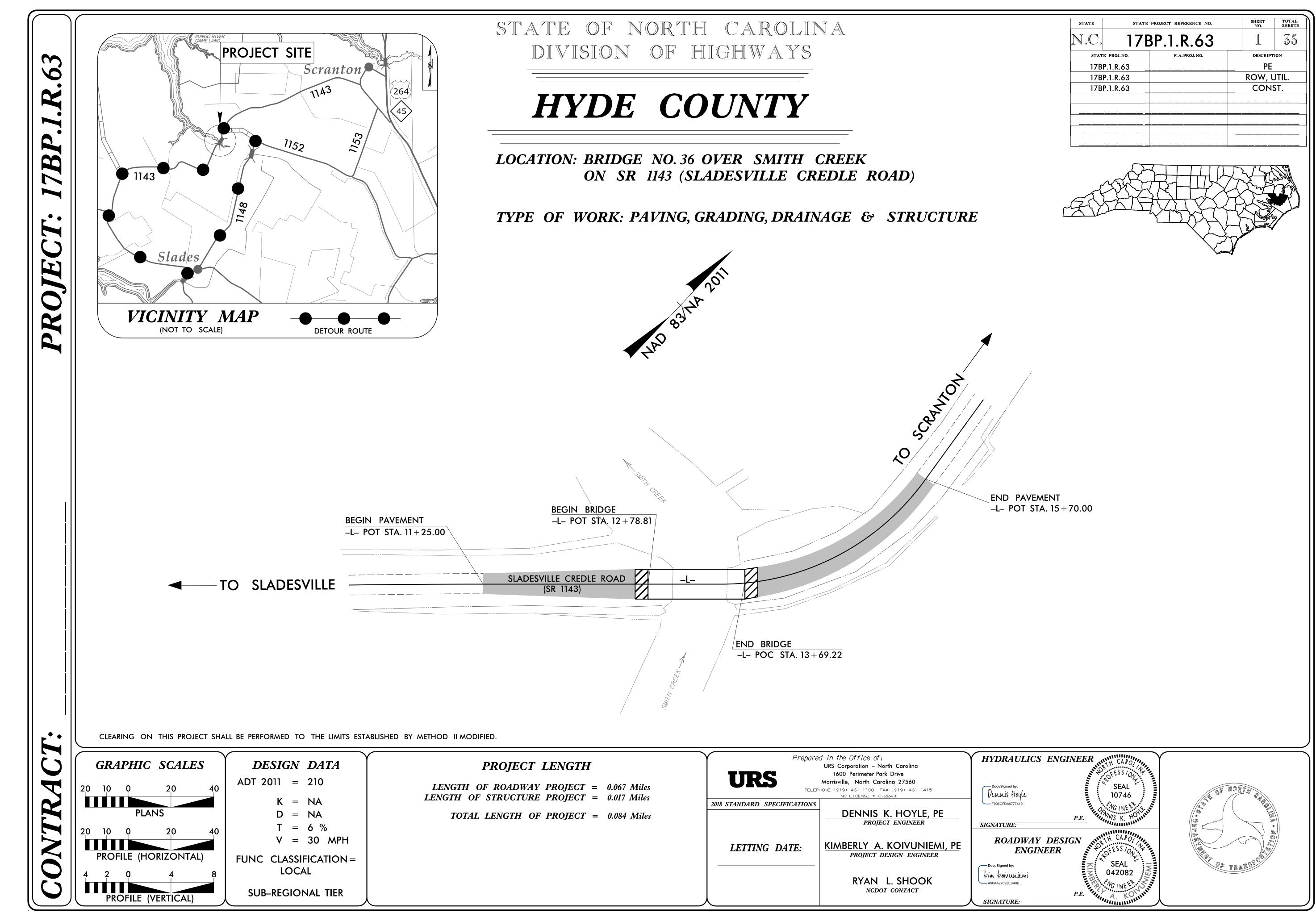
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This file or an individual page shall not be considered a certified document.



 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.1.R.63
 1-A

INDEX OF SHEETS

SHEET NUMBER

TITLE

	··· -
1	TITLE SHEET
1-A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS
1-B	CONVENTIONAL SYMBOLS
2	TYPICAL SECTIONS, PAVEMENT SCHEDULE , AND WEDGING DETAIL
2-A	DETAIL FOR STRUCTURE ANCHOR UNIT, TYPE III
2C-1	DETAIL FOR CONCRETE FLUME IN 2'-6" C&G
3-A	SUMMARY OF DRAINAGE QUANTITIES, SUMMARY OF GUARDRAIL,
	EARTHWORK SUMMARY, REMOVAL OF EXISTING ASPHALT PAVEMENT SUMMARY,
	AND BREAKING OF EXISTING ASPHALT PAVEMENT SUMMARY
4	PLAN SHEET & PROFILE
TMP-1	TRAFFIC MANAGEMENT PLAN
EC-1 THRU EC-4, RF-1	EROSION CONTROL PLANS
X-1 THRU X-4	CROSS-SECTIONS
S-1 THRU S-17	STRUCTURE PLANS
N/A	STRUCTURE STANDARD NOTES

GENERAL NOTES

GENERAL NOTES: 2018 SPECIFICATIONS EFFECTIVE: 01-16-2018 REVISED:

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II (MODIFIED).

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT IS CENTURY LINK (TELEPHONE) AND TIDELAND ELECTRIC (POWER LINE).

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

STANDARD DRAWINGS

2018 ROADWAY ENGLISH STANDARD DRAWINGS

EFF. 01-16-2018 REV.

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE

DIVISION 2 - EARTHWORK

00.02 Method of Clearing - Method II

25.02 Guide for Grading Subgrade - Secondary and Local

Method of Obtaining Superelevation - Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation

DIVISION 4 - MAJOR STRUCTURES

422.02 Reinforced Bridge Approach Fills - Type II Modiefied Approach Fill

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

840.00 Concrete Base Pad for Drainage Structures

840.20 Frames and Wide Slot Flat Grates

840.35 Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates

340.46 Traffic Bearing Precast Drainage Structure

846.04 Drop Inlet Installation in Shoulder Berm Gutter

862.01 Guardrail Placement

862.02 Guardrail Installation

846.04 Drop Inlet Installation in Shoulder Berm Gutter

PROJECT REFERENCE NO.	SHEET
17BP.I.R.63	/-

*S.U.E. = Subsurface Utility Engineering

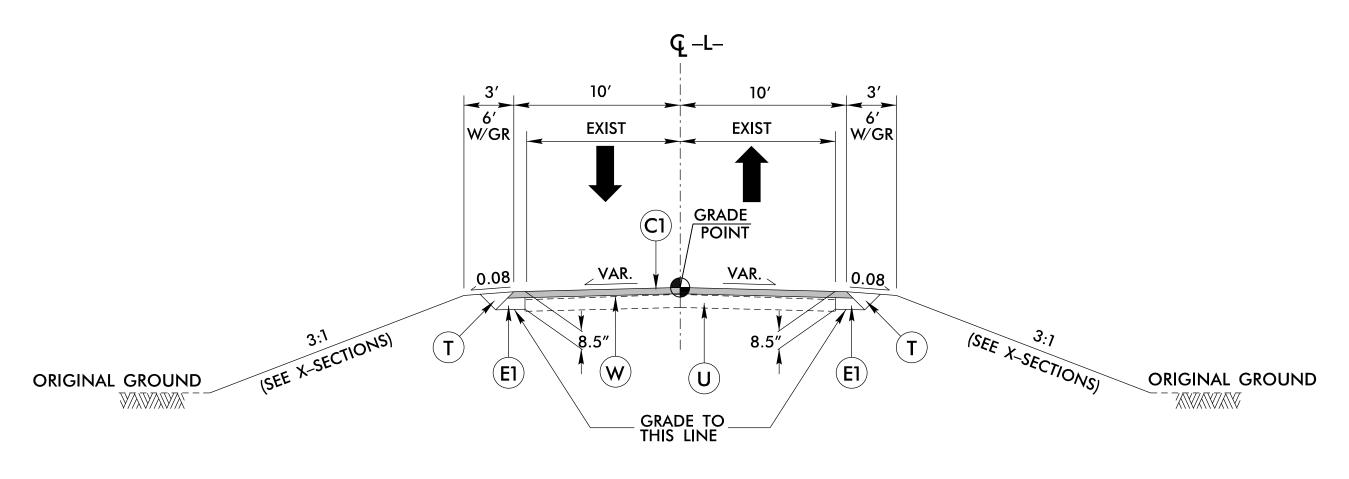
CONVENTIONAL PLAN SHEET SYMBOLS

State Line ————————————————————————————————————	
County Line	
ownship Line	
City Line	
Reservation Line ————————————————————————————————————	
Property Line	
existing Iron Pin	
Property Corner	
Property Monument	ECM
Parcel/Sequence Number ————————————————————————————————————	— (I23)
Existing Fence Line	
Proposed Woven Wire Fence	—
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	→
existing Wetland Boundary	
Proposed Wetland Boundary	
Existing Endangered Animal Boundary ———	
Existing Endangered Plant Boundary	
Known Soil Contamination: Area or Site ——	
Potential Soil Contamination: Area or Site —	-
BUILDINGS AND OTHER CULT	
Gas Pump Vent or U/G Tank Cap	
Sign ————————————————————————————————————	_
A / - II	\circ
Vell	
Small Mine	-
Small Mine Foundation	→→
Small Mine Foundation Area Outline	→→□□
Small Mine Foundation Area Outline Cemetery	— ★— □— □† □
Small Mine Foundation Area Outline	— ★— □— □† □
Small Mine Foundation Area Outline Cemetery	→→→†→↓
Small Mine Foundation Area Outline Cemetery Suilding Church	
Small Mine Foundation Area Outline Cemetery Suilding School	
Small Mine Foundation Area Outline Cemetery Suilding School Church Dam	
Small Mine Foundation Area Outline Cemetery Suilding School Church Dam HYDROLOGY:	
Small Mine Foundation Area Outline Cemetery Building School Church Dam FYDROLOGY: Stream or Body of Water	
Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir	
Small Mine Foundation Area Outline Cemetery Building School Church Dam FYYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Urisdictional Stream	-
Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir urisdictional Stream Suffer Zone 1	—
Small Mine Foundation Area Outline Cemetery Building Church Church Church Church Oam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir urisdictional Stream Buffer Zone 1 Buffer Zone 2	—
Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir urisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow	—
Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir urisdictional Stream Buffer Zone 1 Suffer Zone 2 Flow Arrow Disappearing Stream	—
Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir urisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream	→
Small Mine Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir urisdictional Stream Buffer Zone 1 Suffer Zone 2 Flow Arrow Disappearing Stream	→ ★

RAILROADS:			
Standard Gauge	CSX TRANSPORTATION	Orchard —	유 유 유 · 유
RR Signal Milepost ————————————————————————————————————	. O MILEPOST 35		Vineyard
Switch —	. SWITCH	Vineyard ————————————————————————————————————	Villeydi d
RR Abandoned		EXISTING STRUCTURES:	
RR Dismantled		MAJOR:	
RIGHT OF WAY:		Bridge, Tunnel or Box Culvert	CONC
Baseline Control Point	•	Bridge Wing Wall, Head Wall and End Wall -	- CONC WW
Existing Right of Way Marker	\triangle	MINOR:	
Existing Right of Way Line		Head and End Wall	CONC HW
Proposed Right of Way Line	$\frac{R}{W}$	Pipe Culvert	
Proposed Right of Way Line with Iron Pin and Cap Marker	$-\frac{R}{W}$	Footbridge ————————————————————————————————————	>
Proposed Right of Way Line with	R	Drainage Box: Catch Basin, DI or JB	СВ
Concrete or Granite R/W Marker Proposed Control of Access Line with		Paved Ditch Gutter	
Concrete C/A Marker		Storm Sewer Manhole ————————————————————————————————————	(\$)
Existing Control of Access		Storm Sewer ———————————————————————————————————	ss
Proposed Control of Access			
Existing Easement Line	——E——	UTILITIES:	
Proposed Temporary Construction Easement –	Е	POWER:	1
Proposed Temporary Drainage Easement —	TDE	Existing Power Pole ————————————————————————————————————	•
Proposed Permanent Drainage Easement ——	PDE	Proposed Power Pole ————————————————————————————————————	O 1
Proposed Permanent Drainage / Utility Easemer	nt —— DUE——	Existing Joint Use Pole	I
Proposed Permanent Utility Easement ———	PUE	Proposed Joint Use Pole	-0-
Proposed Temporary Utility Easement ———	TUE	Power Manhole	P
Proposed Aerial Utility Easement ————	AUE	Power Line Tower	
Proposed Permanent Easement with		Power Transformer	
Iron Pin and Cap Marker		U/G Power Cable Hand Hole	
ROADS AND RELATED FEATURE	ES:	H-Frame Pole	
Existing Edge of Pavement		Recorded U/G Power Line	·
Existing Curb		Designated U/G Power Line (S.U.E.*)	— — — P— — — -
Proposed Slope Stakes Cut		TELEPHONE:	
Proposed Slope Stakes Fill	F		_
Proposed Curb Ramp		Existing Telephone Pole	<u> </u>
Existing Metal Guardrail		Proposed Telephone Pole	
Proposed Guardrail ————	<u> </u>	Telephone Manhole	
Existing Cable Guiderail		Telephone Booth	
Proposed Cable Guiderail		Telephone Pedestal	
Equality Symbol	lacktriangle	Telephone Cell Tower	
Pavement Removal —————		U/G Telephone Cable Hand Hole ———	
VEGETATION:		Recorded U/G Telephone Cable	
Single Tree		Designated U/G Telephone Cable (S.U.E.*)	
Single Shrub		Recorded U/G Telephone Conduit	
Hedge ———————————————————————————————————		Designated U/G Telephone Conduit (S.U.E.*)	
Woods Line		Recorded U/G Fiber Optics Cable ————	T FO

Orchard —	상 상 상 상
Vineyard ————————————————————————————————————	Vineyard
EVICTING CTDINGTIDES.	
EXISTING STRUCTURES:	
MAJOR: Pridge Tunnel or Pay Culvert	CONC
Bridge, Tunnel or Box Culvert	
Bridge Wing Wall, Head Wall and End Wall –	J COINC WW (
MINOR: Head and End Wall ——————————————————————————————————	CONC HW
Pipe Culvert	
Footbridge >	
	СВ
Drainage Box: Catch Basin, DI or JB Paved Ditch Gutter	
Storm Sewer Manhole ————	(\$)
Storm Sewer Mannole	_
Sionii Sewei	
UTILITIES:	
POWER:	
Existing Power Pole	•
Proposed Power Pole	6
Existing Joint Use Pole	
Proposed Joint Use Pole	-6-
Power Manhole	P
Power Line Tower	
Power Transformer ———————————————————————————————————	\square
U/G Power Cable Hand Hole	
H-Frame Pole	•—•
Recorded U/G Power Line	P
Designated U/G Power Line (S.U.E.*)	P
TELEPHONE:	
Existing Telephone Pole	-● -
Proposed Telephone Pole	- O-
Telephone Manhole	
Telephone Booth	3
Telephone Pedestal	I
Telephone Cell Tower	<u>,</u>
U/G Telephone Cable Hand Hole	Η _Η
Recorded U/G Telephone Cable	
Designated U/G Telephone Cable (S.U.E.*)	
Recorded U/G Telephone Conduit	
Designated U/G Telephone Conduit (S.U.E.*)	TC

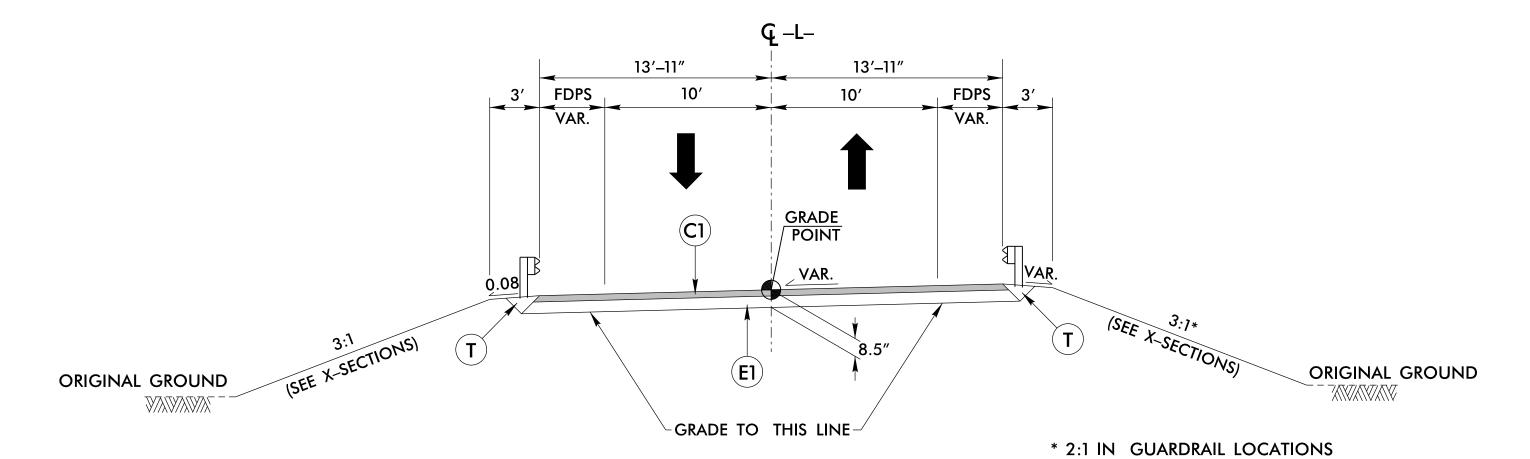
WATER:	
Water Manhole	W
Water Meter	
Water Valve	\otimes
Water Hydrant	÷
Recorded U/G Water Line	w
Designated U/G Water Line (S.U.E.*)	w
Above Ground Water Line	A/G Water
TV:	
TV Satellite Dish	K
TV Pedestal ————————————————————————————————————	C
TV Tower —	\bigotimes
U/G TV Cable Hand Hole	H _H
Recorded U/G TV Cable —	ту
Designated U/G TV Cable (S.U.E.*)	
Recorded U/G Fiber Optic Cable ————	TV F0
Designated U/G Fiber Optic Cable (S.U.E.*)—	- — — TV FO— — —
GAS:	
Gas Valve	\Diamond
Gas Meter —	\Diamond
Recorded U/G Gas Line	G
Designated U/G Gas Line (S.U.E.*)	— — — c — — –
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	
Sanitary Sewer Cleanout ————————————————————————————————————	(+)
U/G Sanitary Sewer Line ——————	ss
Above Ground Sanitary Sewer —	A/G Sanitary Sewer
Recorded SS Forced Main Line	FSS
Designated SS Forced Main Line (S.U.E.*) —	— — — FSS— — — —
MISCELLANEOUS:	
Utility Pole —	•
Utility Pole with Base ——————	
Utility Located Object ——————	\odot
Utility Traffic Signal Box —	S
Utility Unknown U/G Line —————	
U/G Tank; Water, Gas, Oil ———————————————————————————————————	
Underground Storage Tank, Approx. Loc. ——	(UST)
A/G Tank; Water, Gas, Oil —————	
Geoenvironmental Boring	*
U/G Test Hole (S.U.E.*)	
Abandoned According to Utility Records —	AATUR
End of Information ————————————————————————————————————	E.O.I.



TYPICAL SECTION NO. 1

USE TYPICAL SECTION NO. 1

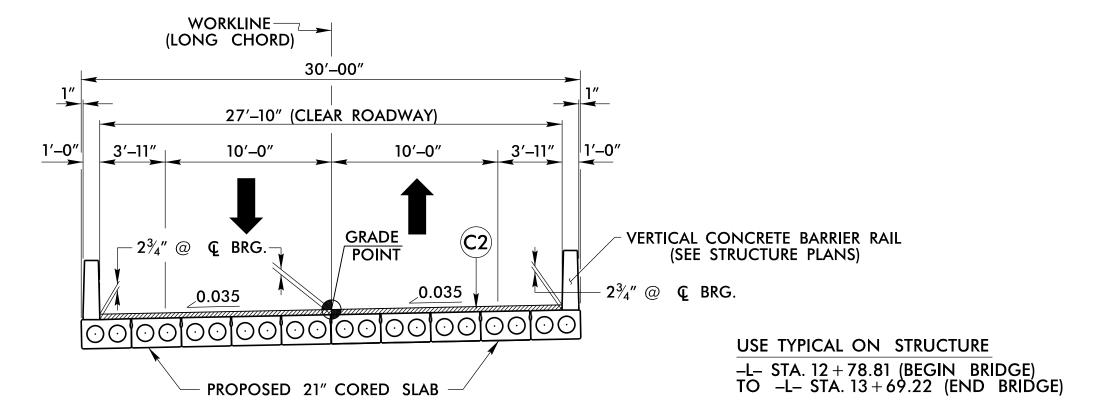
-L- STA. 11+25.00 TO -L- STA. 12+00.00
-L- STA. 15+00.00 TO -L- STA. 15+70.00



TYPICAL SECTION NO. 2

USE TYPICAL SECTION NO. 2

-L- STA. 12+00.00 TO -L- STA. 12+78.81 (BEGIN BRIDGE)
-L- STA. 13+69.22 (END BRIDGE) TO -L- STA. 15+00.00



TYPICAL SECTION ON STRUCTURE (SEE STRUCTURE PLANS)

	PAVEMENT SCHEDULE
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT TO EXCEED 1.5" IN DEPTH.
E1	PROP. APPROX. 5.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 627 LBS. PER SQ. YD.
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.OC, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH.
Т	EARTH MATERIAL.
U	EXISTING PAVEMENT.
V	TIE-IN MILLING.
W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL)

PROJECT REFERENCE NO.

17BP.I.R.63

2

HYDE COUNTY BRIDGE NO. 470036

ROADWAY DESIGN PAVEMENT DESIGN ENGINEER

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Prepared by

URS Corporation - North Carolina

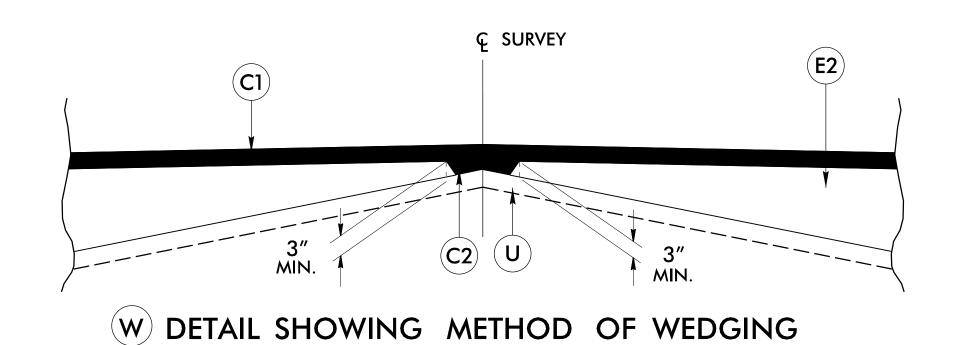
1600 Perimeter Park Drive, Suite 400

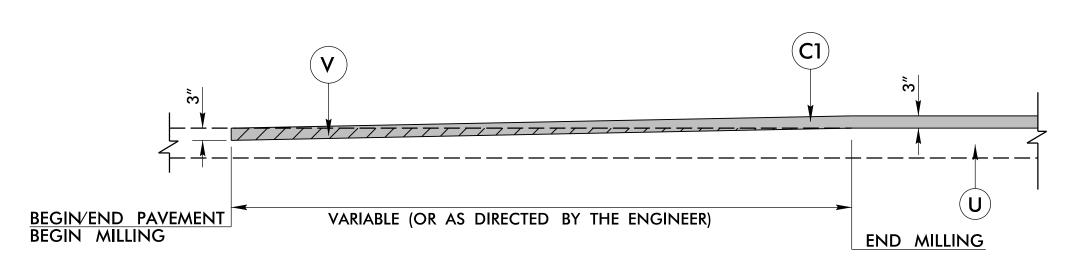
Morrisville, NC 27560

PHONE(919)461-1100 FAX(919)461-1415

NC LIC. # C-2243

NOTE: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.





V TIE-IN MILLING DETAIL

NOTE: END MILLING WHERE PROPOSED GRADE
IS 3" ABOVE EXISTING PAVEMENT

Changed C1 and C2 to S9.5B. Changed E1 and E2 to B25.0C.

Toster of the state of the stat

PROJECT REFERENCE NO. SHEET NO. 17BP.1.R.63 2-A

BEAM POST

THRIE

WTR SECTION ELEVATION VIEW

1" DIA. HOLES (TYP. FOR ANCHOR BOLTS

THRIE-BEAM SECTION

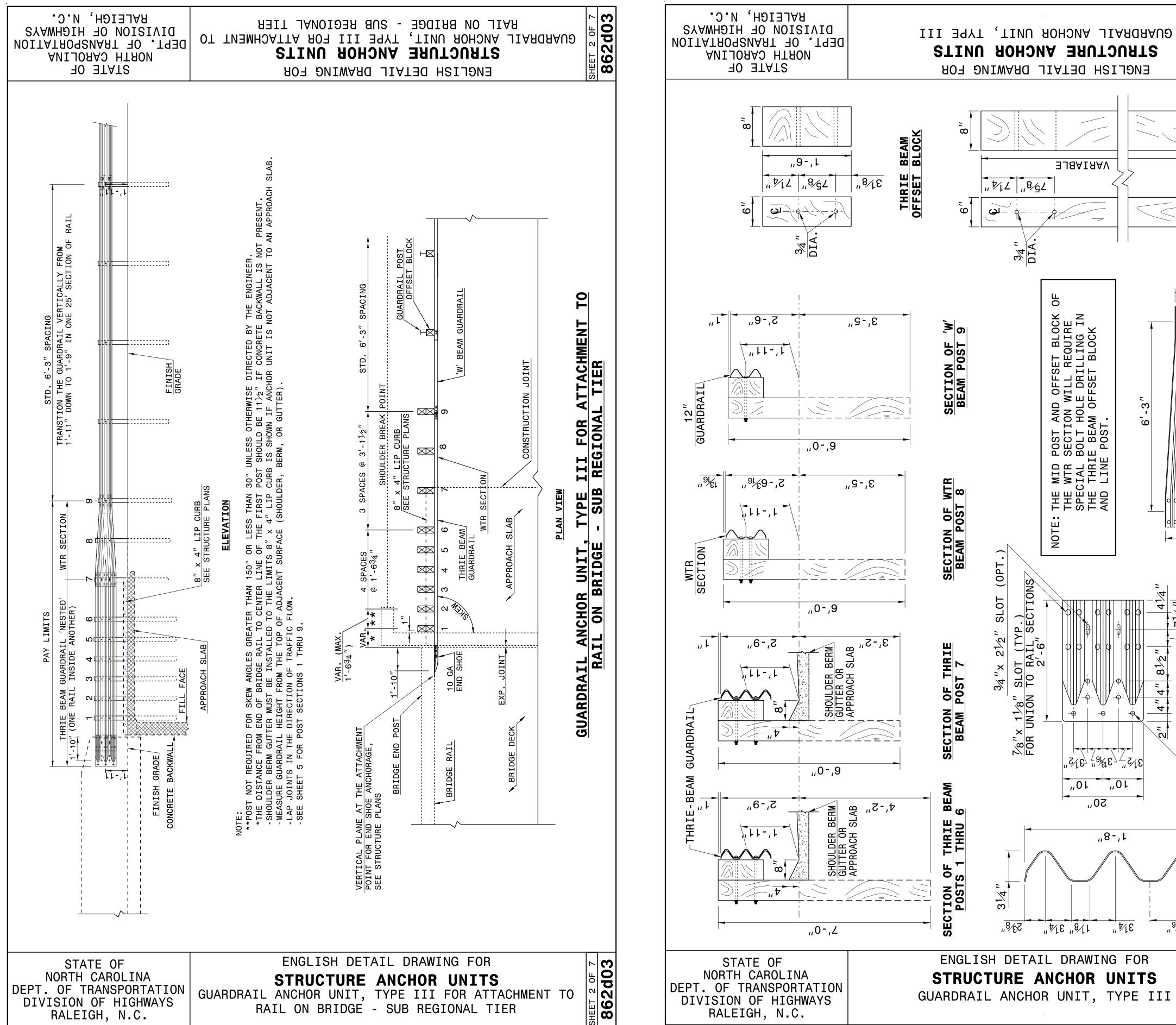
SEAL 022966 Joel Howerton

7/31/2015

CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J HOWERTON	DATE: 06-22-12
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
ETLE SPEC .	



STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO
RAIL ON BRIDGE - SUB REGIONAL TIER

STRUCTURE ANCHOR UNITS GUARDRAIL ANCHOR UNIT, TYPE III

 COMPUTED BY:
 JBJ
 DATE:
 07-08-2015

 CHECKED BY:
 LHJ
 DATE:
 07-09-2015

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO. 17BP.1.R.63 3-A

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

LINE & STATION	SET Z **	SET	SET	SET	SET	SET	SET	SET	DRAINAGE PIPE (RCP, CSP, CAAP, HDPE, or PVC)	C.S. PIPE R.C. PIPE CLASS III		R.C. PIPE CLASS IV	ENDWALLS 338.01 OR STD. 838.11 S NOTED OTHERWISE) FORCED ENDWALLS	DRAINAGE STRUCTURE	QUANTITIES FOR DRAINAGE STRUCTURES NOTE: TOTAL LIN. FT. FOR PAY QUANTITY SHALL BE A+(1.3 X B) 8. Q	STD. 852.06 STD. 852.06 CONCRETE TRANSITIONAL SECTION SETTIONAL SECTION STD. 840.16		10.18 OR 840.27 E W/2 GRATES STD. 840.20		tUG, С.Ү. STD. 840.71	ABBREVIATIONS C.A.A. CORRUGATED ALUMINIUM ALLOY C.B. CATCH BASIN C.S. CORRUGATED STEEL D.I. DROP INLET G.D.I. GRATED DROP INLET H.D.P.E HIGH DENSITY POLYETHYLENE J.B. JUNCTION BOX M.H. MANHOLE N.S. NARROW SLOT P.V.C. POLYVINYL CHLORIDE
SIZE THICKNESS OR GAUGE	OFFS	OM STRUC EVATION ELEVATION ELEVATION MUM REQUIRED	12" 15" 18" 24" 30" 36" UN LON LON LON LON LON LON LON LON LON LO	17 USE H 64 64 64 79 79	12" 15" 18" 24" 30" 60"	12" 15" 18" NI	E DRAIN PIPE STD. 838 (UNLESS	MASONRY	THRU 10' AND ABOVE 3. 840.01 OR S	GRATE TYPE	TD. 852.04 OR TD. 852.05 TD. 840.14 OR RAME AND GRATI	TYPE "B" STD. 84 (W.S. FLAT) FRAMI	CRETE FLUME	C. & BRICK PIPE P	R.C. REINFORCED CONCRETE T.B.D.I. TRAFFIC BEARING DROP INLET T.B.J.B. TRAFFIC BEARING JUNCTION BOX W.S. WIDE SLOT						
		TOP EL INVERT NVERT NVERT NVERT NVERT NVERT N N N N N N N N N N N N N N N N N N		z z 		15" SID	<u>81</u> Cγ Cγ	CY	EACH IIN. FT. IIN. FT. C. B. ST.	E F G	D.I. S1	G.D.I.	CONCOO	O LIN. FT.	REMARKS						
11 + 90.00	14.0 LT												1		REFER TO DETAIL 2C-1 CONCRETE FLUME						
TOTAL													1								

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.
G = GATING IMPACT ATTENUATOR TYPE 350

NG = NON-GATING IMPACT ATTENUATOR TYPE 350

GUARDRAIL SUMMARY

NG = NO	N-GATING IMPACT	ATTENUATOR TYPE 350	0																					
SURVEY	BEG. STA.	END STA.	LOCATION		LENGTH		WARRA	WARRANT POINT		"N" TOTAL	FLARE	FLARE LENGTH		W		ANCHORS				IMPACT ATTENUATOR		REMOVE	REMOVE AND STOCKPILE	DEV.A.DV.C
LINE	BEG. SIA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.		TRAILING END	APPROACH END	TRAILING END	XI MOD	TYPE GREU III TL-2	M-350	XIII CAT-1	VI BIC	AT-1 EA G NG		REMOVE EXISTING GUARDRAIL	STOCKPILE EXISTING GUARDRAIL	REMARKS	
-L-	11 + 53.81	12 + 78.81	LT	125.00′			BRIDGE		VAR.	VAR.		25.00′		1.00′		1 1								
-L-	12+03.81	12 + 78.81	RT	75.00′				BRIDGE	VAR.	VAR.	25.00′		1.00′			1 1								
-L-	13 + 70.49	14 + 45.49	LT	75.00′				BRIDGE	VAR.	VAR.	25.00′		1.00′			1 1								
-L-	13+67.92	14 + 42.92	RT	75.00′			BRIDGE		VAR.	VAR.		25.00′		1.00′		1 1								
_			TOTAL	350.00′												4 4								
		DEDUCT	TION FOR ANCHORS	–175.00 ′												DEDUCTION FO	OR ANCH	ORS:						
			PROJECT TOTAL	175.00′											(GREU TL-2) 4 @ 25' = 100'									
			SAY	175.00′											(TYPE IIII) 4 @ 18.75' = 75'									
		ADDITIONAL GUARDRA	AIL POST = 2 EACH												TOTAL DEDUCTIONS = 175'									

SUMMARY OF EARTHWORK

IN CUBIC YARDS

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT+%	BORROW	WASTE
-L-					
11+25.00 TO 12+78.81 (BEGIN BRIDGE)	2		185	183	
13+69.22 (END BRIDGE) TO 15+70.00	1		299	298	
SUB-TOTAL	3		484	481	
LOSS DUE TO CLEARING & GRUBBING					
PROJECT TOTAL	3		484	481	
5% TO REPLACE TOP SOIL ON BORROW PIT				25	
GRAND TOTAL	3			506	
SAY	10			600	

REMOVAL OF EXISTING ASPHALT PAVEMENT

LINE	BEGIN STATION	END STATION	LOCATION	SQ. YD.
-L-	12 + 00.00	12 + 79.00	CL	162.49
-L-	13+69.03	13 + 89.00	CL	42.12
-L-	14 + 50.00	15 + 00.00	CL	111.86
			TOTAL	316.47
			SAY	320

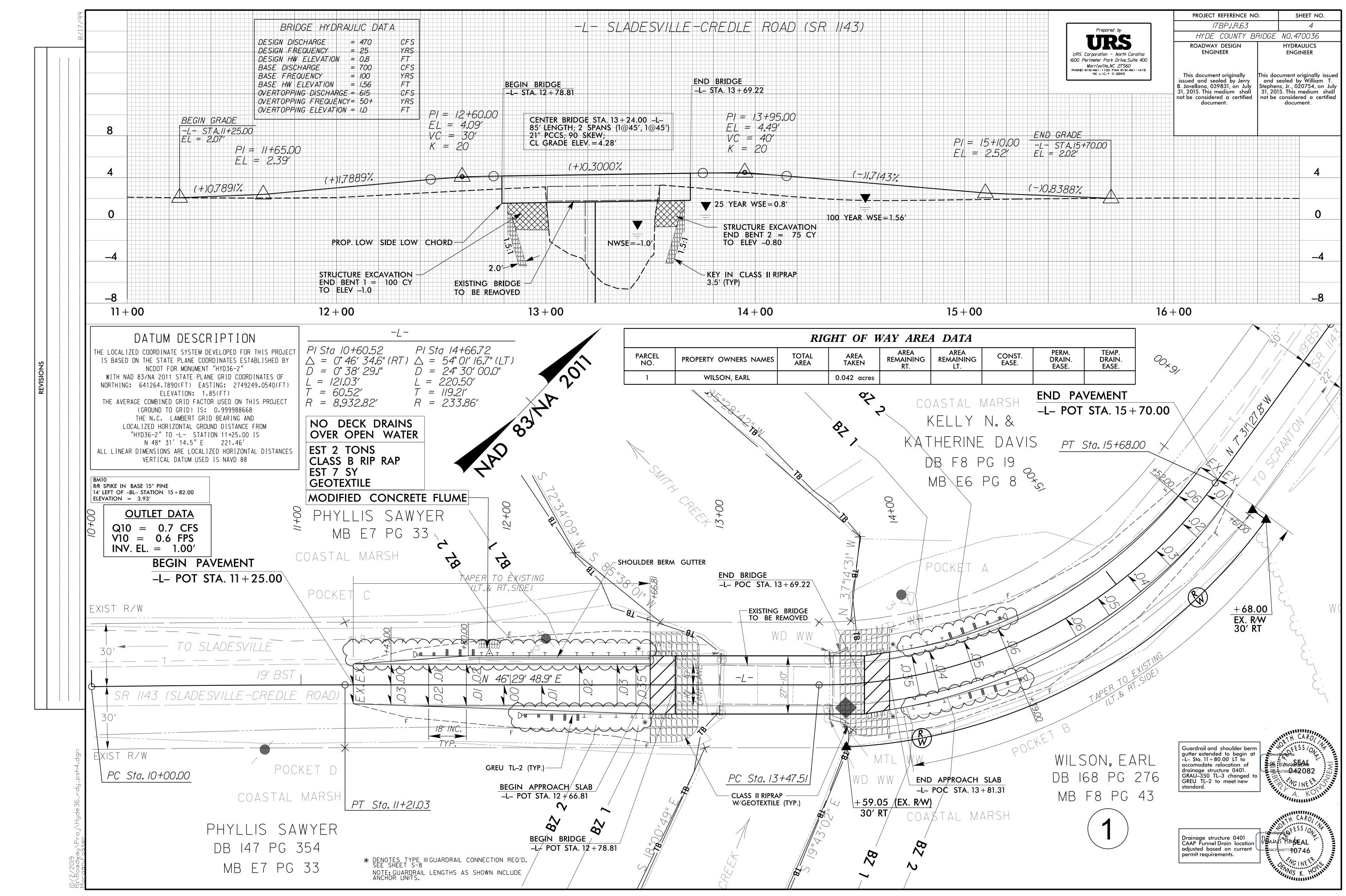
BREAKING OF EXISTING ASPHALT PAVEMENT

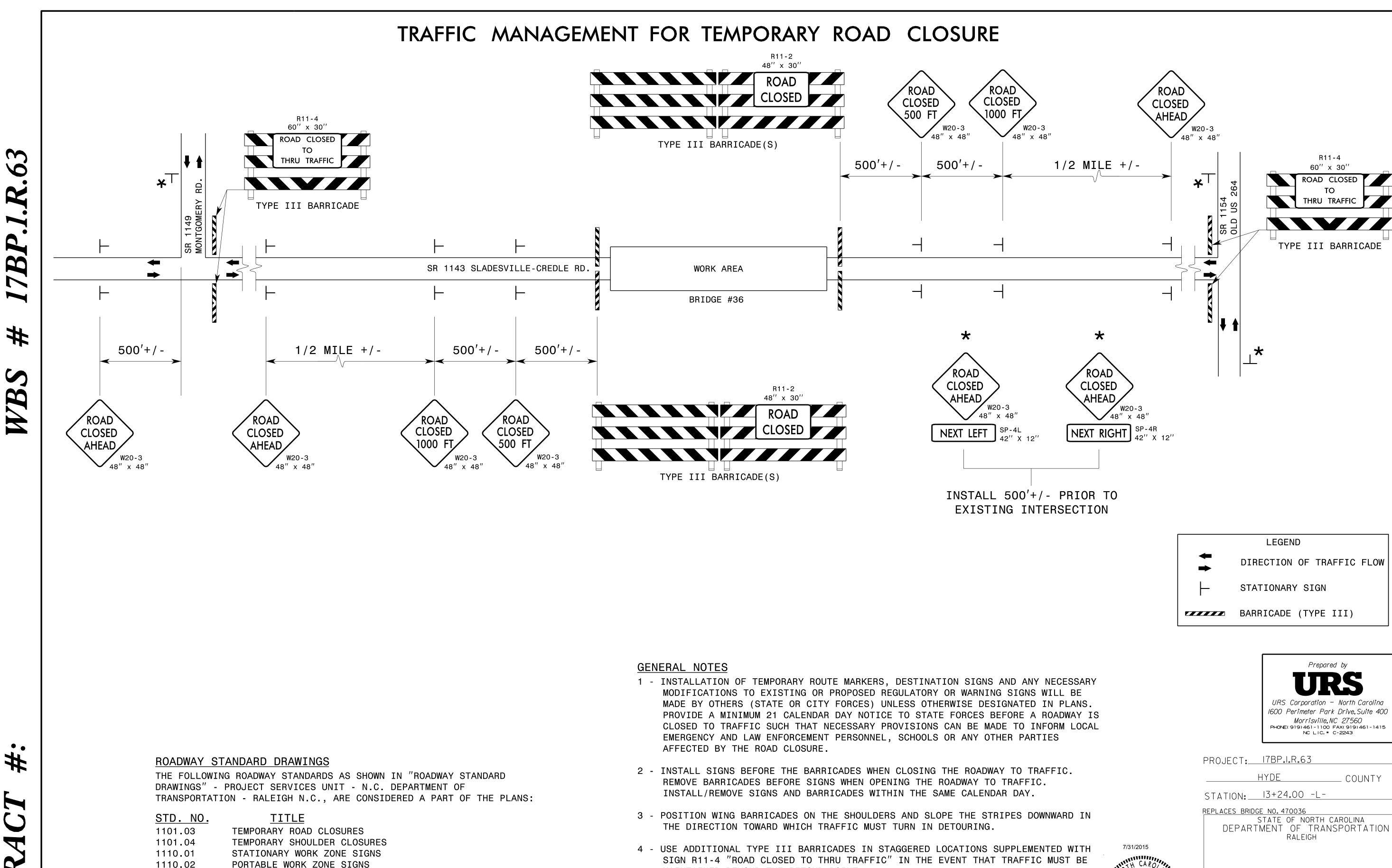
LINE	BEGIN STATION	END STATION	LOCATION	SQ. YD.
- L-	13 + 89.00	14 + 50.00	CL	134.00
			TOTAL	134.00
			SAY	140

SHOULDER BERM GUTTER

LINE	BEGIN STATION	END STATION	LENGTH
-L-	11 + 80.00	12 + 66.81	86.81
		TOTAL	86.81
		SAY	87

NOTE: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading".





MAINTAINED BEYOND THE DETOUR POINT.

5 - SEE STANDARD SPECIFICATION 1089-1 FOR WORK ZONE SIGNS.

6 - SEE STANDARD SPECIFICATION 1089-2 FOR WORK ZONE SIGN SUPPORTS.

TRAFFIC MANAGEMENT PLAN

27'-10" CLEAR ROADWAY - 90° SKEW

DATE: NO. BY:

TMP-1

TOTAL

SHEETS

SEAL

039831

Jerry Javellana

CONES

BARRICADES

ORIENTATION OF GROUND MOUNTED SIGNS

1135.01

1145.01

DRAWN BY: ____LHM_

CHECKED BY: _____JBJ__

904.10

DATE: 06/26/15

- DATE: <u>07/03/15</u>

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

PROJECT: 17BP.I.R.63

HYDE COUNTY

STATION: 13+24.00 -L
REPLACES BRIDGE NO. 470036

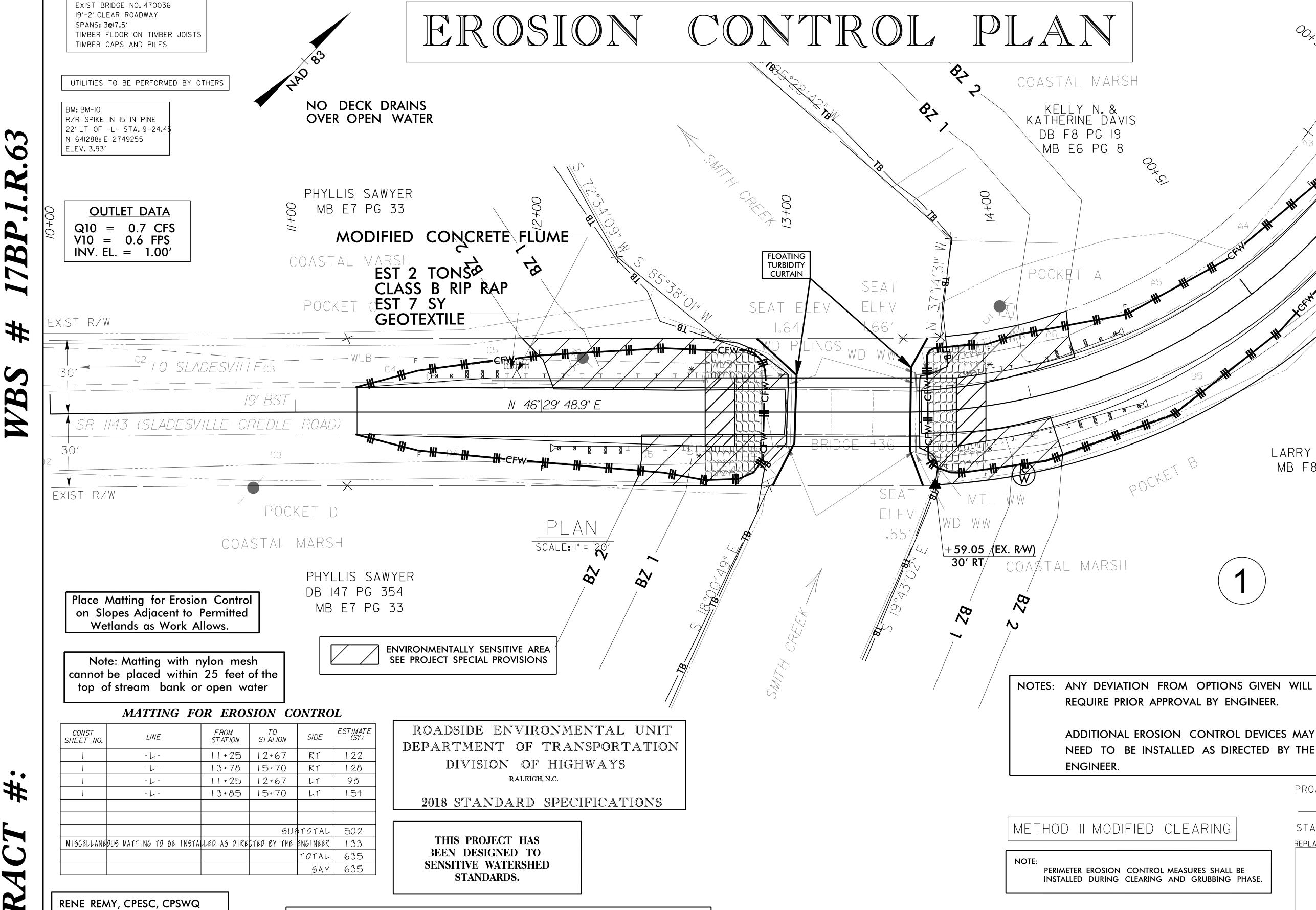
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

EROSION CONTROL PLAN
BRIDGE #470036
ON SR II43
OVER SMITH CREEK

27'-IO" CLEAR ROADWAY - 90° SKEW

NO. BY: DATE: NO. BY: DATE:

TOTAL SHEETS



Symbol

—____CFW—____CFW—___

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY

WITH THE REGULATIONS SET FORTH BY THE

NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE APRIL 1, 2019

ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

Description

Temporary Silt Fence

Coir Fiber Wattle Barrier

Floating Turbidity Curtain

1605.01

 $N /\!\!/ A$

+68.00

EX. R/W 30' RT

Prepared by

1600 Perimeter Park Drive, Suite 400

Morrisville, NC 27560 PHONE(919)461-1100 FAX(919)461-1415 NC LIC. # C-2243

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

EROSION CONTROL PLAN

BRIDGE #470036 OVER SMITH CREEK

27'-10" CLEAR ROADWAY - 90° SKEW

DATE: NO. BY:

_ COUNTY

EC–2

TOTAL

SHEETS

PROJECT: 17BP.I.R.63

REPLACES BRIDGE NO. 470036

HYDE

STATION: 13+24.00 -L-

LARRY EGGERT

MB F8 PG 19

T

LEVEL III NAME

3135 EXP. 12/31/20

DRAWN BY: MDB

CHECKED BY: <u>WTS</u>

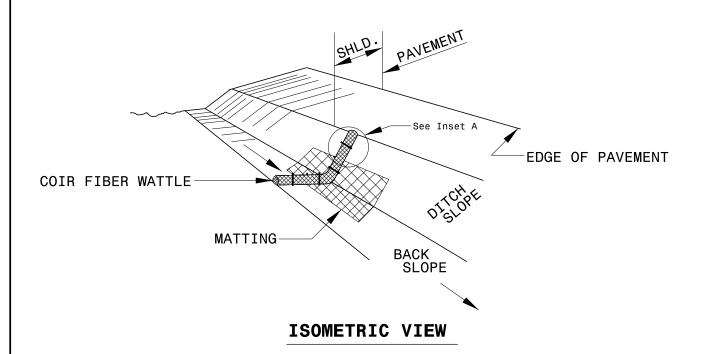
LEVEL III CERTIFICATION NO.

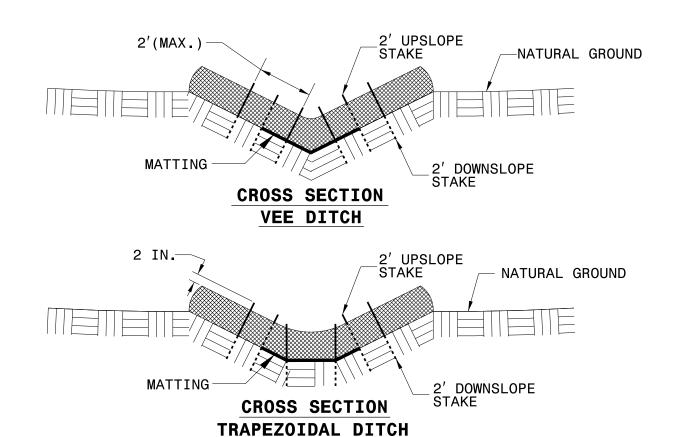
__ DATE: <u>06.22.15</u>

__ DATE: <u>06.22.15</u>

PROJECT REFERENCE NO. SHEET NO. 17BP.J.R.63

COIR FIBER WATTLE DETAIL





NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

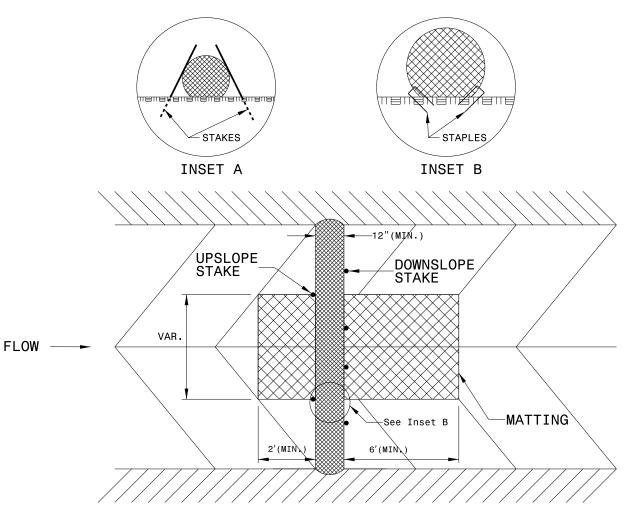
 $\underline{\text{ONLY}}$ INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.



TOP VIEW

SILT FENCE COIR FIBER WATTLE BREAK DETAIL

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

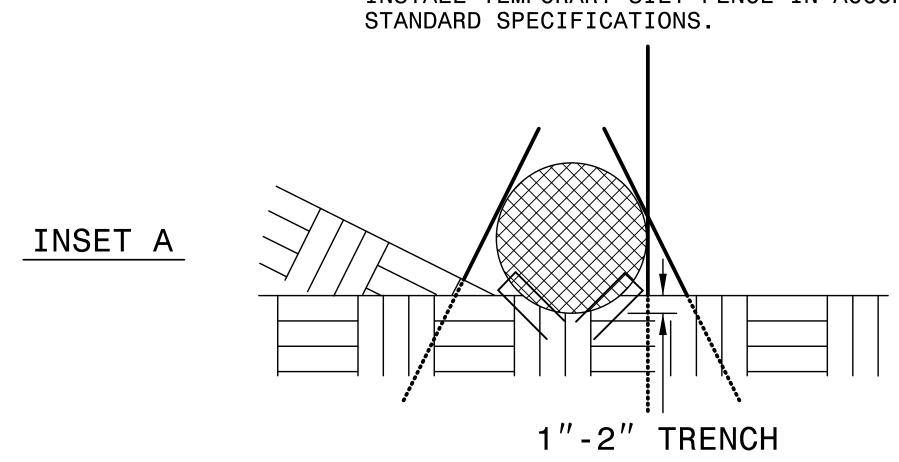
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.



VIEW FROM SLOPE

-FILL

WATTLE

MATERIAL

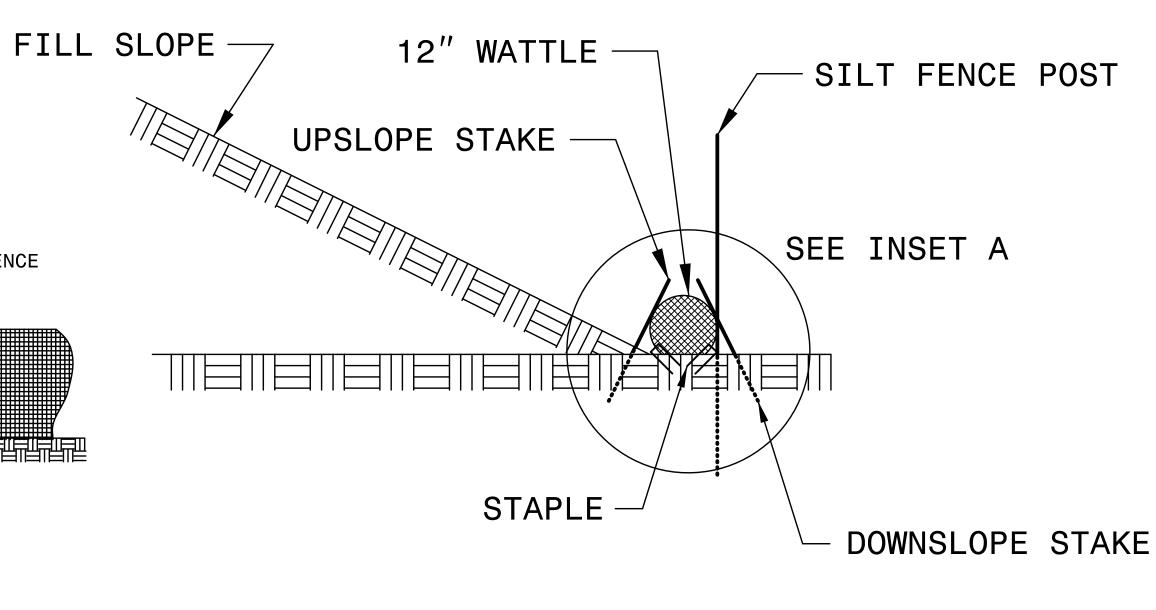
ROAD

SILT

FENCE

GRADE

ISOMETRIC VIEW



SIDE VIEW

PROJECT: <u>I7BP.I.R.63</u>

<u>HYDE</u> COUNTY

STATION: <u>I3+24.00 -L-</u>

REPLACES BRIDGE NO. 470036

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

EROSION CONTROL PLAN

BRIDGE #470036 ON SR II43 OVER SMITH CREEK

27'-10" CLEAR ROADWAY - 90° SKEW

REVISIONS

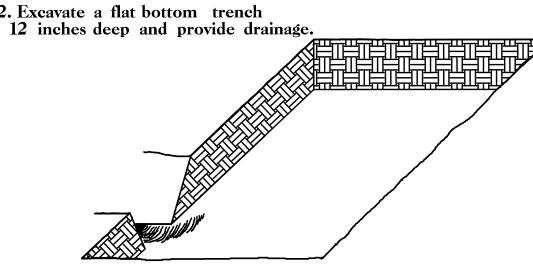
SHEET NO.

PLANTING DETAILS

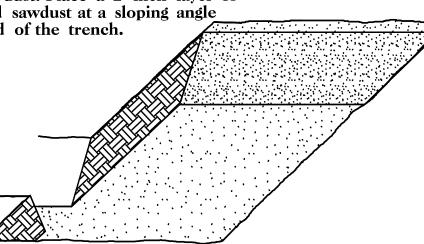
SEEDLING / LINER BAREROOT PLANTING DETAIL

HEALING IN

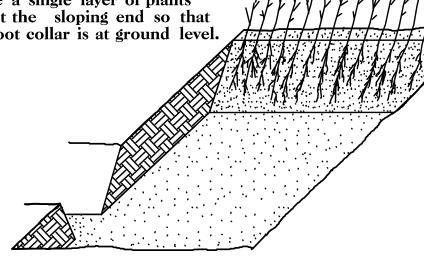
. Locate a healing-in site in a shady, well protected area.



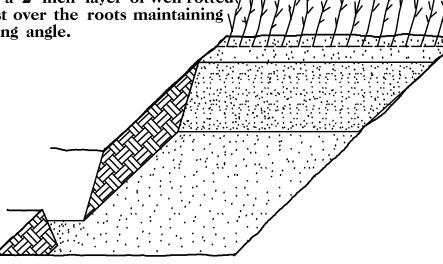
3. Backfill the trench with 2 inches well rotted sawdust. Place a 2 inch layer of well rotted sawdust at a sloping angle at one end of the trench.



4. Place a single layer of plants against the sloping end so that the root collar is at ground level.

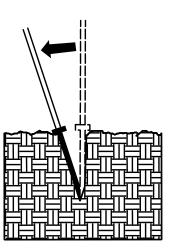


5. Place a 2 inch layer of well rottedy sawdust over the roots maintaining a sloping angle.

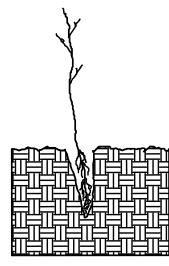


6. Repeat layers of plants and sawdust as necessary and water thoroughly.

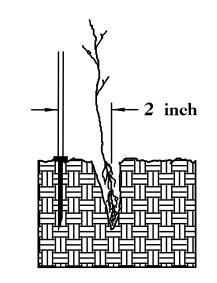
DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR



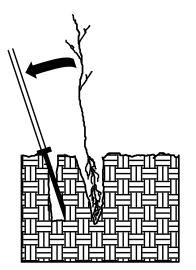
1. Insert planting bar as shown and pull handle toward planter.



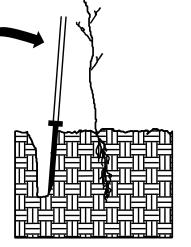
2. Remove planting bar and place seedling at



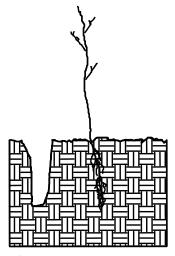
3. Insert planting bar 2 inches toward planter



4. Pull handle of bar toward planter, firming soil at bottom.



firming soil at top.



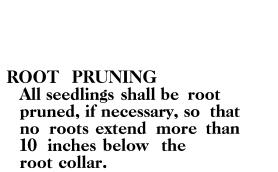
6. Leave compaction hole open. Water

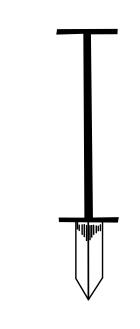
PLANTING NOTES:

PLANTING BAG During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR Planting bar shall have a blade with a triangular cross section, and shall be 12 inches long, 4 inches wide and 1 inch thick at center.





REFORESTATION DETAIL SHEET

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

REFORESTATION

TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION

25% BETULA NIGRA

MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

25% LIRIODENDRON TULIPIFERA TULIP POPLAR 12 in - 18 in BR 25% PLATANUS OCCIDENTALIS **SYCAMORE** 12 in - 18 in BR 25% FRAXINUS PENNSYLVANICA **GREEN ASH** 12 in - 18 in BR

RIVER BIRCH

PROJECT: 17BP.I.R.63

_ COUNTY

12 in - 18 in BR

STATION: 13+24.00 -L-

REPLACES BRIDGE NO. 470036

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION RALEIGH

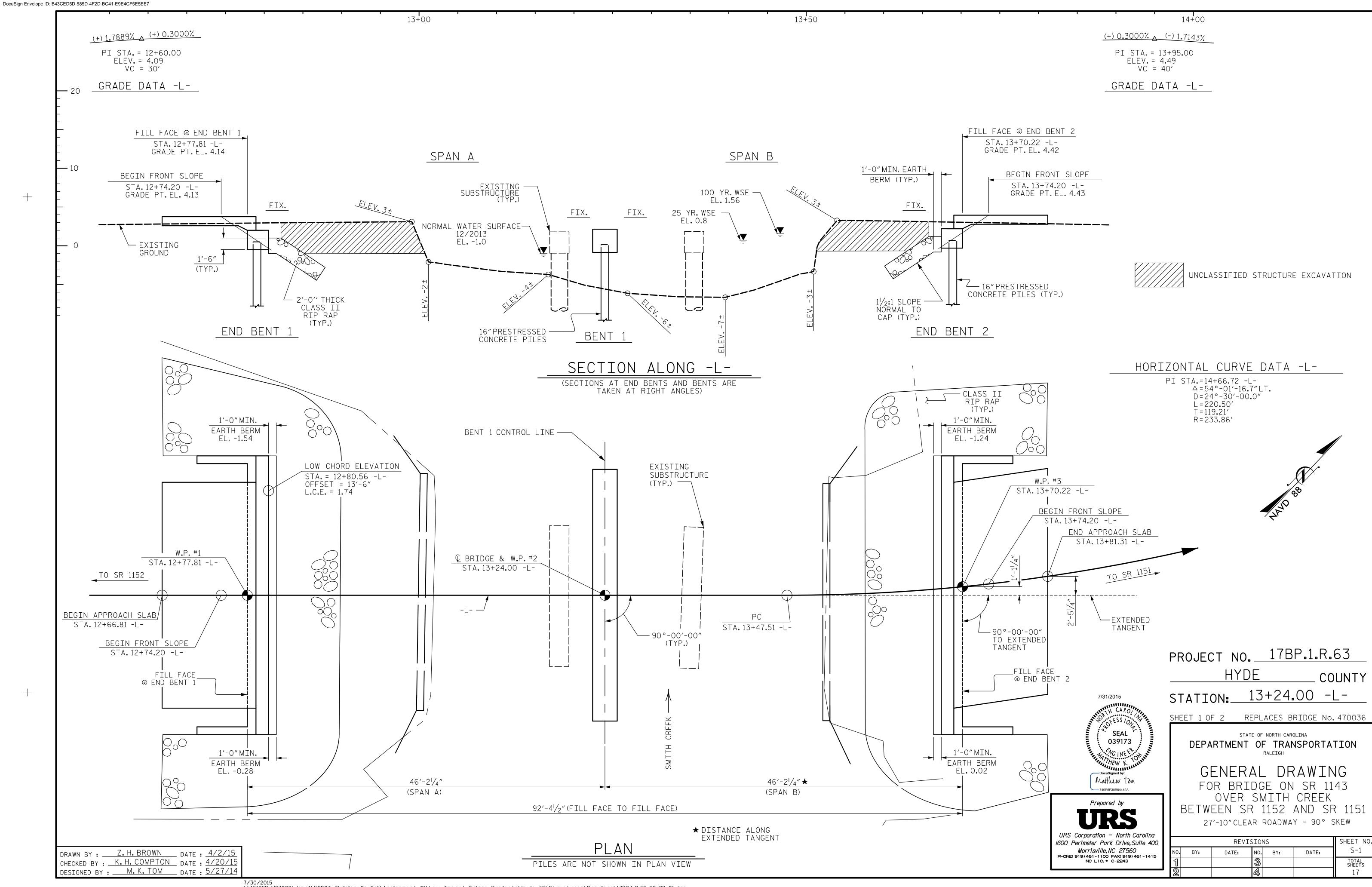
EROSION CONTROL PLAN

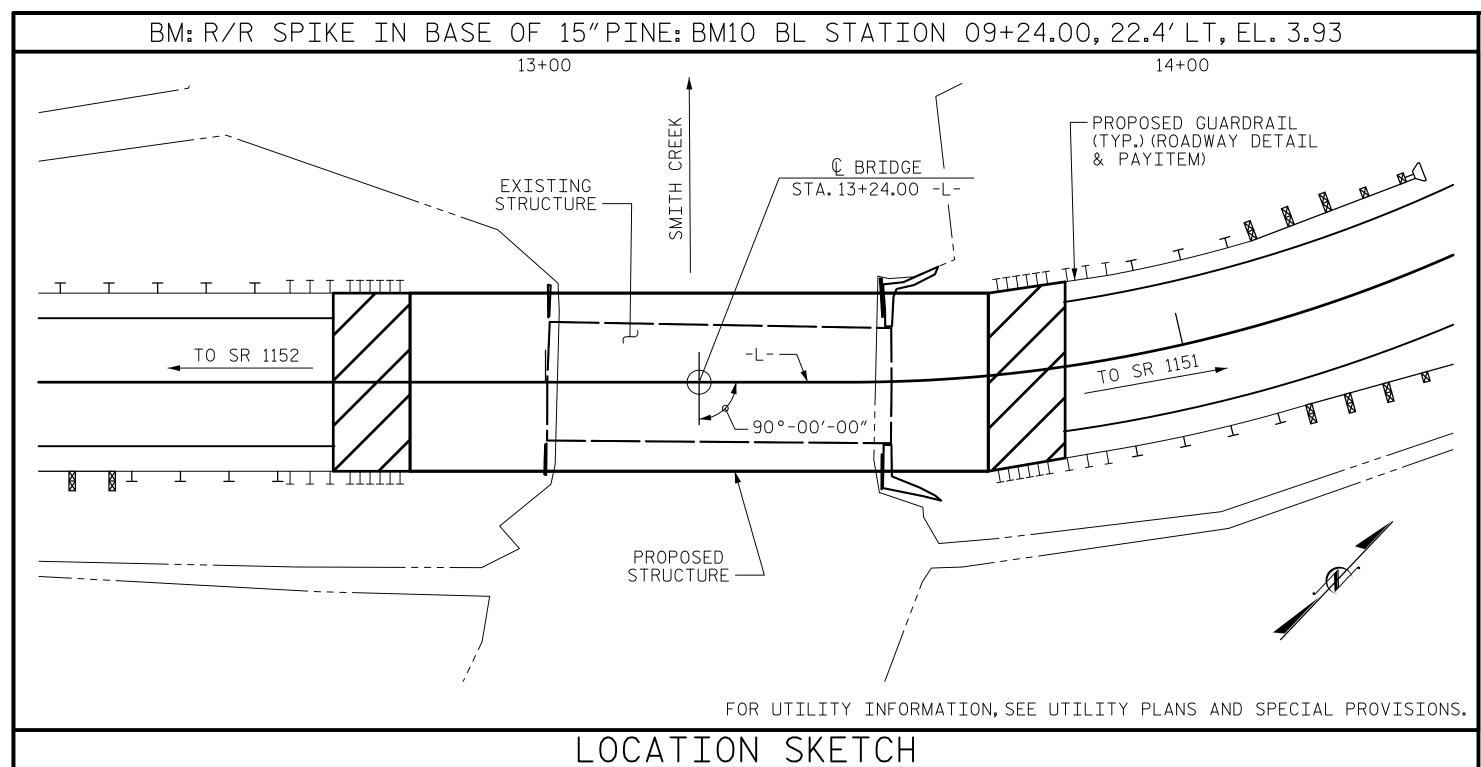
BRIDGE #470036 ON SR 1143 OVER SMITH CREEK

27'-10" CLEAR ROADWAY - 90° SKEW

NO. BY: DATE: NO. BY: DATE: 1 |

TOTAL





TOTAL BILL OF MATERIAL								
	REMOVAL OF EXISTING STRUCTURE AT STA 13+24.00 -L-	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS AA CONCRETE	BRIDGE APPROACH SLABS	EPOXY COATED REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR 16" PRESTRESSED CONCRETE PILES
	LUMP SUM	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EACH
SUPERSTRUCTURE								
END BENT 1					13.7		1983	5
BENT 1					9.3		1909	7
END BENT 2					13.7		1983	5
TOTAL	LUMP SUM	LUMP SUM	1	LUMP SUM	36.7	LUMP SUM	5875	17

	TOTAL BILL OF MATERIAL									
		PRESTRESSED CRETE PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAILS	RIP RIP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PF	/-0"X 1/-9" RESTRESSED DRED SLABS	PREDRILLING FOR PILES
	No.	LIN.FT.	EACH	LIN.FT.	TON	SQ. YDS.	LUMP SUM	No.	LIN.FT.	LIN.FT.
SUPERSTRUCTURE				90				20	900	
END BENT 1	5	400	3		70	80				100
BENT 1	7	630	4							105
END BENT 2	5	375	3		70	80				100
TOTAL	17	1405	10	90	140	160	LUMP SUM	20	900	305

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASTHO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS STRUCTURE CONTAINS THE NECESSARY CORROSION PROTECTION REQUIRED FOR A CORROSIVE SITE.

- FOR UTILITY INFORMATION, SEE UTILITY PLANS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCATED FOR A DISTANCE OF 18 FT EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICAITONS.

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE COLUMNS, BENT CAPS, PILE CAPS, AND FOOTINGS, AND SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR.

ALL BAR SUPPORTS USED IN THE BARRIER RAIL, BENT CAPS, END BENTS CAPS AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICAITONS.

THE CONCRETE IN THE BENT CAP, END BENT CAPS, AND PILES OF BENT NO. 1 AND END BENTS NO. 1 & 2 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB OF CEMENT. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF 3 SPANS TOTALING 52'-0", WITH TIMBER FLOORS AND TIMBER JOISTS, ON TIMBER CAPS WITH TIMBER PILES, AND 19'-2' CLEAR ROADWAY TO BE REMOVED). THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECASSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

- FOR CALCIUM NITRITE CORROSION INHIBITOR, SEE SPECIAL PROVISIONS.
- FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

HYDRAULIC DATA

DESIGN DISCHARGE	= 470 CFS.
FREQUENCY OF DESIGN FLOOD	= 25 YR.
DESIGN HIGH WATER ELEVATION	= 0.8 FT.
DRAINAGE AREA	= 2.92 SQ. MI.
BASE FLOOD DISCHARGE (Q100)	= 700 CFS.
BASE HIGH WATER ELEVATION	= 1.56 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE=	615 CFS
FREQUENCY OF OVERTOPPING FLOOD=	50(+) YF
OVERTOPPING FLOOD ELEVATION=	1.OFT.

Prepared by

URS Corporation — North Carolina
1600 Perimeter Park Drive, Suite 400
Morrisville, NC 27560
PHONE(919)461-1100 FAX(919)461-1415
NC LIC. * C-2243



FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENTS No.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 75 TONS PER PILE.

DRIVE PILES AT END BENTS No.1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 125 TONS PER PILE.

PILES AT BENT No.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 105 TONS PER PILE.

DRIVE PILES AT BENT No.1 TO A REQUIRED DRIVING RESISTANCE OF 185 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESITANCE FOR DOWNDRAG OR SCOUR.

INSTALL PILES AT BENT No.1 TO A TIP ELEVATION NO HIGHER THAN -40

THE SCOUR CRITICAL ELEVATION FOR BENT No.1 IS ELEVATION -12 FT. SCOUR CRITICAL ELEVATION IS USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 15 TO 25 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT No.1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 35 TO 45 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT No.1. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS (AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION).

STEEL PILE TIPS ARE REQUIRED FOR PRESTRESSED CONCRETE PILES AT END BENTS No.1 AND 2, AND BENT No.1. FOR STEEL PILE TIPS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PREDRILLING FOR PILES IS REQUIRED AT END BENTS No.1 AND 2, AND BENT No.1. PREDRILL PILE LOCATIONS TO ELEVATION -20 FT WITH EQUIPMENT THAT WILL RESULT IN A MAXIMUM PREDRILLING DIAMETER OF 23". FOR PREDRILLING FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

SPUDDING MAY BE USED INSTEAD OF PREDRILLING AT END BENT No.1 AND 2, AND BENT No.1.

TEMPORARY STEEL CASINGS ARE REQUIRED FOR PREDRILLING AT END ENTS No. 1 AND 2, AND BENT No. 1.

PROJECT NO. 17BP.1.R.63

HYDE county

STATION: 13+24.00 -L-

SHEET 2 OF 2 REPLACES BRIDGE No. 470036

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
FOR BRIDGE ON SR 1143
OVER SMITH CREEK
BETWEEN SR 1152 AND SR 1151
27'-10"CLEAR ROADWAY - 90° SKEW

		SHEET NO.				
10.	BY:	DATE:	NO.	BY:	DATE:	S-2
1			3			TOTAL SHEETS
2			4			17

DRAWN BY: Z. H. BROWN DATE: 4/2/15
CHECKED BY: K. H. COMPTON DATE: 4/20/15
DESIGNED BY: M. K. TOM DATE: 5/27/14

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) ROLLING RATING GIRDER GIRDER CONT DIST, LEFT SPAN DIST, LEFT SPAN DI: FA(1.088 1.75 0.277 1.34 0.539 1.23 2.2 0.277 N/A EL 22 45′ 1.09 45′ 22 HL-93(Inv)45′ 0.80 1.590 0.539 1.59 45′ HL-93(Opr)1.35 0.277 1.74 45′ EL 22 EL 2.2 N/A DESIGN LOAD 36.000 1.336 48.104 0.539 1.45 0.277 1.34 45′ 45′ 22 HS-20(Inv) 1.75 0.277 1.65 45′ EL 22 2.2 0.80 RATING 0.539 2.2 HS-20(0pr) 36.000 1.882 67.763 1.35 0.277 2.14 45′ EL 22 1.88 45′ N/A EL 4.01 13.500 2.611 35.252 0.277 4.02 0.539 0.277 2.61 45′ EL 45′ 2.2 45′ 22 SNSH 22 0.80 EL 0.539 2.94 3.25 20.000 2.108 42.166 0.277 EL 45′ 0.277 2.11 45′ SNGARBS2 45′ 22 EL 2.2 0.80 22 0.539 2.77 17.6 22.000 2.067 45.466 0.277 3.15 0.277 2.07 45' 22 SNAGRIS2 45′ EL 45′ 2.2 0.80 EL 0.539 27.250 35.527 1.304 0.277 45′ EL 22 2.01 2.2 0.277 1.30 45′ 22 SNCOTTS3 2.01 0.80 1.4 EL SNAGGRS4 34.925 1.150 40.181 0.277 1.77 45′ EL 22 0.539 1.74 45′ 2.2 0.80 0.277 1.15 45′ 22 EL 35.550 0.277 1.73 0.539 1.79 0.277 1.12 1.121 45′ EL 22 45′ 2.2 45′ 22 SNS5A 39.841 EL 0.80 0.539 39.950 1.056 42.175 0.277 1.63 1.67 0.277 1.06 45′ 22 SNS6A 45′ EL 22 45′ 2.2 EL 0.80 0.539 22 SNS7B 42.000 1.006 42.268 0.277 1.55 45′ EL 22 1.68 45′ 2.2 0.80 0.277 1.01 45′ LEGAL LOAD 0.539 1.96 33.000 1.296 42.759 0.277 45′ 0.277 1.30 45′ TNAGRIT3 45′ EL 22 EL 2.2 0.80 22 RATING 0.539 1.88 0.277 1.31 TNT4A 33.075 1.309 43.305 0.277 2.02 45′ EL 22 45′ EL 2.2 0.80 45′ EL 22 0.539 1.83 TNT6A 41.600 1.099 45.712 0.277 1.69 45′ EL 22 45′ 2.2 0.80 0.277 1.10 45′ 22 EL 1.69 42.000 1.120 47.043 0.277 1.73 45′ EL 22 0.539 45′ 2.2 0.277 1.12 45′ 22 TNT7A EL 0.80 0.539 1.61 45′ 22 42.000 1.166 48.975 0.277 1.8 45′ EL 22 2.2 0.80 0.277 1.17 TNT7B 1.4 EL 0.539 1.55 43.000 1.111 47.757 0.277 45' 0.80 0.277 45′ 22 TNAGRIT4 1.71 45′ EL 22 2.2 1.11 1.4 EL 46.505 1.033 0.277 1.59 0.539 1.59 0.277 1.03 TNAGT5A 45.000 EL 22 2.2 0.80 45′ 22 45′ EL 1.56 1.009 1.4 0.277 2.2

22 0.539

1.47

0.80 0.277 1.01

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$ LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.1.R.63 HYDE _ COUNTY

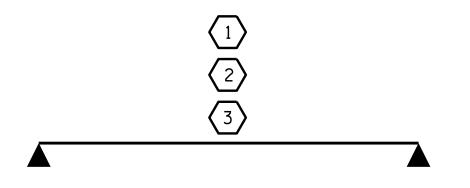
STATION: 13+24.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD LRFR SUMMARY FOR 45' CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-3
		3			TOTAL SHEETS
		4			17



_RFR SUMMARY FOR SPANS 'A' & 'B'

ASSEMBLED BY: M.M. AHMED DATE: 3-2-15 CHECKED BY: P.N. HOLDER DATE: 3-17-15 DRAWN BY : CVC 6/10

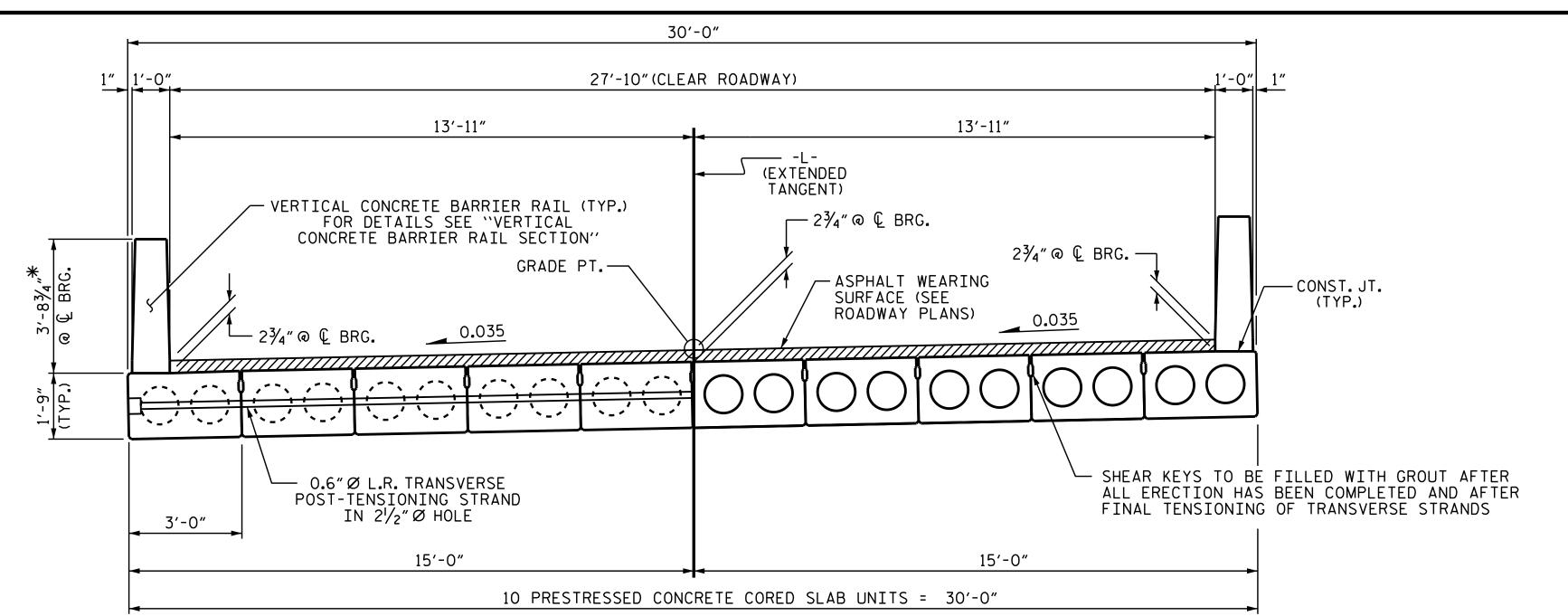
CHECKED BY : DNS 6/10

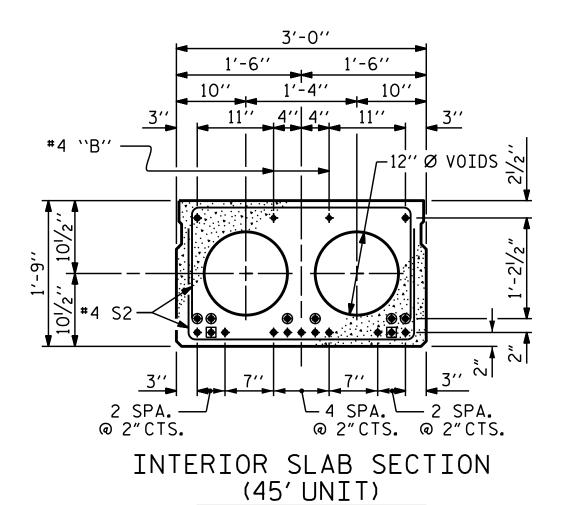
Greg Dickey 3/27/2015

45.000

TNAGT5B

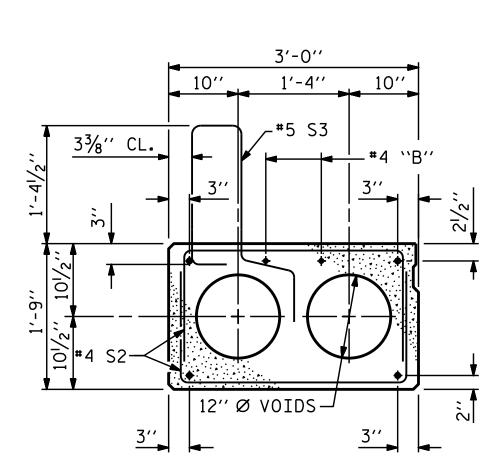
45.408





(13 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT

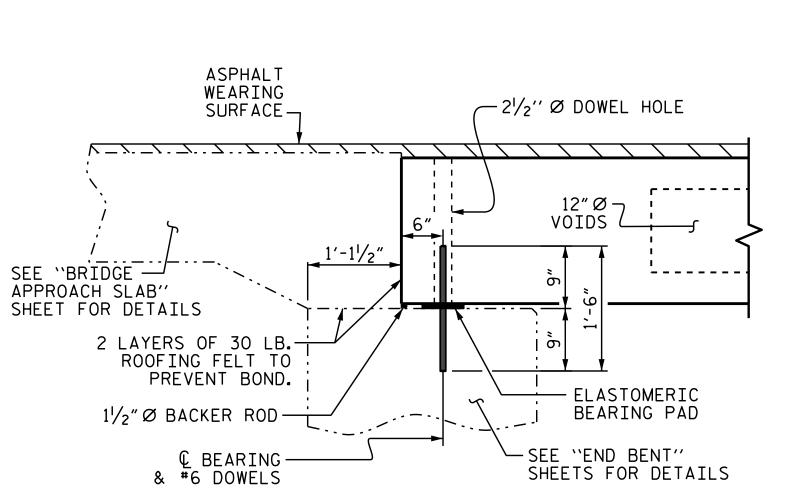


EXT. SLAB SECTION (FOR PRESTRESSED STRAND LAYOUT, SEE

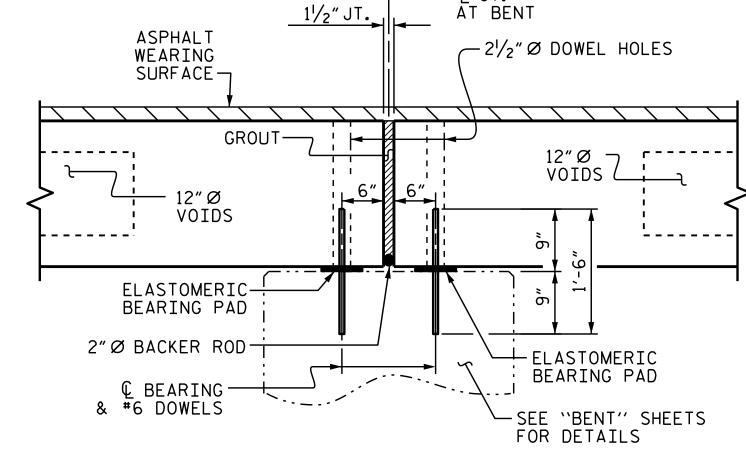
INTERIOR SLAB SECTION.)

TYPICAL SECTION

*- THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.



FIXED END



FIXED END

FIXED END

PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.—

THREADED INSERT DETAIL

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O"FROM END OF CORED SLAB UNIT SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

PROJECT NO. <u>17BP.1.R.63</u> HYDE COUNTY

STATION: 13+24.00 -L-

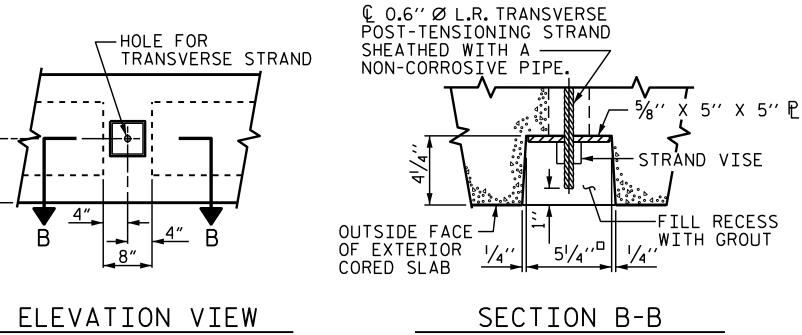
SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT

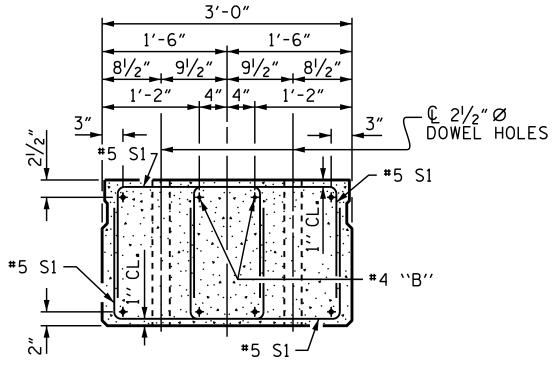
		REVI	SIONS	S		SHEET NO.
р.	BY:	DATE:	NO.	BY:	DATE:	S-4
			3			TOTAL SHEETS
2			4			17

SECTION AT END BENT SECTION AT BENT



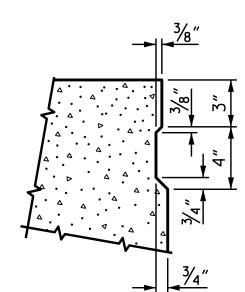
GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

DATE : 3-2-15 ASSEMBLED BY : M.M. AHMED CHECKED BY : P.N. HOLDER DATE : 3-17-15 DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09



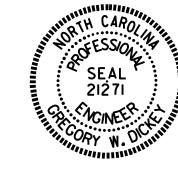
END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

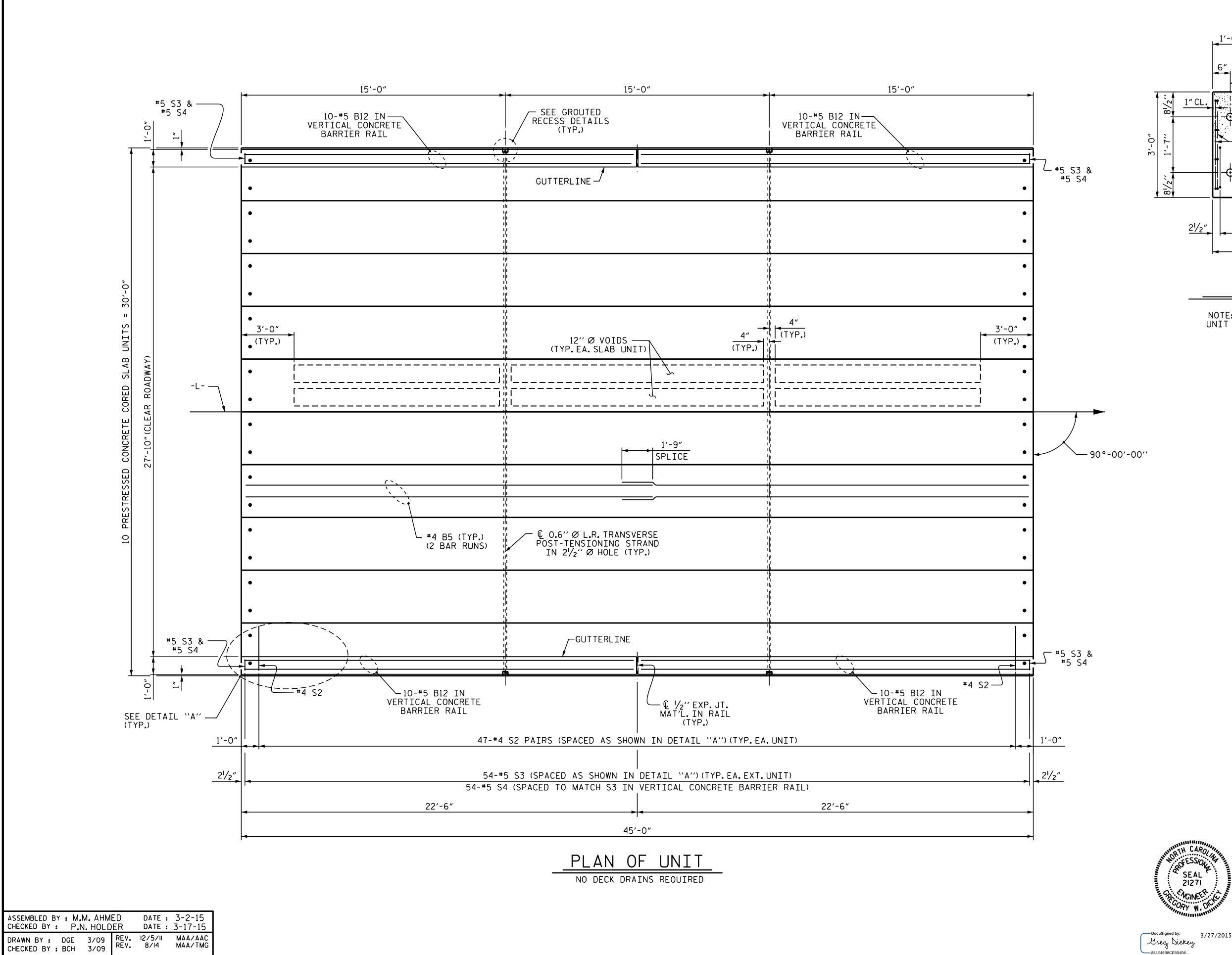


SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



Greg Dickey



#4 S2 PAIRS 7-#4 S2 PAIRS @ @ 1'-0"CTS. 9"CTS. DOWEL HOLES VOIDS #5 S3 @ 1'-0"CTS. 8-#5 S3 @ 6"CTS. 3'-0"

(TYPICAL EACH END OF UNIT)
NOTE: EXTERIOR UNIT SHOWN - INTERIOR
UNIT SIMILAR EXCEPT OMIT #5 S3 BARS.

DETAIL "A"

PROJECT NO. 17BP.1.R.63 HYDE COUNTY

STATION: 13+24.00 -L-

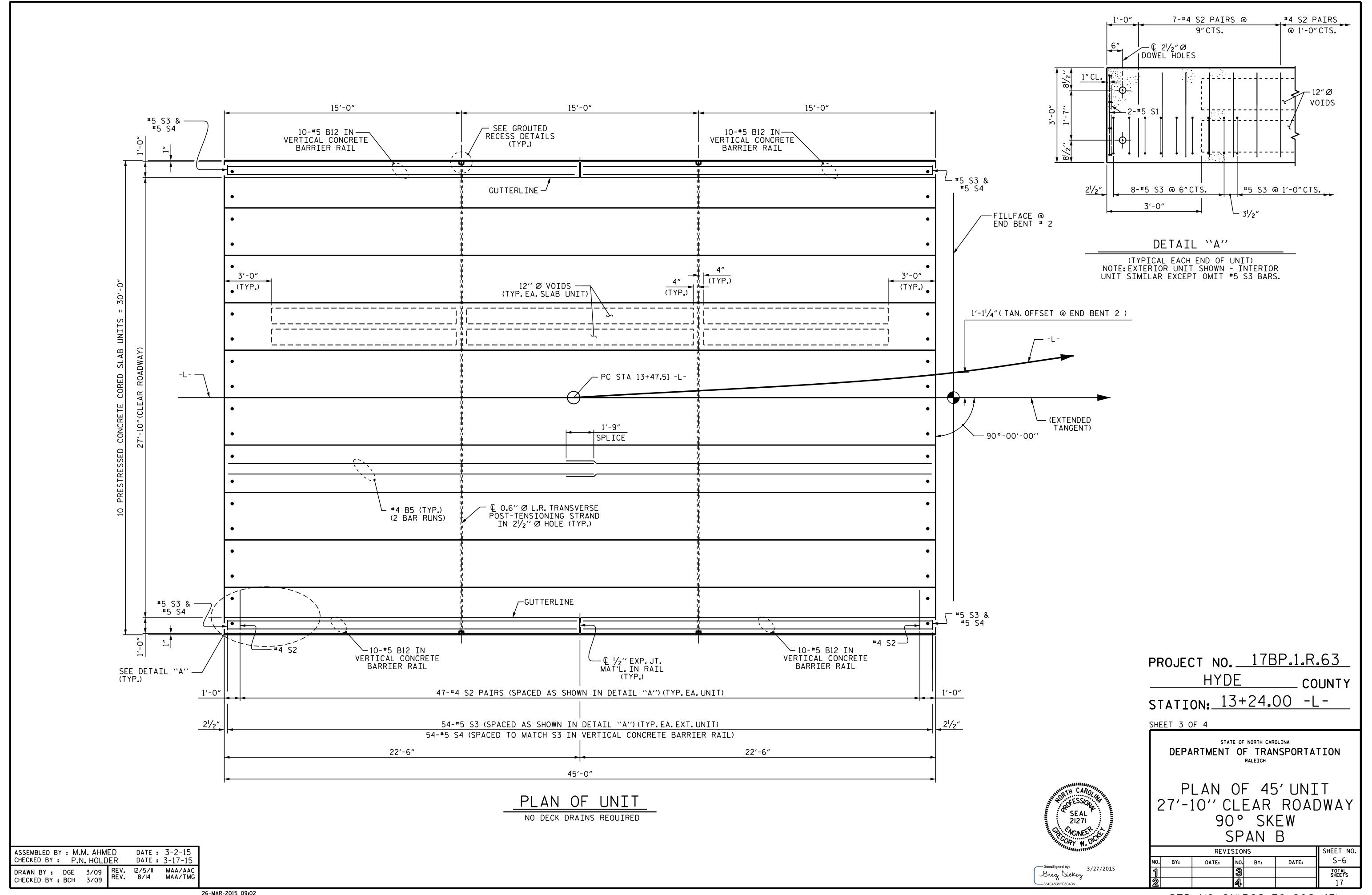
SHEET 2 OF 4

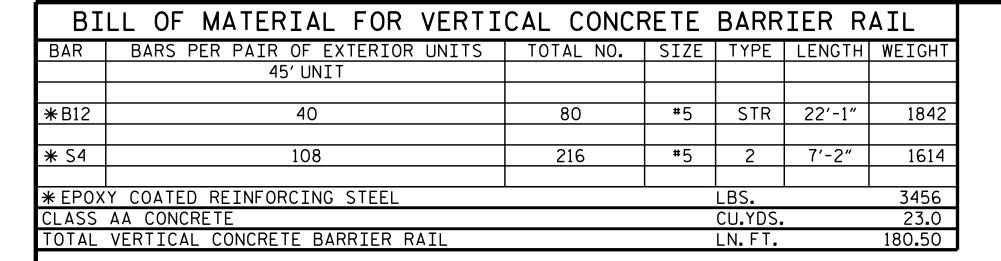
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PLAN OF 45' UNIT 27'-10" CLEAR ROADWAY 90° SKEW SPAN A

REVISIONS S-5 DATE:

Greg Dickey





SLABS REQUIRED					
NUMBER	LENGTH	TOTAL LENGTH			
4	45'-0"	180'-0"			
16	45'-0"	720'-0"			
20		900'-0"			
	NUMBER 4	NUMBER LENGTH 4 45'-0"			

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
45' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	7⁄8″ ∤
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	l∕8″ ∤
FINAL CAMBER	3/₄″ Å

**	INCLUDES	FUTURE	WEARING	SURFACE

TT	9 7 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	3'-4" 7 ³ / ₄ "
\neg	S1 1'-9" S2 2'-8" 3 "5-,1" ALL BAR DIMENSIONS ARE OUT TO	OUT

BAR TYPES

BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT									
				EXTERI	OR UNIT	INTERI	OR UNIT		
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
B5	4	#4	STR	23'-3"	62	23′-3″	62		
S1	8	# 5	3	4'-3"	35	4'-3"	35		
S2	94	#4	3	5′-4″	335	5′-4″	335		
* S3	54	# 5	1	5′-7″	314				
REINFO	RCING S	STEEL	LB:	5.	432		432		
	* EPOXY COATED REINFORCING STEEL LBS. 314								
5000 F	S.I.CO	NCRETE	CU. YDS).	6 . 5		6. 5		
0.6"Ø	L.R. STR	ANDS	No).	13		13		

0.6" Ø L.R.

58,600

43,950

2"CL.MIN.

10"

-#5 S4

(TYP.)

 $2\frac{3}{8}$ " CL.

-#5 S3 ×

VERTICAL DIM. VARIES

#5 S3 (SEE "PLAN OF UNIT" FOR SPACING)

GRADE 270 STRANDS

(SQUARE INCHES) ULTIMATE STRENGTH

(LBS.PER STRAND)

APPLIED PRESTRESS

(LBS.PER STRAND)

3'-8¾" "CUTTERLINE / RAIL HEIGHT'

VARIES THICKNE

GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
45' UNITS	2"	3′-8″

GROUT-

SECTION T-T

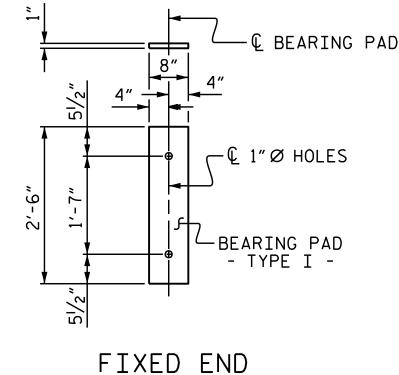
AT OPEN JOINT AT BENT

¾″ CHAMFEF

END VIEW

CHAMFER.

42 DIAT 12



ELASTOMERIC BEARING DETAILS

(TYPE I - 40 REQ'D)

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

(THIS IS TO BE USED WHERE FOAM JOINT IS NOT USED) 2'-0" 4-#5 S3 6" 4-#5 S3 #5 S3 & S4 `& S4 @ [& S4 @ 10" FIELD BEND-"B" BARS 6"CTS. \FIELD CUT CONCRETE RELEASE STRENGTH FIELD CUT-#5 S4 UNIT #5 S3-FIELD-CUT 45' UNITS #5 S4 CONST. JT.

SIDE VIEW

VERTICAL CONCRETE BARRIER RAIL SECTION

21/2"

↓ ½"EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS.

(NOTE: OMIT EXP. JT. MAT'L.

WHEN SLIP FORM IS USED)

CHAMFER

ELEVATION AT EXPANSION JOINTS

SECTION S-S

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY

WHEN SLIP FORM IS USED)

© OPEN JT. IN RAIL @ BENT

CHAMFER

DATE : 3-2-15 ASSEMBLED BY : M.M. AHMED CHECKED BY: P.N. HOLDER DATE: 3-17-15 DRAWN BY: DGE 5/09
CHECKED BY: BCH 6/09
REV. II/14

CONST. JT. —

END OF RAIL DETAILS

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PROJECT NO. <u>17BP.1.R.</u>63 HYDE COUNTY STATION: 13+24.00 -L-

SHEET 4 OF 4

PSI

4000

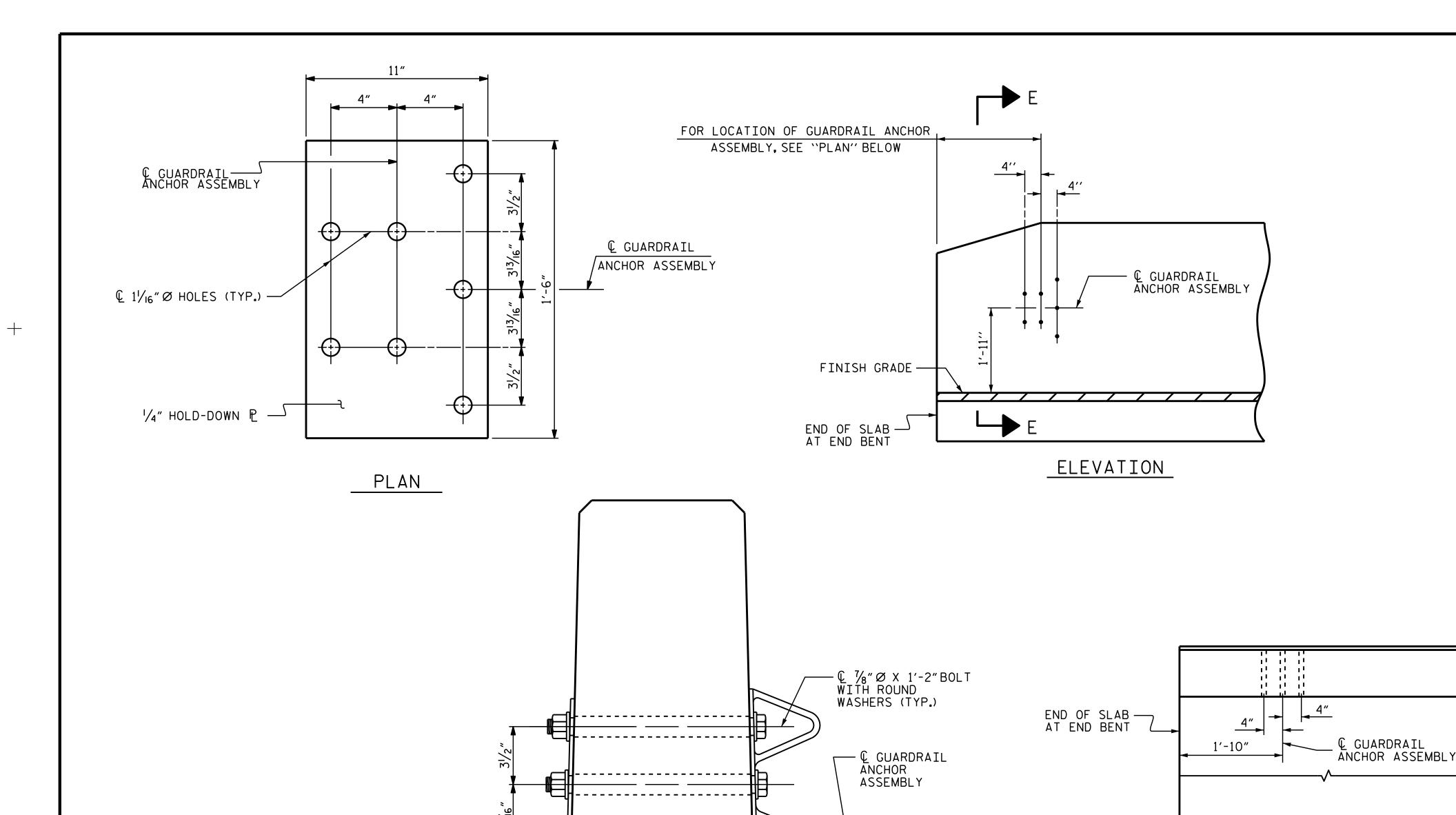
SEAL 21271

NCINEER

Greg Dickey

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-7
1			3			TOTAL SHEETS
2			4			17



SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

 $-1^{1}/_{4}$ " Ø HOLE (TYP.)

PLAN

LOCATION OF
ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.

€ GUARDRAIL ANCHOR ASSEMBLY

1'-10"

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

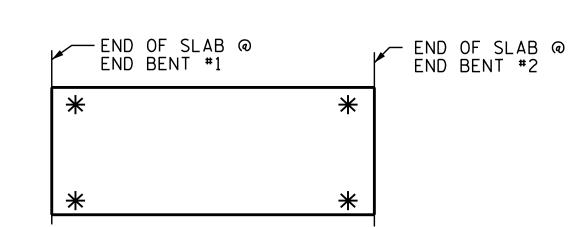
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.1.R.63

HYDE COUNTY

STATION: 13+24.00 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

STANDARD
GUARDRAIL ANCHORAGE
FOR VERTICAL CONCRETE
BARRIER RAIL

REVISIONS

DATE: NO. BY: DATE:

SHEET NO. S-8

TOTAL SHEETS
17

SEAL 21271

CONEER W. DICKER

Docusigned by: 3/27/2015

Lickey

26-MAR-2015 09:02 S:\DPG1\Division1\17BP.1.R.63\FinalPlans\17BP.1.R.63_SD_CS.dgn gdickey

ASSEMBLED BY : M.M. AHMED CHECKED BY : P.N. HOLDER

DRAWN BY : MAA 5/10

CHECKED BY : GM 5/10

DATE: 3-2-15

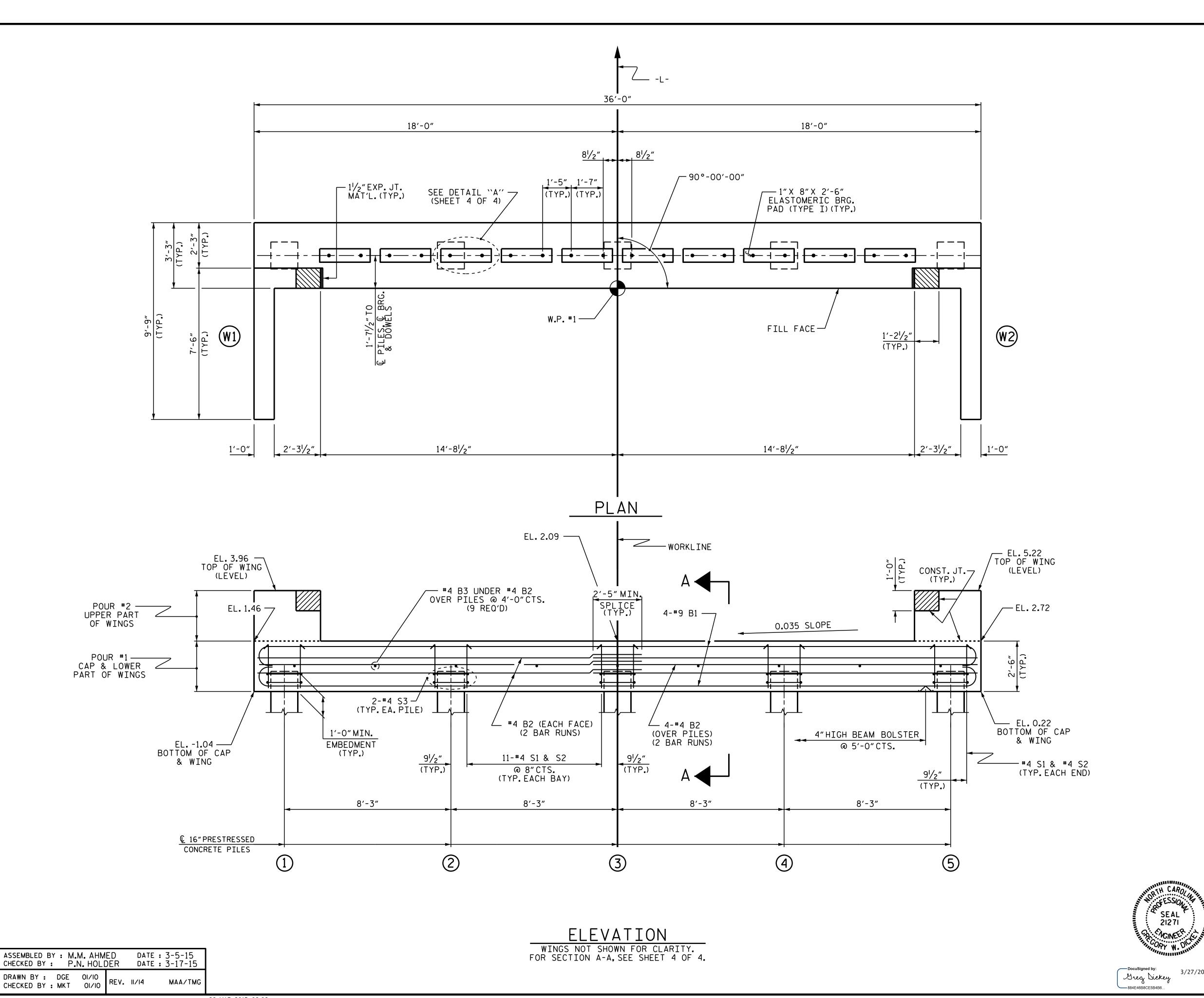
DATE: 3-17-15

ADDED 5/6/10 REV. 10/1/11

REV. 12/5/II

¼" HOLD-DOWN ₽—

(SHT 1) STD. NO. GRA3



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

ALL BAR SUPPORTS AND REINFORCING STEEL SHALL BE EPOXY COATED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

TOP ELE	OF PILE VATIONS
1	0.03
2	0.32
3	0.61
4	0.90
5	1.19

PROJECT NO. <u>17BP.1.R.63</u>

<u>HYDE</u> <u>county</u>

STATION: <u>13+24.00 -L-</u>

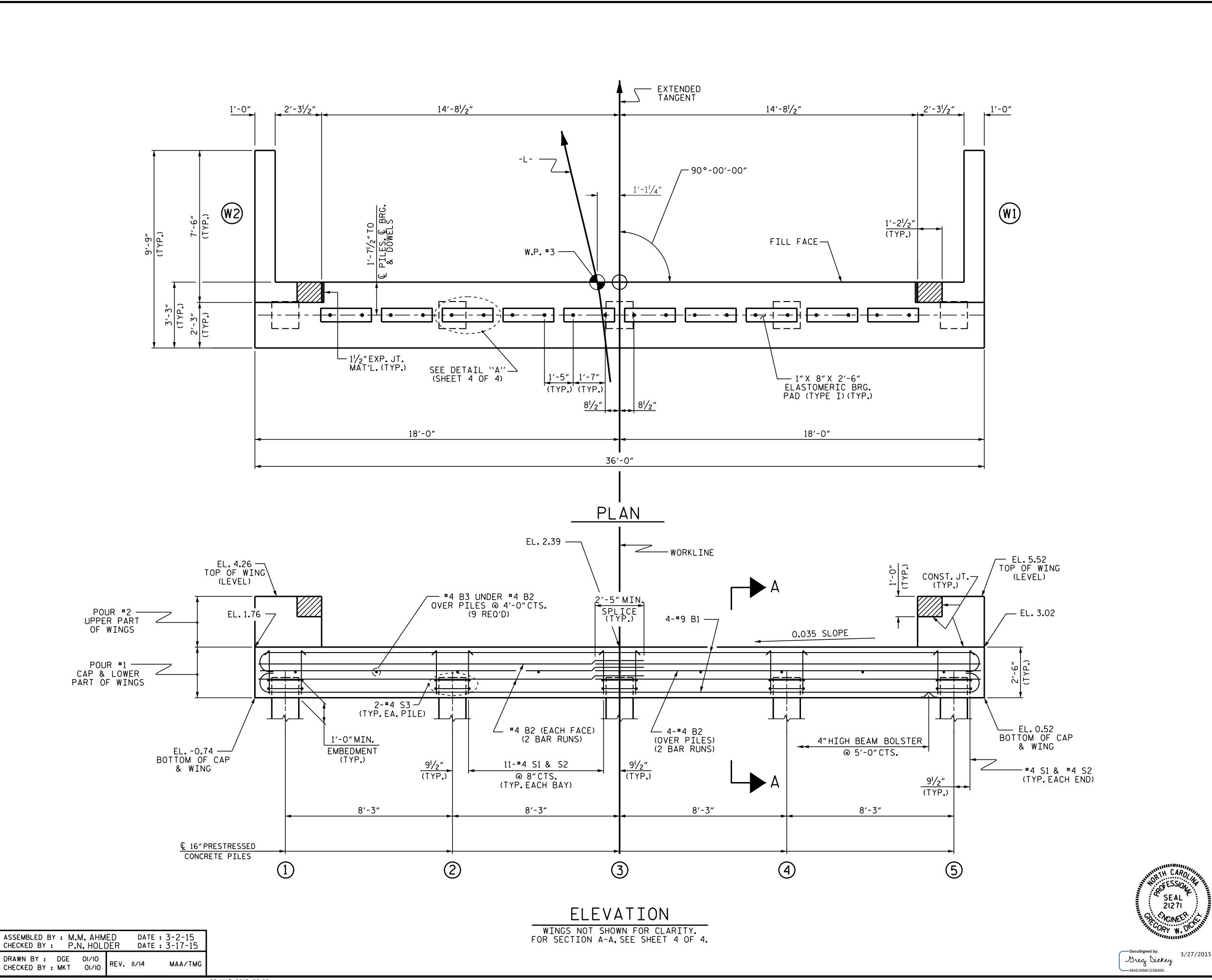
SHEET 1 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 1

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-9
1			3			TOTAL SHEETS
2			4			17



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

ALL BAR SUPPORTS AND REINFORCING STEEL SHALL BE EPOXY COATED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

TOP ELE	OF PILE VATIONS
1	0.33
2	0.62
3	0.91
4	1.20
5	1.49

PROJECT NO. 17BP.1.R.63

HYDE COUNTY

STATION: 13+24.00 -L-

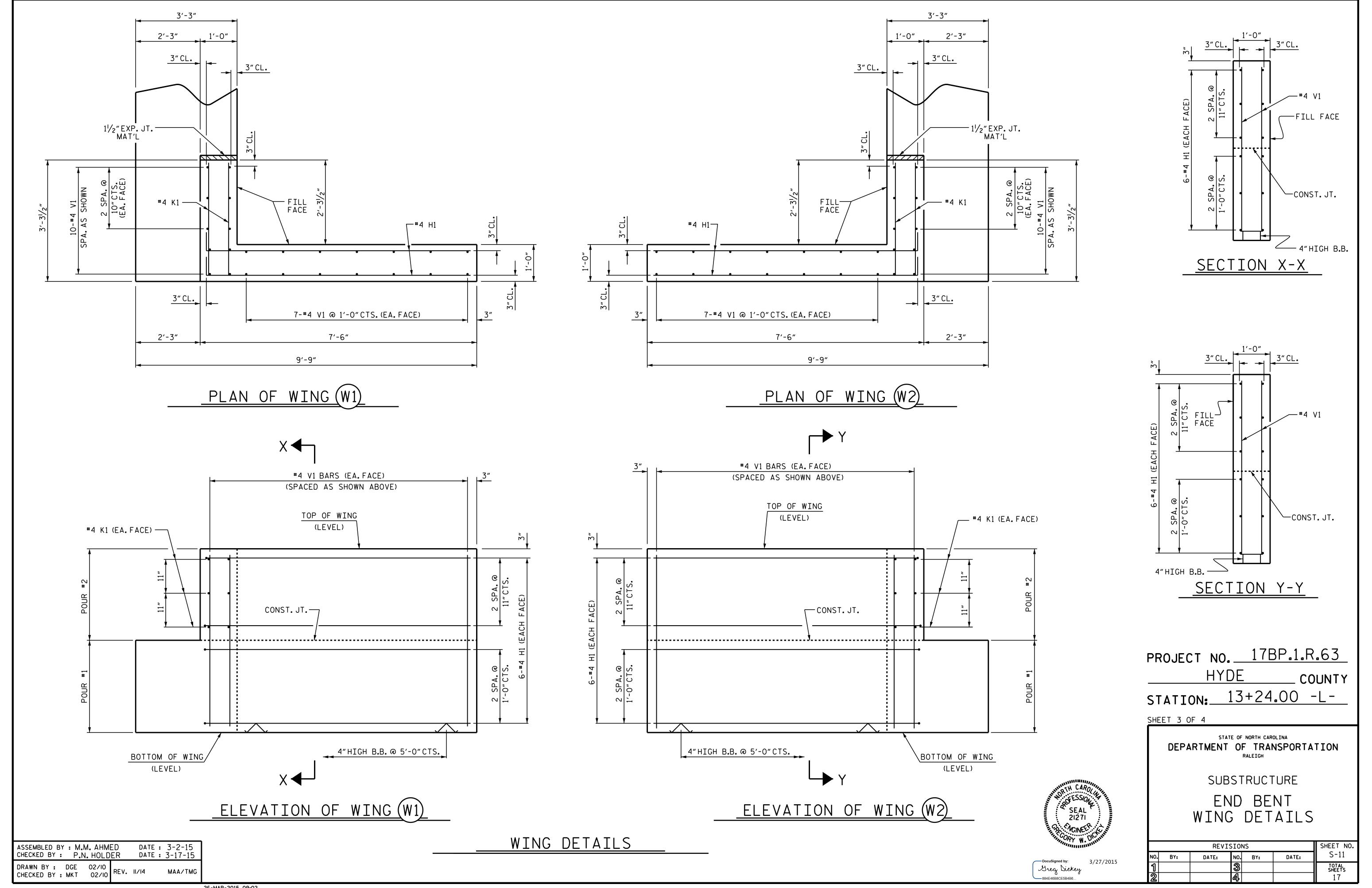
SHEET 2 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

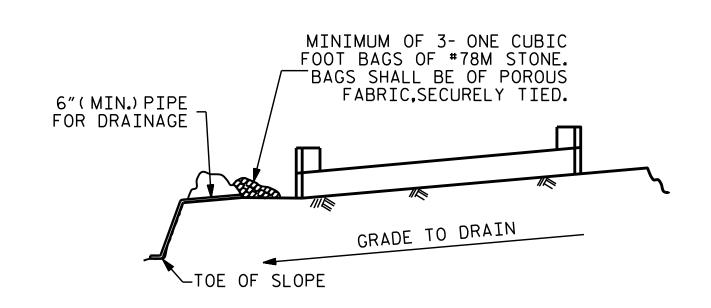
END BENT No. 2

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-10
1			3			TOTAL SHEETS
2			4			17



26-MAR-2015 09:02 S:\DPG1\Division1\17BP.1.R.63\FinalPlans\17BP.1.R.63_SD_CS.dgn gdickey

STD. NO. EB_30_90S

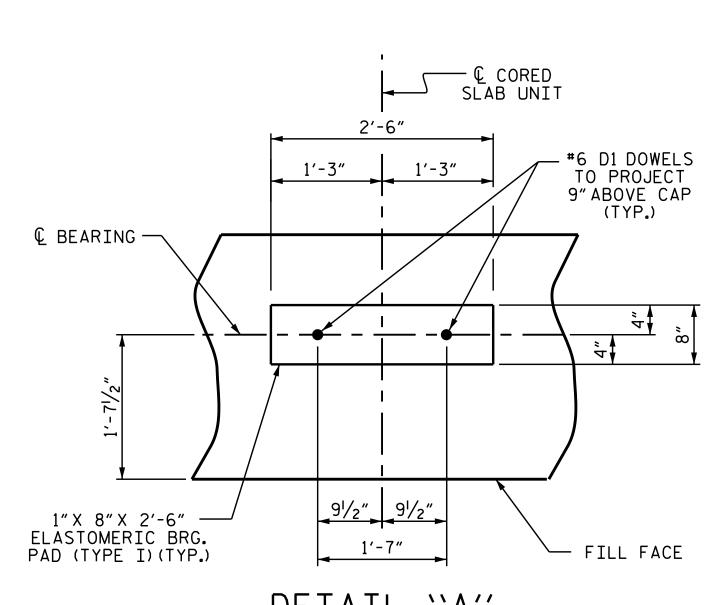


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

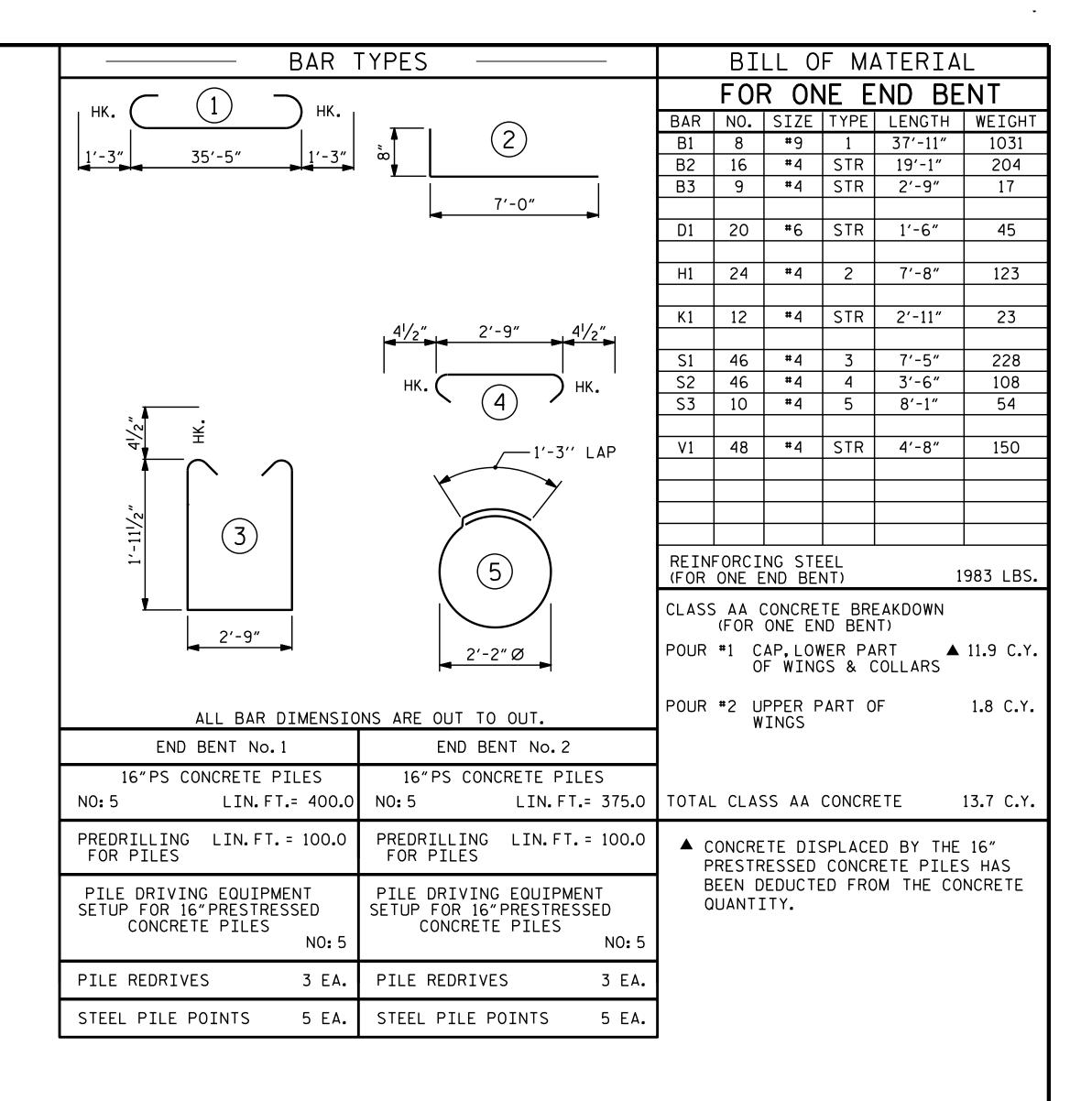
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

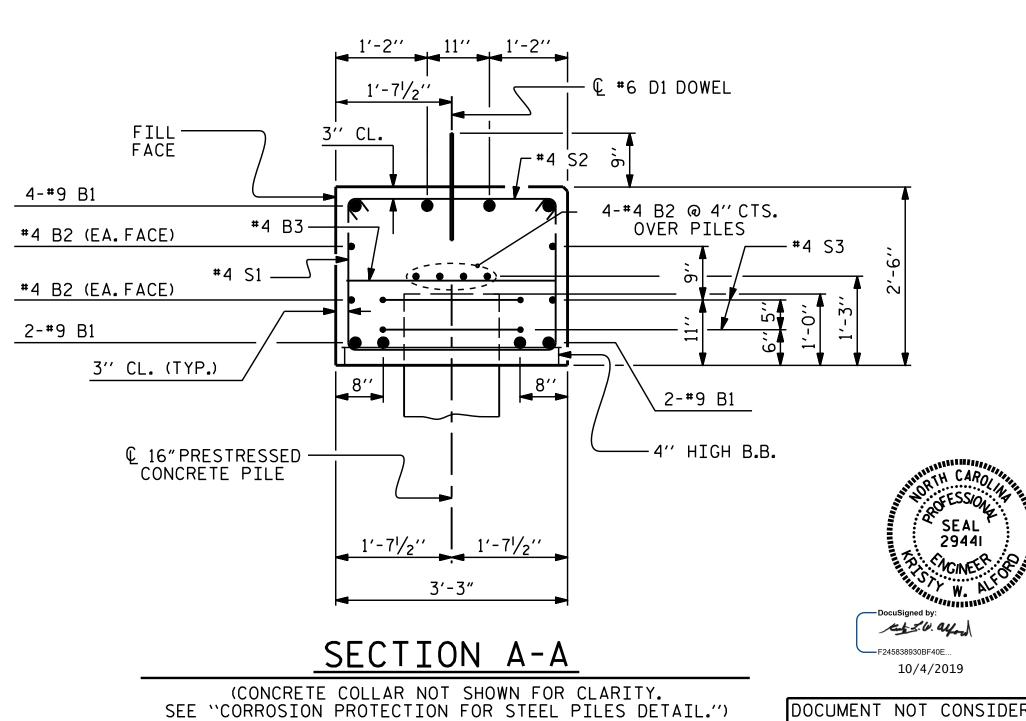
TEMPORARY DRAINAGE AT END BENT



ASSEMBLED BY : K.W. ALFORD DATE: 10/19 CHECKED BY : P.K. NEWTON DATE: 10/19 DRAWN BY: DGE 12/09 CHECKED BY: MKT 01/10

DETAIL "A" (END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)





PROJECT NO. <u>17BP.1.R.63</u> HYDE COUNTY STATION: 13+24.00 -L-

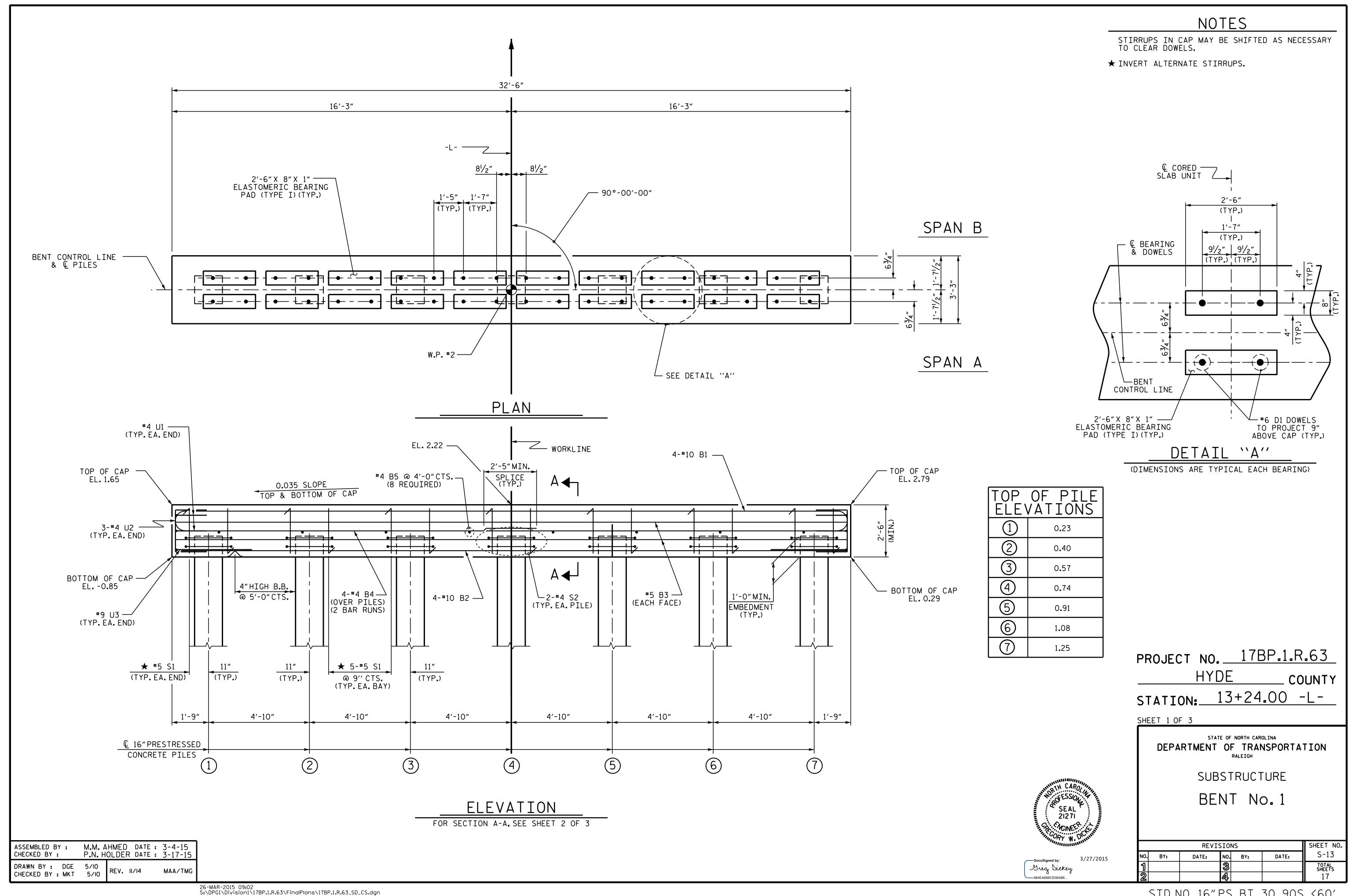
SHEET 4 OF 4

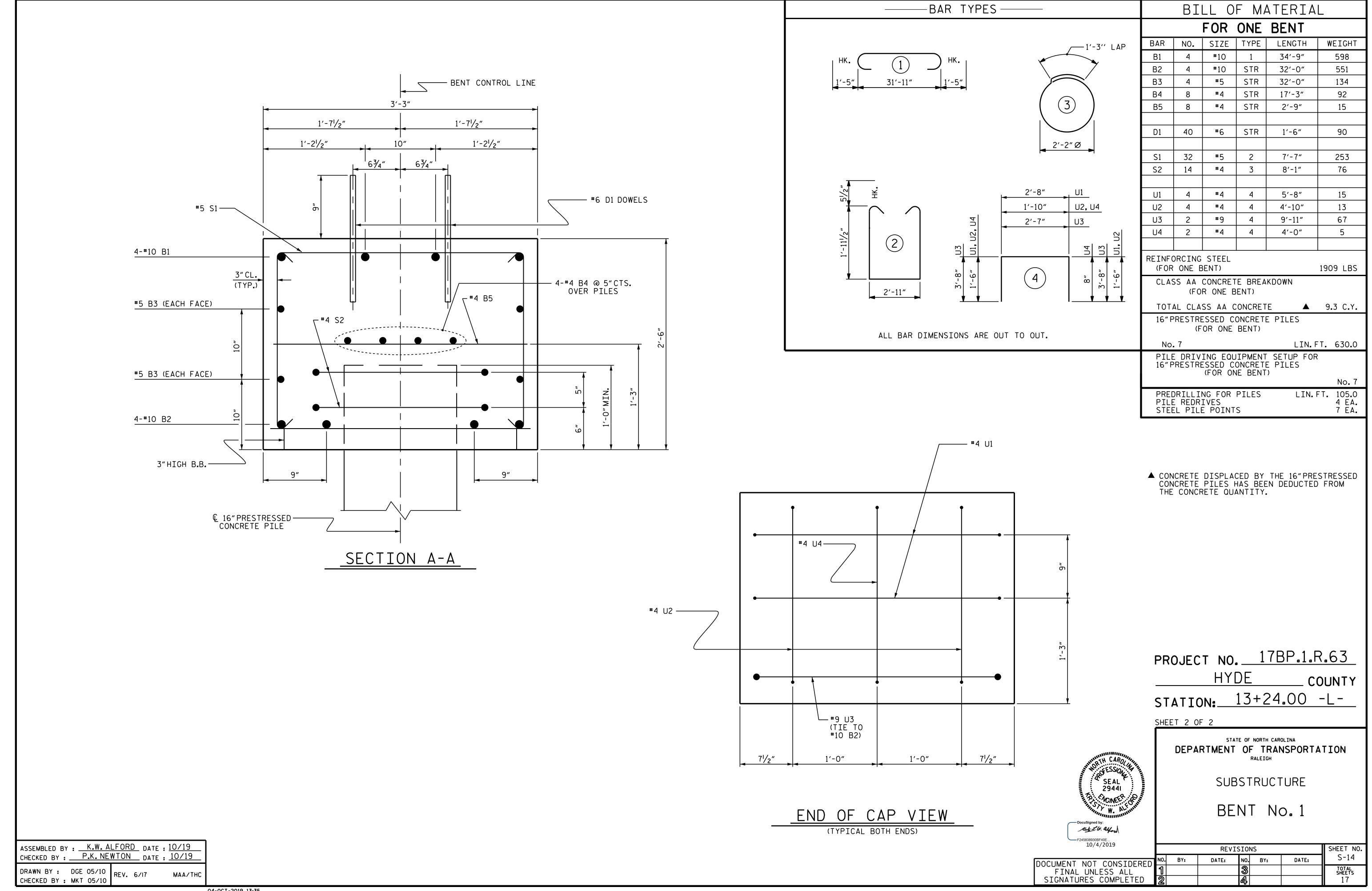
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

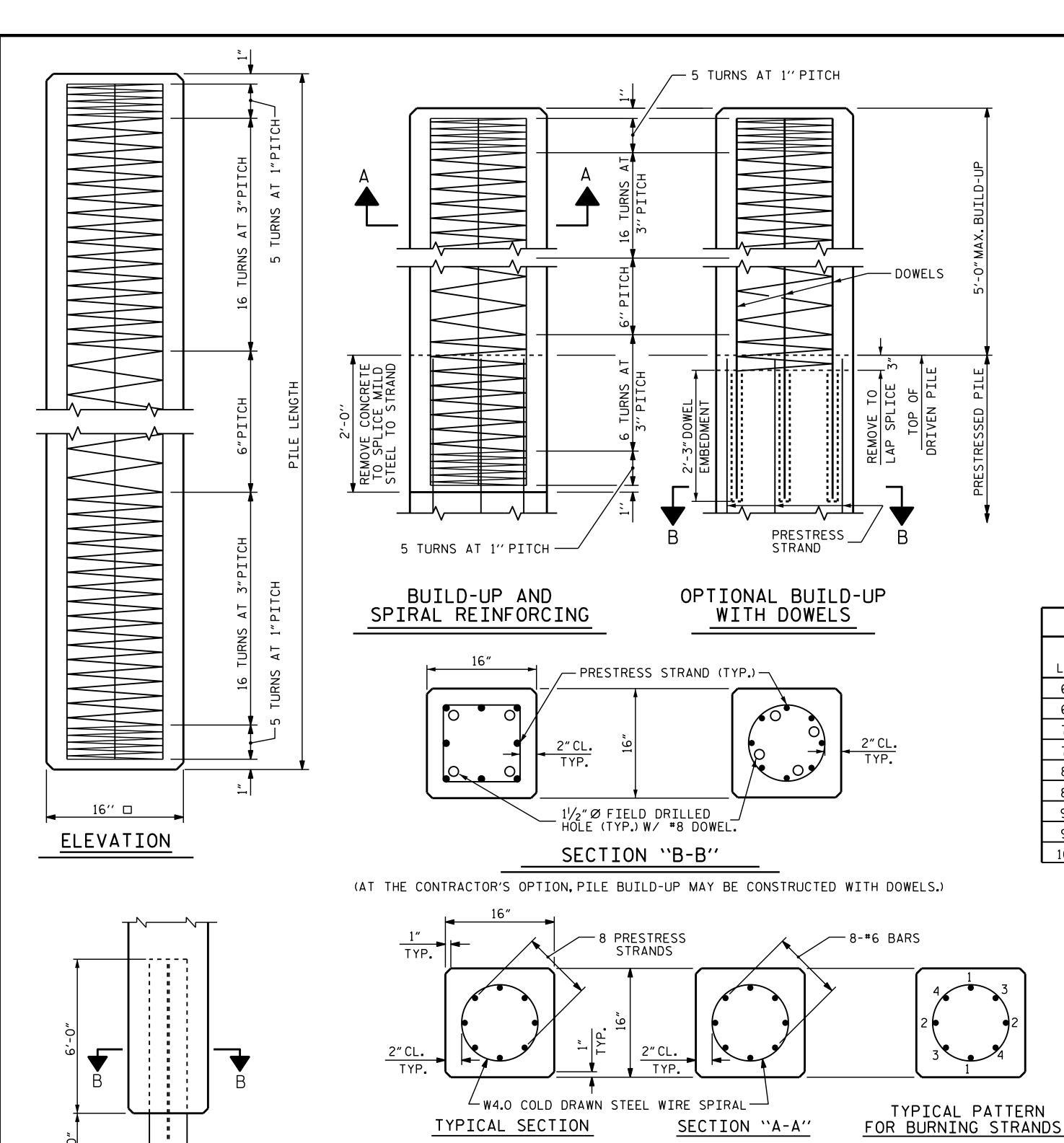
SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

4 - 4 - 4 - 4 - 4							
10/4/2019			REVI	SION	۱S		SHEET NO.
DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S-12
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			17







— HP 8 × 36 OR W 8 X 35

W4.0 COLD DRAWN
STEEL WIRE SPIRAL

- PRESTRESSING

STRANDS

WMC/GM

MAA/GM

TYP.

2"CL.

TYP.

ELEVATION

SECTION B-B

PILE TIP DETAILS

ASSEMBLED BY: M.M. AHMED DATE: 3-4-15 CHECKED BY: P.N. HOLDER DATE: 3-17-15

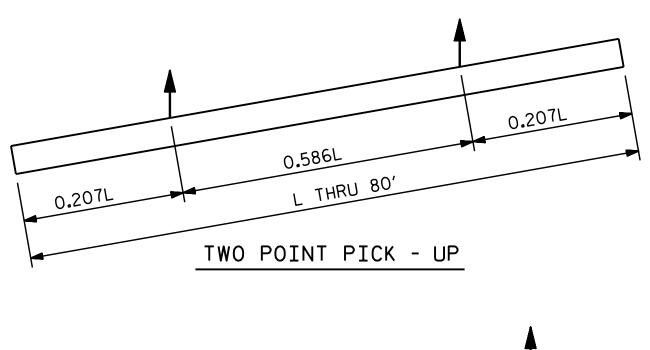
FOR 16" SQUARE PRESTRESSED CONCRETE PILE

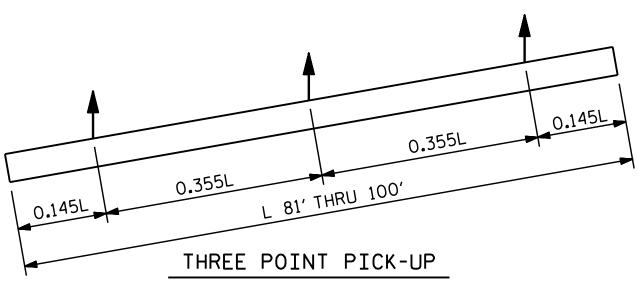
REV.11/30/10

HP 8 x 36 — OR W 8 X 35

DRAWN BY: RH 9/98

CHECKED BY: LES 10/98 REV. 10/1/11 REV. 12/14





PICK - UP POINTS

QUANTITIES FOR ONE 16"PRESTRESSED PILE								
	CONCRETE	PILE WT.	TWO POIN	T PICK-UP	THREE POIN	IT PICK-UP		
LENGTH	CU. YDS.	TONS	0.207L	0 . 586L	0.145L	0 . 355L		
60'-0"	3.92	7.94	12′-5″	35′-2"				
65'-0"	4.25	8.60	13'-5 ¹ / ₂ "	38'-1"				
70'-0"	4.57	9.26	14'-6"	41'-0"				
75′-0"	4.90	9.92	15′-6 ¹ / ₂ "	43′-11″				
80'-0"	5.23	10.58	16′-7″	46′-10″				
85′-0′′	5 . 55	11.24			12'-4"	30′-2″		
90'-0''	5.88	11.91			13'-1"	32'-0"		
95′-0′′	6.21	12.58			13'-9"	33'-9"		
100'-0''	6.53	13.22			14'-6"	35′-6″		

STRAND DATA:

SIZE	GRADE	AREA	ULTIMATE STRENGTH	APPLIED PRESTRESS FORCE
1/2"	270 L.R.	0.153	41,300# PER STRAND	30,980# PER STRAND
0.6"	270 L.R.	0.217	58,600# PER STRAND	43,940# PER STRAND

NOTES

PRESTRESSED CONCRETE STRENGTH : f'c = 7,500 PSI
BUILD-UP CONCRETE STRENGTH : f'c = 7,500 PSI

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS CONFORMING TO AASHTO M203. STRAND SAMPLING REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

AT THE CONTRACTOR'S OPTION, 1/2" OR 0.6" STRANDS MAY BE USED IN EITHER STRAND CONFIGURATION SHOWN IN THE TYPICAL SECTION DETAIL. MIXING OF STRAND SIZE IS NOT ALLOWED.

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED.

TRANSFER THE LOAD FROM THE ANCHORAGES TO THE PILE AFTER THE CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN OPPOSITE PAIRS AS INDICATED IN THE TYPICAL PATTERN SHOWN. FOR ANY NUMBER OF STRANDS, BURN IN OPPOSITE PAIRS AND SYMMETRICALLY ABOUT BOTH THE VERTICAL AND HORIZONTAL AXES. STRANDS 1-1 SHALL BE BURNED BEFORE 2-2, ETC. NOT MORE THAN 4 STRANDS, SAY 3-3 AND 4-4, MAY BE BURNED AT ANY ONE SECTION BEFORE THESE SAME PAIRS OF STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS ARE TO BE INDICATED WITH A 2" WIDE BLACK MARK.

DRIVE PILES USING A METHOD APPROVED BY THE ENGINEER, WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

DRIVING OF THE BUILT-UP PILE WILL NOT BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 5,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

THE WATER/CEMENT RATIO FOR CONCRETE PILES SHALL NOT EXCEED 0.40.

PRESTRESSED PILES SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONCRETE IN THE PILES OF BENT NO.1 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

DOWEL INSTALLATION FOR OPTIONAL BUILD-UP

GROUT COMPRESSIVE STRENGTH: f'c= 5,000 PSI

BEFORE DRILLING DOWEL HOLES, REMOVE THE UPPER 3"OF CONCRETE FROM THE TOP OF THE PILE WITHOUT DAMAGE TO THE REINFORCING STEEL. THE REMOVAL PLANE SHOULD BE NORMAL TO THE EDGE OF THE PILE.

DOWEL HOLES SHALL BE POSITIONED TO MAINTAIN $\frac{1}{2}$ CLEAR TO ALL EXISTING PRESTRESSING STRANDS IN THE CONCRETE PILE.

FIELD DRILLED HOLES SHALL BE CLEAN AND FREE OF ANY OBSTRUCTIONS BEFORE GROUTING OF DOWELS. DOWEL BARS SHALL BE INSTALLED AND GROUTED WITH AN APPROVED NON-SHRINK GROUT.

THE SPIRAL REINFORCING IN ALL BUILD-UPS SHALL BE W4.0 COLD DRAWN WIRE WHICH SHALL BE SECURED TO THE LONGITUDINAL REINFORCEMENT TO MAINTAIN PITCH.

THE SPIRAL REINFORCING IN THE BUILD-UP AND THE PRESTRESSED CONCRETE PILE SHALL BE SPLICED BY OVERLAPPING A MIN. OF ONE TURN.

PROJECT NO. 17BP.1.R.63

HYDE county

STATION: 13+24.00 -L-

SHEET 3 OF 3

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

16" PRESTRESSED CONCRETE PILE



Docusigned by: 3/27/2015

Lickey

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	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-15
		3			TOTAL SHEETS
		4			17

SECTION "A-A"

1/2" OR 0.6" Ø GRADE 270 L.R. PRESTRESS STRANDS

'OR 0.6" Ø GRADE 270 L.R. PRESTRESS STRANDS

-8-#6 BARS

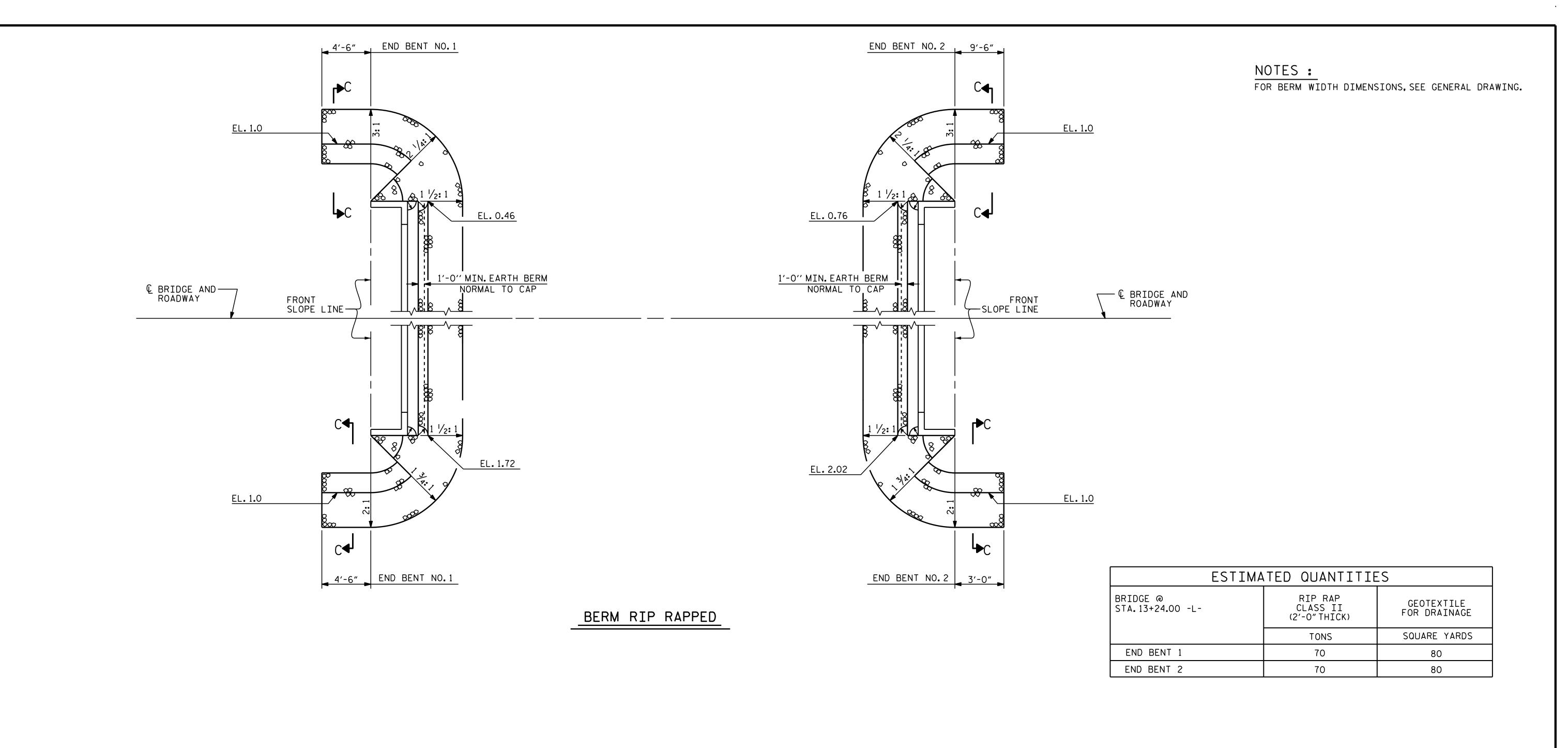
TYPICAL PATTERN

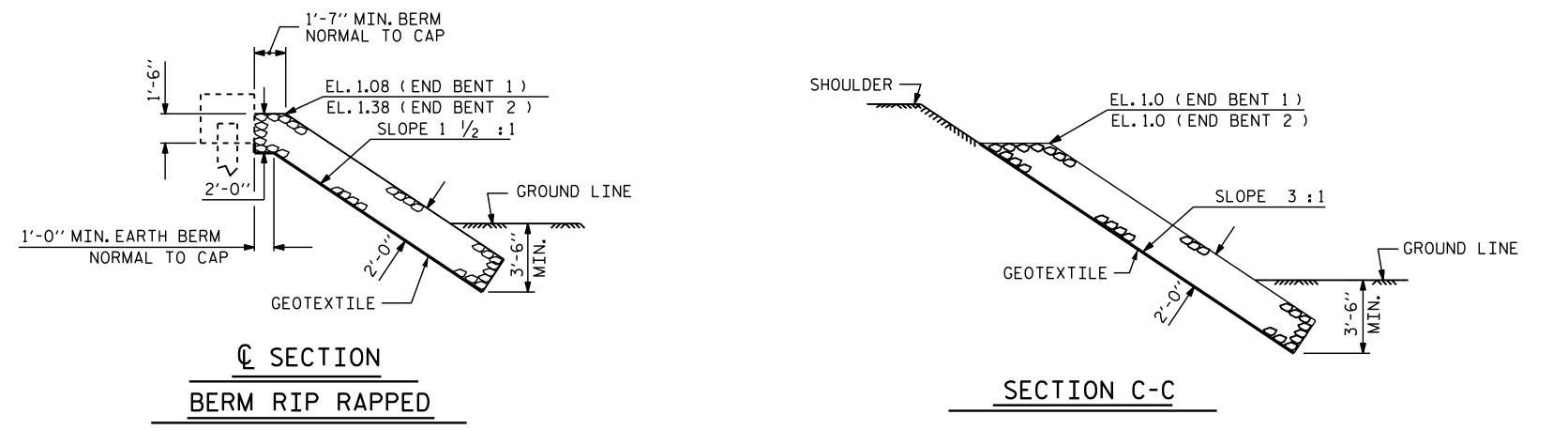
FOR BURNING STRANDS

8 PRESTRESS STRANDS

TYPICAL SECTION

TYP.





PROJECT NO. 17BP.1.R.63

HYDE county

STATION: 13+24.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

---RIP RAP DETAILS---

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-16

1 3 TOTAL SHEETS
17

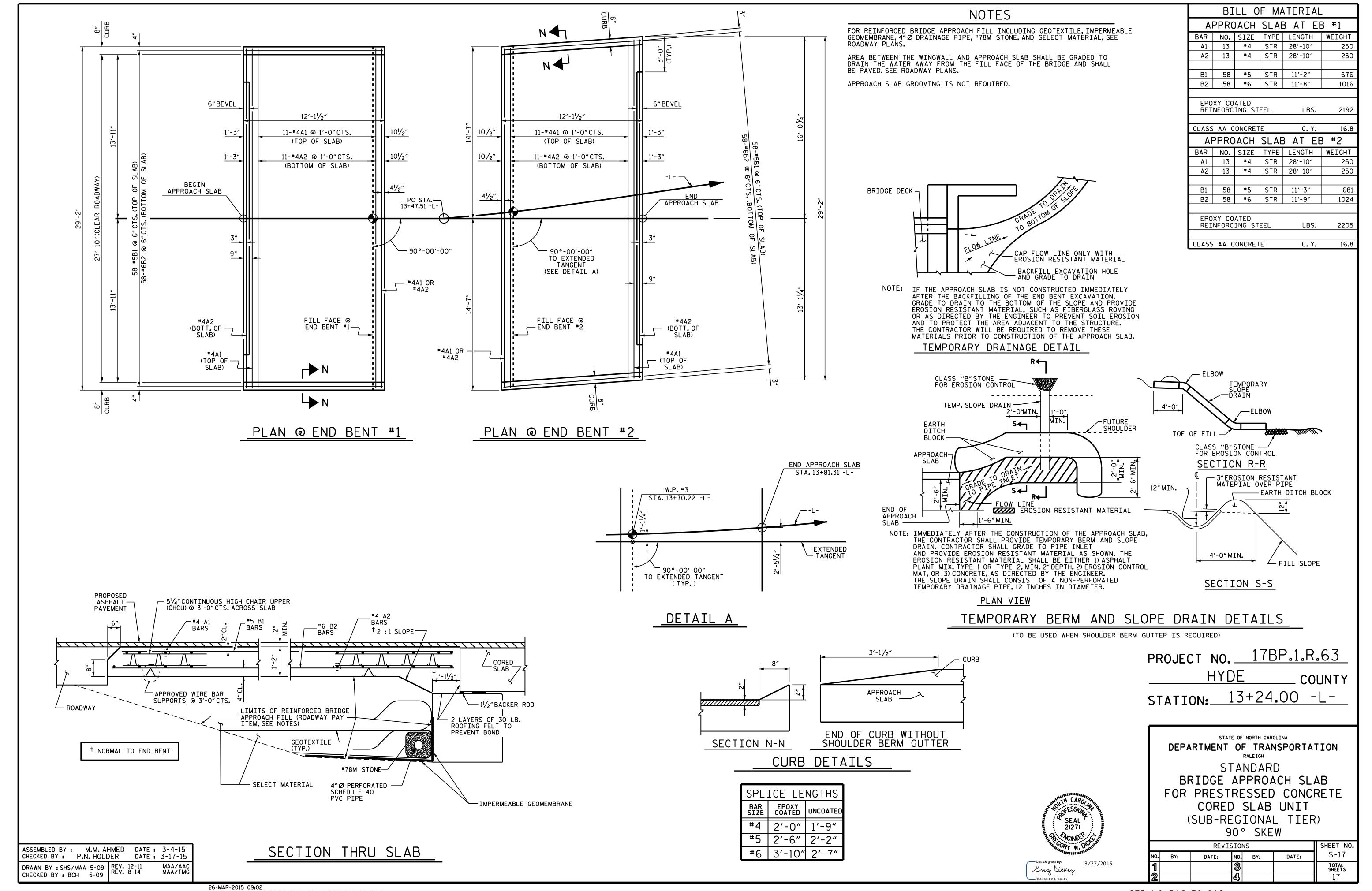
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ASSEMBLED BY: M.M. AHMED DATE: 3-4-15 CHECKED BY: P.N. HOLDER DATE: 3-17-15

DRAWN BY: REK 1/84 CHECKED BY: RDU 1/84 REV. 5/I/06R REV. I0/I/II REV. I2/2I/II TLA/GM MAA/GM MAA/GM



STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS ---- SEE PLANS STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION - GRADE 60 - - - 24.000 LBS. PER SQ. IN. CONCRETE IN SHEAR - - - - - - - - - - - SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ---- 375 LBS. PER SQ. IN. EQUIVALENT FLUID PRESSURE OF EARTH ---- 30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{1}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

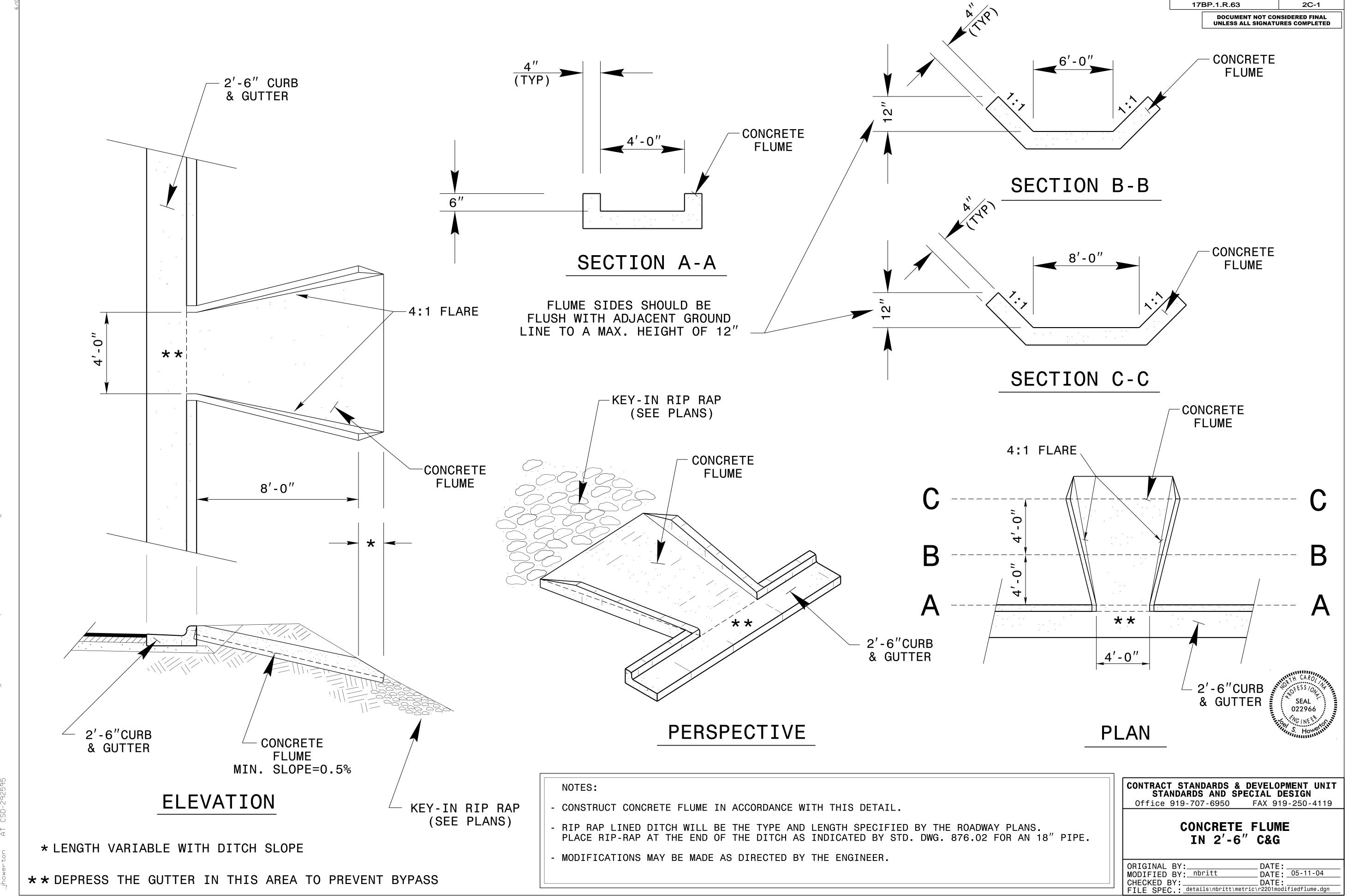
METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

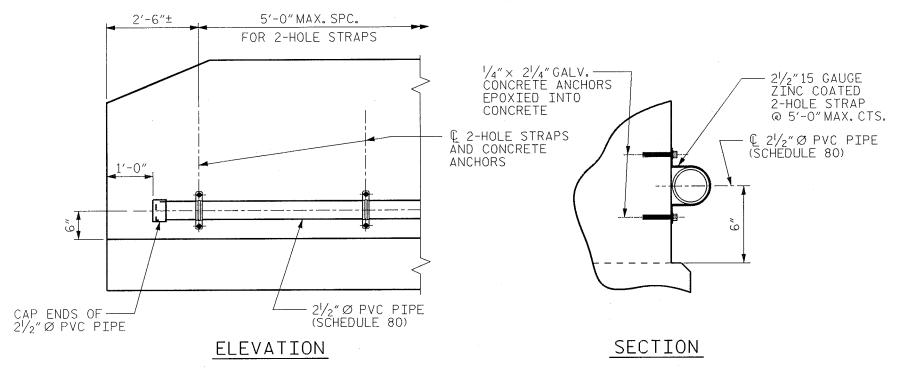
JANUARY, 1990



PROJECT REFERENCE NO.

SHEET NO.

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FIBER OPTIC CONDUIT SYSTEM DETAILS

 $2\frac{1}{2}$ % SCHEDULE 80 PVC PIPE ATTACHED TO THE BACK OF BOTH RAILS FOR FUTURE FIBER OPTIC CABLE.

ADD TO NOTES:

FOR FIBER OPTIC CONDUIT SYSTEM, SEE SPECIAL PROVISIONS.

