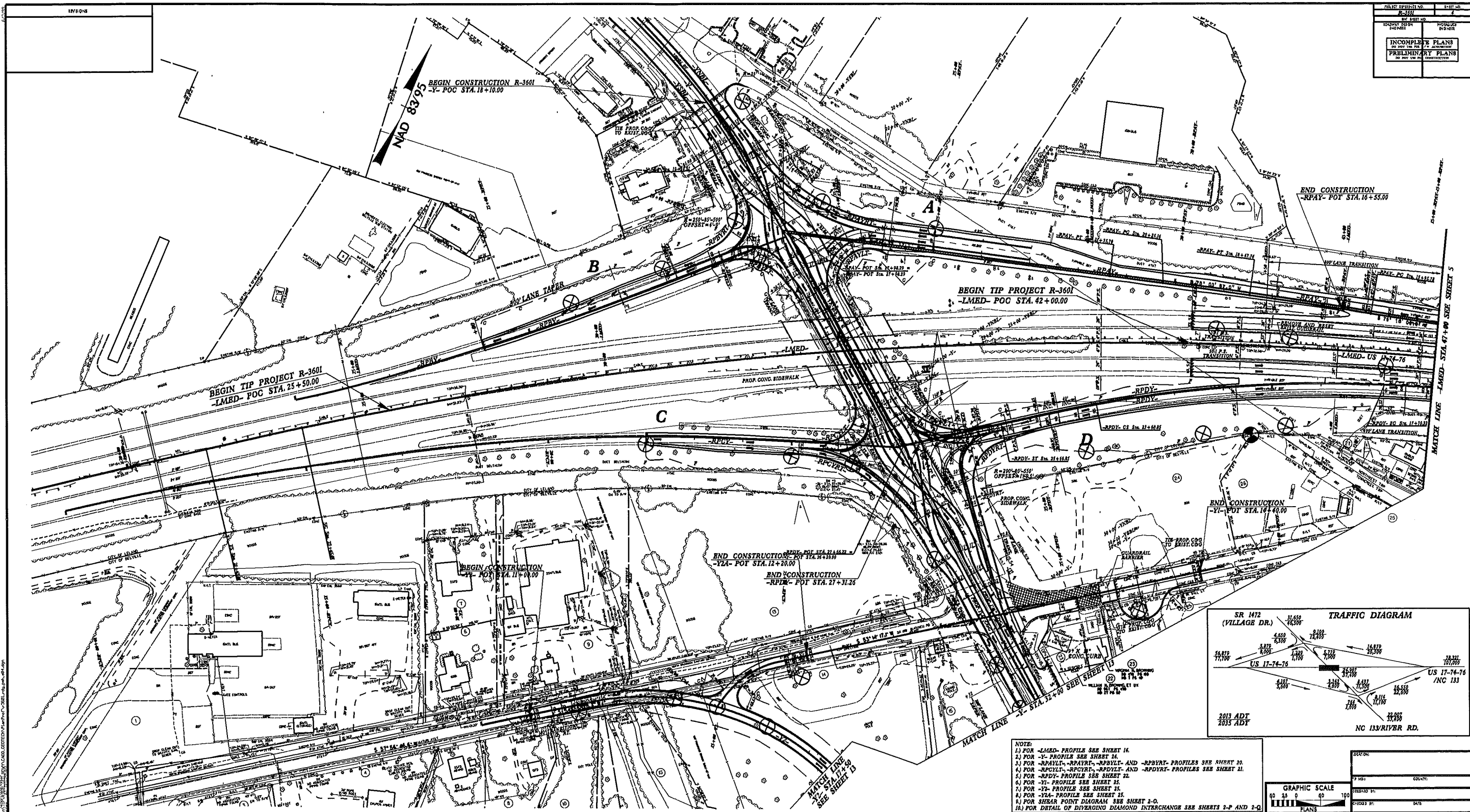
Alluvial soils were encountered beneath the embankment and within the floodplain of the Brunswick and Alligator Rivers. They are comprised of 1± to 20± feet of loose to medium dense sand (A-2-4, A-3), 1± to 3± feet of soft sandy silt and sandy clay (A-4, A-6), and 1± to 15± feet of muck, sandy and silty clay with little to moderate organic material, and sand with trace to moderate organic material.

Laboratory analysis of these soils show organic percentages ranging from 2% to 95% and moisture contents ranging from 28% to 197%.

Respectfully Submitted,

Tyler C. Bottoms, L.G.
Project Engineering Geologist

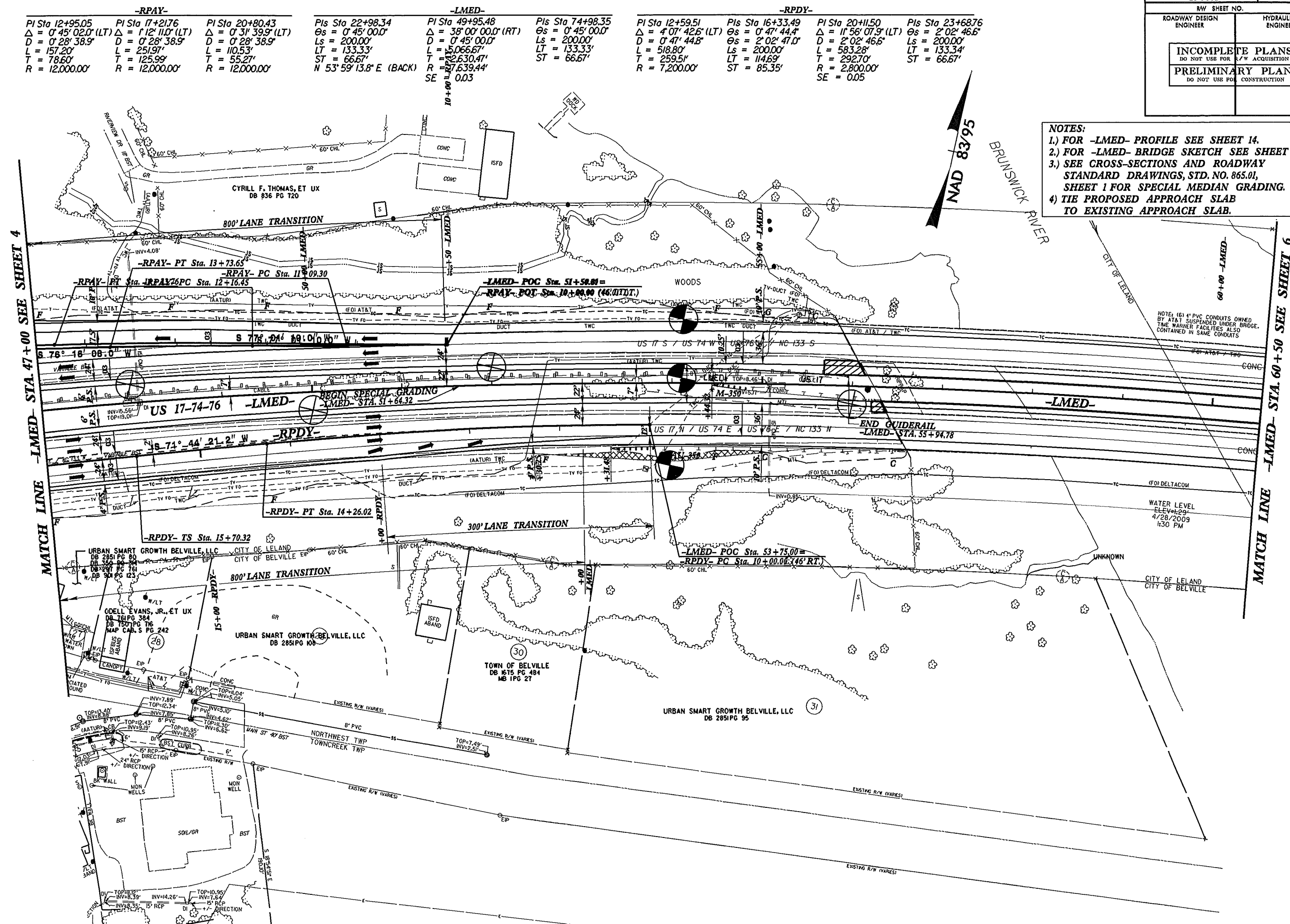
PROJECT REFERENCE NO.	SHEET NO.
R-360I	3B

EARTHWORK BALANCE SHEET



NOTES:

- 1.) FOR -LMED- PROFILE SEE SHEET 14.
- 2.) FOR -LMED- BRIDGE SKETCH SEE SHEET 2-M
- 3.) SEE CROSS-SECTIONS AND ROADWAY STANDARD DRAWINGS, STD. NO. 865.01, SHEET 1 FOR SPECIAL MEDIAN GRADING.
- 4.) TIE PROPOSED APPROACH SLAB TO EXISTING APPROACH SLAB.



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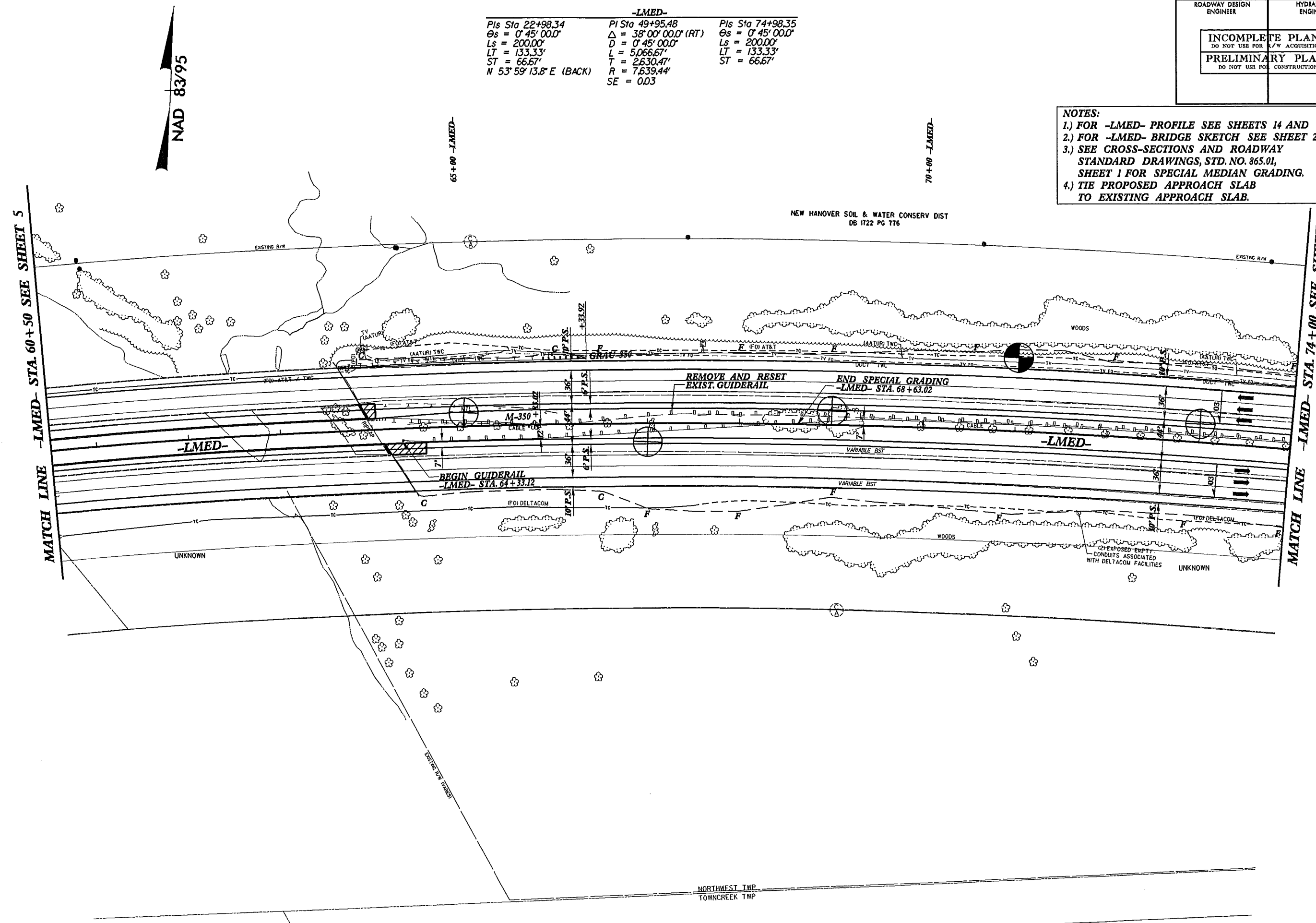
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REVISIONS

PROJECT REFERENCE NO.	SHEET NO.
R-3601	6
RDW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

-LMED-		
Pls Sta 22+98.34	Pls Sta 49+95.48	Pls Sta 74+98.35
$\Theta_s = 0^\circ 45' 00.0''$	$\Delta = 38^\circ 00' 00.0''$ (RT)	$\Theta_s = 0^\circ 45' 00.0''$
$L_s = 200.00'$	$D = 0^\circ 45' 00.0''$	$L_s = 200.00'$
$LT = 133.33'$	$L = 5,066.67'$	$LT = 133.33'$
$ST = 66.67'$	$T = 2,630.47'$	$ST = 66.67'$
$N 53^\circ 59' 13.8'' E$ (BACK)	$R = 7,639.44'$	
	$SE = 0.03$	

NOTES:
1.) FOR -LMED- PROFILE SEE SHEETS 14 AND 15.
2.) FOR -LMED- BRIDGE SKETCH SEE SHEET 2-M.
3.) SEE CROSS-SECTIONS AND ROADWAY STANDARD DRAWINGS, STD. NO. 865.01, SHEET 1 FOR SPECIAL MEDIAN GRADING.
4.) TIE PROPOSED APPROACH SLAB TO EXISTING APPROACH SLAB.



8/17/99

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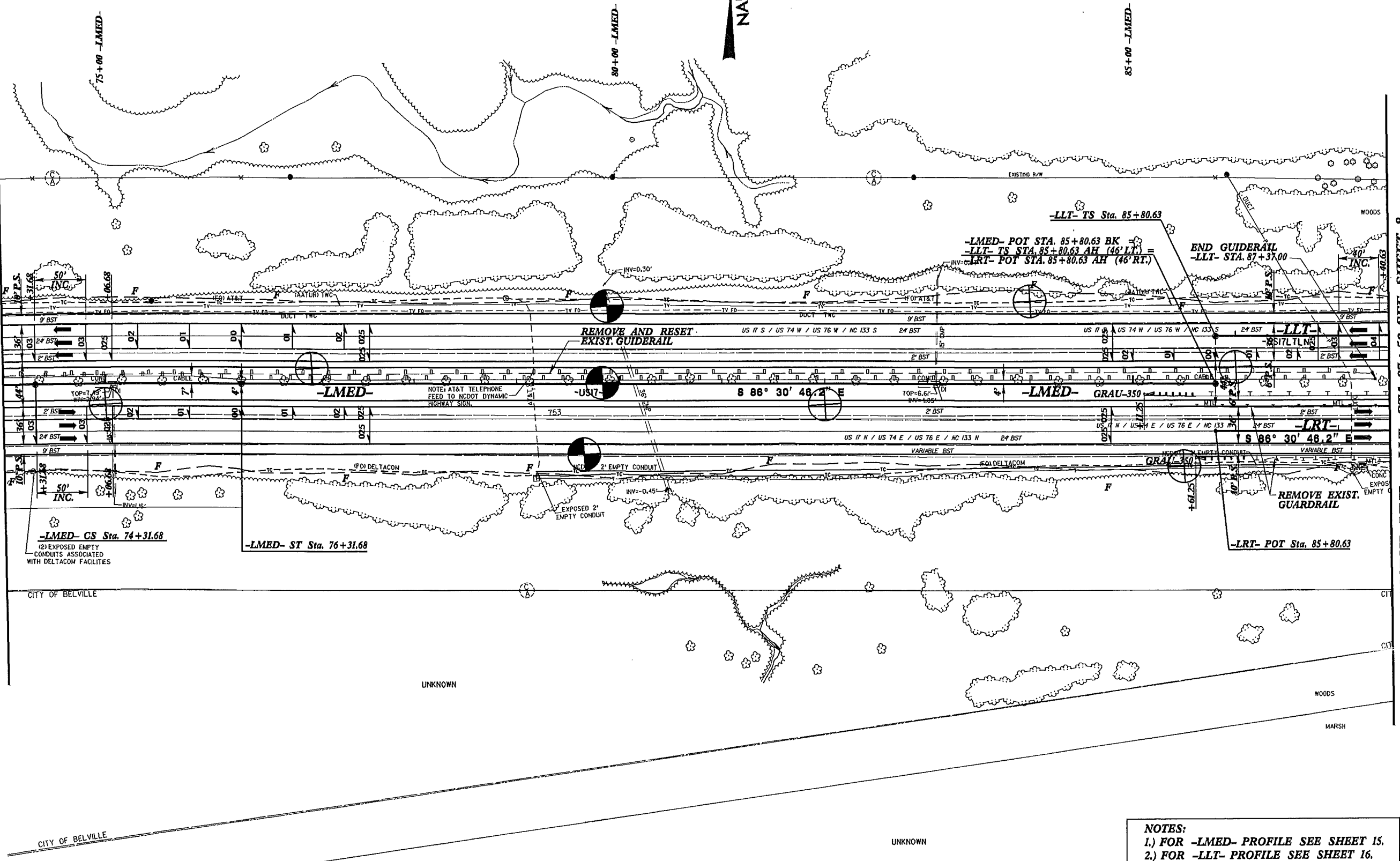
REVISIONS

PROJECT REFERENCE NO.		SHEET NO.
R-3601		7
RWY SHEET NO.		
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION		

MATCH LINE -LMED- STA. 74+00 SEE SHEET 6

-LMED-
Pls Sta 22+98.34 PI Sta 49+95.48 Pls Sta 74+98.35
 $\theta_s = 0' 45' 00.0''$ $\Delta = 38' 00' 00.0''$ (RT) $\theta_s = 0' 45' 00.0''$
 $L_s = 200.00'$ $D = 0' 45' 00.0''$ $L_s = 200.00'$
 $LT = 133.33'$ $L = 5,066.67'$ $LT = 133.33'$
 $ST = 66.67'$ $T = 2,630.47'$ $ST = 66.67'$
 $N 53' 59' 13.8'' E$ (BACK) $R = 7,639.44'$
SE = 0.03

-LLT-
Pls Sta 87+13.96 PI Sta 92+45.65 Pls Sta 97+76.19
 $\theta_s = 0' 45' 00.0''$ $\Delta = 6' 58' 00.0''$ (LT) $\theta_s = 0' 45' 00.0''$
 $L_s = 200.00'$ $D = 0' 45' 00.0''$ $L_s = 200.00'$
 $LT = 133.33'$ $L = 928.89'$ $LT = 133.33'$
 $ST = 66.67'$ $T = 465.02'$ $ST = 66.67'$
 $S 86' 30' 46.2'' E$ (BACK) $R = 7,639.44'$
SE = 0.05



MATCH LINE -LLT- STA. 87+50 SEE SHEET 8

NOTES:
1.) FOR -LMED- PROFILE SEE SHEET 15.
2.) FOR -LLT- PROFILE SEE SHEET 16.
3.) FOR -LRT- PROFILE SEE SHEET 18.

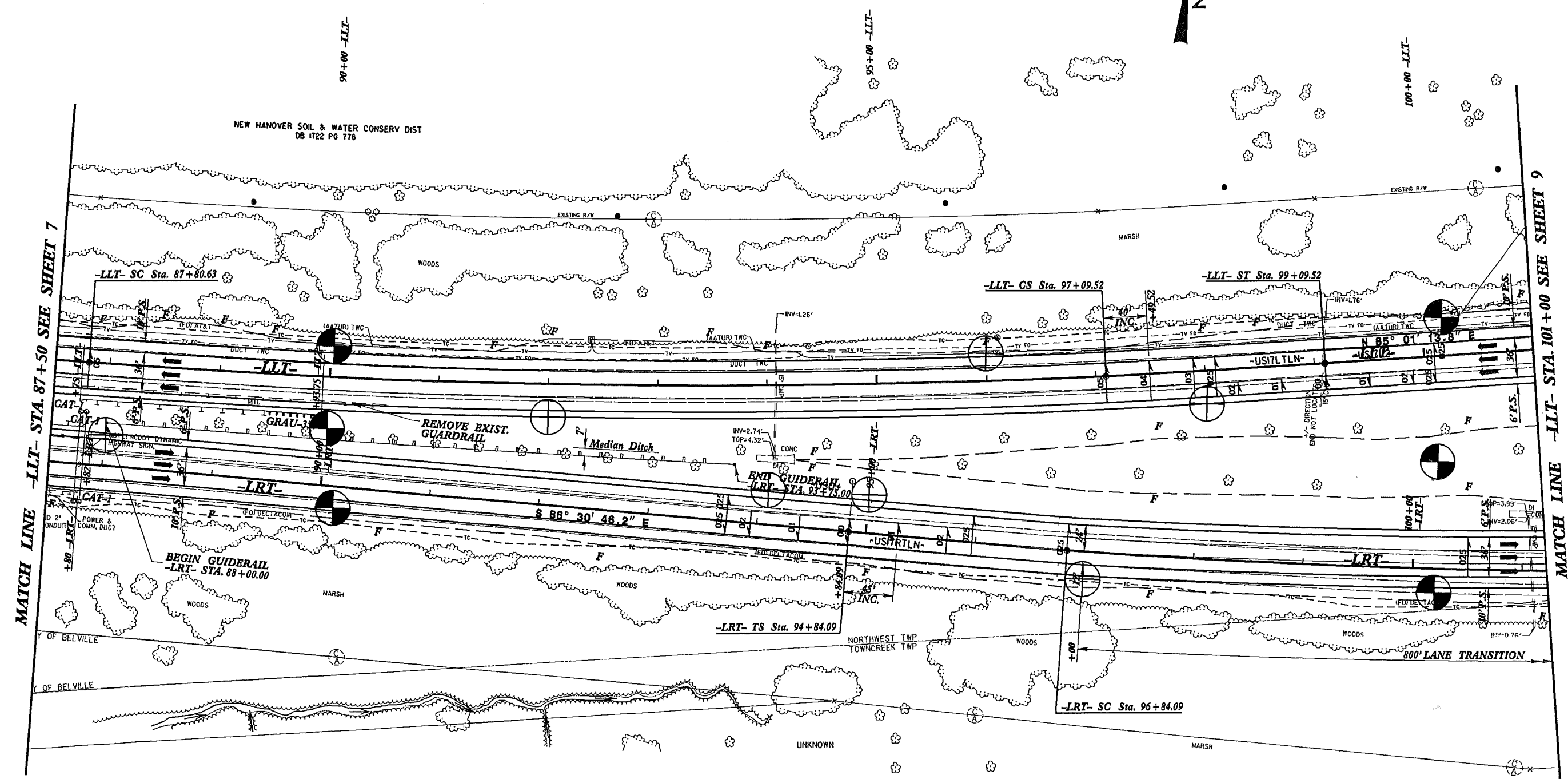
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REVISIONS

-LLT-			-LRT-		
Pls Sta 87+13.96	Pls Sta 92+45.65	Pls Sta 97+76.19	Pls Sta 96+17.42	Pls Sta 101+49.11	Pls Sta 106+79.65
$\Theta_s = 0' 45' 00.0''$	$\Delta = 6' 58' 00.0''$ (LT)	$\Theta_s = 0' 45' 00.0''$	$\Theta_s = 0' 45' 00.0''$	$\Delta = 6' 58' 00.0''$ (LT)	$\Theta_s = 0' 45' 00.0''$
Ls = 200.00'	D = 0' 45' 00.0'	Ls = 200.00'	Ls = 200.00'	D = 0' 45' 00.0'	Ls = 200.00'
LT = 133.33'	L = 928.89'	LT = 133.33'	LT = 133.33'	L = 928.89'	LT = 133.33'
ST = 66.67'	T = 465.02'	ST = 66.67'	ST = 66.67'	T = 465.02'	ST = 66.67'
S 86° 30' 46.2" E (BACK)	R = 7,639.44'	SE = 0.05		R = 7,639.44'	
				SE = 0.025	

PROJECT REFERENCE NO. R-3601	SHEET NO. 8
RAW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



NOTES:
1.) FOR -LLT- PROFILE SEE SHEET 16.
2.) FOR -LRT- PROFILE SEE SHEET 18.

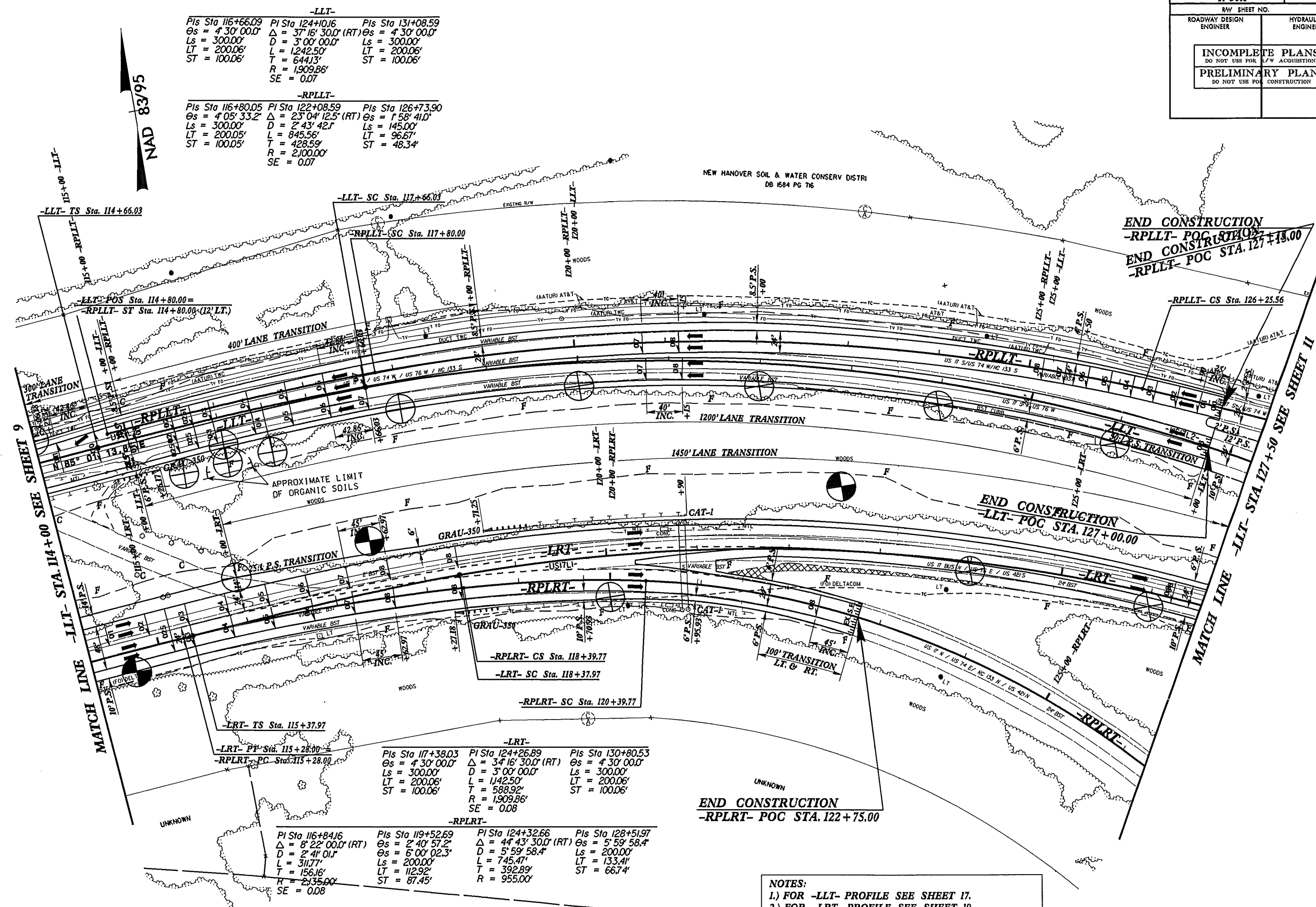


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REVISIONS

PROJECT REFERENCE NO.	SHEET NO.
R-3601	10
RDW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



-LLT-		
PI Sta 116+66.09	PI Sta 124+10.16	PI Sta 131+08.59
$\Theta_s = 4' 30'' 00.0''$	$\Delta = 37' 16'' 30.0''$ (RT)	$\Theta_s = 4' 30'' 00.0''$
$L_s = 300.00'$	$D = 3' 00'' 00.0''$	$L_s = 300.00'$
$LT = 200.06'$	$L = 1242.50'$	$LT = 200.06'$
$ST = 100.06'$	$T = 644.13'$	$ST = 100.06'$
	$R = 1909.86'$	
	$SE = 0.07$	

-RPLLT-		
PI Sta 116+80.05	PI Sta 122+08.59	PI Sta 126+73.90
$\Theta_s = 4' 05'' 33.2''$	$\Delta = 23' 04'' 12.5''$ (RT)	$\Theta_s = 1' 58'' 41.0''$
$L_s = 300.00'$	$D = 2' 43'' 42.7''$	$L_s = 145.00'$
$LT = 200.05'$	$L = 845.56'$	$LT = 96.67'$
$ST = 100.05'$	$T = 428.59'$	$ST = 48.34'$
	$R = 2,100.00'$	
	$SE = 0.07$	

-LRT-		
PI Sta 117+38.03	PI Sta 124+26.89	PI Sta 130+80.53
$\Theta_s = 4' 30'' 00.0''$	$\Delta = 34' 16'' 30.0''$ (RT)	$\Theta_s = 4' 30'' 00.0''$
$L_s = 300.00'$	$D = 3' 00'' 00.0''$	$L_s = 300.00'$
$LT = 200.06'$	$L = 1242.50'$	$LT = 200.06'$
$ST = 100.06'$	$T = 588.92'$	$ST = 100.06'$
	$R = 1909.86'$	
	$SE = 0.08$	

-RPLRT-			
PI Sta 116+84.16	PI Sta 119+52.69	PI Sta 124+32.66	PI Sta 128+51.97
$\Delta = 8' 22'' 00.0''$ (RT)	$\Theta_s = 2' 40'' 57.2''$	$\Delta = 44' 43'' 30.0''$ (RT)	$\Theta_s = 5' 59'' 58.4''$
$D = 2' 41'' 01.1''$	$\Theta_s = 6' 00'' 02.3''$	$D = 5' 59'' 58.4''$	$L_s = 200.00'$
$L = 311.77'$	$L_s = 200.00'$	$L = 745.47'$	$LT = 133.41'$
$T = 156.16'$	$LT = 112.92'$	$T = 392.89'$	$ST = 66.74'$
$R = 2,135.00'$	$ST = 87.45'$	$R = 955.00'$	
$SE = 0.08$			

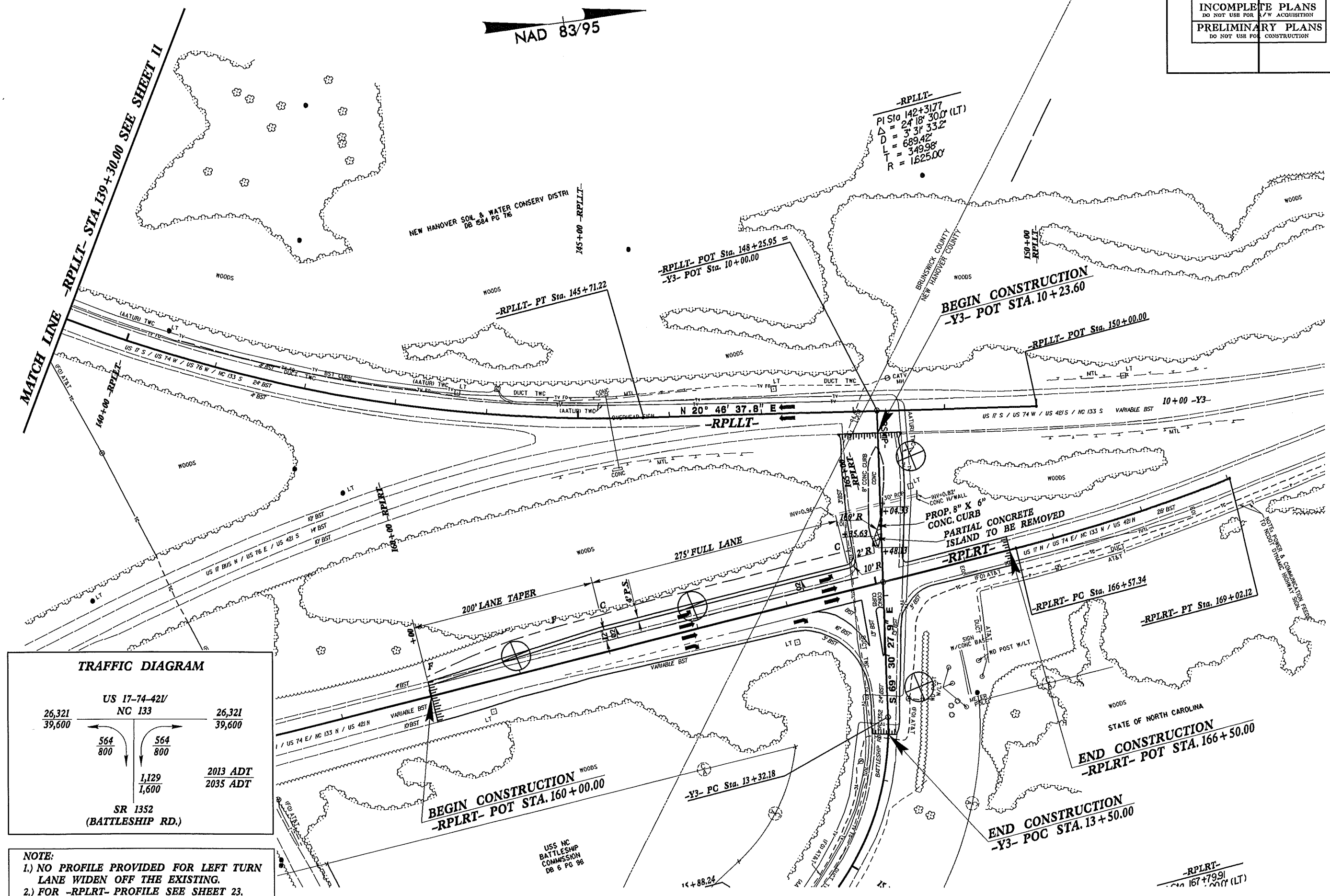
- NOTES:
- 1.) FOR -LLT- PROFILE SEE SHEET 17.
 - 2.) FOR -LRT- PROFILE SEE SHEET 19.
 - 3.) FOR -RPLRT- PROFILE SEE SHEETS 22 AND 23.
 - 4.) FOR -RPLLT- PROFILE SEE SHEET 23.

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REVISIONS

PROJECT REFERENCE NO.	SHEET NO.
R-3601	12
R/W SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

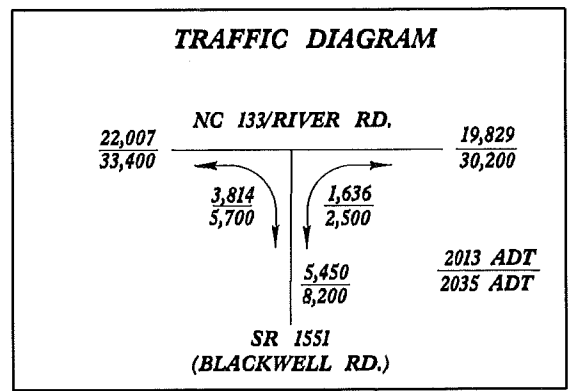
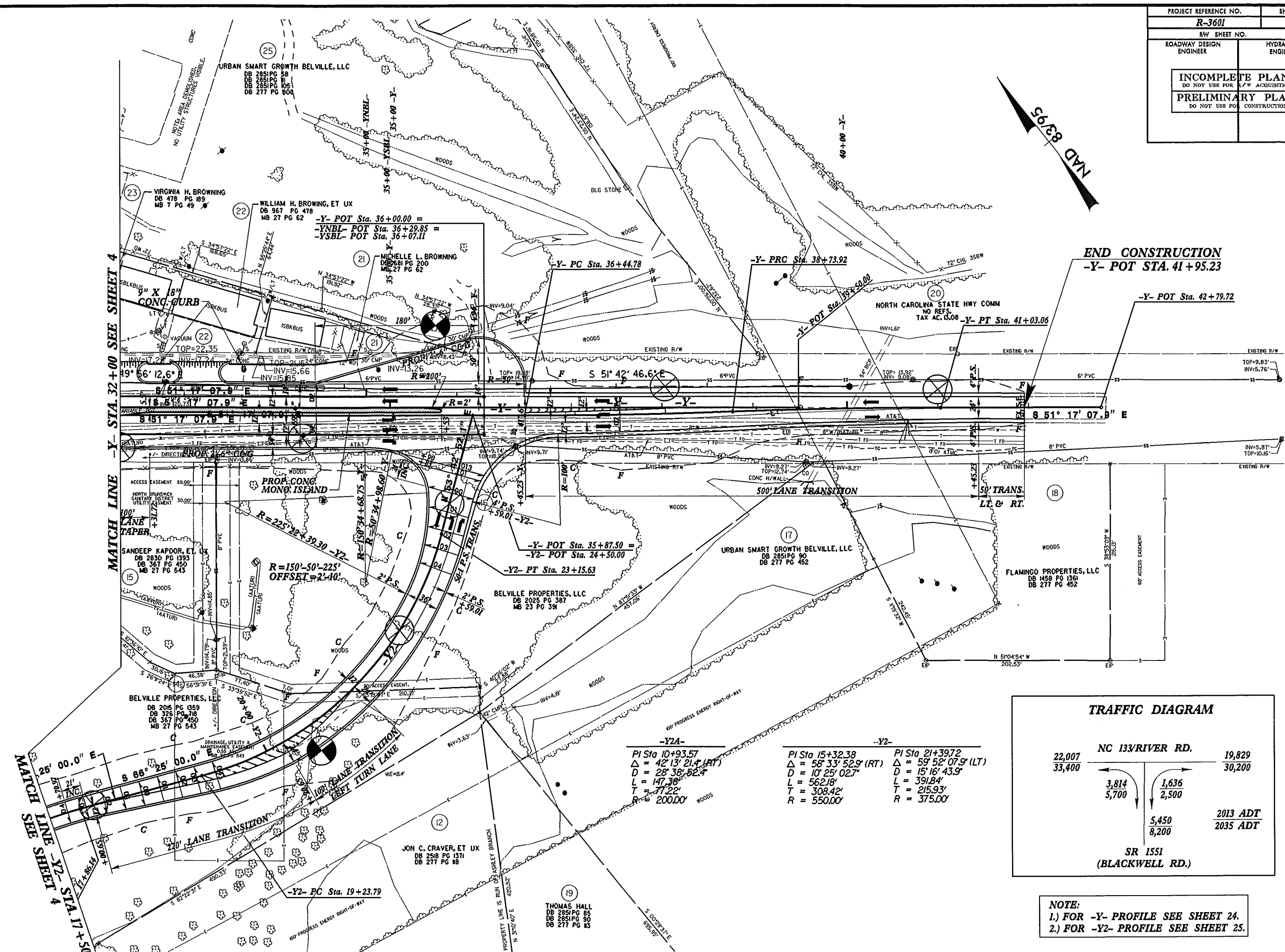


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REVISIONS

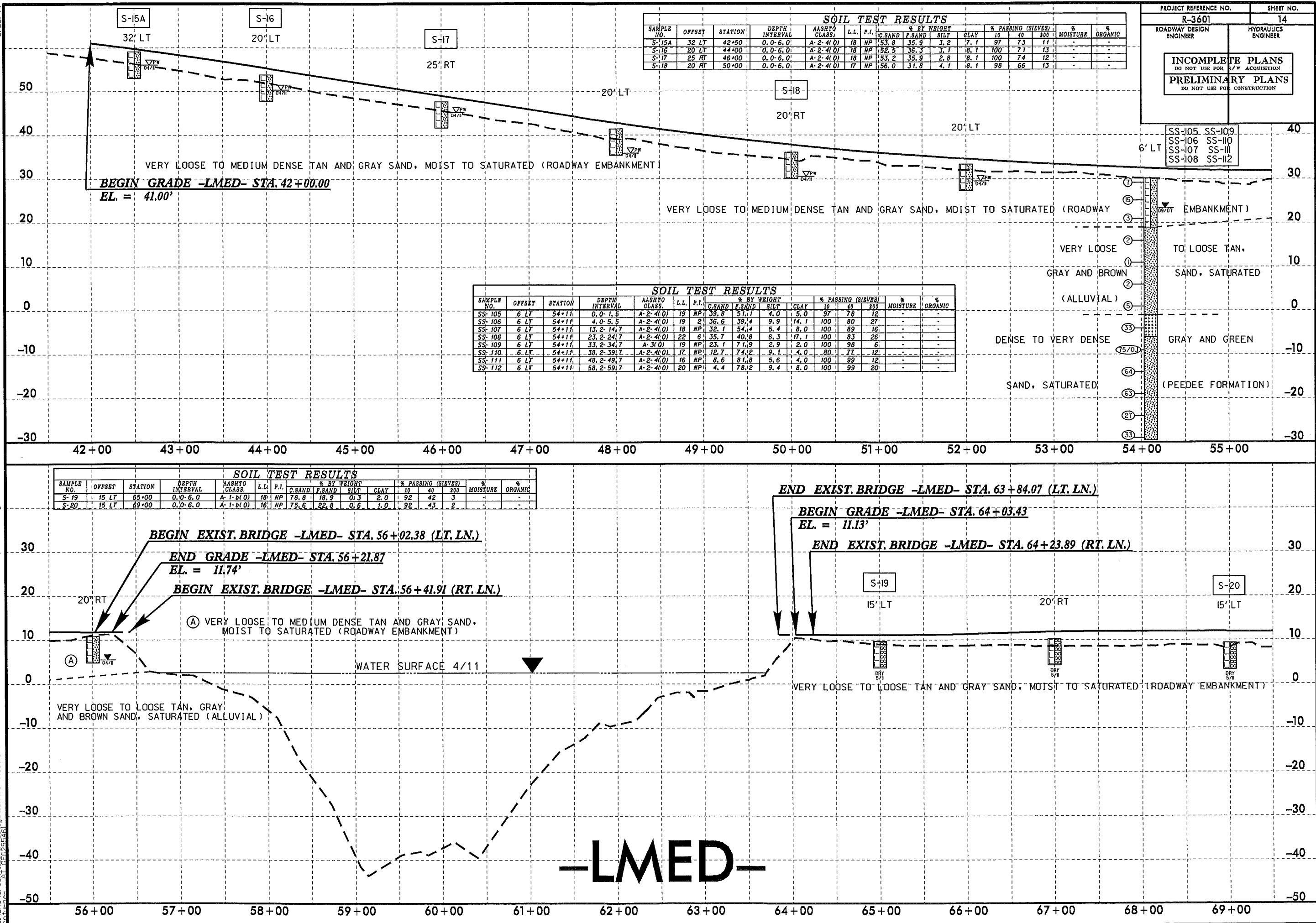
PROJECT REFERENCE NO.	SHEET NO.
R-3601	13
RDW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



NOTE:
1.) FOR -Y- PROFILE SEE SHEET 24.
2.) FOR -Y2- PROFILE SEE SHEET 25.

5/28/99

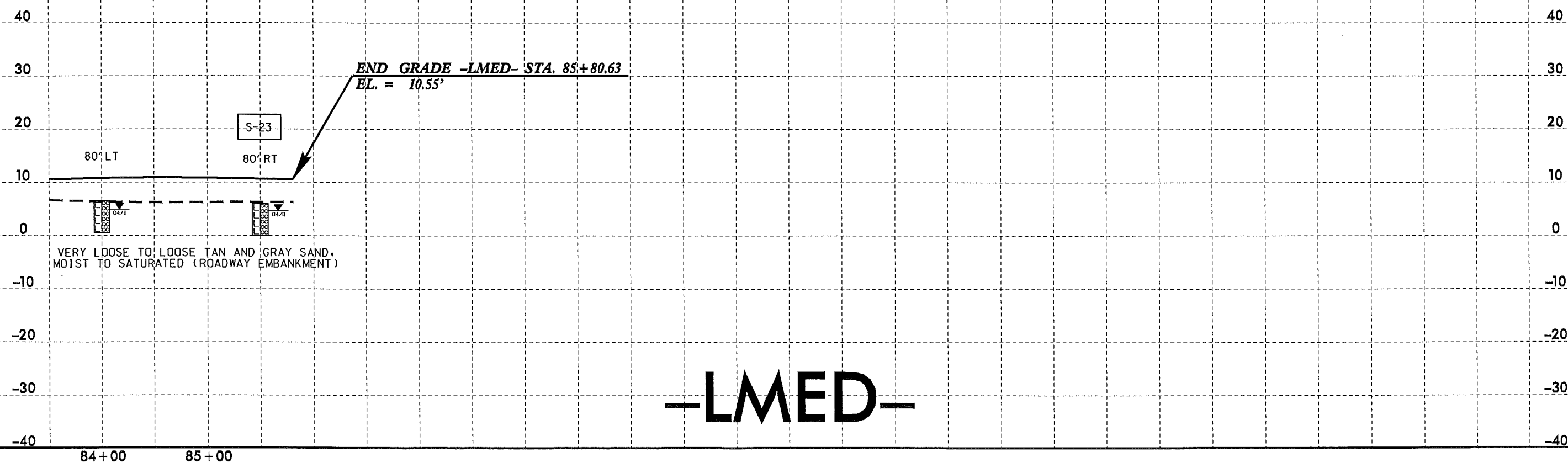
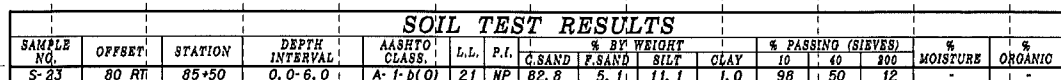
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PROJECT REFERENCE NO.		SHEET NO.	
R-3601		15	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

INCOMPLETE PLANS
DO NOT USE FOR A/R/W ACQUISITION

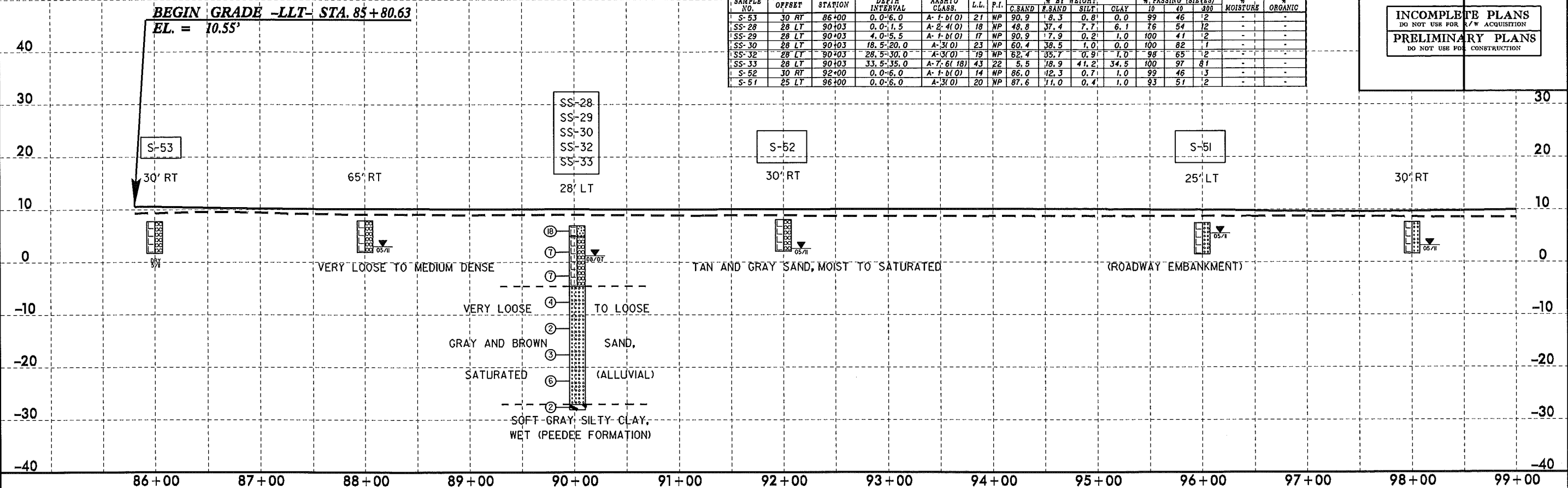
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION



5/28/99

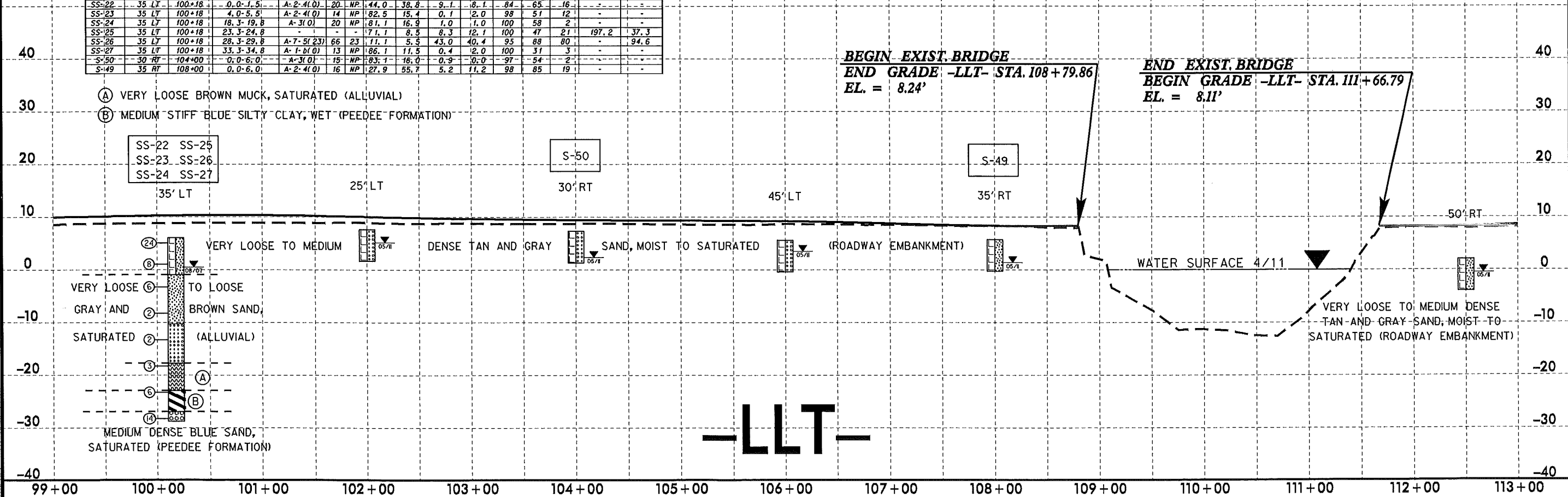
PROJECT REFERENCE NO.		SHEET NO.	
R-3601		16	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	C.SAND	F.SAND	SILT	CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC
S-53	30 RT	86+00	0.0-6.0	A-1-b(0)	21	NP	90.9	8.3	0.8	0.0	99	46	-
SS-28	28 LT	90+03	0.0-1.5	A-2-4(0)	18	NP	48.8	37.4	7.7	6.1	76	54	-
SS-29	28 LT	90+03	4.0-5.5	A-1-b(0)	17	NP	90.9	7.9	0.2	1.0	100	41	-
SS-30	28 LT	90+03	18.5-20.0	A-3(0)	23	NP	60.4	38.5	1.0	0.0	100	82	-
SS-32	28 LT	90+03	28.5-30.0	A-3(0)	19	NP	62.4	35.7	0.9	1.0	98	65	-
SS-33	28 LT	90+03	33.5-35.0	A-7-6(18)	43	22	5.5	18.9	41.2	34.5	100	97	81
S-52	30 RT	92+00	0.0-6.0	A-1-b(0)	14	NP	86.0	12.3	0.7	1.0	99	46	-
S-51	25 LT	96+00	0.0-6.0	A-3(0)	20	NP	87.6	11.0	0.4	1.0	93	51	-



SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	LL	P.I.	C.SAND	F.SAND	SILT	CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC
SS-22	35 LT	100+18	0.0-1.5	A-2-4(0)	20	NP	44.0	38.8	9.1	8.1	84	65	16
SS-23	35 LT	100+18	4.0-5.5	A-2-4(0)	14	NP	82.5	15.4	0.1	2.0	98	51	12
SS-24	35 LT	100+18	18.3-19.8	A-3(0)	20	NP	81.1	16.9	1.0	1.0	100	58	2
SS-25	35 LT	100+18	23.3-24.8	-	-	-	71.1	8.5	8.3	12.1	100	47	21
SS-26	35 LT	100+18	28.3-29.8	A-7-5(23)	66	23	11.1	5.5	43.0	40.4	95	88	80
SS-27	35 LT	100+18	33.3-34.8	A-1-b(0)	13	NP	86.1	11.5	0.4	2.0	100	31	3
S-50	30 RT	104+00	0.0-6.0	A-3(0)	15	NP	83.1	16.0	0.9	0.0	97	54	2
S-49	35 RT	108+00	0.0-6.0	A-2-4(0)	16	NP	27.9	55.7	5.2	11.2	98	85	19

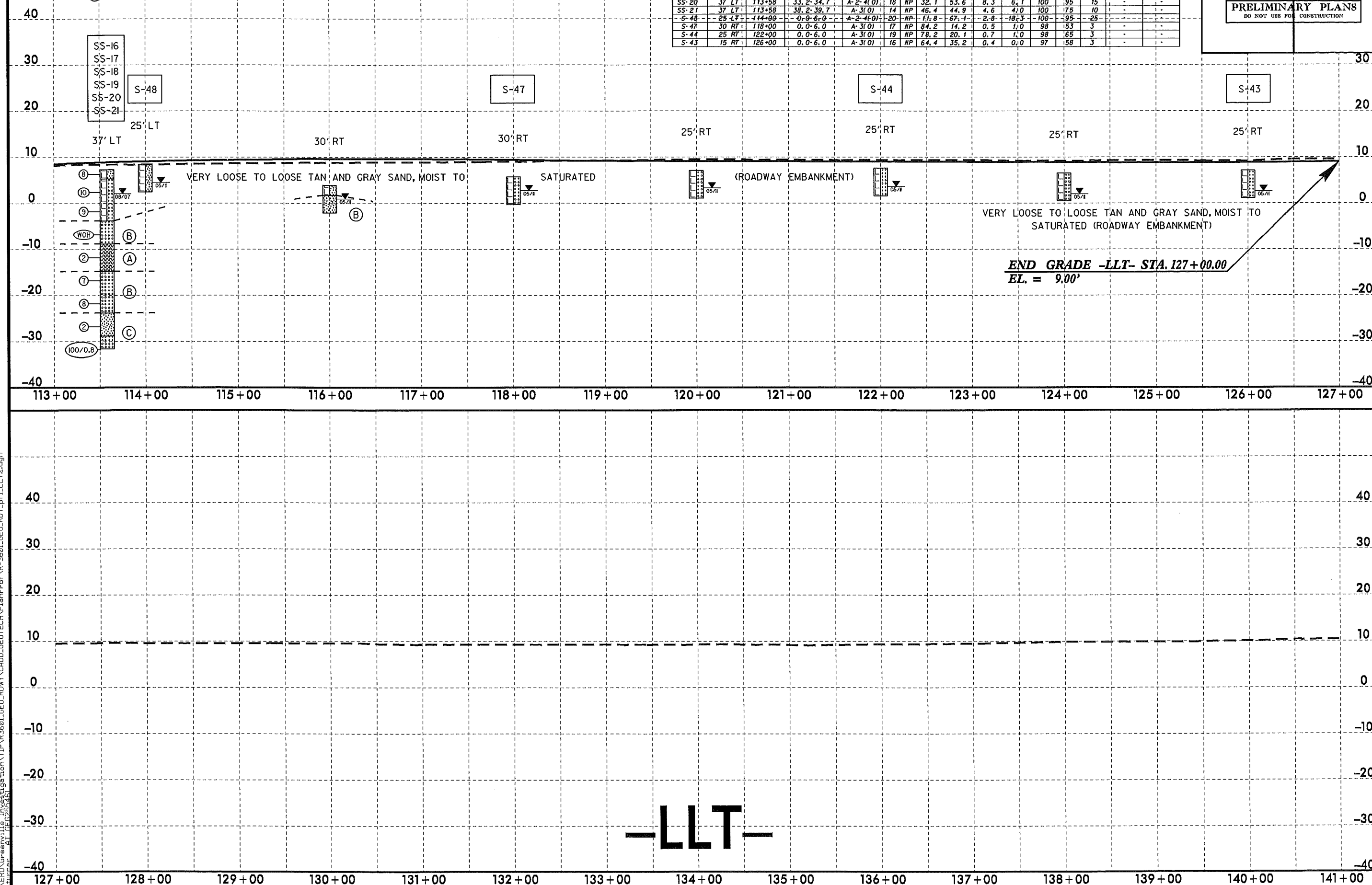
- (A) VERY LOOSE BROWN MUCK, SATURATED (ALLUVIAL)
(B) MEDIUM STIFF BLUE SILTY CLAY, WET (PEEDEE FORMATION)



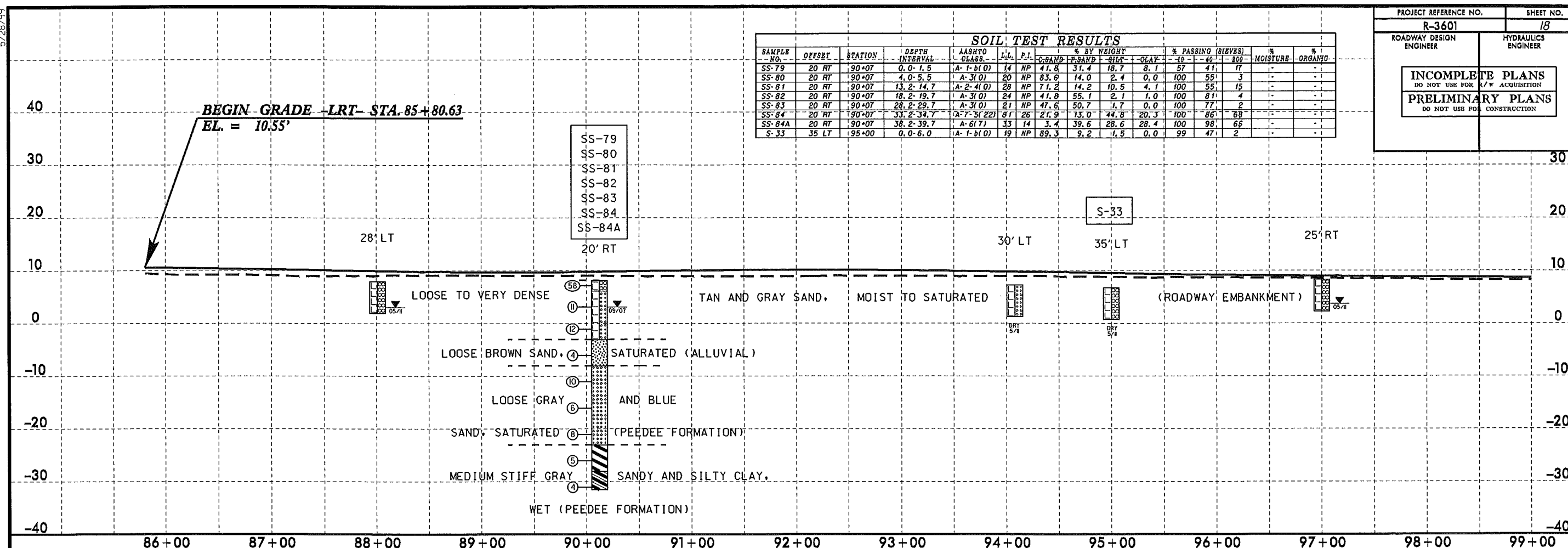
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- | SOIL TEST RESULTS | | | | | | | | | | | | | | | |
|-------------------|--------|---------|----------------|---------------|------|------|-------------|--------|------|--------------------|-----|----|------------|-----------|---|
| SAMPLE NO. | OFFSET | STATION | DEPTH INTERVAL | AASHTO CLASS. | L.L. | P.I. | % BY WEIGHT | | | % PASSING (SIEVES) | | | % MOISTURE | % ORGANIC | |
| | | | | | | | SAND | F.SAND | SILT | CLAY | 10 | 40 | 200 | | |
| SS-16 | 37 LT | 113+58 | 0.0 - 1.5 | A-2-4(0) | 19 | NP | 44.8 | 37.5 | 9.6 | 8.1 | 75 | 56 | 15 | - | - |
| SS-17 | 37 LT | 113+58 | 8.2 - 9.7 | A-3(0) | 16 | NP | 79.5 | 18.1 | 0.4 | 2.0 | 100 | 60 | 3 | - | - |
| SS-18 | 37 LT | 113+58 | 18.2 - 19.7 | - | - | - | - | - | - | - | - | - | - | 99 | - |
| SS-19 | 37 LT | 113+58 | 23.2 - 24.7 | A-3(0) | 12 | NP | 84.5 | 12.9 | 0.6 | 2.0 | 100 | 65 | 3 | - | - |
| SS-20 | 37 LT | 113+58 | 33.2 - 34.7 | A-2-4(0) | 18 | NP | 32.1 | 53.6 | 8.3 | 6.1 | 100 | 95 | 15 | - | - |
| SS-21 | 37 LT | 113+58 | 38.2 - 39.7 | A-3(0) | 14 | NP | 46.4 | 44.9 | 4.6 | 4.0 | 100 | 75 | 10 | - | - |
| S-48 | 25 LT | 114+00 | 0.0 - 6.0 | A-2-4(0) | 20 | NP | 11.8 | 67.1 | 2.8 | 18.3 | 100 | 95 | 25 | - | - |
| S-47 | 30 RT | 118+00 | 0.0 - 6.0 | A-3(0) | 17 | NP | 84.2 | 14.2 | 0.5 | 1.0 | 98 | 53 | 3 | - | - |
| S-44 | 25 RT | 122+00 | 0.0 - 6.0 | A-3(0) | 19 | NP | 78.2 | 20.1 | 0.7 | 1.0 | 98 | 65 | 3 | - | - |
| S-43 | 15 RT | 126+00 | 0.0 - 6.0 | A-3(0) | 16 | NP | 64.4 | 35.2 | 0.4 | 0.4 | 97 | 58 | 3 | - | - |

PROJECT REFERENCE NO.		SHEET NO.	
R-3601		17	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			

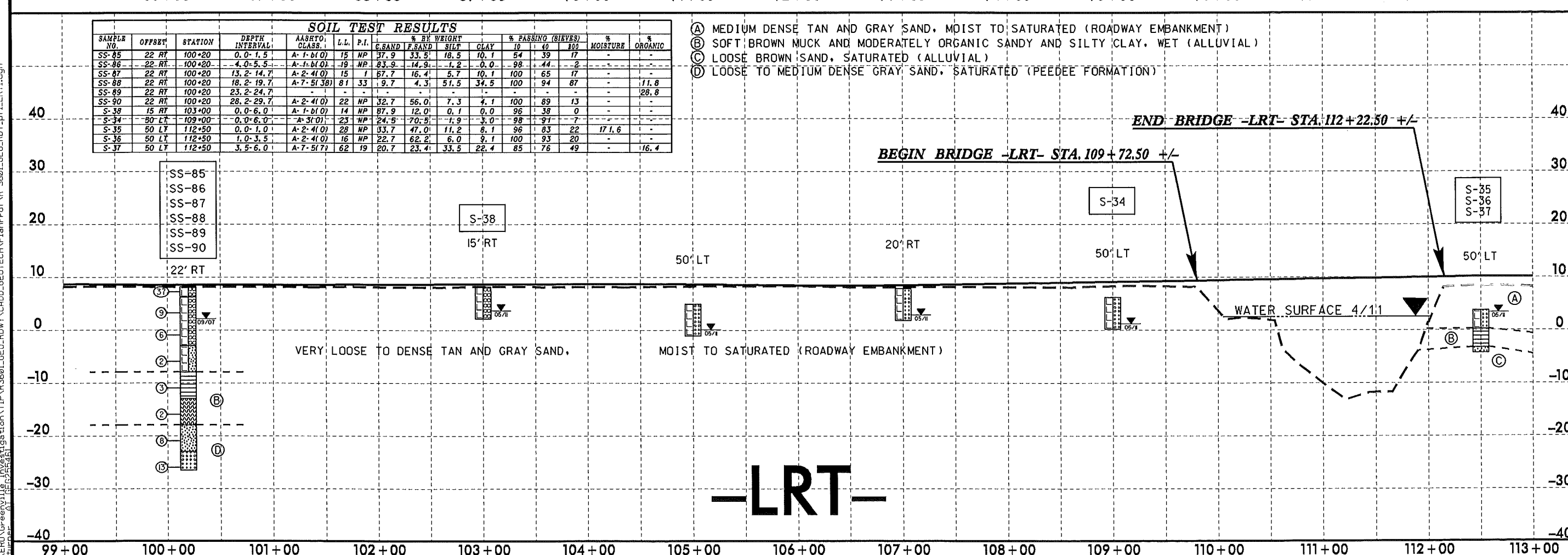


SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.L.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							G.SAND	F.SAND	SILT	CLAY	40	60	200		
SS-79	20 RT	90+07	0.0 - 1.5	A-1-b(1)	14	NP	41.8	3.4	16.7	8.1	57	41	17	-	-
SS-80	20 RT	90+07	4.0 - 5.5	A-3(0)	20	NP	83.6	14.0	2.4	0.0	100	55	3	-	-
SS-81	20 RT	90+07	13.2 - 14.7	A-2-4(0)	28	NP	71.2	14.2	10.5	4.1	100	55	15	-	-
SS-82	20 RT	90+07	18.2 - 19.7	A-3(0)	24	NP	41.8	55.1	2.1	1.0	100	81	4	-	-
SS-83	20 RT	90+07	28.2 - 29.7	A-3(0)	21	NP	47.6	50.7	1.7	0.0	100	77	2	-	-
SS-84	20 RT	90+07	33.2 - 34.7	A-7-5(22)	81	26	21.9	13.0	44.8	20.3	100	86	68	-	-
SS-84A	20 RT	90+07	38.2 - 39.7	A-6(7)	33	14	3.4	39.6	28.6	28.4	100	98	66	-	-
SS-33	35 L.T	95+00	0.0 - 5.0	A-6-b(1)	39	NP	89.3	9.3	1.6	0.0	89	42	2	-	-



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C.SAND	F.SAND	SILT	CLAY	10	40			#20
SS-25	22 RT	100+20	0.0 - 1.5	A-1-b(0)	15	NP	37.9	33.5	18.5	10.1	54	39	17	-	-
SS-26	22 RT	100+20	4.0 - 5.5	A-1-b(0)	19	NP	83.9	14.9	1.2	0.0	98	44	2	-	-
SS-87	22 RT	100+20	13.2 - 14.7	A-2-4(0)	15	1	67.7	16.4	5.7	10.1	100	65	17	-	-
SS-88	22 RT	100+20	18.2 - 19.7	A-7-5(38)	81	33	9.7	4.3	51.5	34.5	100	94	87	-	11.8
SS-89	22 RT	100+20	23.2 - 24.7	-	-	-	-	-	-	-	-	-	-	-	28.8
SS-90	22 RT	100+20	28.2 - 29.7	A-2-4(0)	22	NP	32.7	56.0	7.3	4.1	100	89	13	-	-
S-38	15 RT	103+00	0.0 - 6.0	A-1-b(0)	14	NP	87.9	12.0	0.1	0.0	96	38	0	-	-
S-39	50 LT	105+00	0.0 - 6.0	A-3(0)	23	NP	24.5	70.5	1.9	3.0	98	97	3	-	-
S-35	50 LT	112+50	0.0 - 1.0	A-2-4(0)	28	NP	53.7	47.0	11.2	8.1	86	83	22	17.6	-
S-36	50 LT	112+50	1.0 - 5.5	A-2-4(0)	16	NP	22.7	62.2	6.0	9.1	100	93	20	-	-
S-36	50 LT	112+50	5.5 - 8.0	A-2-5(2)	28	1P	22.7	62.2	7.3	23.1	65	75	20	-	16.4

- (A) MEDIUM DENSE TAN AND GRAY SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)
(B) SOFT BROWN MUCK AND MODERATELY ORGANIC SANDY AND SILTY CLAY, WET (ALLUVIAL)
(C) LOOSE BROWN SAND, SATURATED (ALLUVIAL)
(D) LOOSE TO MEDIUM DENSE GRAY SAND, SATURATED (PEEDEE FORMATION)



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
							C.SAND	F.SAND	SILT	CLAY	10	40			200
S-39	45 RT	120+00	0.0 - 0.6	A-1-b(0)	13	NP	82.0	15.8	1.2	1.0	96	48	4	-	-
SS-115	70 LT	122+00	0.0 - 1.0	A-2-4(0)	28	NP	56.9	23.0	10.0	10.1	100	74	21	-	6.2
SS-116	70 LT	122+00	1.0 - 1.5	A-2-4(0)	15	NP	61.9	21.9	5.1	11.1	100	74	17	-	-
SS-117	70 LT	122+00	7.0 - 8.5	C	-	-	-	-	-	-	-	-	-	-	24.1
SS-118	70 LT	122+00	12.0 - 13.5	A-1-b(0)	15	NP	89.1	9.5	1.4	0.0	100	50	3	-	-
S-42	15 RT	126+00	0.0 - 0.6	A-3(0)	20	NP	57.6	42.3	0.1	0.0	99	67	1	-	-

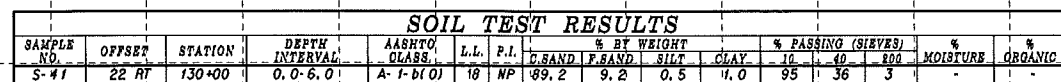
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R-3601		19	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

INCOMPLETE PLANS

DO NOT USE FOR A/W ACQUISITION

PRELIMINARY PLANS

DO NOT USE FOR CONSTRUCTION



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PROJECT REFERENCE NO.
R-3601

ROADWAY DESIGN
ENGINEER

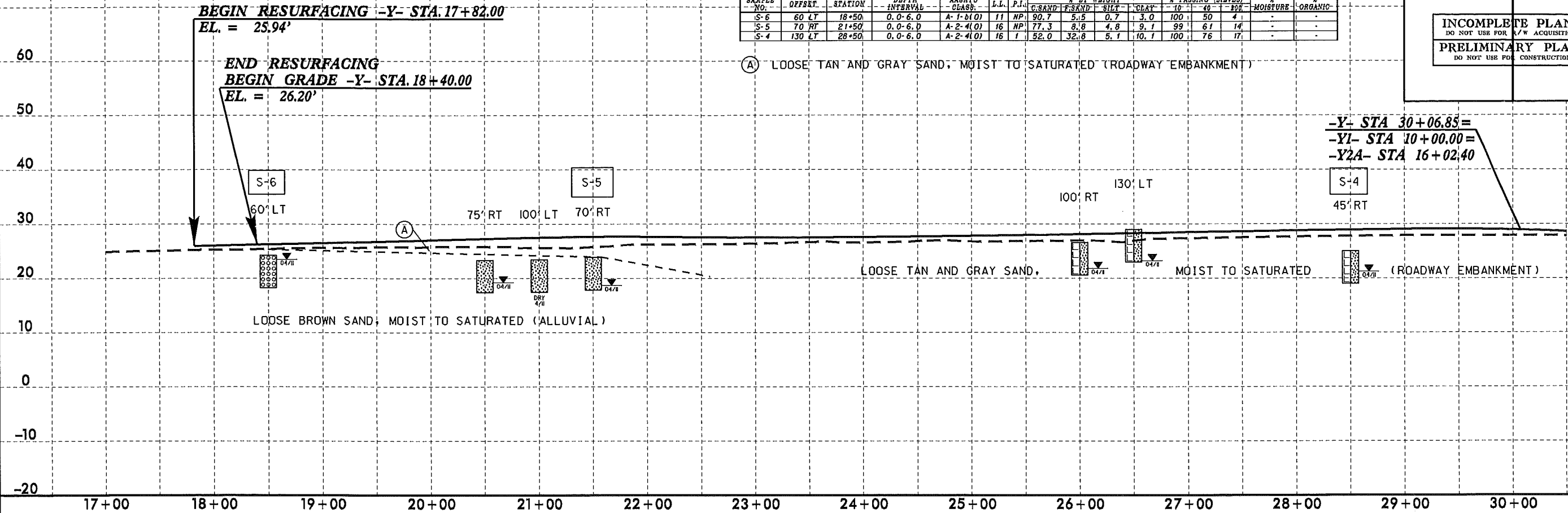
SHEET NO.
20

HYDRAULICS
ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR A/W ACQUISITION

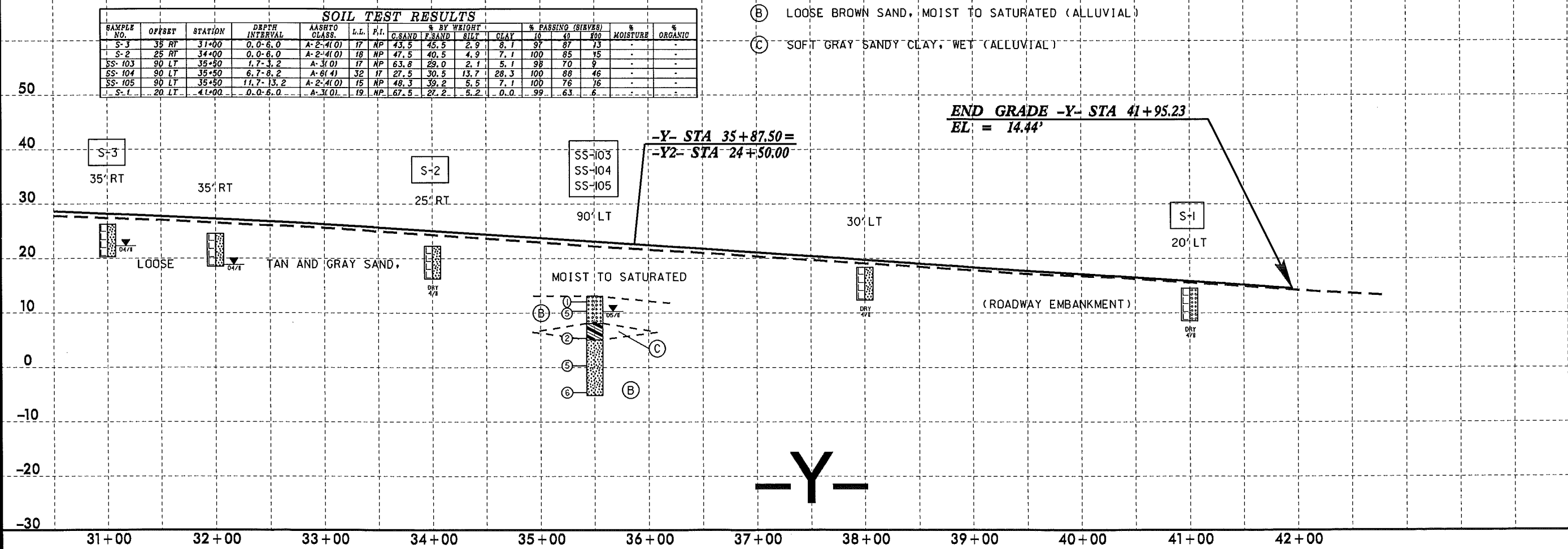
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE
							C.SAND	F.SAND	SILT	CLAY	10	40	
S-6	60' LT	18+50	0.0-6.0	A-1-B(0)	11	NP	90.7	5.5	0.7	3.0	100	50	4
S-5	70' RT	21+50	0.0-6.0	A-2-4(0)	16	NP	77.3	8.8	4.8	9.1	99	61	14
S-4	130' LT	28+50	0.0-6.0	A-2-4(0)	16	1	52.0	32.8	5.1	10.1	100	76	17



(A) LOOSE TAN AND GRAY SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)

SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE
							C.SAND	F.SAND	SILT	CLAY	10	40	
S-3	35' RT	31+00	0.0-6.0	A-2-4(0)	17	NP	43.5	45.5	2.9	8.1	97	87	13
S-2	25' RT	34+00	0.0-6.0	A-2-4(0)	18	NP	47.5	40.5	4.9	7.1	100	85	15
SS-103	90' LT	35+50	1.7-3.2	A-3(0)	17	NP	63.8	29.0	2.1	5.1	98	70	9
SS-104	90' LT	35+50	6.7-8.2	A-6(4)	32	17	27.5	30.5	13.7	28.3	100	88	46
SS-105	90' LT	35+50	11.7-13.2	A-2-4(0)	15	NP	48.3	39.2	5.5	7.1	100	76	16
S-1	20' LT	41+00	0.0-6.0	A-3(0)	19	NP	67.5	27.2	5.2	0.0	99	63	6

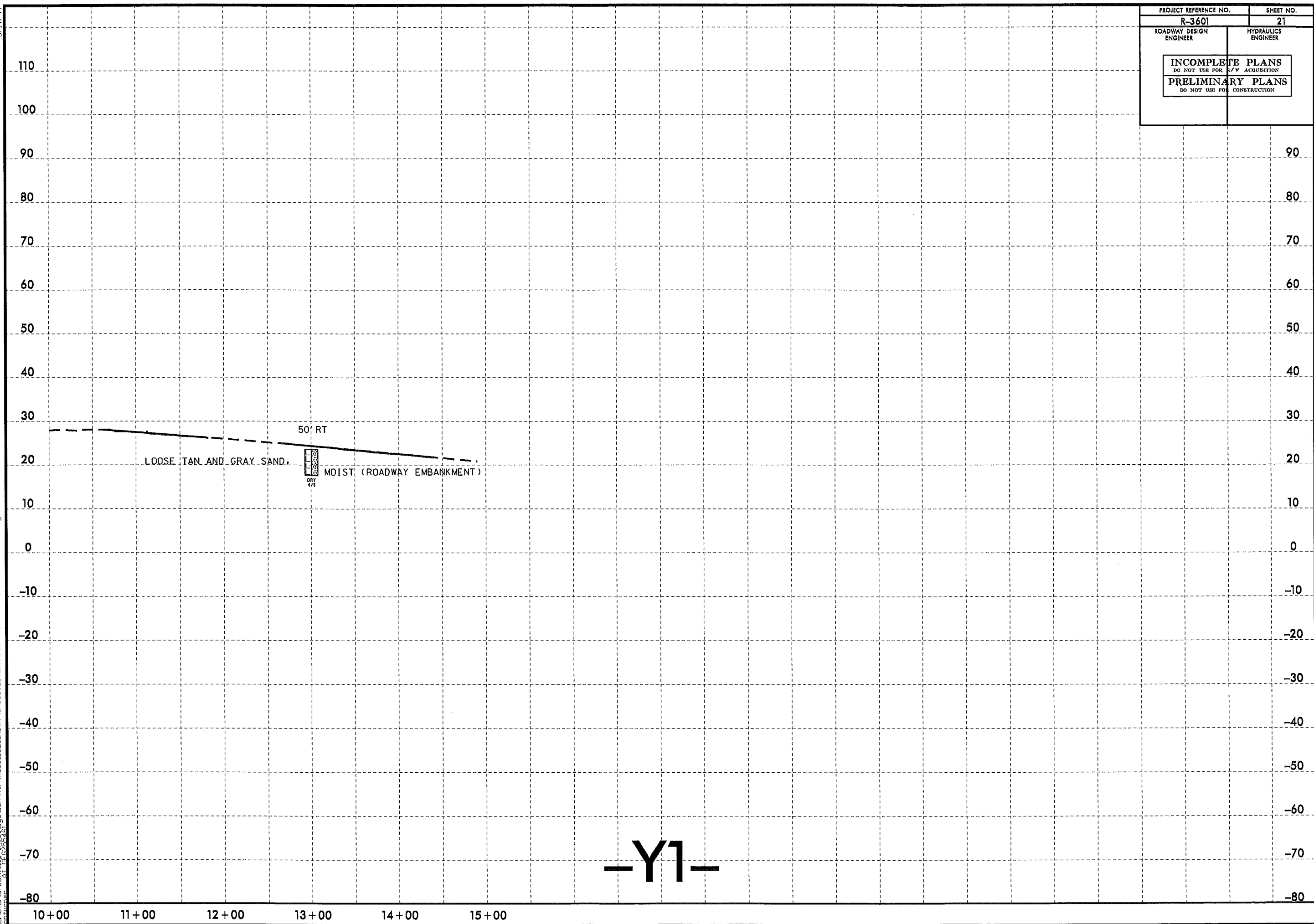


(B) LOOSE BROWN SAND, MOIST TO SATURATED (ALLUVIAL)

(C) SOFT GRAY SANDY CLAY, WET (ALLUVIAL)

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PROJECT REFERENCE NO.		SHEET NO.	
R-3601		21	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



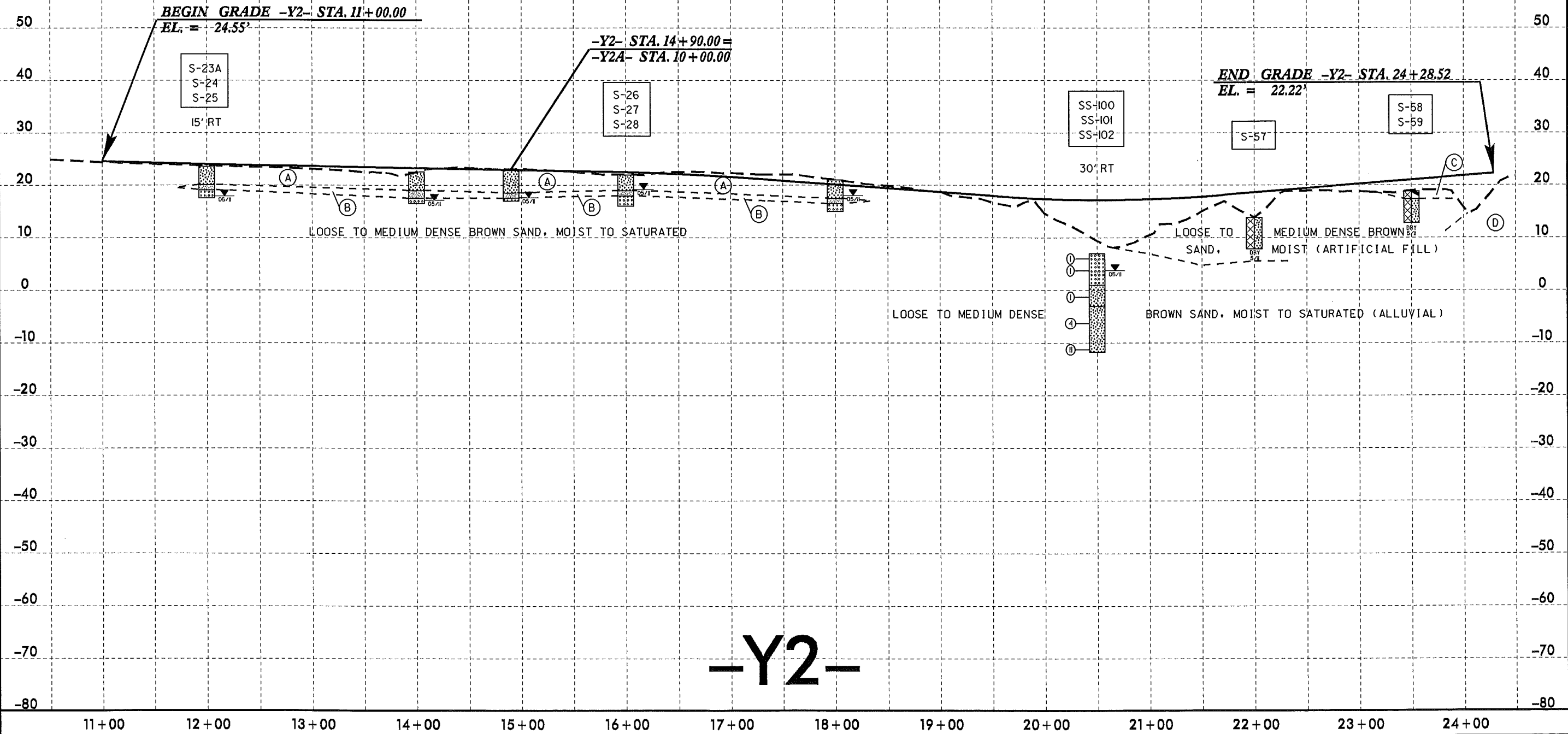
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LEADERGREEN

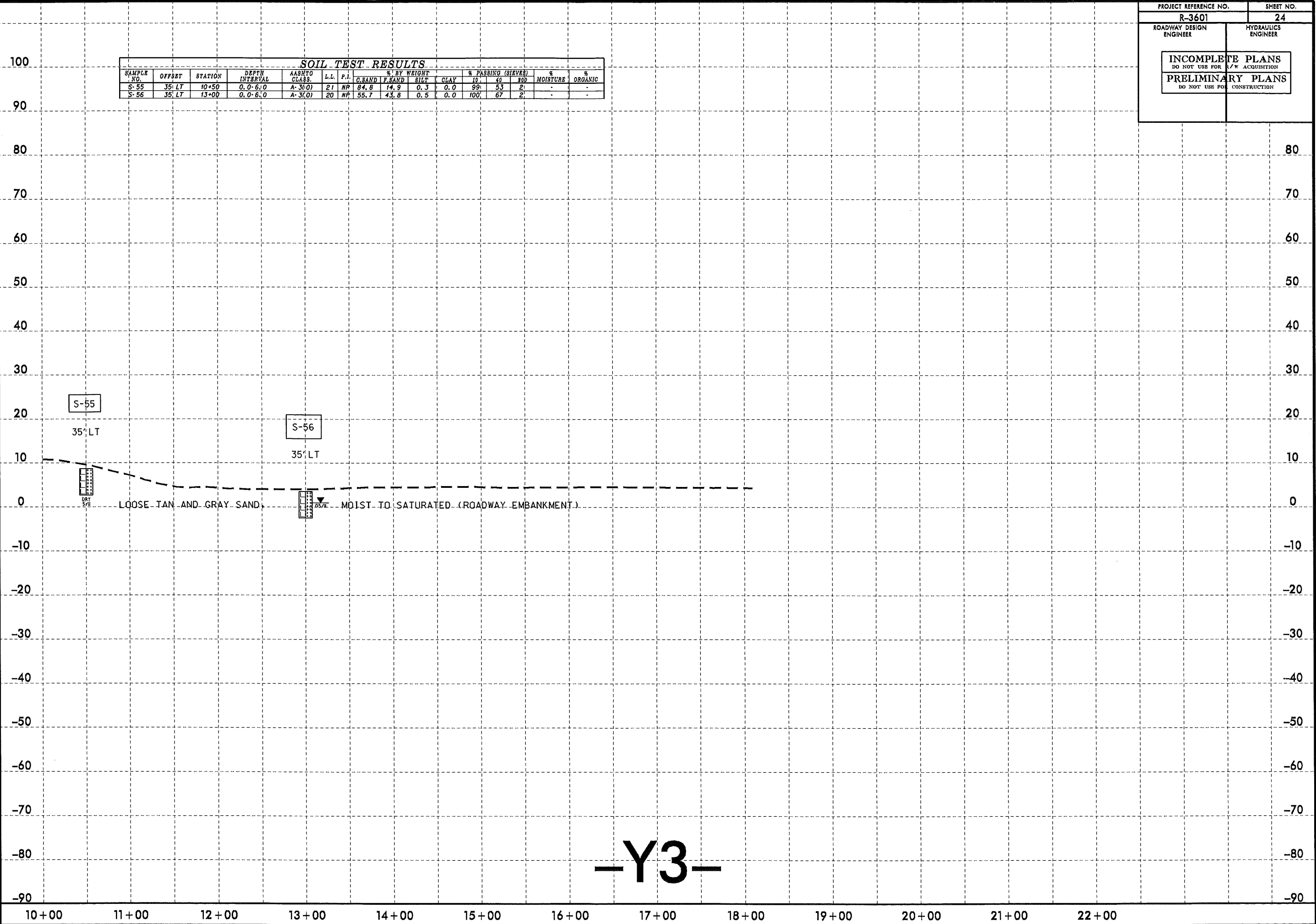
SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		
							C.SAND	F.SAND	SILT	CLAY	10	40	200
S-23A	15' RT	12+00	0.0-3.5	A-2-4(0)	15	NP	57.5	27.6	7.9	7.0	100	75	19
S-24	15' RT	12+00	3.5-4.5	A-4(2)	25	6	8.2	49.8	18.9	23.0	100	98	64
S-25	15' RT	12+00	4.5-6.0	A-3(0)	19	NP	37.7	58.2	1.1	3.0	100	90	6
S-26	CL	16+00	0.0-3.0	A-2-4(0)	16	NP	40.8	42.0	6.2	11.0	100	85	19
S-27	CL	16+00	3.0-4.0	A-4(1)	23	6	18.8	44.2	17.9	19.0	100	96	55
S-28	CL	16+00	4.0-6.0	A-3(0)	16	NP	59.4	32.9	3.7	4.0	100	91	10
SS-100	30' RT	20+50	2.5-3.8	A-3(0)	18	NP	65.7	29.1	1.2	4.1	100	84	7
SS-101	30' RT	20+50	7.5-8.8	A-2-4(0)	16	NP	51.9	35.8	5.2	7.1	100	86	16
SS-102	30' RT	20+50	12.5-13.8	A-2-4(0)	18	NP	62.9	27.5	3.5	6.1	97	60	11
S-57	CL	22+00	0.0-6.0	A-2-4(0)	18	NP	45.9	40.0	4.9	9.1	97	77	14
S-58	CL	23+50	0.0-1.5	A-7-6(15)	42	28	24.2	14.4	20.7	40.7	85	63	-
S-59	CL	23+50	1.5-6.0	A-2-4(0)	19	NP	67.4	20.4	6.1	6.1	90	52	12

PROJECT REFERENCE NO.	SHEET NO.
R-3601	22
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION	
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

- (A) LOOSE BROWN SAND, MOIST TO SATURATED (ALLUVIAL)
(B) SOFT BROWN SANDY SILT, MOIST TO WET (ALLUVIAL)
(C) SOFT BROWN SANDY CLAY, MOIST (ARTIFICIAL FILL)
(D) LOOSE TAN AND GRAY SAND, MOIST TO SATURATED (ROADWAY EMBANKMENT)



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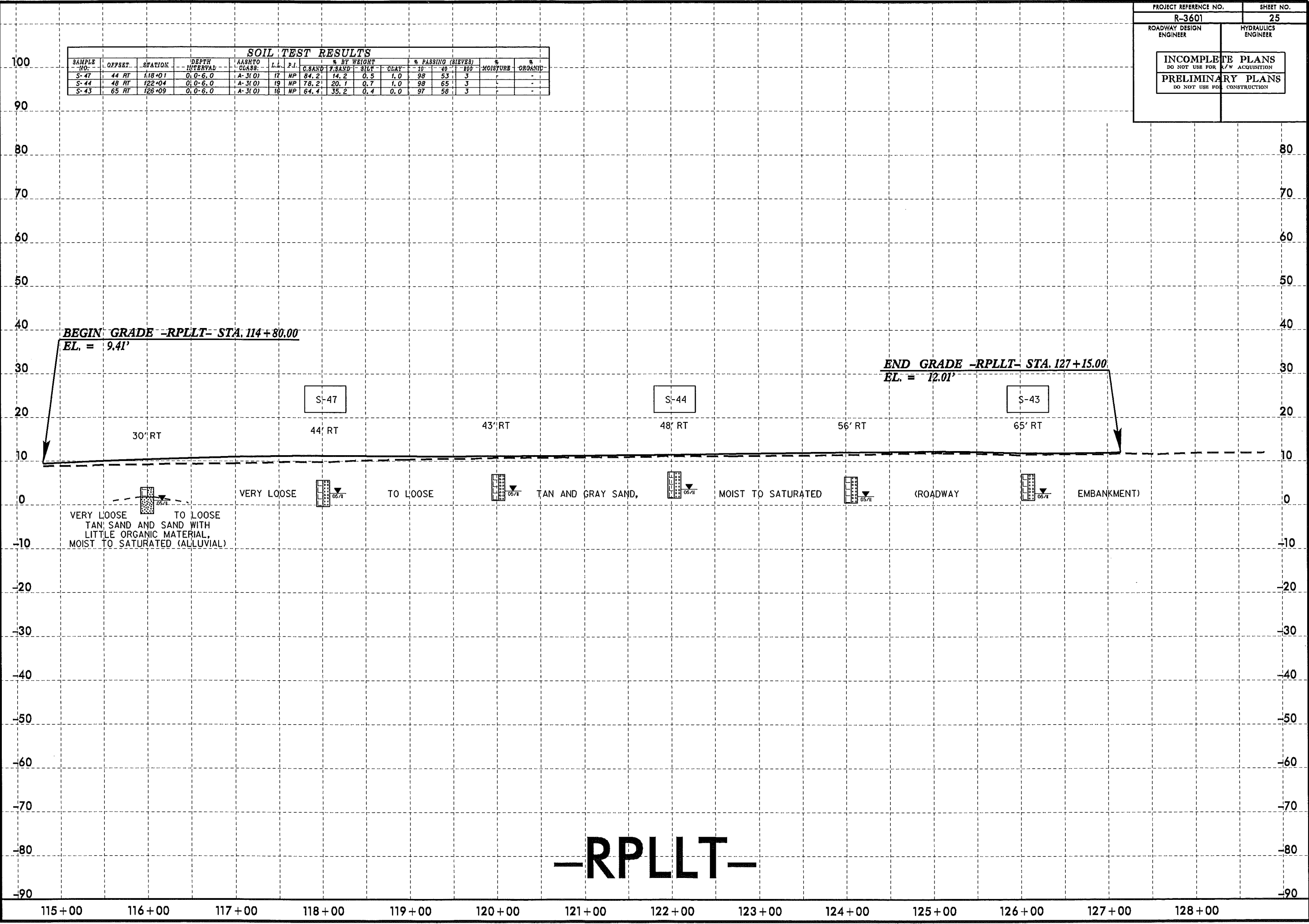


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PROJECT REFERENCE NO.		SHEET NO.	
R-3601		25	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR R/W ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		
							C.SAND	F.SAND	SILT	CLAY	10	40	200
S-47	44 RT	118+01	0'-0" - 6'-0"	A-3(0)	17	NP	84.2	14.2	0.5	1.0	98	53	3
S-44	48 RT	122+04	0'-0" - 6'-0"	A-3(0)	19	NP	78.2	20.1	0.7	1.0	98	65	3
S-43	65 RT	126+09	0'-0" - 6'-0"	A-3(0)	16	NP	64.4	35.2	0.4	0.0	97	58	3



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SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	U.C.	C.SAND	F.SAND	SILT	CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC	
S-40	50' LT	116+00	0.0-0.0	A-2.4(0)	18	NP	37.0	47.2	3.7	12.2	100	84	1.1	
SS-111	55' LT	117+50	0.0-1.5	A-2.4(0)	19	NP	60.4	29.9	0.6	5.1	98	70	1.1	
SS-112	55' LT	117+50	1.5-3.6	A-3(0)	15	NP	61.5	32.5	0.9	5.1	92	66	7	17.2
SS-113	55' LT	117+50	12.1-13.6	-	-	-	-	-	-	-	-	-	-	-
SS-114	55' LT	117+50	17.1-18.6	A-3(0)	20	NP	73.8	19.7	2.4	4.0	100	68	8	-
S-39	45' RT	120+00	0.0-6.0	A-1.6(0)	13	NP	82.0	15.8	1.2	1.0	96	48	4	-

(A) VERY LOOSE MODERATELY ORGANIC SAND,
MOIST TO SATURATED (ALLUVIAL)

PROJECT REFERENCE NO.
R-3601

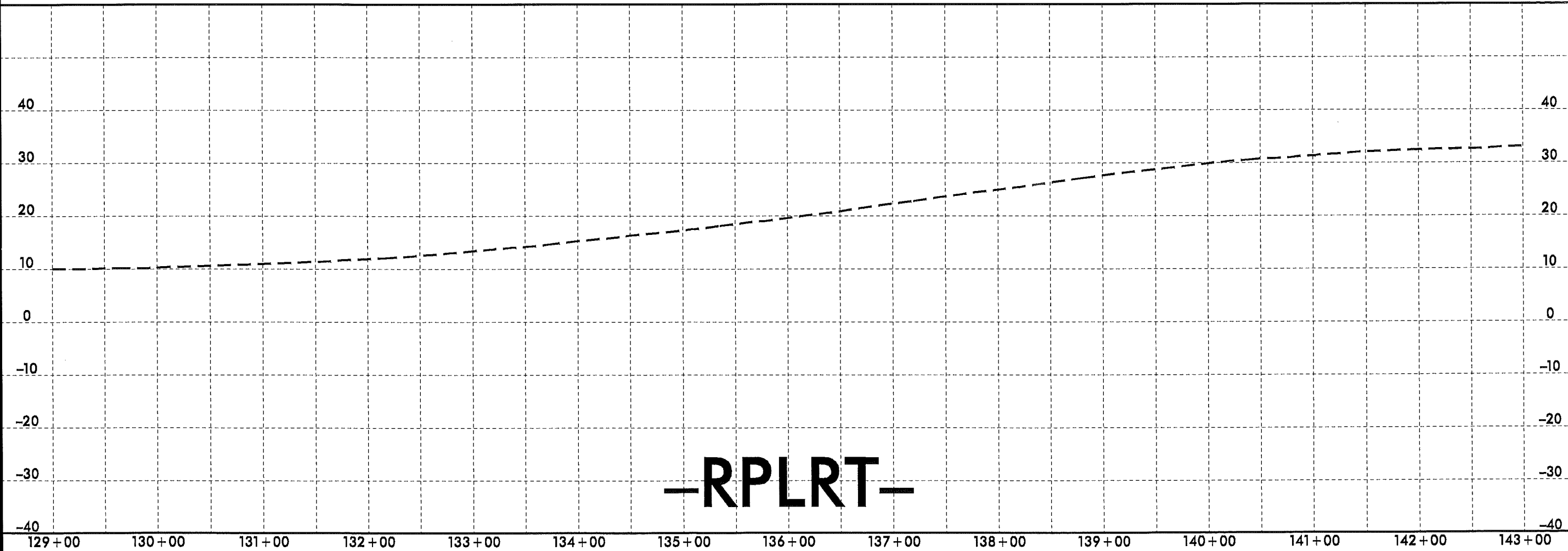
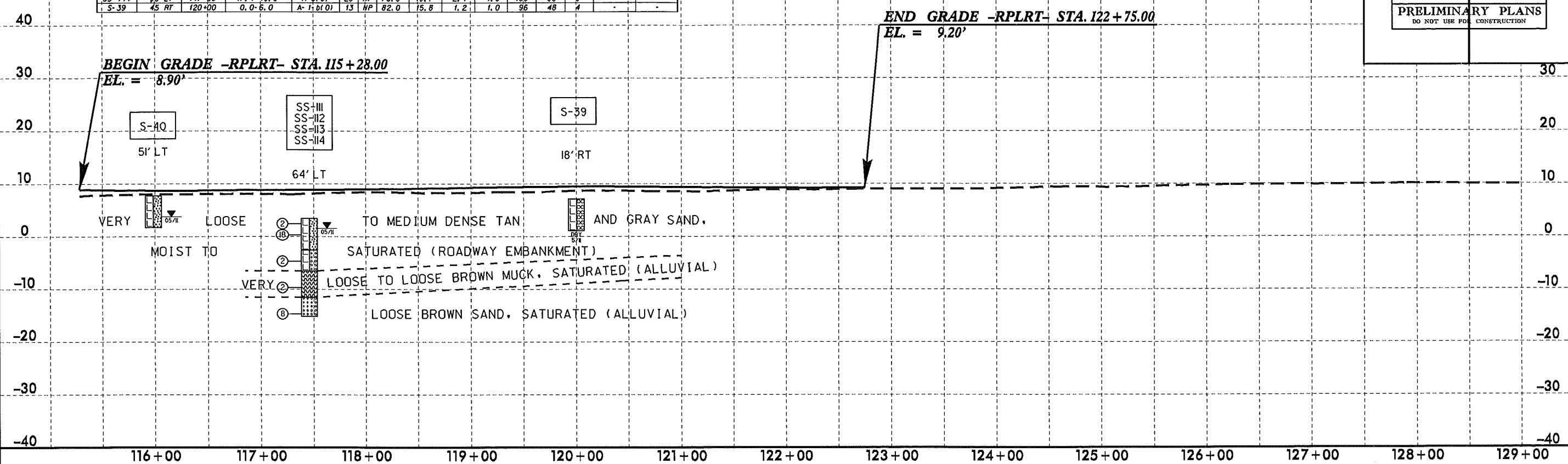
ROADWAY DESIGN
ENGINEER

SHEET NO.
26

HYDRAULICS
ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR A/W ACQUISITION

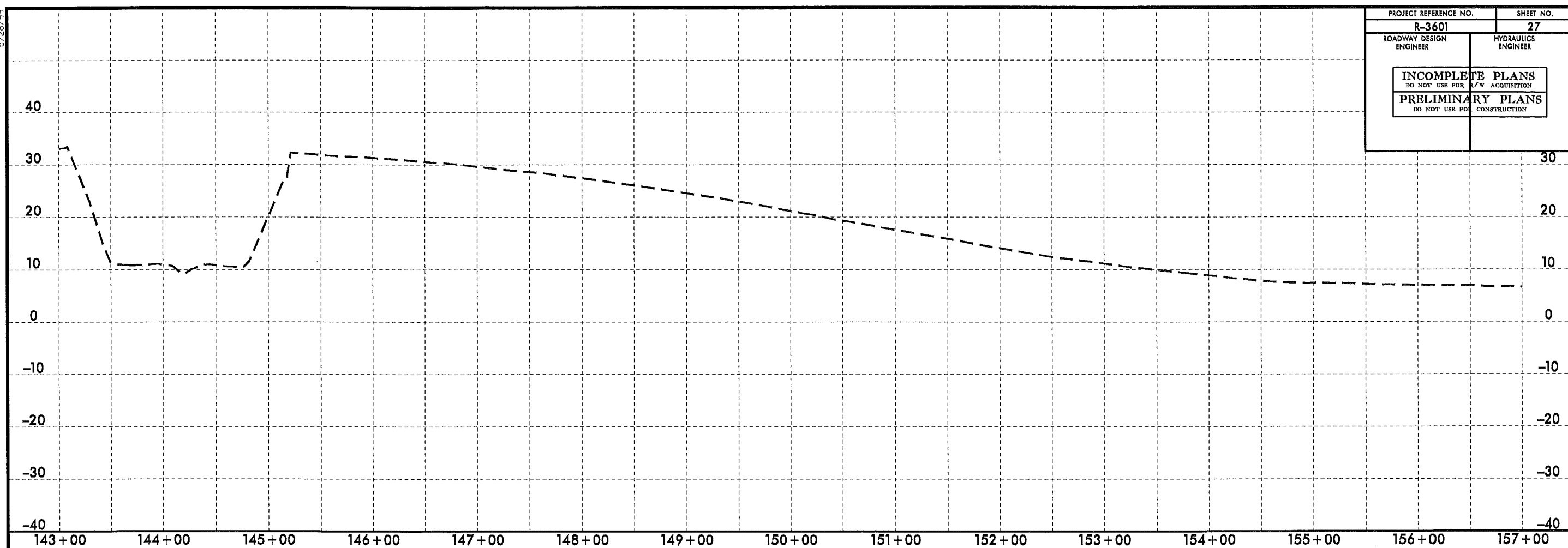
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION



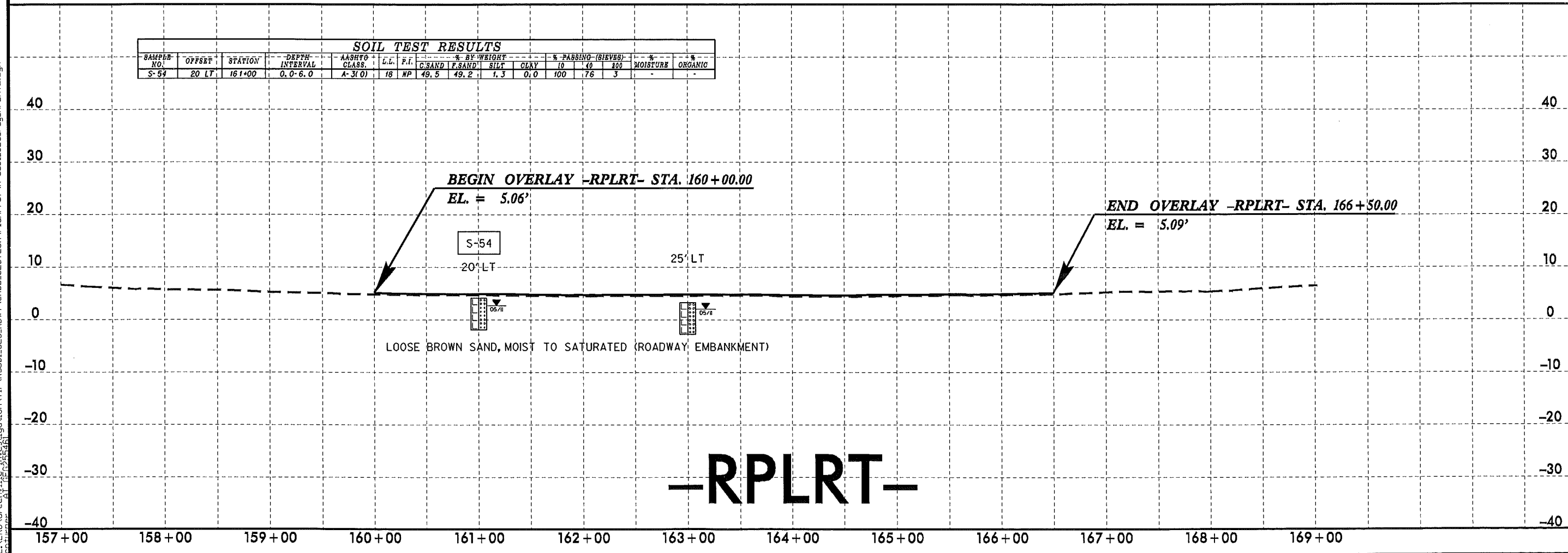
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3601

PROJECT REFERENCE NO. R-3601		SHEET NO. 27	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC
S-54	20' LT	161+00	0.0-6.0	A-3(0)	18	NP	49.5	49.2	1.3	0.0	100	76	3

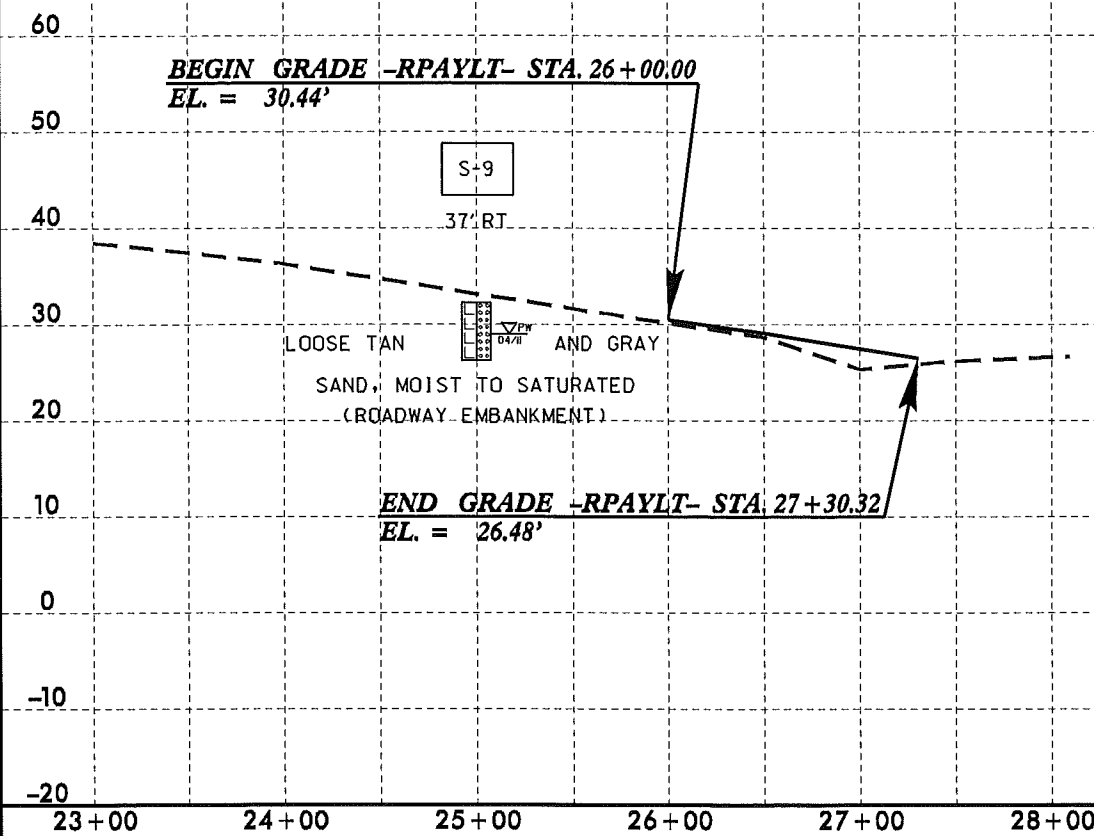


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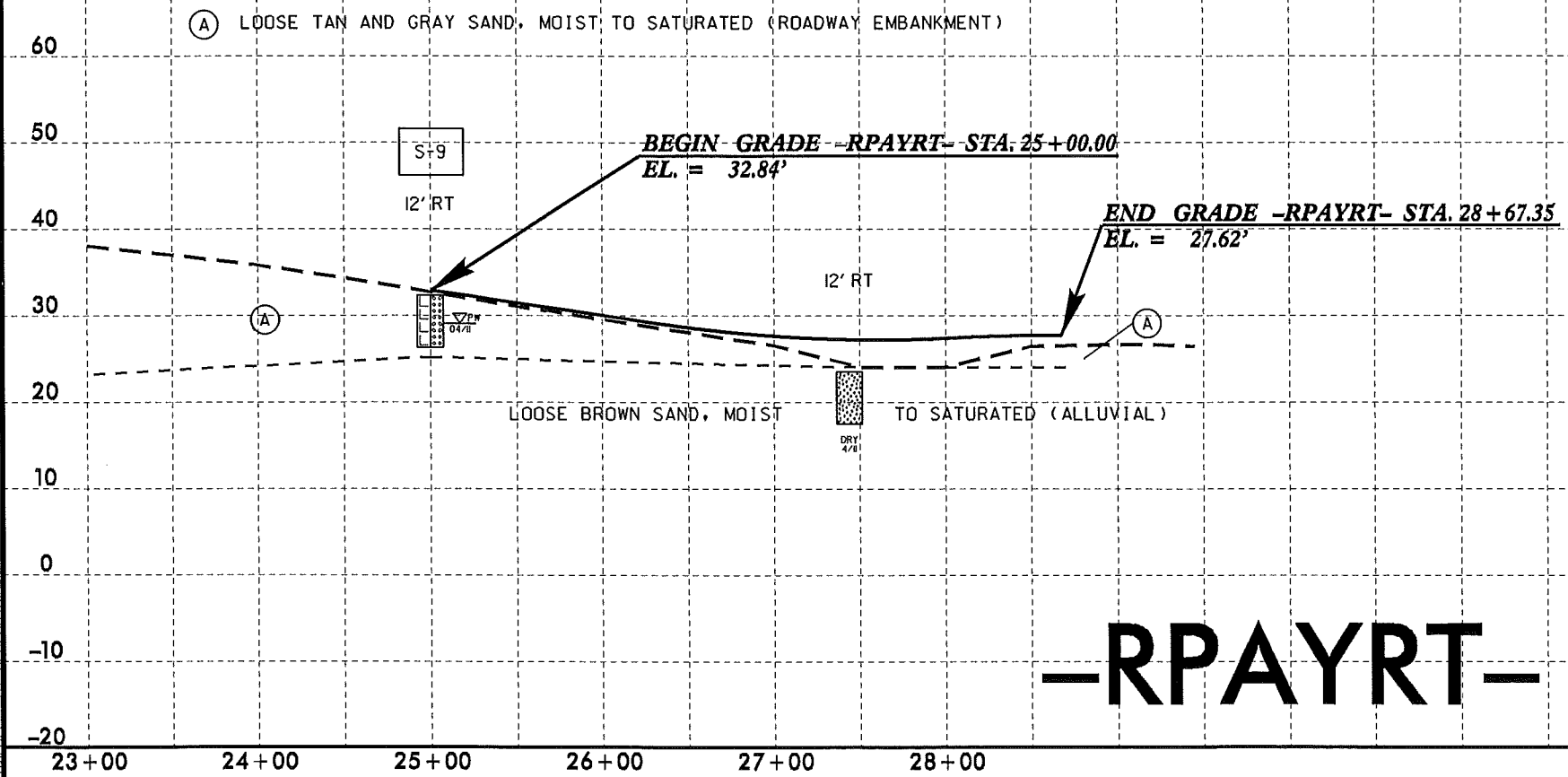
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R-3601		28	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	% BY WEIGHT					% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
					L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	10	40			200
S-9	45 RT	25+00	0.0-6.0	A-3(0)	15	NP	14.1	81.3	1.5	3.0	99	91	7	-	-



-RPAYLT-

SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)		%	%
							% SAND	% F.SAND	% SILT	% CLAY	-10	-40	-200	MOISTURE
S-9	45 RT	25+00	0.0-6.0	A-3(0)	15	NP	14.1	81.3	1.5	3.0	99	91	7	.

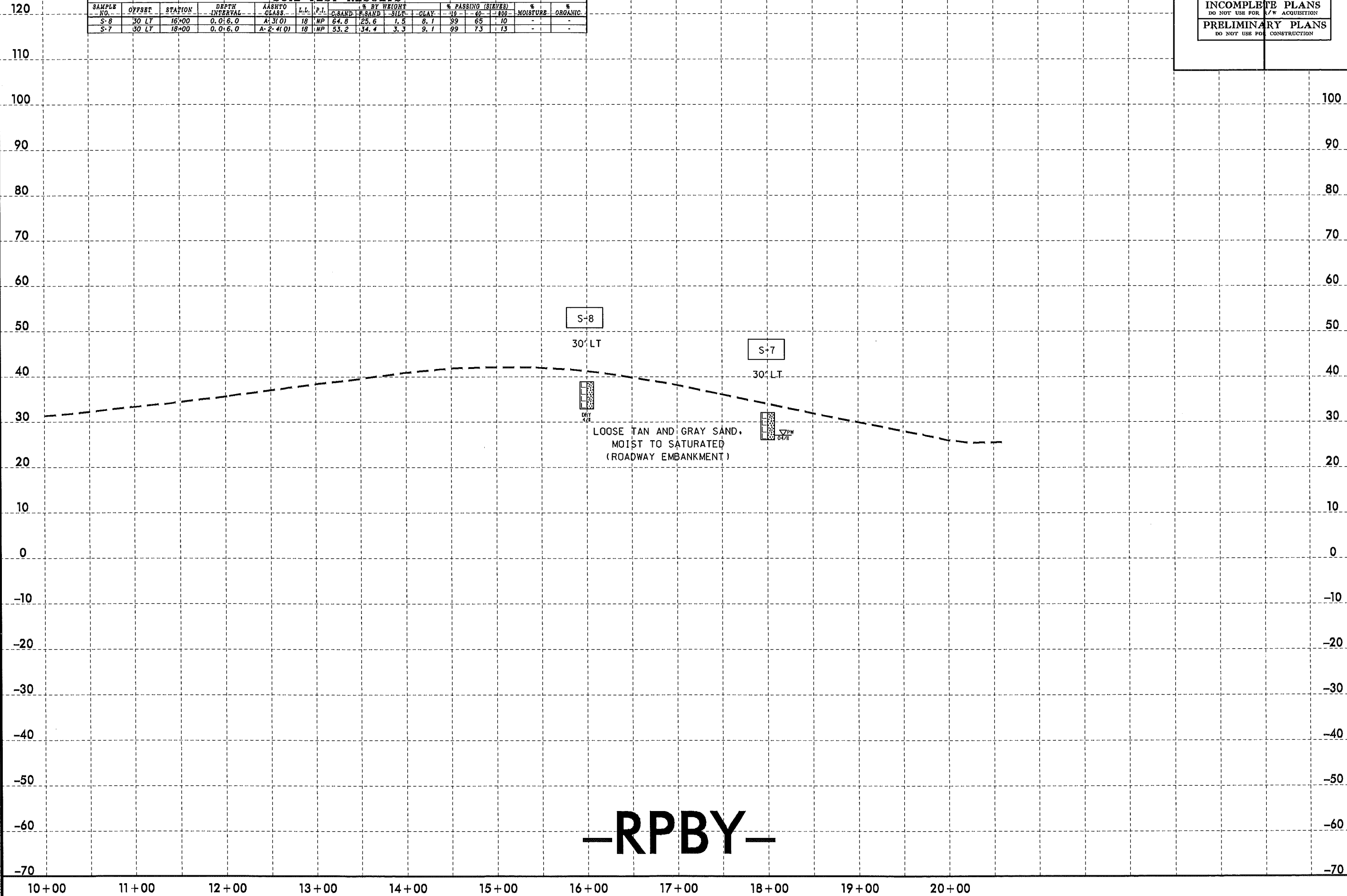


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Author: AT 11/25/2011

SOIL TEST RESULTS																
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS		L.L.	P.I.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE	% ORGANIC	
				CL	NP			C SAND	F SAND	SILT	CLAY	#10	#40			#200
S-8	30 LT	16+00	0.0' 6.0	A-3(0)	18	NP	64.8	25.6	1.5	8.1	99	65	10	-	-	
S-7	30 LT	18+00	0.0' 6.0	A-2-4(0)	18	NP	53.2	34.4	3.3	9.1	99	73	13	-	-	

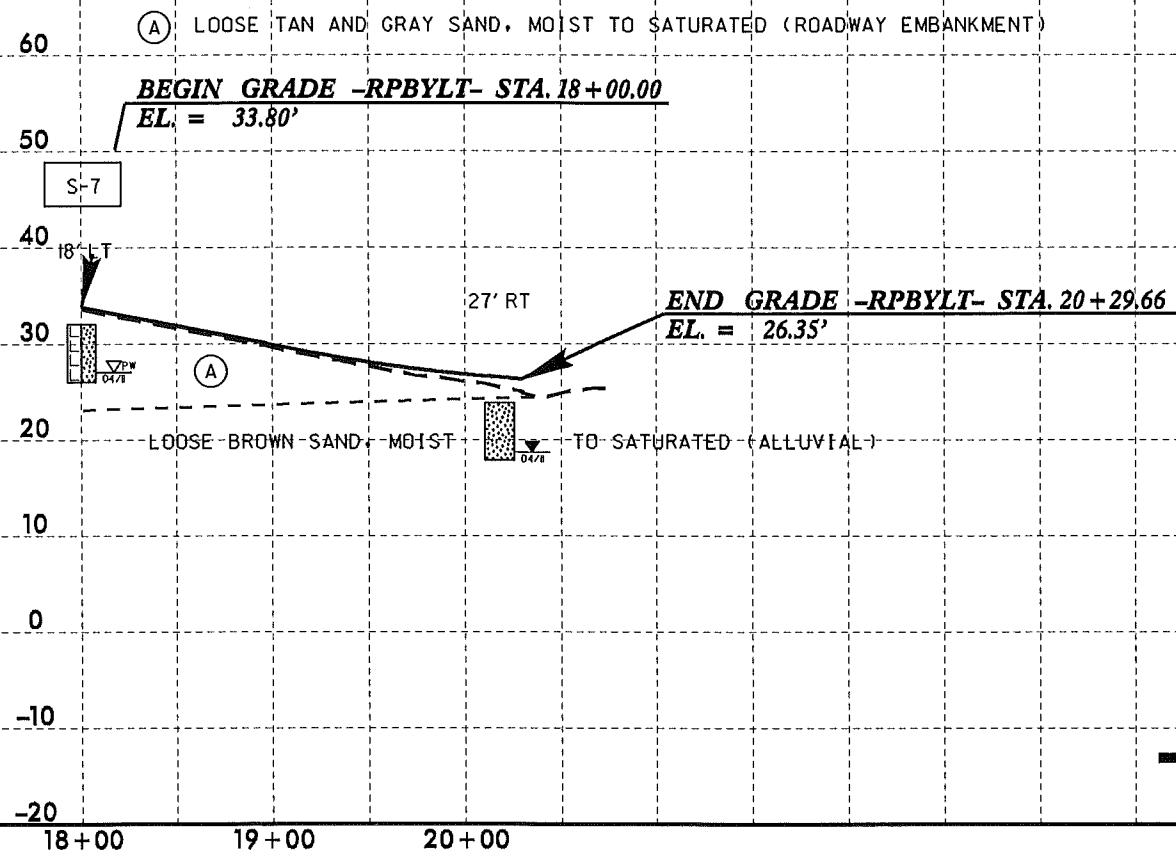
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R-3601		29	
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INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	



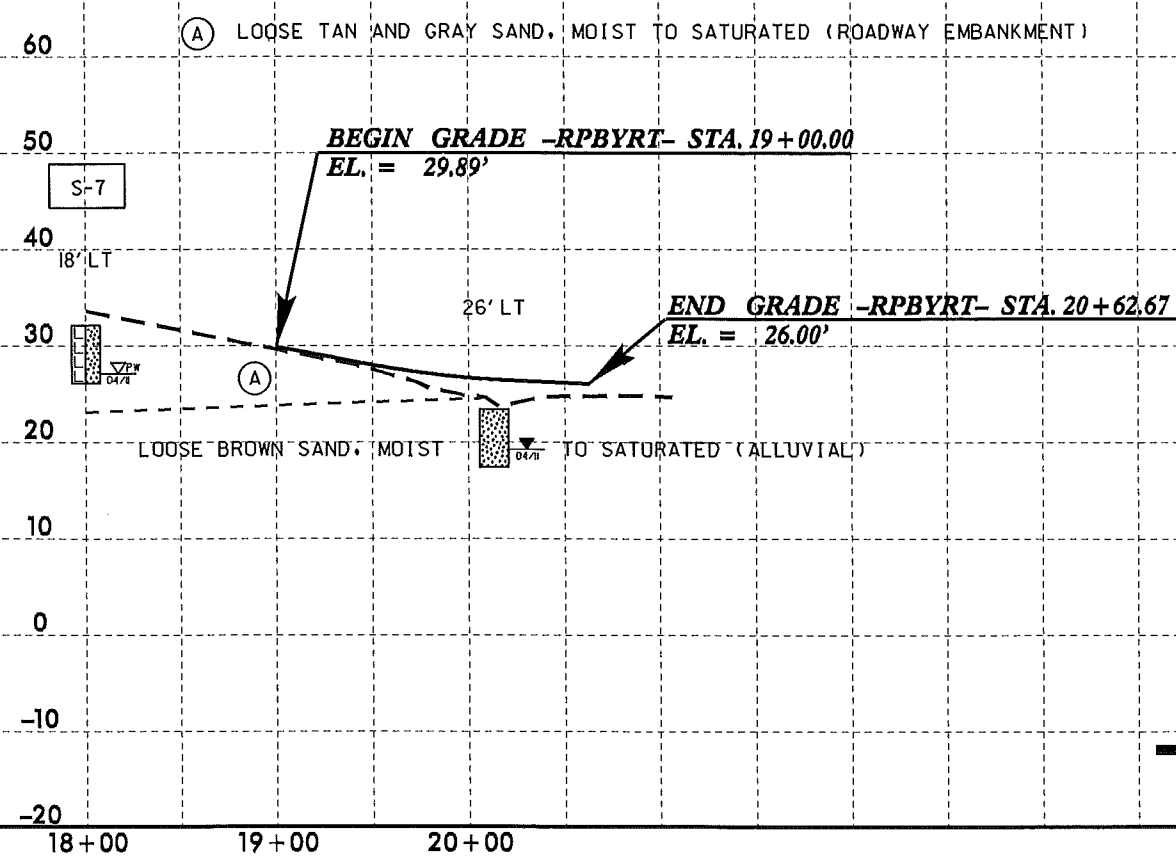
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PROJECT REFERENCE NO.		SHEET NO.	
R-3601		30	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.L.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE
							C.SAND	F.SAND	SILT	CLAY	10	40	600
S-7	30 LT	18+00	0.0-6.0	A-2-4(0)	18	NP	53.2	34.4	3.3	9.1	99	73	13



SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.L.	% BY WEIGHT			% PASSING (SIEVES)			% MOISTURE
							C.SAND	F.SAND	SILT	CLAY	10	40	600
S-7	30 LT	18+00	0.0-6.0	A-2-4(0)	18	NP	53.2	34.4	3.3	9.1	99	73	13

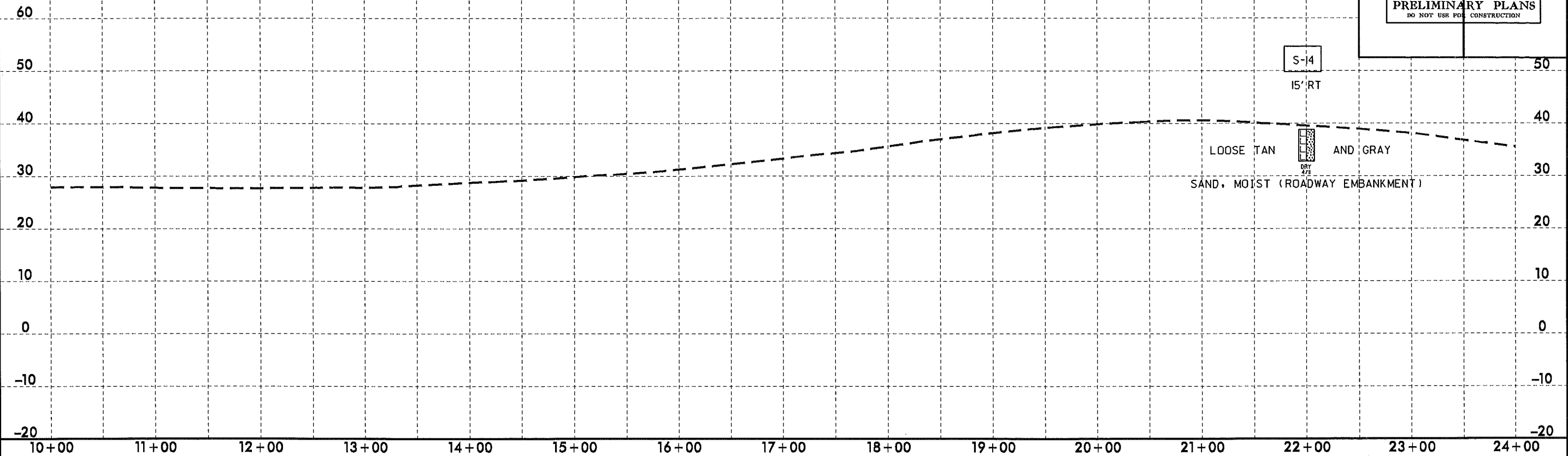


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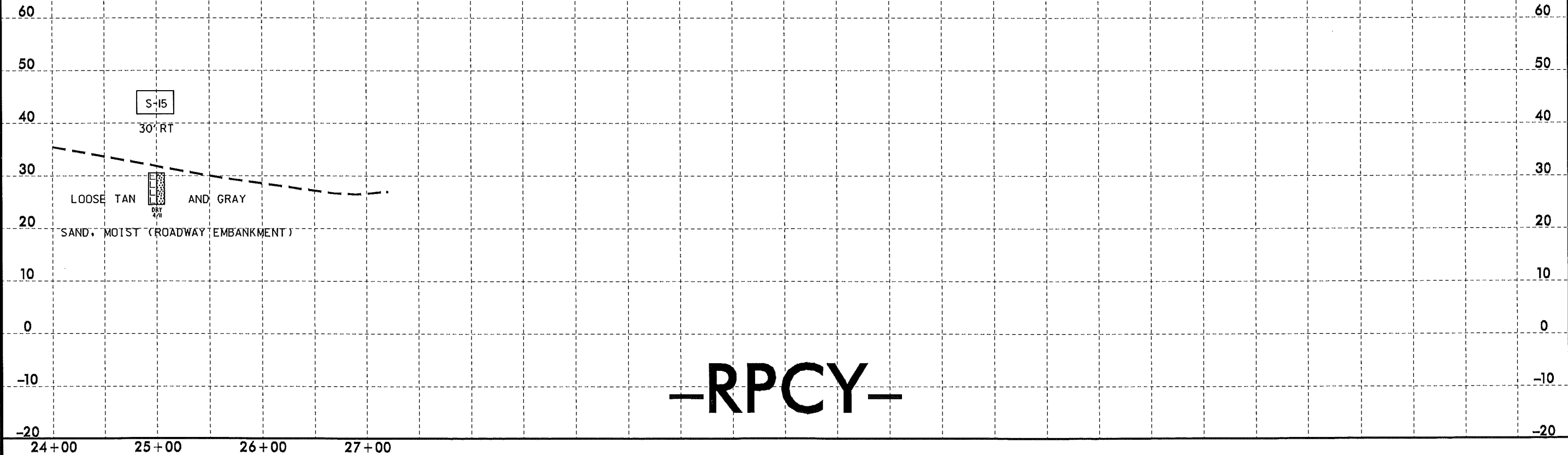
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PROJECT REFERENCE NO.		SHEET NO.	
R-3601		31	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION	

SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-14	15 RT	22+00	0.0-6.0	A-2-4(0)	18	NP	55.0	34.8	1.1	9.1	100	79	11	.	.



SOIL TEST RESULTS															
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% PASSING (SIEVES)			% MOISTURE	% ORGANIC
							C.SAND	F.SAND	SILT	CLAY	10	40	200		
S-15	15 RT	25+00	0.0-6.0	A-2-4 (0)	20	NP	59.9	29.9	1.1	9.1	100	71	11	-	-

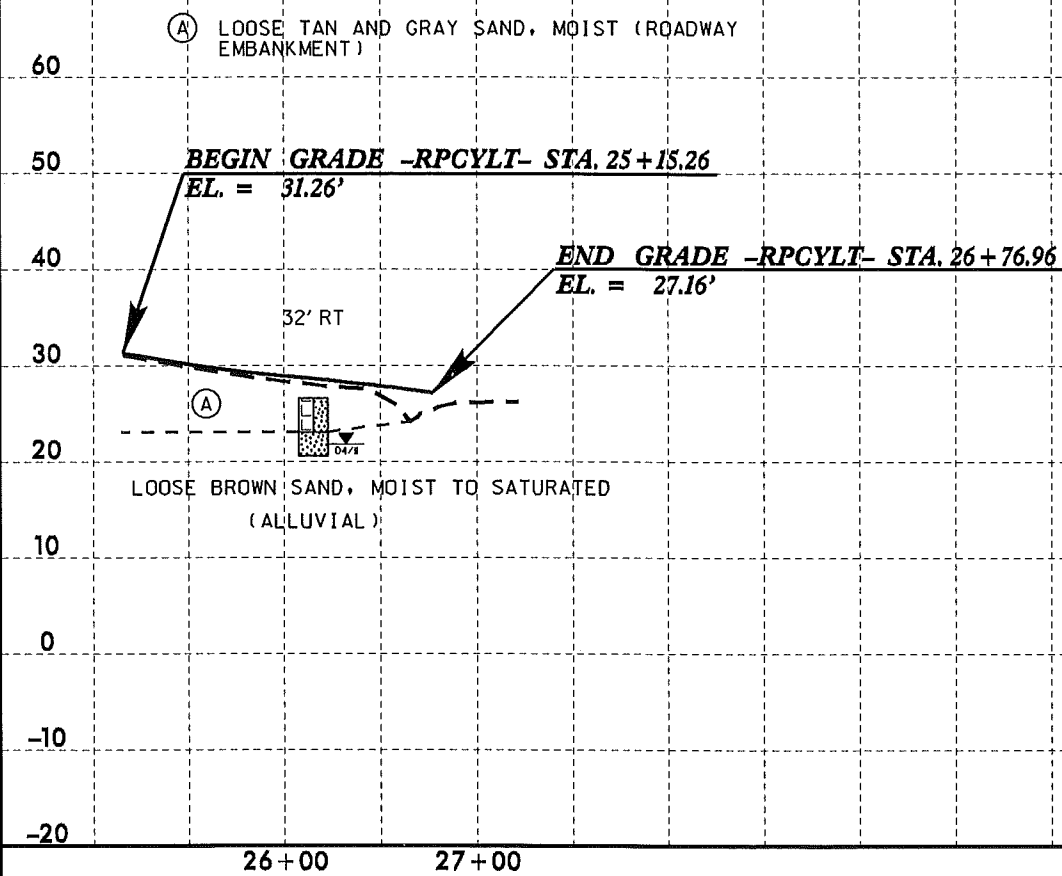


-RPCY-

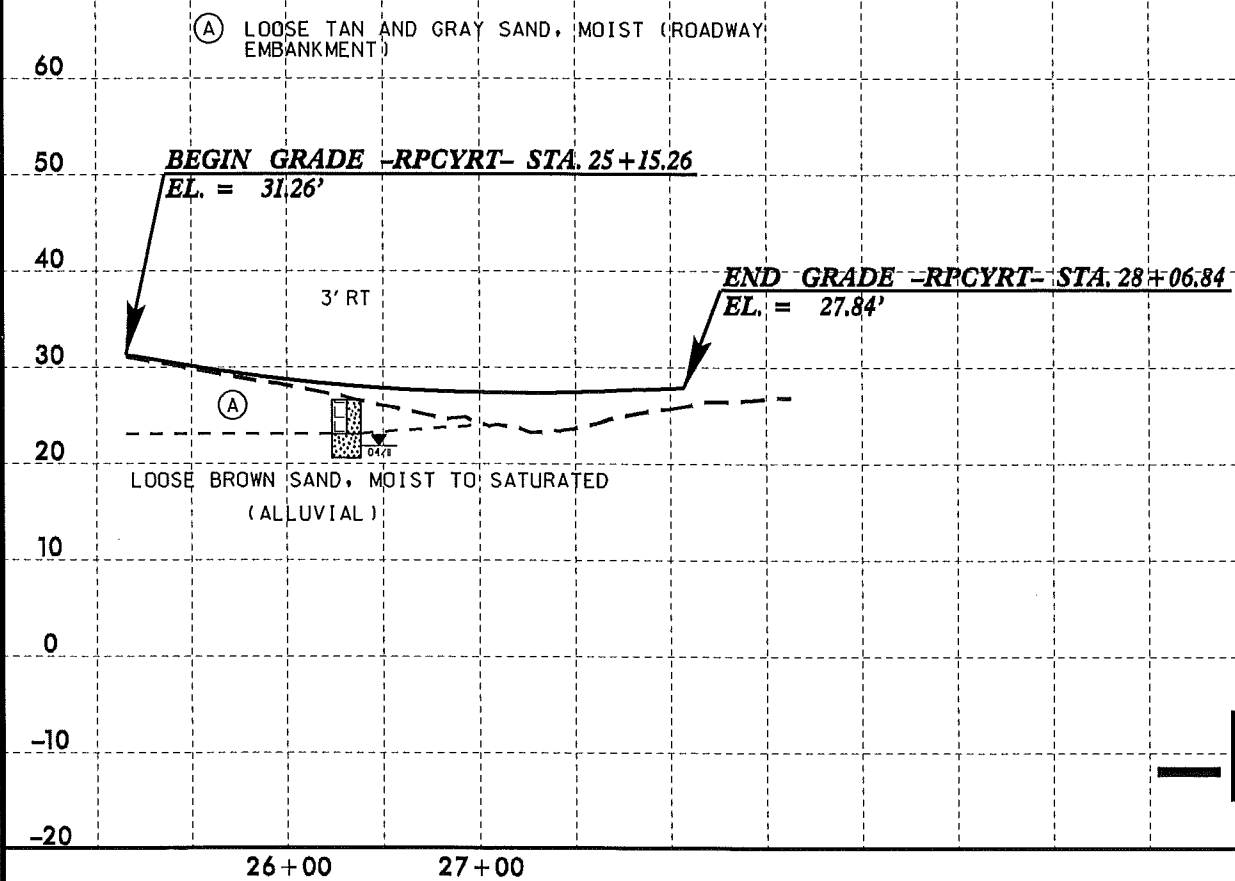
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06/28/99

PROJECT REFERENCE NO.		SHEET NO.
R-3601		32
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION		
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION		



-RPCYLT-



-RPCYRT-

5/28/99

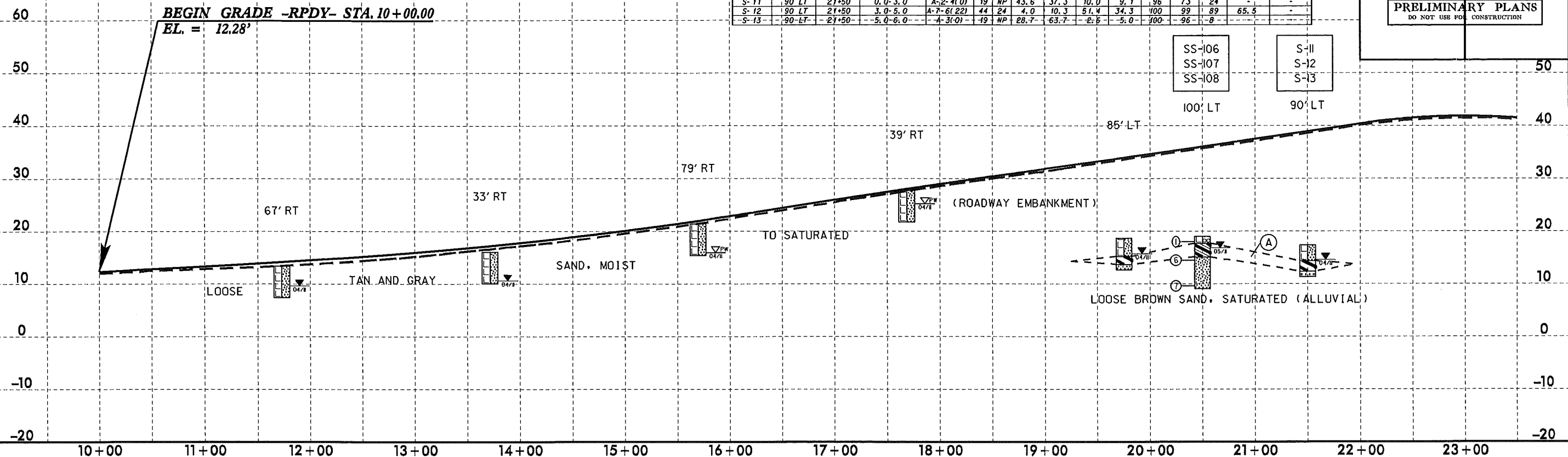
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Author: AT 10/25/99

(A) VERY SOFT GRAY SAND AND SILTY CLAY,
MOIST TO WET (ALLUVIAL)

BEGIN GRADE -RPDY- STA. 10+00.00
EL. = 12.28'

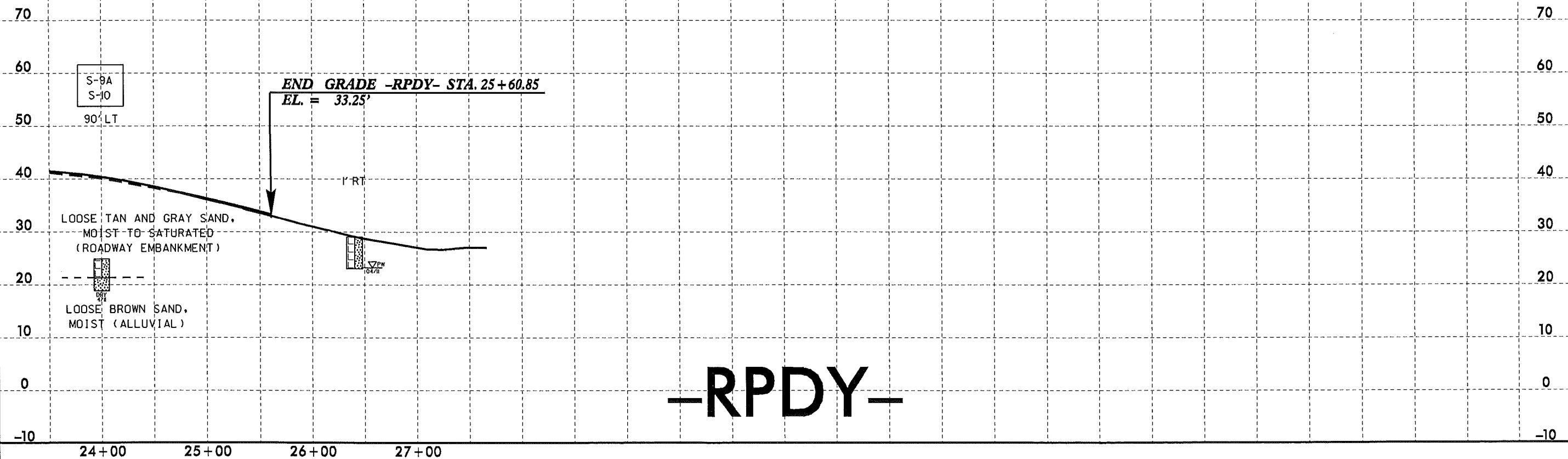
SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% SAND	% FINE SAND	% SILT	% CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC
SS-106	100' LT	20+50	0.0-1.3	A-1-b(1)	17	NP	83.5	14.0	0.4	2.0	100	48	4
SS-107	100' LT	20+50	3.4-3.9	A-6(14)	39	20	13.5	13.9	38.2	34.3	100	92	76
SS-108	100' LT	20+50	3.9-4.9	A-2-4(1)	17	NP	41.1	45.2	4.6	9.1	100	90	15
S-11	90' LT	21+50	0.0-3.0	A-2-4(1)	19	NP	43.6	37.3	10.0	9.1	96	73	24
S-12	90' LT	21+50	3.0-5.0	A-7-6(22)	44	24	4.0	10.3	51.4	34.3	100	99	89
S-13	90' LT	21+50	5.0-6.0	A-3(1)	19	NP	28.7	63.7	2.6	5.0	100	96	8

PROJECT REFERENCE NO.		SHEET NO.	
R-3601		33	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR A/W ACQUISITION			
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION			



SOIL TEST RESULTS													
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	% SAND	% FINE SAND	% SILT	% CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC
S-9A	90' LT	24+00	0.0-3.9	A-2-4(1)	14	NP	47.0	35.0	5.9	12.1	100	80	20
S-10	90' LT	24+00	3.9-6.0	A-2-4(1)	27	8	54.5	12.3	13.0	20.2	100	75	34

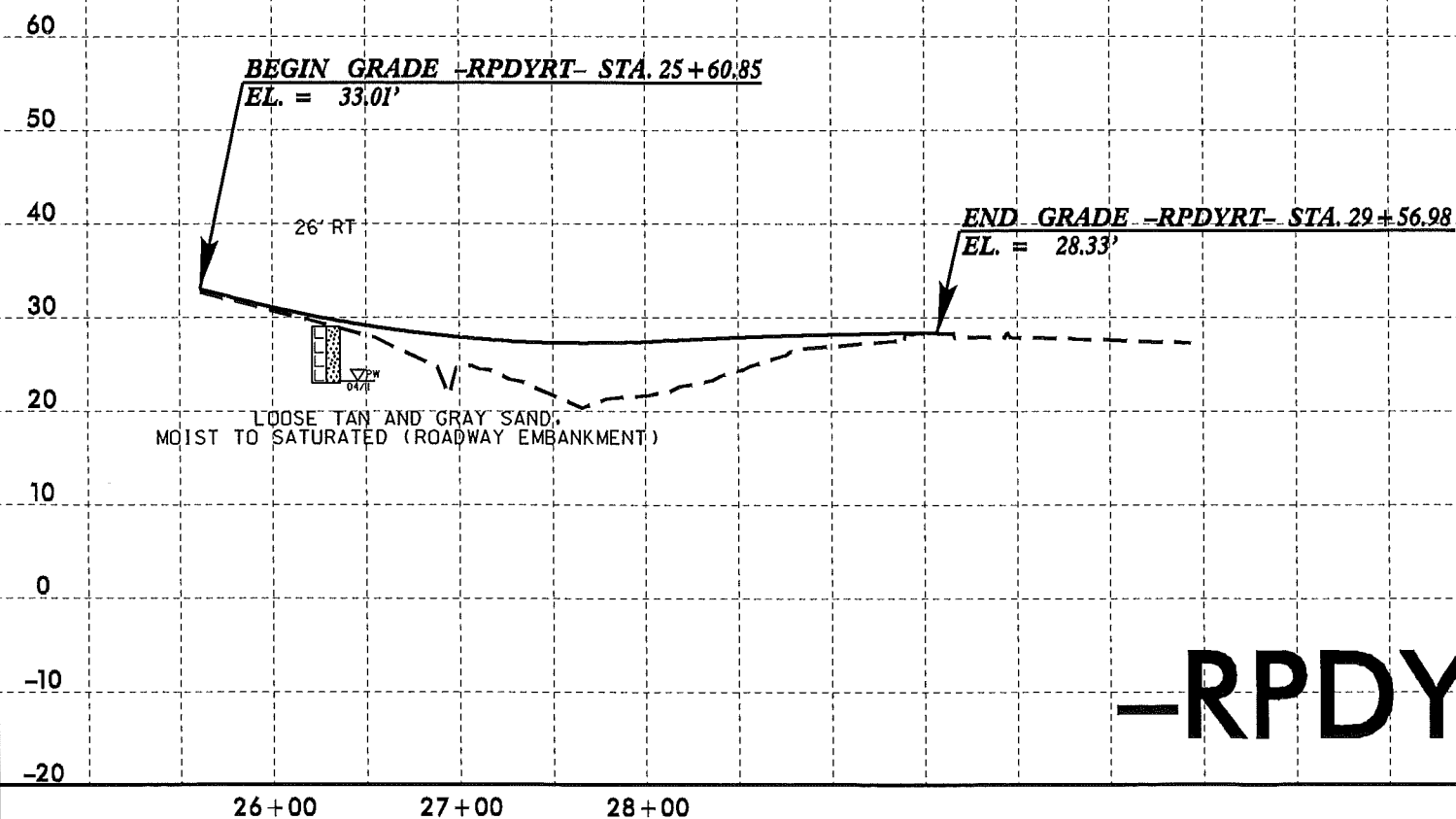
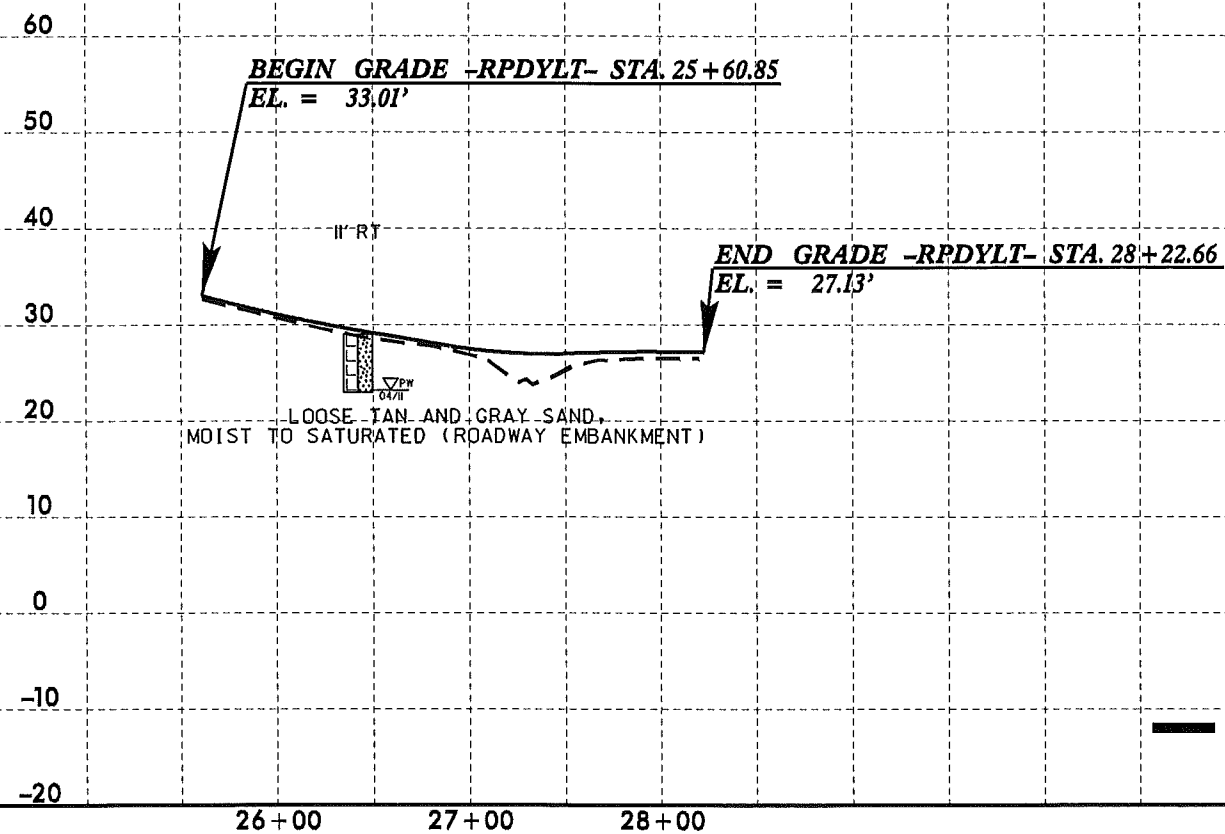
END GRADE -RPDY- STA. 25+60.85
EL. = 33.25'



-RPDY-

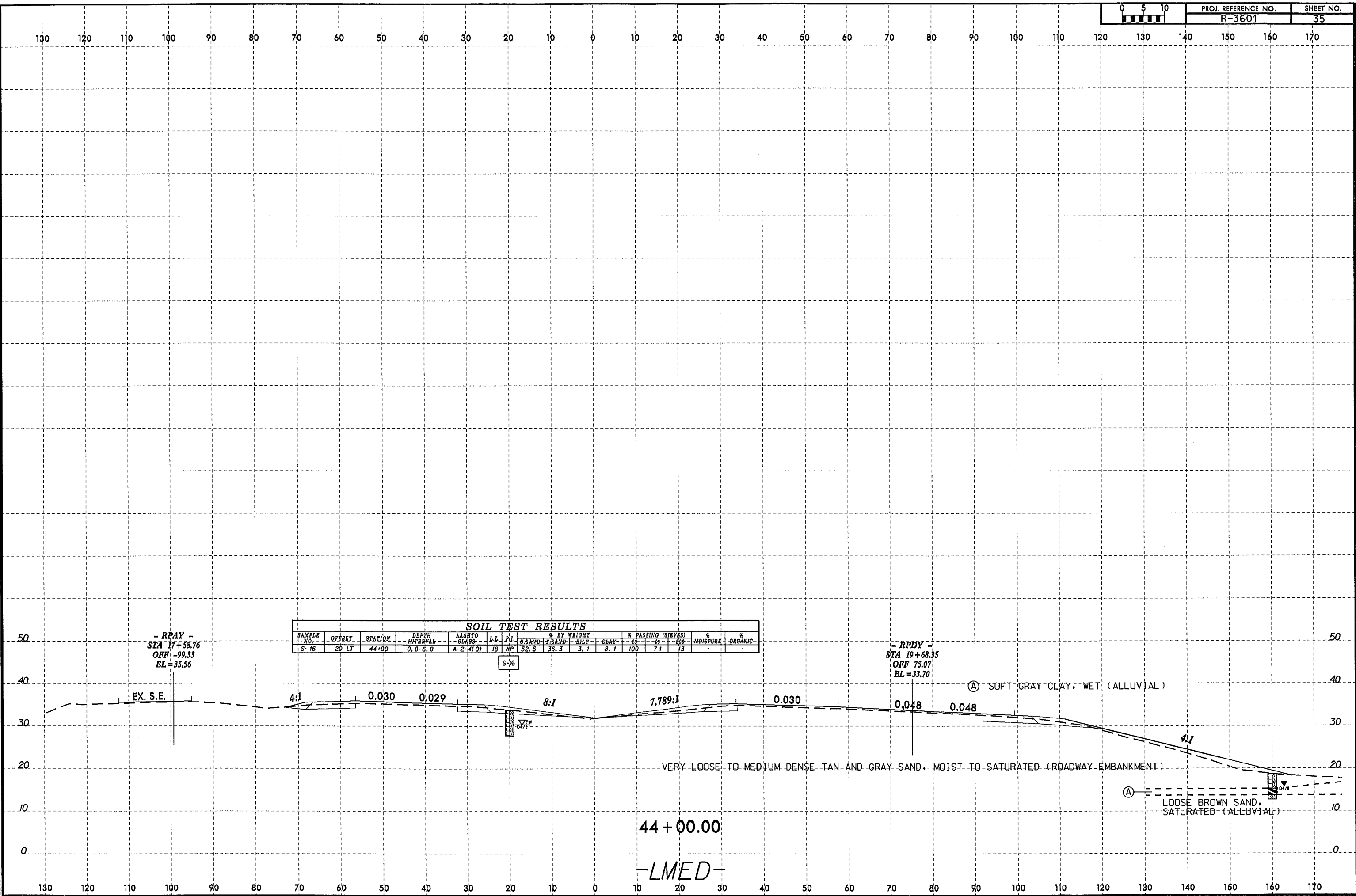
5/28/99

PROJECT REFERENCE NO.		SHEET NO.
R-3601		34
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER	
INCOMPLETE PLANS DO NOT USE FOR ACQUISITION		
PRELIMINARY PLANS DO NOT USE FOR CONSTRUCTION		



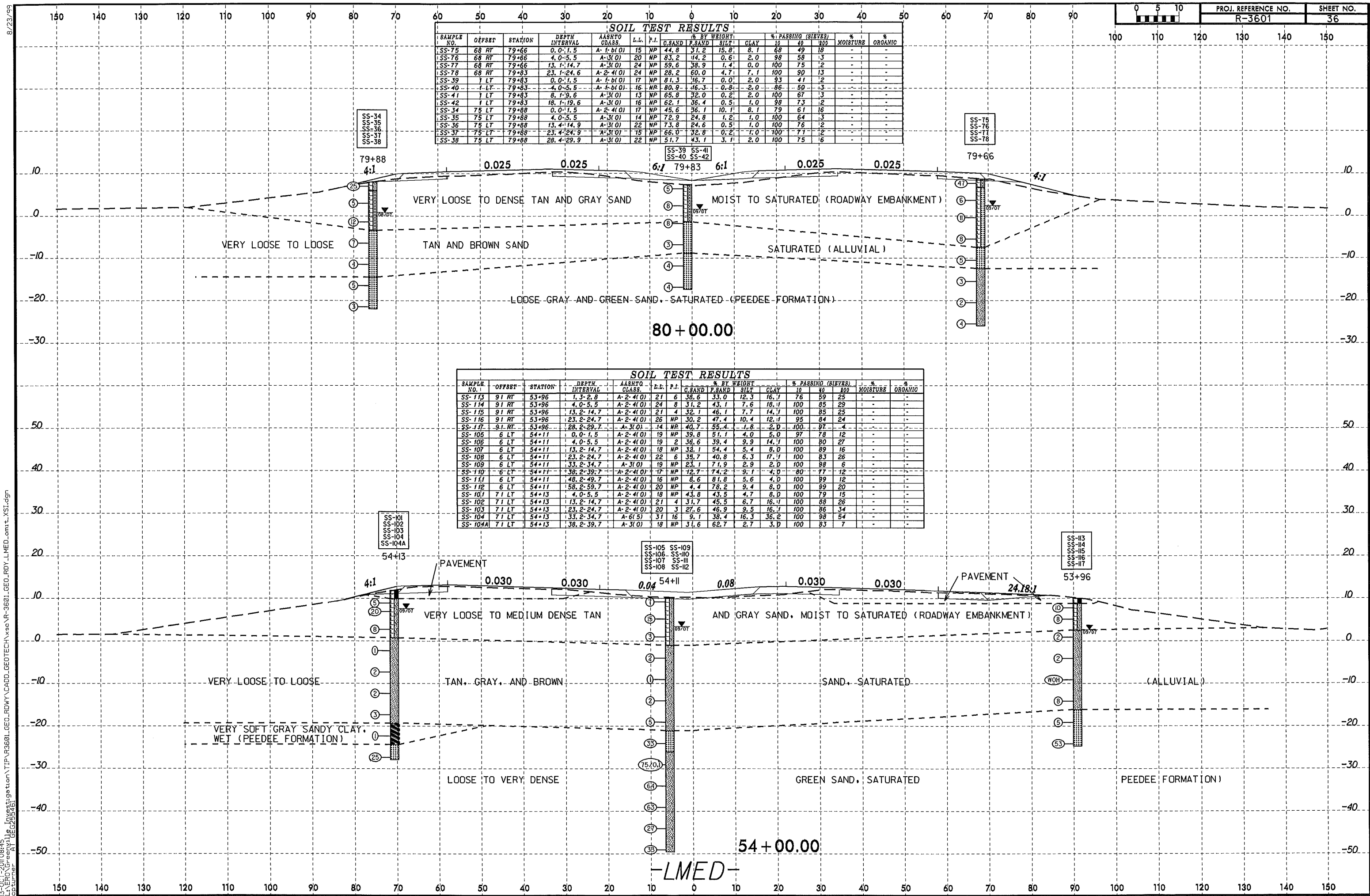
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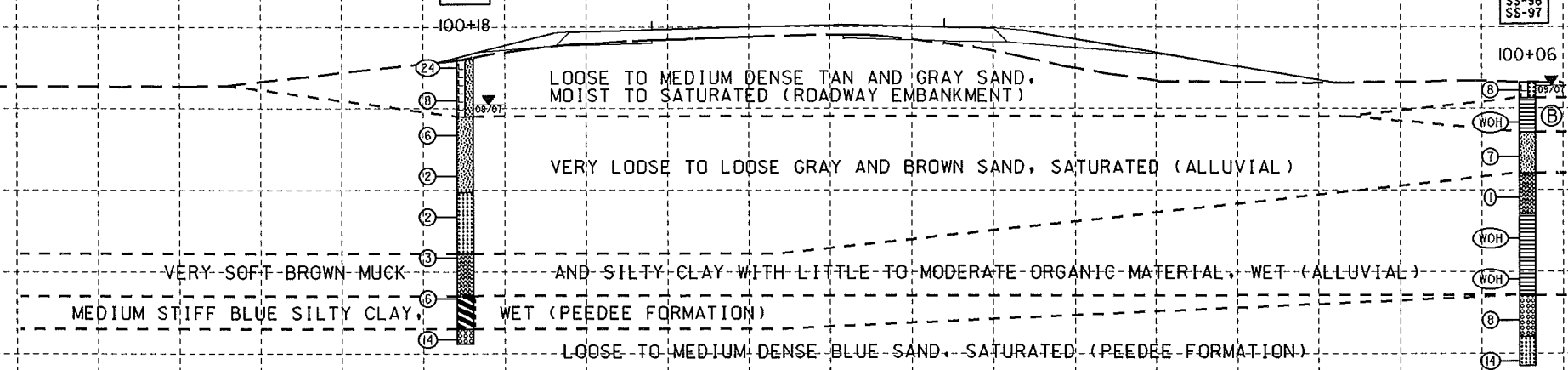
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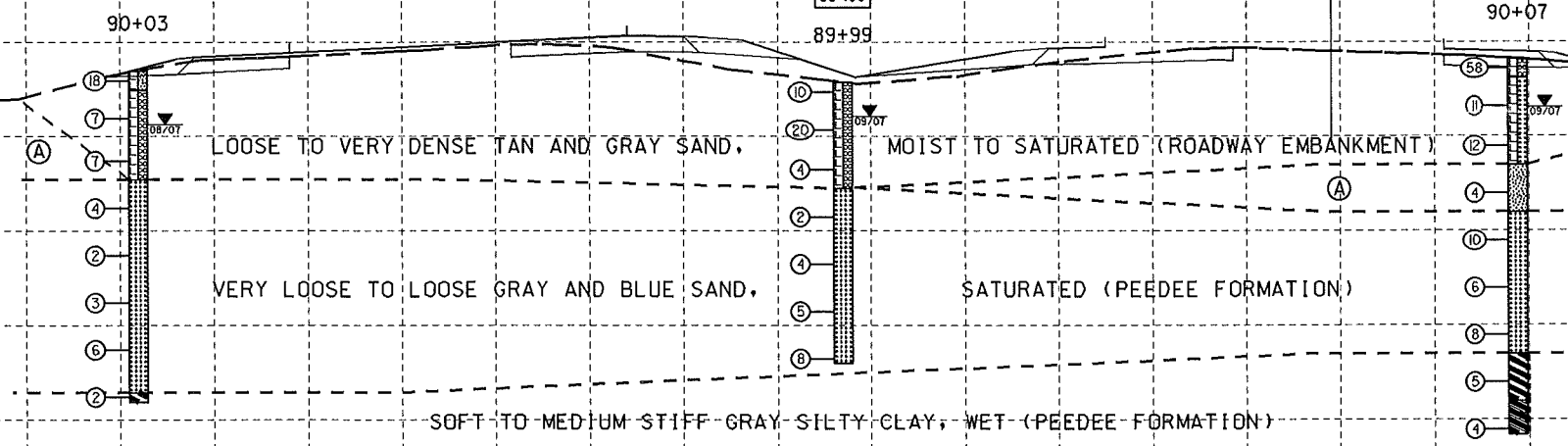
SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC	
SS-91	96 RT	100+06	0.0-1.5	A-3(0)	21	NP	76.9	20.5	2.6	0.0	95	61	3	
SS-92	96 RT	100+06	4.0-5.5	A-7-5(22)	79	34	31.2	7.3	37.1	24.3	100	82	62	174.2
SS-93	96 RT	100+06	18.2-9.7	A-2-4(0)	18	NP	68.4	20.3	5.3	6.1	100	65	12	
SS-94	96 RT	100+06	13.2-14.7	-	-	-	73.8	11.0	15.2	0.0	90	33	15	60.7
SS-95	96 RT	100+06	18.2-19.7	A-7-5(46)	76	40	1.6	8.1	37.5	52.7	100	99	94	121.3
SS-96	96 RT	100+06	23.2-24.7	A-7-6(31)	60	35	1.6	20.3	55.5	42.6	100	99	82	-
SS-97	96 RT	100+06	28.2-29.7	A-1-b(0)	24	NP	70.5	26.3	1.2	2.0	96	45	4	-
SS-22	35 LT	100+18	0.0-1.5	A-2-4(0)	20	NP	44.0	38.8	9.1	8.1	84	65	16	-
SS-23	35 LT	100+18	4.0-5.5	A-2-4(0)	14	NP	82.5	15.4	0.1	2.0	98	51	12	-
SS-24	35 LT	100+18	18.3-19.8	A-3(0)	20	NP	81.1	16.9	1.0	1.0	100	47	21	197.2
SS-25	35 LT	100+18	23.3-24.8	-	-	-	71.1	8.5	8.3	12.1	100	47	21	197.2
SS-26	35 LT	100+18	28.3-29.8	A-7-5(23)	66	23	11.1	5.5	43.0	40.4	95	88	80	94.6
SS-27	35 LT	100+18	33.3-34.8	A-1-b(0)	13	NP	86.1	11.5	0.4	2.0	100	31	3	-

(B) VERY SOFT BROWN SANDY CLAY WITH MODERATE ORGANIC MATERIAL, WET (ALLUVIAL)



SOIL TEST RESULTS														
SAMPLE NO.	OFFSET	STATION	DEPTH INTERVAL	AASHTO CLASS	L.L.	P.I.	C.SAND	F.SAND	SILT	CLAY	% PASSING (SIEVES)	% MOISTURE	% ORGANIC	
SS-98	47 RT	89+99	0.0-1.5	A-1-b(0)	21	NP	63.5	15.5	0.0	1.0	98	45	1	-
SS-100	47 RT	89+99	23.2-24.7	A-3(0)	24	NP	23.1	74.0	2.8	0.0	100	90	4	-
SS-28	28 LT	90+03	0.0-1.5	A-2-4(0)	18	NP	48.8	37.4	7.7	6.1	78	54	12	-
SS-29	28 LT	90+03	4.0-5.5	A-1-b(0)	17	NP	90.9	7.9	0.2	1.0	100	41	2	-
SS-30	28 LT	90+03	18.5-20.0	A-3(0)	23	NP	60.4	38.5	1.0	0.0	100	82	1	-
SS-32	28 LT	90+03	28.5-30.0	A-3(0)	19	NP	62.4	35.7	0.9	1.0	98	65	2	-
SS-33	28 LT	90+03	33.5-35.0	A-7-6(18)	43	22	5.5	18.9	41.2	34.5	100	97	81	-
SS-79	119 RT	90+07	0.0-1.5	A-1-b(0)	14	NP	41.8	31.4	18.7	8.1	57	41	17	-
SS-80	119 RT	90+07	4.0-5.5	A-3(0)	20	NP	83.6	14.0	2.4	0.0	100	55	3	-
SS-81	119 RT	90+07	13.2-14.7	A-2-4(0)	28	NP	71.2	14.2	10.5	4.1	100	55	15	-
SS-82	119 RT	90+07	18.2-19.7	A-3(0)	24	NP	41.8	55.1	2.1	1.0	100	81	4	-
SS-83	119 RT	90+07	28.2-29.7	A-3(0)	21	NP	47.6	50.7	1.7	0.0	100	77	2	-
SS-84	119 RT	90+07	33.2-34.7	A-7-5(22)	81	26	21.9	13.0	44.8	20.3	100	86	68	21.8
SS-84A	119 RT	90+07	38.2-39.7	A-6(7)	33	14	3.4	39.6	28.6	28.4	100	98	68	-

(A) LOOSE BROWN SAND, SATURATED (ALLUVIAL)



- LRT -
STA 90+04.04
OFF 98.98
EL=9.74

SS-79
SS-80
SS-81
SS-82
SS-83
SS-84
SS-84A

90+00.00

-LLT-

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