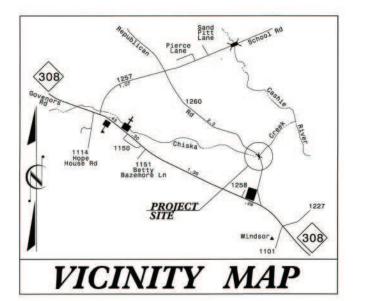
See Sheet 1-A For Index of Sheets



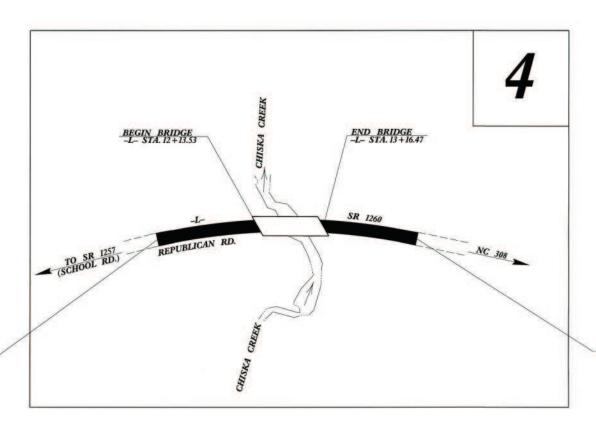
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

BERTIE COUNTY

LOCATION: BRIDGE NO. 43 OVER CHISKA CREEK ON SR 1260 (REPUBLICAN RD.)

TYPE OF WORK: GRADING, DRAINAGE, PAVING & STRUCTURE

STATE	STATE I	PROJECT REPERENCE NO.	NO.	SHEETS
N.C.	17E	3P.1.R.18	1	
STATE PE	IOJ. NO.	P. A. PROJ. NO.	DESCRIPT	TION
17BP.1	R.18		PE, RW,	UTIL.
			CON	ST.
			12 40.00	
			_	
			-	-



BEGIN PROJECT 17BP.1.R.18 -L- STA. 10 + 55.00

END PROJECT 17BP.1.R.18 -L- STA. 14+65.00

ES OWN

GRAPHIC SCALES 50 25 0 50 25 0 PROFILE (HORIZONTAL)

PROFILE (VERTICAL)

D = 50 %T = 8 % * V = 45 MPH* TTST = 3% DUAL = 5% FUNC CLASS = LOCAL SUBREGIONAL TIER

DESIGN DATA

DHV = 10 %

ADT 2009 = 570

ADT 2032 = 1125

PROJECT LENGTH

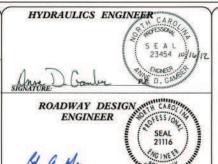
LENGTH ROADWAY TIP PROJECT 17BP.1.R.18 LENGTH STRUCTURES TIP PROJECT 17BP.1.R.18 TOTAL LENGTH TIP PROJECT 17BP.1.R.18

= 0.019 ML= 0.078 MI.

ETHERILL 2012 STANDARD SPECIFICATIONS RIGHT OF WAY DATE: EDWARD G. WETHERILL, PE

LETTING DATE:

BOB A. MAY, PE





GENERAL NOTES

GENERAL NOTES:

2012 SPECIFICATIONS

EFFECTIVE: 01-17-12

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II - MODIFIED.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01.

UNDERDRAINS:

UNDERDRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.03 AT

GUARDRAIL .

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

TEMPORARY SHORING:

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC WILL BE PAID FOR AS "EXTRA WORK" IN ACCORDANCE WITH SECTION 104-7.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE ROANOKE ELECTRIC CORPORATION AND CENTURYLINK

ANY RELOCATION OF EXISTING UTILITIES WILL ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

PROJECT REFERENCE NO. SHEET NO.



LIST OF STANDARDS

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch – N. C. Department of Transportation – Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO.

DIVISION 2 - EARTHWORK

200.02

225.02

Method of Clearing – Method II, Modified Guide for Grading Subgrade – Secondary and Local Method of Obtaining Superelevation – Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation

DIVISION 4 - MAJOR STRUCTURES

Reinforced Bridge Approach Fills - Sub Regional Tier

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

Pipe Underdrain and Blind Drain Concrete Base Pad for Drainage Structures Frames and Narrow Slot Flat Grates 815.03

S-1 THRU S-20

840.00

840.35 Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates 840.46

Traffic Bearing Precast Drainage Structure Concrete Curb, Gutter and Curb & Gutter 846.01

Drop Inlet Installation in Shoulder Berm Gutter

862.01 **Guardrail Placement**

862.02 Guardrail Installation Structure Anchor Units

INDEX OF SHEETS

SHEET NUMBER	SHEET
1	TITLE SHEET
1-A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS
1-B	CONVENTIONAL SYMBOLS
2	PAYEMENT SCHEDULE, TYPICAL SECTIONS, AND WEDGING DETAILS
3	SUMMARY OF DRAINAGE QUANTITIES, SUMMARY OF GUARDRAIL, EARTHWORK SUMMARY, ASPHALT PAYEMENT REMOVAL SUMMARY AND SHOULDER BERM GUTTER SUMMARY
4	PLAN & PROFILE SHEET
TCP-1	TRAFFIC CONTROL PLANS
EC-1 THRU EC-3B	EROSION CONTROL PLANS
UO-1	UTILITIES BY OTHERS PLANS
X-1A	CROSS-SECTION SUMMARY SHEET
X-1 THRU X-4	CROSS-SECTIONS

STRUCTURE PLANS (BRIDGE #43)

PROJECT	REFERENCE	NO.
17E	BPJ.RJB	

*S.U.E. = Subsurface Utility Engineering

BOUNDARIES AND PROPERTY:

CONVENTIONAL PLAN SHEET SYMBOLS

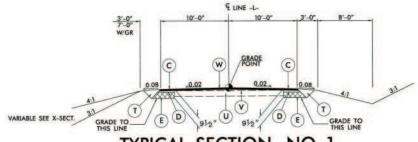
State Line	
County Line	
Township Line	
City Line	
Reservation Line	
Property Line	4
Existing Iron Pin	EIP
Property Corner	
Property Monument	ECM
Parcel/Sequence Number	(123)
Existing Fence Line	xx
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	EAB
Existing Endangered Plant Boundary	- EPH-
Known Soil Contamination: Area or Site	
Potential Soil Contamination: Area or Site	
BUILDINGS AND OTHER CULT	TURE:
Gas Pump Vent or U/G Tank Cap	
Sign —	
Well	- W
Small Mine	- *
Foundation	*1000
Area Outline	
Cemetery	
Building	
SCHOOL	
Church — — — — — — — — — — — — — — — — — — —	215
HYDROLOGY: Stream or Body of Water —	
U. I. D. I. D.	
Hydro, Pool or Reservoir	
Jurisdictional Stream	
Jurisdictional Stream Buffer Zone 1	
Jurisdictional Stream Buffer Zone 1 Buffer Zone 2	BZ 1 ———————————————————————————————————
Jurisdictional Stream	BZ 2
Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream	BZ 1 BZ 2
Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring	BZ 1
Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream	BZ 1 BZ 2 BZ 2

Standard Gauge	1111111
RR Signal Milepost	CSX TRANSPORTATIO
Switch	WILEPOST 35
RR Abandoned	SWITCH
RR Dismantled	
RIGHT OF WAY:	
	•
Baseline Control Point	^
Existing Right of Way Marker	\triangle
Existing Right of Way Line	R
Proposed Right of Way Line	(W)
Proposed Right of Way Line with Iron Pin and Cap Marker	
Proposed Right of Way Line with Concrete or Granite RW Marker	
Proposed Control of Access Line with Concrete C/A Marker	® \$
Existing Control of Access	(<u>\$</u>)
Proposed Control of Access	<u> </u>
Existing Easement Line	——E——
Proposed Temporary Construction Easement -	—
Proposed Temporary Drainage Easement —	TDE
Proposed Permanent Drainage Easement —	PDE
Proposed Permanent Drainage / Utility Easement	
Proposed Permanent Utility Easement —	PUE
Proposed Temporary Utility Easement —	TUE
Proposed Aerial Utility Easement	—— AUE—
Proposed Permanent Easement with Iron Pin and Cap Marker	•
ROADS AND RELATED FEATURE	S:
Existing Edge of Pavement	
Existing Curb	
Proposed Slope Stakes Cut	<u>c</u>
Proposed Slope Stakes Fill	<u>F</u>
Proposed Curb Ramp	CR
Existing Metal Guardrail	
Proposed Guardrail	
Existing Cable Guiderail	_ в _ п _ п
Proposed Cable Guiderail	
Equality Symbol	•
Pavement Removal	XXXXX
VEGETATION:	T V V V
Single Tree	ch:
Single Shrub	٥
Hedge	~~~~~~

Orchard ————————————————————————————————————	0 0 0 0
rineyard ————————————————————————————————————	Vineyard
EXISTING STRUCTURES:	
MAIOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall	
NINOR:	
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge	
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole	(5)
Storm Sewer	s
7.7.111 (47.7.7.1	
UTILITIES:	
OWER:	
Existing Power Pole	•
Proposed Power Pole	6
Existing Joint Use Pole	•
Proposed Joint Use Pole	-
Power Manhole	©
Power Line Tower	
Power Transformer	M
U/G Power Cable Hand Hole	
H-Frame Pole	• •
Recorded U/G Power Line	P
Designated U/G Power Line (S.U.E.*)	
ELEPHONE:	
Existing Telephone Pole	•
Proposed Telephone Pole	-0-
Telephone Manhole	1
Telephone Booth	2
Telephone Pedestal	T
Telephone Cell Tower	
U/G Telephone Cable Hand Hole	FH
Recorded U/G Telephone Cable	
Designated U/G Telephone Cable (S.U.E.*)	
Recorded U/G Telephone Conduit	
Designated U/G Telephone Conduit (S.U.E.*)	
Recorded U/G Fiber Optics Cable	
Designated U/G Fiber Optics Cable (S.U.E.*)	

WATER:	
Water Manhole	(4)
Water Meter	
Water Valve	
Water Hydrant	
Recorded U/G Water Line	
Designated U/G Water Line (S.U.E.*) Above Ground Water Line	1.3274
Above Ground Water Line	A/G Water
TV:	
TV Satellite Dish	K
TV Pedestal	
TV Tower	
U/G TV Cable Hand Hole	円田
Recorded U/G TV Cable	
Designated U/G TV Cable (S.U.E.*)	
Recorded U/G Fiber Optic Cable	
Designated U∕G Fiber Optic Cable (S.U.E.*)—	tv Fq
GAS:	
Gas Valve	
Gas Meter	
Recorded U/G Gas Line	
Designated U/G Gas Line (S.U.E.*)	
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	
Sanitary Sewer Cleanout	
U/G Sanitary Sewer Line	
Above Ground Sanitary Sewer	
Recorded SS Forced Main Line	
Designated SS Forced Main Line (S.U.E.*) —	
MISCELLANICOLIS.	
MISCELLANEOUS: Utility Pole	
Utility Pole with Base	
Utility Located Object	
Utility Traffic Signal Box	0
	(5)
Utility Unknown U/G Line	
U/G Tank; Water, Gas, Oil	Man Toronto
Underground Storage Tank, Approx. Loc.	(UST)
A/G Tank; Water, Gas, Oil	
Geoenvironmental Boring	●
U/G Test Hole (S.U.E.*)	•
Abandoned According to Utility Records —	AATUR
End of Information —	E.O.I.

Detail Showing Method of Wedging



TYPICAL SECTION NO. 1

USE TYPICAL SECTION NO. 1 AS FOLLOWS:

-L. STA. 10+55.00 TO -L. STA. 11+80.00

-L. STA. 13+40.00 TO -L. STA. 14+65.00

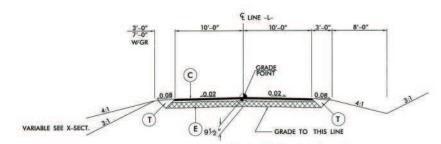
NOTE: MILL EXISTING PAVEMENT AT THE FOLLOWING LOCATIONS:

-L- STA. 10+55.00 TO -L- STA. 11+80.65

-L- STA. 13+62.31 TO -L- STA. 14+65.00

	PAVEMENT SCHEDULE (FINAL PAVEMENT DESIGN)
С	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C1	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT TO EXCEED 1½" IN DEPTH.
D	PROP. APPROX. 2½" ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE I19.0B, AT AN AVERAGE RATE OF 285 LBS. PER SQ. YD.
D1	PROP. VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 119.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH, TO BE PLACED IN LAYERS NOT LESS THAN 2½" IN DEPTH OR GREATER THAN 4" IN DEPTH.
E	PROP. APPROX. 4" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 456 LBS. PER SQ. YD.
E1	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5½" IN DEPTH.
T	EARTH MATERIAL.
U	EXISTING PAVEMENT.
٧	MILLING BITUMINOUS PAVEMENT. 3" TO 0" DEPTH.

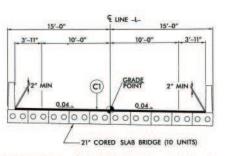
NOTE: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.



TYPICAL SECTION NO. 2

USE TYPICAL SECTION NO. 2 AS FOLLOWS:

-L- STA. 11+80.00 TO -L- STA. 12+13.53 (BEGIN BRIDGE)
-L- STA. 13+16.47 (END BRIDGE) TO -L- STA. 13+40.00



TYPICAL SECTION NO. 3

USE TYPICAL SECTION NO. 3 AS FOLLOWS:

-L- STA. 12+13.53 (BEGIN BRIDGE #43) TO -L- STA. 13+16.47 (END BRIDGE #43)

ertie "43/Roadway/Froj/SK"43-Ray-typ.a



59 Jones Franklin Rd. Suite 16 Roleigh, N.C. 27606 Liscense No. F-0377 Bus: 919 851 8071 Fox: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN CNIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO. 17BP.I.R.18

SUMMARY OF EARTHWORK

STATION	STATION	UNCL. EXCAV.	EMBANK.	BORROW	WASTE
10+55.00	12+13.53	6	93	87	
SUBTO	TALS:				
13+16.47	14+65.00	7	24	17	
SUBTO	TALS:	7	24	17	
PROJECT	TOTALS:	13	117	104	
5% TO REPLACE TO	PSOIL ON BORROW	PIT		5	
GRAND	TOTALS:	13	117	109	
SA	y.	50	117	150	

Earthwork quantities are calculated by the Wetherill Engineering, Inc.. These earthwork quantities are based in part on subsurface data provided by the SUMMIT Design Engineering Services, PLLC.

PAVEMENT REMOVAL SUMMARY

SURVEY	STATION	STATION	LOCATION LT/RT/CL	YD'
-L-	11+80.00	12+43.17	CL	147.59
4-	12+96.95	13 + 40.00	CL	99.11
				Tax state
			TOTAL:	246.70
			SAY:	250.00

Note: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Shoulder Borrow, Fine Grading, Clearing and Grubbing, Breaking of Existing Pavement, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading".

SHOULDER BERM GUTTER SUMMARY

SURVEY	STATION	STATION	LENGTH
-L- RT	11+83.00	12+09.79	26.79
		TOTAL:	26.79
		SAY:	30.00

NOTE: Invert Elevations are for Bid Purposes only and shall not be used for project construction stakeout. See "Standard Specifications For Roads and Structures, Section 300–5".

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

ON (LT,RT, OR CU	ILT,RT, OR CL)	STRUCTURE NO.	ATION	NOTEN		EVATION	(1	RCP, C	DRAINAGE PIPE P, CAAP, HDPE, or PYC)			5)	C.S. PIPE (UNLESS NOTED OTHRWISE)							CLASS (UNLESS O			5 III R.C. PIPE THERWISE NOTED)					STD STD (U	. 838.01, 0. 838.11 OR . 838.80 INLESS IOTED HERWISE)	QUANTITIES FOR DRAINAGE	THE TOTAL LF. FOR PA	'A' + (1.3 X C		FRAME, AND STANDAR	GRATES HOOD D 840.03	STD. 840.15	STD. 840.16	840.17 OR 840.26	840.18 OR 840.27 840.19 OR 840.28	SRATE STD. 840.22	IWO GRATES STD. 840.22	VITH GRATE STD. 840.24 TH TWO GRATES STD. 840.2	840.32				danger or our	NO. & SIZE "B" C.Y. STD 840.72	PLUG, C.Y. STD. 840.71	C.B. N.D.I D.I. G.D.I	CA NA DR GR (N.S.) GR	REVIATIONS TCH BASIN RROW DROP DP INLET ATED DROP IN ATED DROP IN RROW SLOTI	NLET	
SIZE	ГОСАПОР	1	TOP ELEV	INVEST E		INVERT EL	12*	15" 1	24"	30" 3	6" 42"	48"	12" 15	18"	24"	30		6"	42"	48"	12"	15" 18	24"	30" 3	36" 42"	48"	NN PIPE	AIN PIPE	C	U. YDS.)' THRU 5.0')	A ,0.0	B &				840.14 OR	ME & GRATE	PE "A" STD.	PE "B" STD.	SAME WITH O	SAME WITH 1	S. FRAME W	840.31 OR	STD. 840.35			SWOOT	TEEL ELBOWS	& BRICK PIPE	J.B. M.H. T.B.D.	I. TRA		G DROP INLE
THICKNESS OR GAUGE		FROM											.064	.064	.064	620.	620	901		.109							15" SIDE DRA	18" SIDE DRA	R.C.P.	C.S.P.	PER EACH (C	0, TH	10.0' AND A	E	TYPE O	F GRATE	D.I. STD.	D.I. FRAN	0	G.D.I. TY	0	G.D.I. F8	GDI.IN	J.B. STD	T.B.D.I.			S BBCC	CORR. S	CONC.	PIPE REA	100	REMARKS	
-L- 11+90	RT 4	101	17.99	9 13.	71																										1	7002	10 0			1000							1		1									
	4	101 402		13.	71 1.	3.60		12																									-	-						-									1		-			
	-			+	-	+		+	-			H			+	+	+			+			-		+		+	+	+			1	+	-					+			+	+								+			
	_		-	-	-			_	-		-		-	-	-		-			-	-	-	-		-		_	-	-	-	1000		-	_	-							_	120			-			+	_	-			

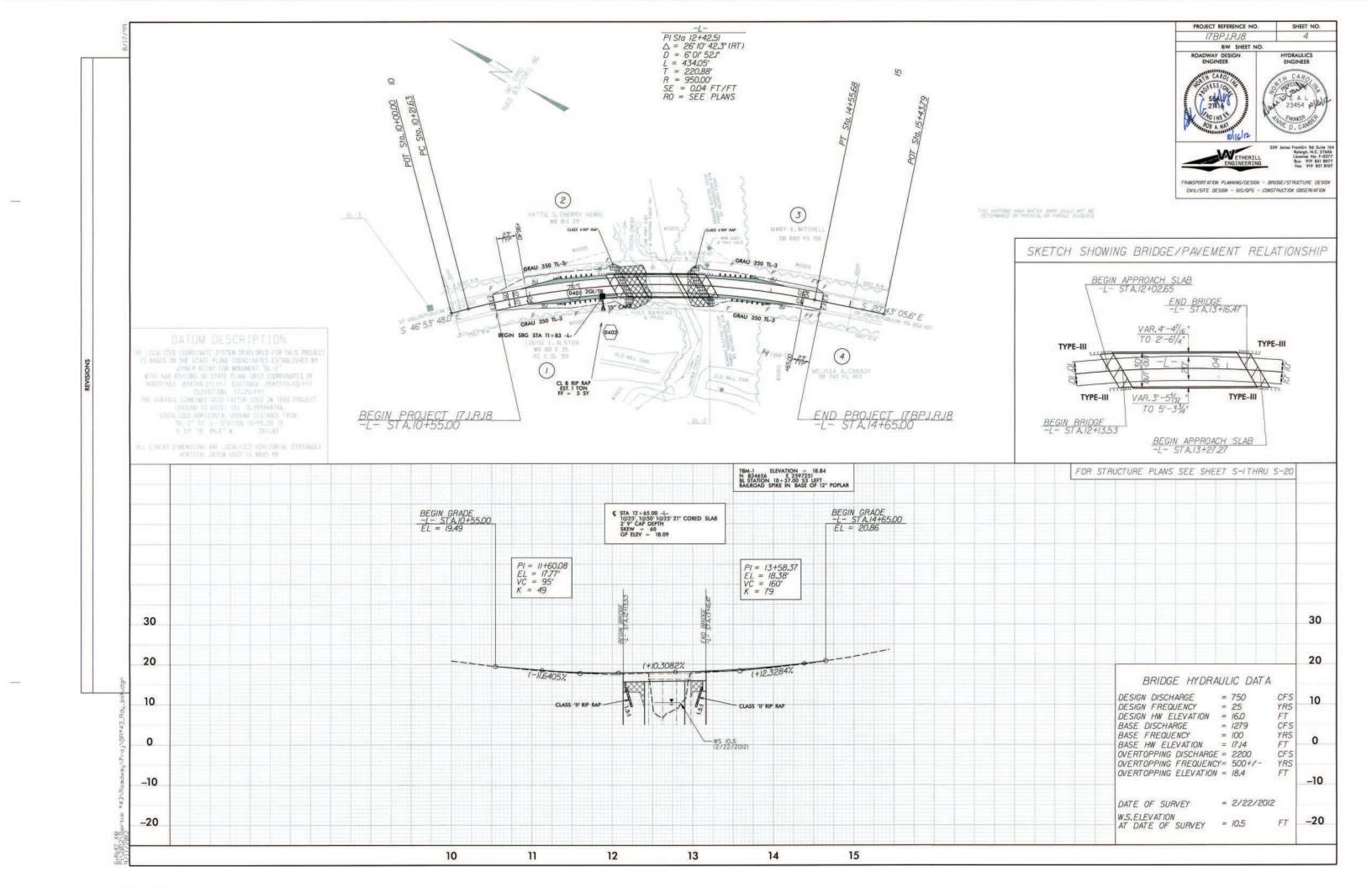
"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

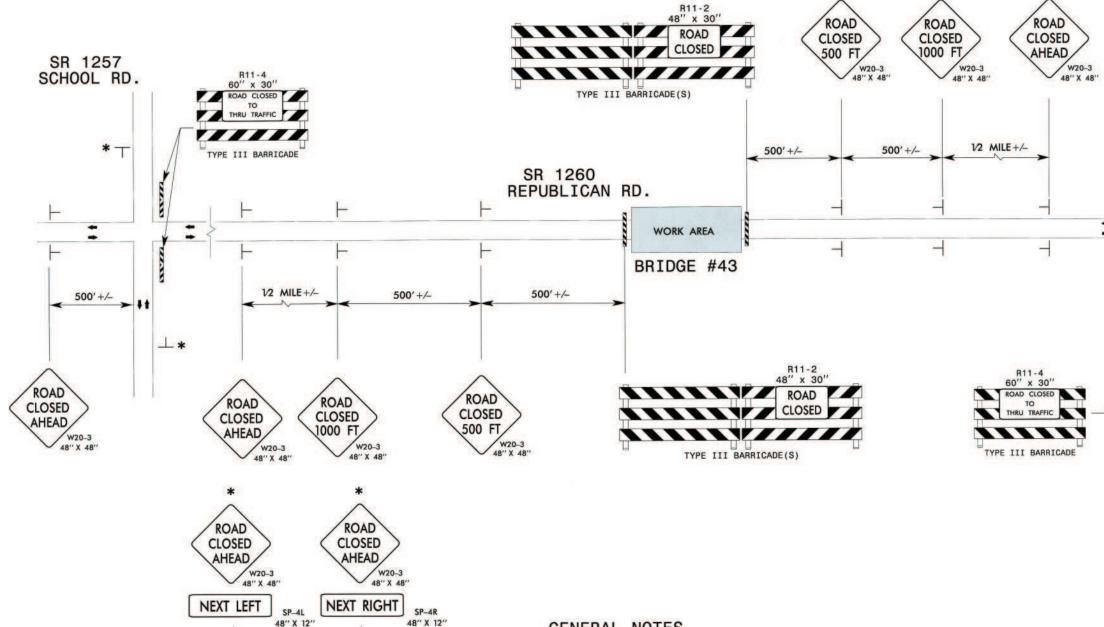
TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

SURVEY	BEG, STA.	END STA.	LOCATION	LENGTH			WARRANT POINT		"N" DIST.	TOTAL	FLARE LENGTH		w		ANCHORS					IMPACT ATTENUATOR TYPE 350	SINGLE	REMOVE EXISTING	REMOVE AND STOCKPILE	REMARKS				
				STRAIGHT	SHOP	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	WIDTH	M SHOUL.	APPROACH END	TRAILING END	APPROACH END	TRAILING	XI MOD	TYPE	GRAU 350	M-350	XIII	CAT-1	MOD	BIC	AT-1	EA G NO	GUARDRAIL	GUARDRAIL	
-1-	11+31.39	12+06.39	LT	75.00				12+06,39	4.36	7.36		50.00		1.00		- 1	1											
4-	11+32.76	12+20.88	RT	88.12			12+20.88		4.30'	7.30'	50.00		1.00'			1	1_											
4-	13+07.84	13+82.84	LT	75.00			13+07.84		3.50	7.00'	50.00'		1.00'			1	1											
-1-	13+26.05	14+01.05	RT	75.00				13+26.05	3.42'	7.00'		50.00'	- 1	1.00		1	1											
PROJECT SU	IBTOTAL			313.12												4	4											
											ADDITION	AL GUARDRAII	POSTS = 5 E	ACH						ANCHOR	DEDUCTION	ONS		1				
LESS ANCH	ORS			(-) 275.00																TYPE III =	4 @ 18	3.75' =	75.00					
PROJECT T	DTAL			38.12'																GRAU 350) = 4@	50.00' =	200.00'					
				111111111111111111111111111111111111111													÷						275.00					
SAY				50.00																								





ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

INSTALL 500' +/- PRIOR TO EXISTING INTERSECTION

STD. NO.

TITLE

1101.03	TEMPORARY ROAD CLOSURES
1101.04	TEMPORARY SHOULDER CLOSURES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1135.01	CONES
1145.01	BARRICADES
904.10	ORIENTATION OF GROUND MOUNTED SIGNS

GENERAL NOTES

- 1. INSTALLATION OF TEMPORARY ROUTE MARKERS, DESTINATION SIGNS, AND ANY NECESSARY MODIFICATIONS TO EXISTING OR PROPOSED REGULATORY OR WARNING SIGNS WILL BE MADE BY OTHERS (STATE OR CITY FORCES) UNLESS OTHERWISE DESIGNATED IN PLANS. PROVIDE A MINIMUM 21 CALENDAR DAY NOTICE TO STATE FORCES BEFORE A ROADWAY IS CLOSED TO TRAFFIC SUCH THAT THE NECESSARY PROVISIONS CAN BE MADE TO INFORM LOCAL EMERGENCY AND LAW ENFORCEMENT PERSONNEL, SCHOOLS OR ANY OTHER PARTIES AFFECTED BY THE ROAD CLOSURE.
- 2. INSTALL SIGNS BEFORE THE BARRICADES WHEN CLOSING THE ROADWAY TO TRAFFIC. REMOVE BARRICADES BEFORE SIGNS WHEN OPENING THE ROADWAY TO TRAFFIC. INSTALL/REMOVE SIGNS AND BARRICADES WITHIN THE SAME CALENDAR DAY.
- 3. POSITION WING BARRICADES ON THE SHOULDERS AND SLOPE THE STRIPES DOWNWARD IN THE DIRECTION TOWARD WHICH TRAFFIC MUST TURN IN DETOURING.
- 4. USE ADDITIONAL TYPE III BARRICADES IN STAGGERED LOCATIONS SUPPLEMENTED WITH SIGN R11-4 "ROAD CLOSED TO THRU TRAFFIC" IN THE EVENT THAT TRAFFIC MUST BE MAINTAINED BEYOND THE DETOUR POINT.
- 5. SEE STANDARD SPECIFICATION 1089-1 FOR WORK ZONE SIGNS.
- 6. SEE STANDARD SPECIFICATION 1089-2 FOR WORK ZONE SIGN SUPPORTS.

LEGEND

DIRECTION OF TRAFFIC FLOW

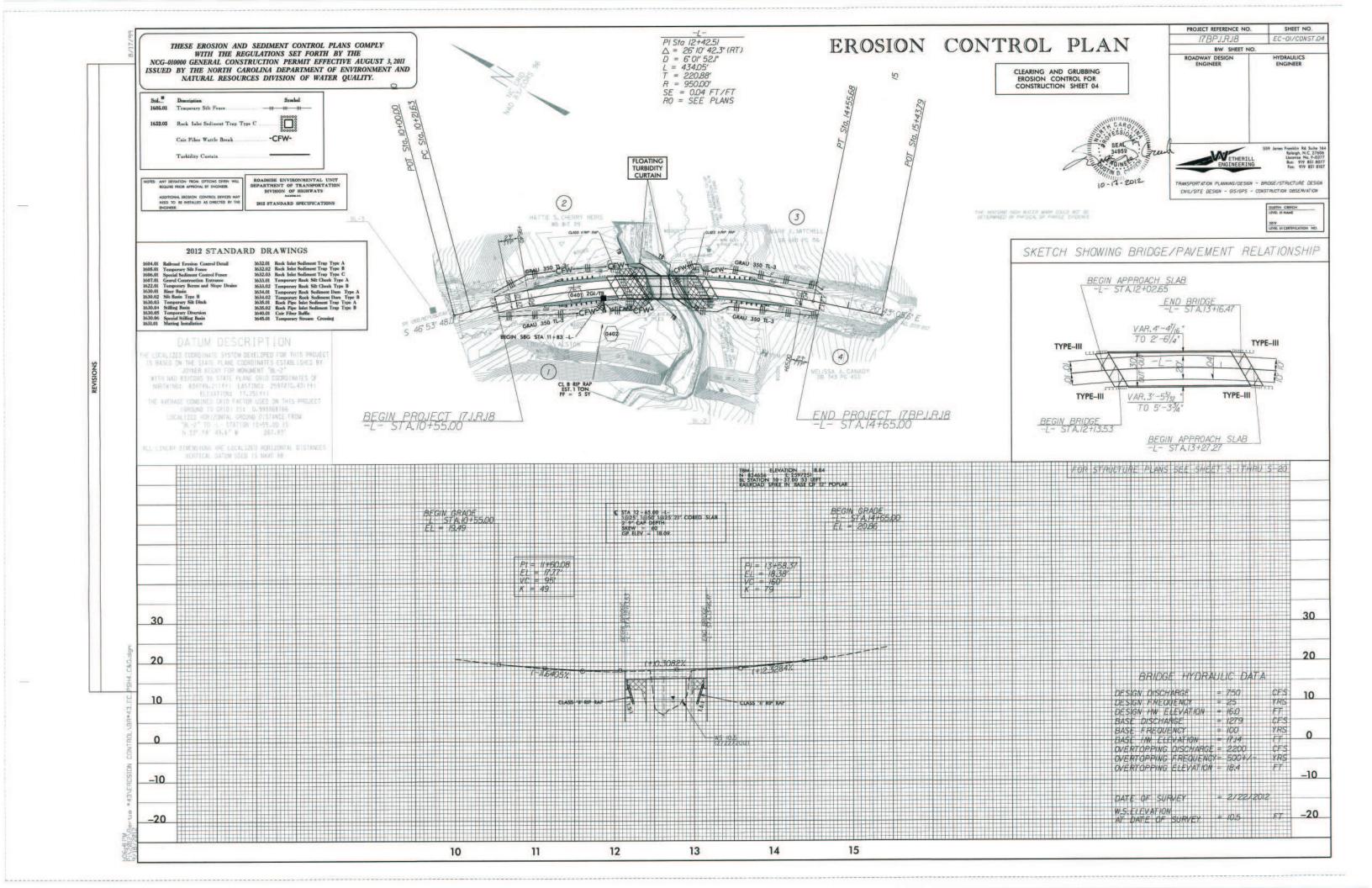
BARRICADE (TYPE III)

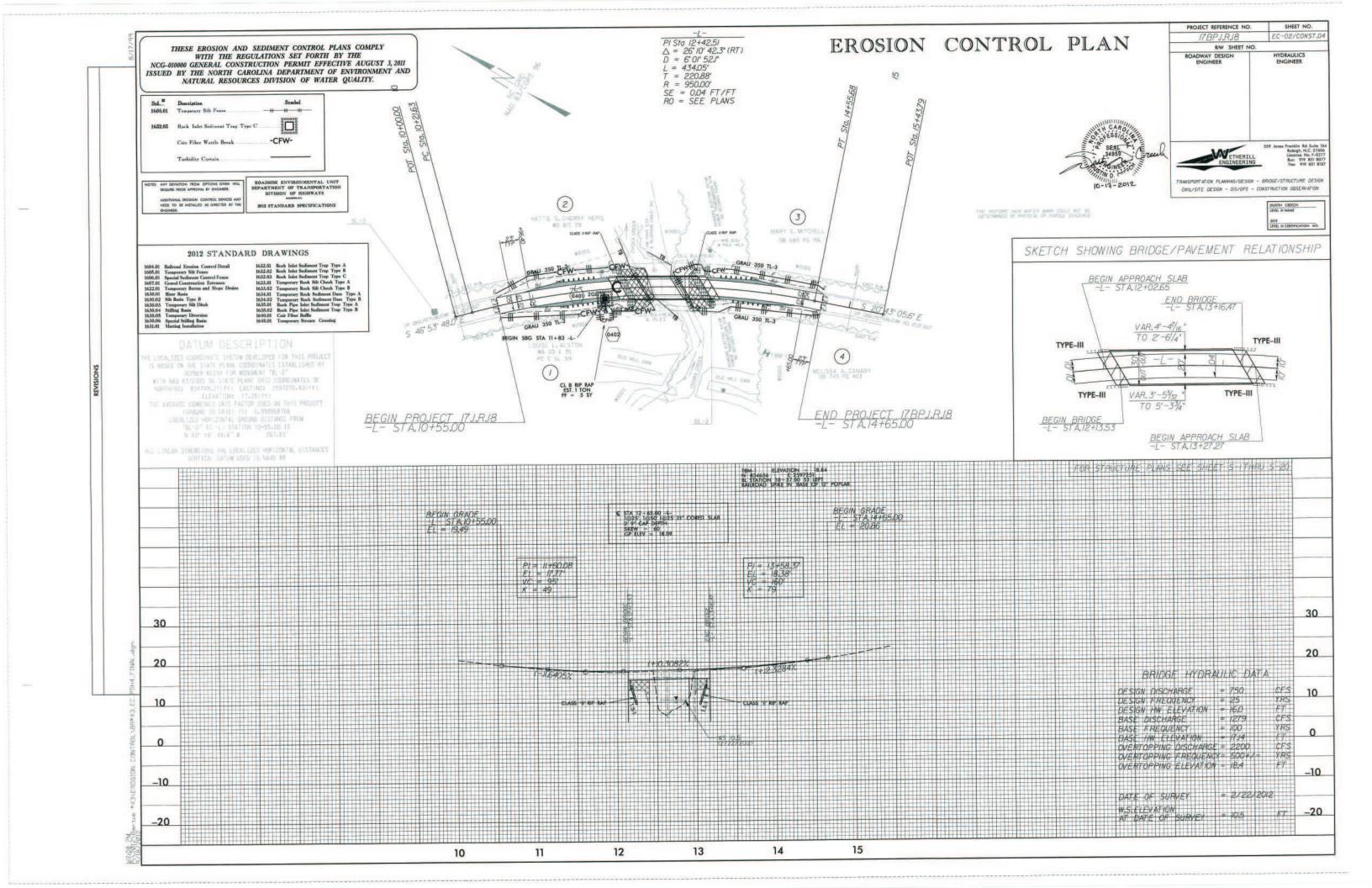
- STATIONARY MOUNTED SIGN



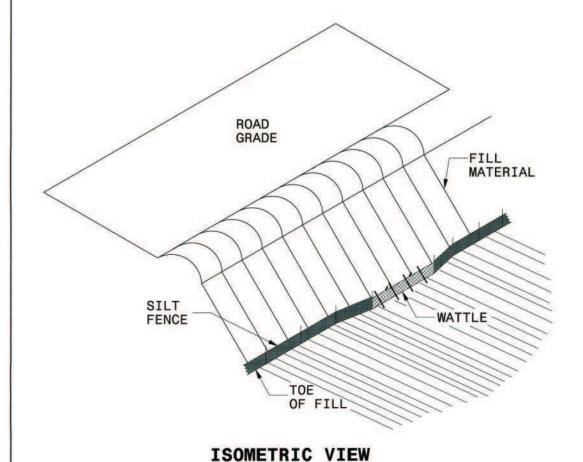


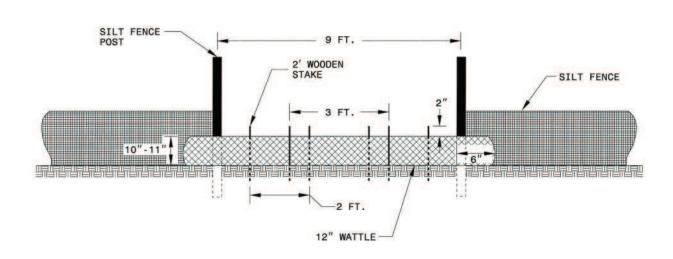
ROAD CLOSURE SR 1260 (REPUBLICAN RD.)





SILT FENCE COIR FIBER WATTLE BREAK DETAIL





VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

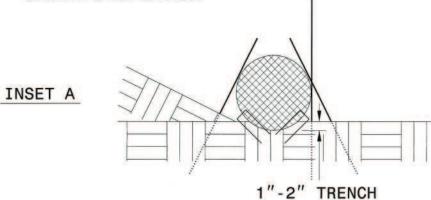
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

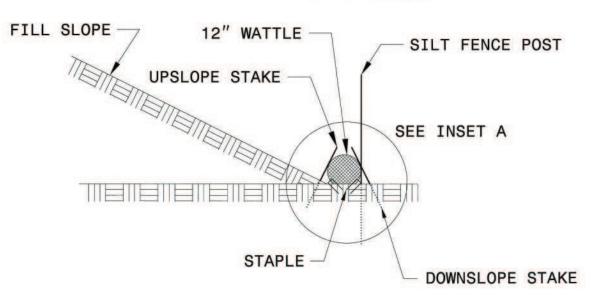
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

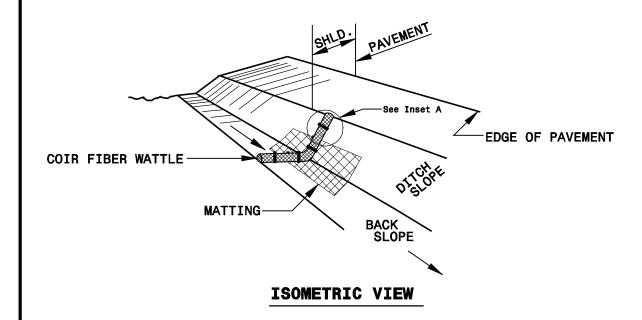


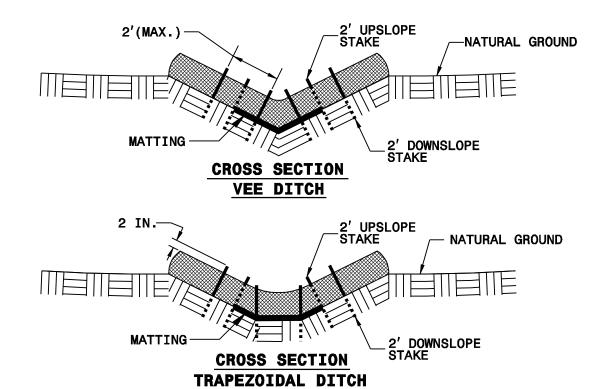


SIDE VIEW

C	N	TR	FTRFR	WATTLE	DETATI
V	V		ITDFI		

PROJECT REFERENCE NO	D. SHEET NO.					
<i>X-XXXX</i>	EC−2G					
R/W SHEET N	10.					
ROADWAY DESIGN ENGINEER	HYDRAULICS BNGINEER					





NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

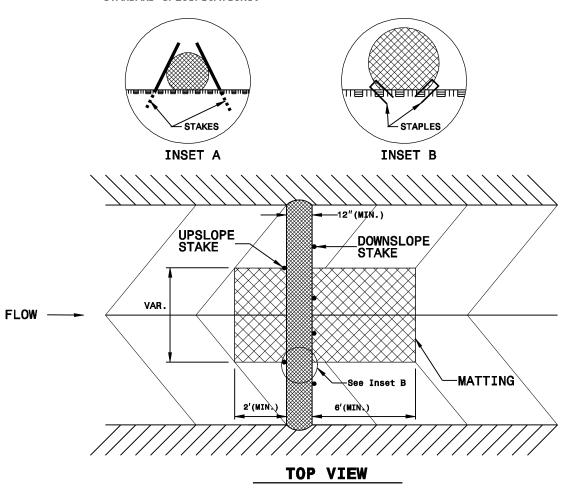
ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

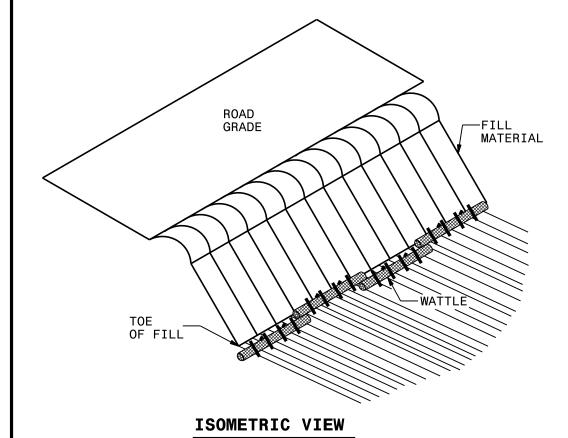
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

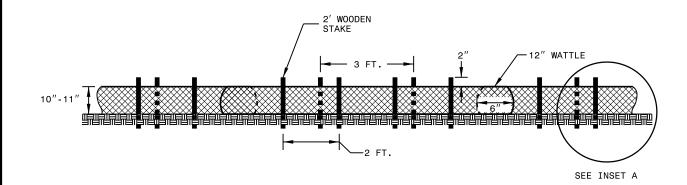
INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.



COIR FIBER WATTLE BARRIER DETAIL

PROJECT REFERENCE NO).	SHEET NO.				
<i>X-XXXX</i>		EC-2H				
R/W SHEET N	NO.					
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER				





FRONT VIEW

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

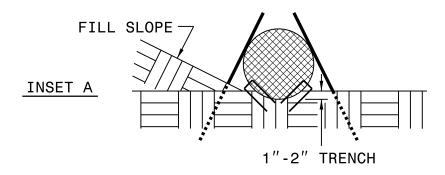
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

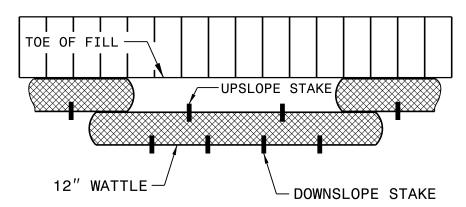
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 20 FT.





TOP VIEW

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION SUMMARY SHEET

MATTING FOR EROSION CONTROL (SLOPES) MATTING FOR EROSION CONTROL

	THE PROPERTY OF THE PROPERTY O	OIL LILO.	32011 0	0111110	L (SLOI LS)		WATTING FOR EROSION CONTROL									
CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)	CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY					
4	- L -	11+00	12+00	LT	69											
4	-L-	11+00	12+18	RT	75											
			SU	BTOTAL	144											
115CELLANEOUS	MATTING TO BE INS	TALLED AS DIRE	CTED BY THE	ENGINEER												
				TOTAL	144											
				SAY	144											

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SHEET NO.				
EC-3B				
HYDRAULICS ENGINEER				

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10' OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50' IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HOW ZONES.

EXISTING POLE (NOT LOCATED) **PROPOSED** - HEMOVE U/G TEL ------ REMOVE NOTE: ALL RELOCATED TELEPHONE TO REMAIN INSIDE EXISTING NCDOT RIGHT OF WAY. PI Sta 12+42.51 \(\triangle = 26 \text{ 10' 42.3'' (RT)} \)
\(D = 6 \text{ 0'' 52.1''}
\(L = 434.05' \) REMOVE EXISTING POLE (NOT LOCATED) NEW POLE T = 220.88' R = 950.00' SE = 0.04 FT/FT RO = SEE PLANS DIRECTIONAL BORE REMOVE EXISTING POLE MINIMUM 10' DEPTH BELOW CREEK BED IN THIS AREA REMOVE EXISTING PEDESTAL AND POLE REMOVE EXISTING THE HISTORIC HIGH WATER MARK COULD NOT BE DETERMINED BY PHYSICAL DR FARGLE EVIDENCE PEDESTAL AND POLE PROPOSED BURIED CABLE MARY E. MITCHELL PROPOSED BURIED CABLE 2' INSIDE EXISTING R/W 08 680 PG 156 2' INSIDE EXISTING R/W NEW POLE BORE UNDER EXISTING ROAD BORE UNDER EXISTING ROAD GRAU 350 TL-3 GRAU 350 TL-3 SR 1280, RE 33' 48.0 E BEGIN SBG STA 11+83 -L-THE LOCALIZED GOORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY JOYNER KERY FOR MONJORNI "BL-2"
WITH NAD B3/CDES 96 STATE PLANE CRID COORDINATES OF MORTHING: 834749.21(ft) EASTING: 2597270.43(ft) ELEVATION: 17.25(ft)
THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.99998766
LOCALIZED HORIZONTAL CROUND DISTANCE FROM "BL-2" TO -L STATION 10-55.00 IS N 33' 18' 49.6" W 261.83' 4 MELISSA A. CANA DB 743 PG 403 NEW POLE Sta. 10+00.00 REMOVE EXISTING POLE VERTICAL DATUM USED IS NAVO 88 BL-1 - N=834176.87, E=2597500.02, ELEV.=44,72 BL-2 - N=834749.21, E=2597270.43, ELEV.=17.25 BL-3 - N=835036.90, E=2597074.78, ELEV.=2L56 EXISTING POLE (NOT LOCATED)

PROJECT REFERENCE ND. SHEET NO. 178PJ.RJ8 UO-1

UTILITIES BY OTHERS

NOTE:
ALL PROPOSED UTILITY WORK
SHOWN ON THIS SHEET WILL
BE DONE BY OTHERS

559 Jones Franklin Rd. Suith Buleigh, N.C., 276 Jones Person No. 7.76 Jones 1919 801 81 Fox: 919 851 81

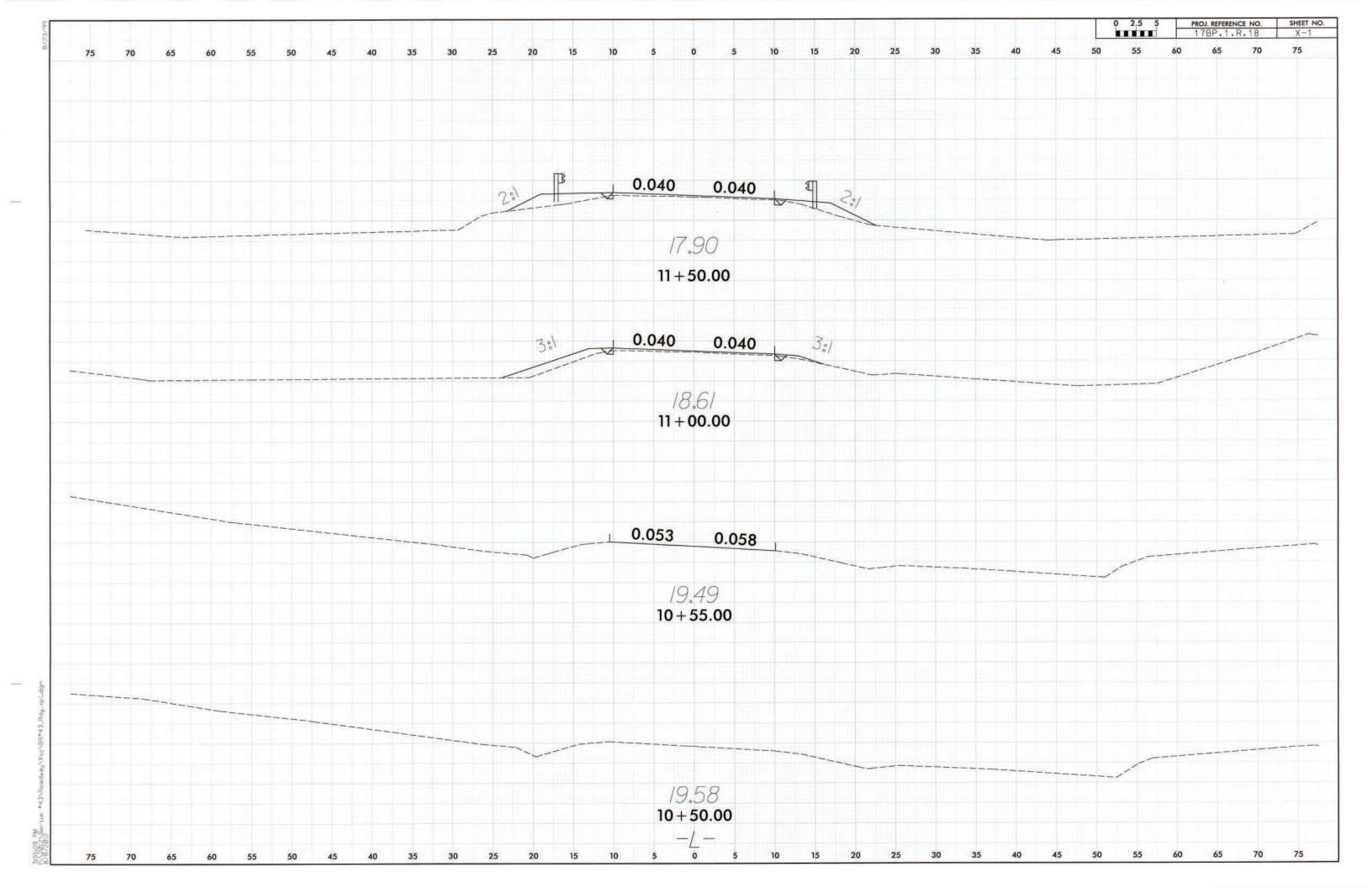
TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN CNIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

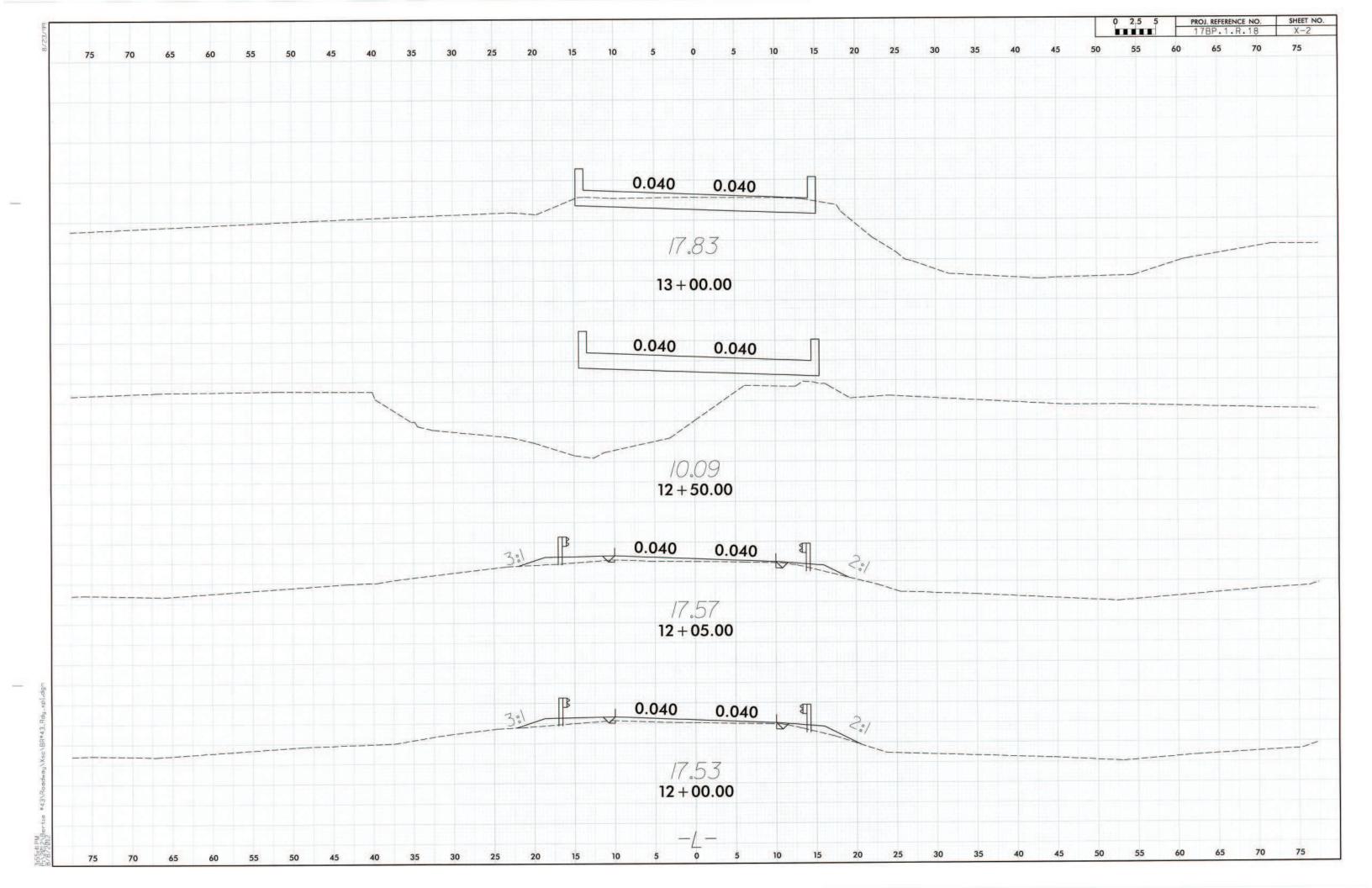
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

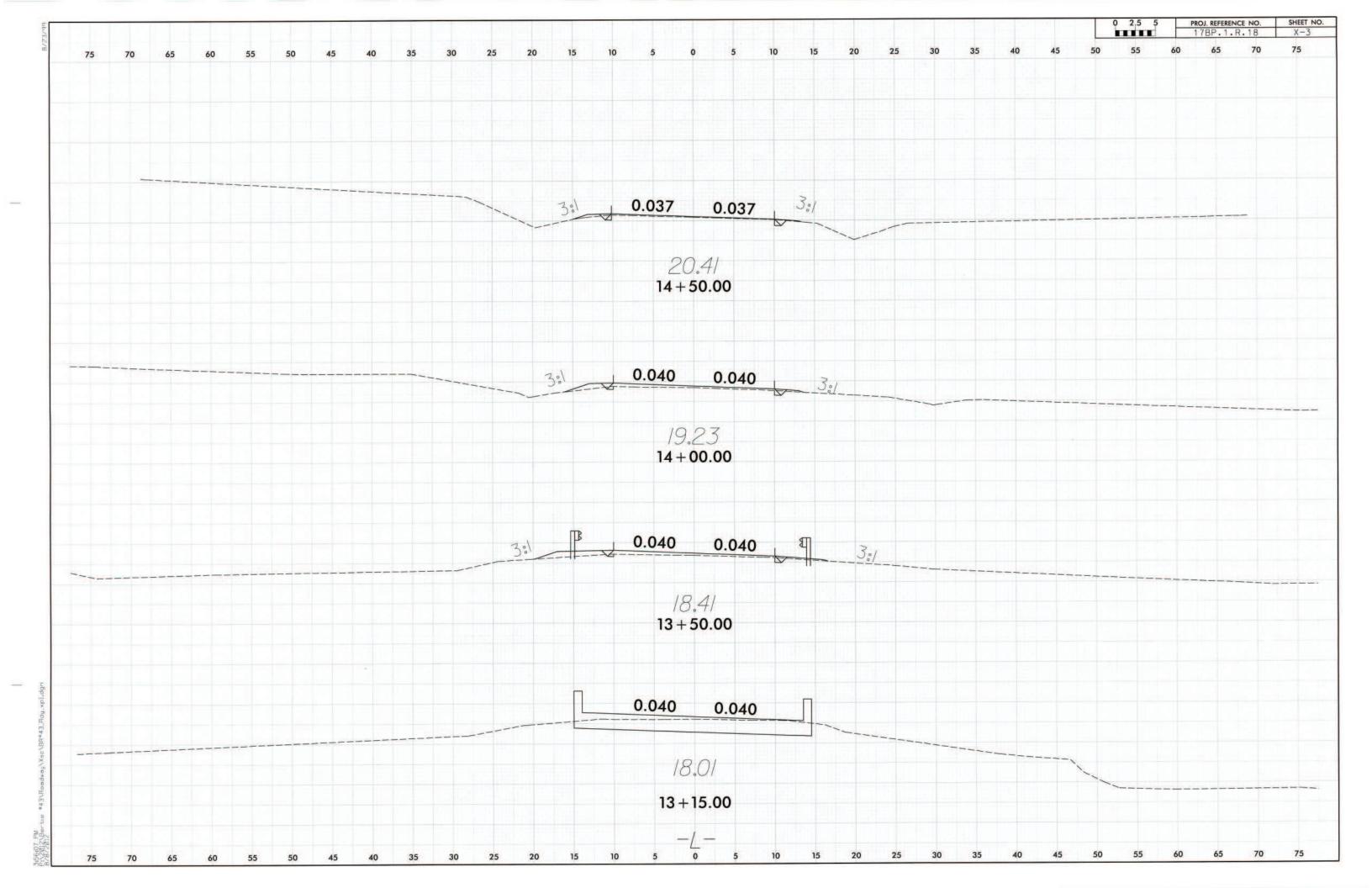
 PROJ. REFERENCE NO.
 SHEET NO.

 17BP.1.R.18
 X-1A

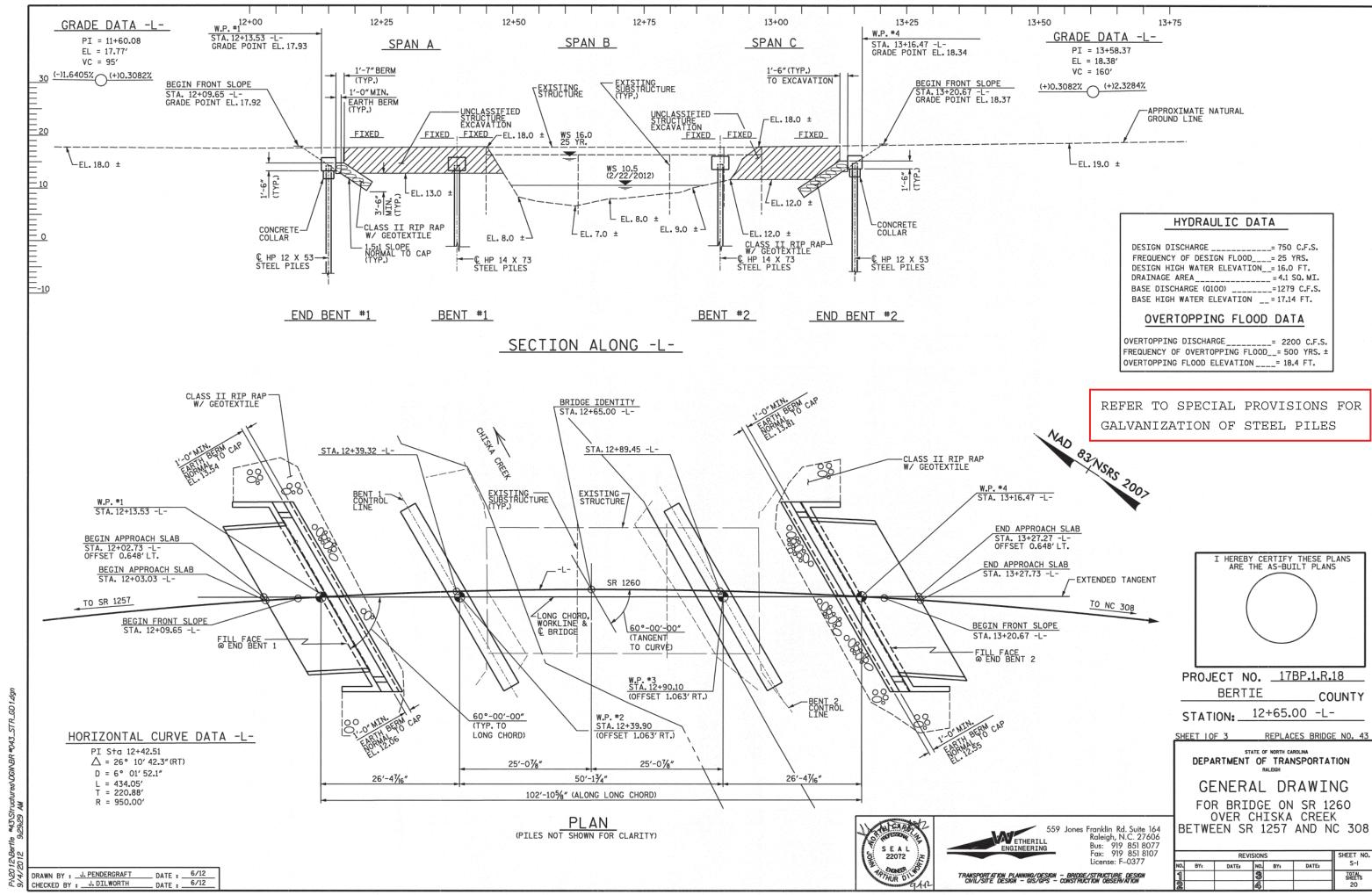
Station	Uncl. Exc.	Embt	Assessment and the second seco	The state of the s	SUMMAR			
lation	Olici. Exc.	Embt						
L	(cu. yd.)	(cu. yd.)					100	
0+55.00	0	0					Approximate quanti	ities only. Unclassified excavation, borrow er borrow, fine grading, clearing and grubbing,
1+00.00	1	10					breaking of existing	pavement and removal of existing pavement
11+50.00	2	30					will be paid for at the	ne lump sum price for "Grading".
2+00.00	2	29						
12+05.00	0	2						
2+13.53	1	3						
3+16.47	0	0						
3+50.00	1	7						
4+00.00	2	7						
4+50.00	2	3						
4+55.68	1	1						
4+65.00	1	1						

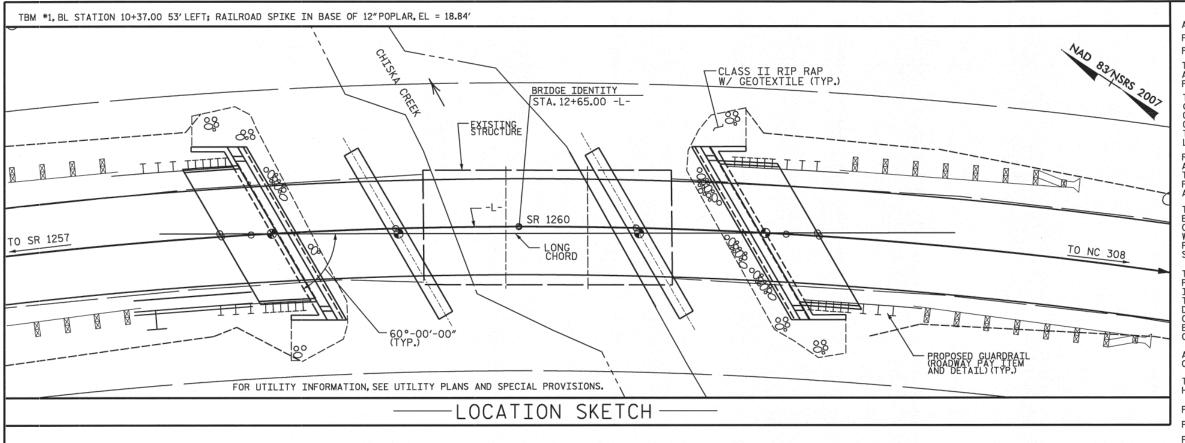












	TOTAL BILL OF MATERIAL																
	REMOVAL OF EXISTING STRUCTURE	PDA TESTING	UNCLASSIFTED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 x 53 EEL PILES	HP GAL STE	14 × 73 VANIZED EL PILES	REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)		ELASTOMERIO BEARINGS	PF	'-0" X 1'-9" RESTRESSED CRETE CORED SLABS
	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	ΝO	LIN.FT.	NO.	LIN. FT.	EACH	LIN.FT.	TONS	SQ. YD.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE	LUMP SUM				LUMP SUM							200.58				30	1000'-0"
END BENT 1			LUMP SUM	14.7		2218	5	245	72 20		2		90	96			
BENT 1				12.0		2350			7	455	2						
BENT 2				12.0		2350	\		7	469	2						
END BENT 2			LUMP SUM	14.7		2218	5	265			2		115	124		22	
TOTAL	LUMP SUM	1	LUMP SUM	53.4	LUMP SUM	9136	10	510	14	924	8	200.58	205	220	LUMP SUM	30	1000'-0"

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING. FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR SEISMIC PERFORMANCE ZONE 1.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 17'-6",1 SPAN @ 17'-1" AND 1 SPAN @ 17'-6" WITH A REINFORCED CONCRETE FLOOR ON TIMBER JOIST SUPERSTRUCTURE AND A CLEAR ROADWAY WIDTH OF 24.2'ON A SUBSTRUCTURE CONSISTING OF TIMBER CAPS ON TIMBER PILES AND LOCATED AT THE PROPOSED STRUCTURE LOCATION SHALL BE REMOVED.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCE BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY 2001.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR PILE DRIVING CRITERIA, SEE SPECIAL PROVISIONS.

REFER TO SPECIAL PROVISIONS FOR GALVANIZATION OF STEEL PILES

FOUNDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 55 TONS PER PILE. DRIVE PILES TO A REQUIRED DRIVING RESISTANCE

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 55 TONS PER PILE. DRIVE PILES TO A REQUIRED DRIVING RESISTANCE OF 95 TONS.

PILES AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE, DRIVE PILES TO A REQUIRED DRIVING RESISTANCE OF 160 TONS. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG OR SCOUR.

INSTALL PILES AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN -11.0 FEET. PILES AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 90 TONS PER PILE. DRIVE PILES TO A REQUIRED DRIVING RESISTANCE OF 160 TONS. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG OR SCOUR.

INSTALL PILES AT BENT NO. 2 TO A TIP ELEVATION NO HIGHER THAN -11.5 FEET. THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 AND BENT NO.2 IS ELEVATION -1.0 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TESTING PILES WITH THE PILE DRIVING ANALYZER (PDA) DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING, FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT THAS BEEN ESTIMATED THAT A HAMMER WITH AN EUDIVALENT RATED ENERGY IN THE RANGE OF 30 TO 33 FT.-KTPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NO. 1. AND END BENT NO. 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40 TO 45 FT.-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT NO.1 AND BENT NO.2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

PROJECT NO. 17BP.1.R.18 BERTIE COUNTY STATION: __12+65.00 -L-

SHEET 2 OF 3 REPLACES BRIDGE NO. 43

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE ON SR 1260 OVER CHISKA CREEK BETWEEN SR 1257 AND NC 308

Fax: 919 851 8107 REVISIONS License: F-0377 S-2 DATE NO. BY: DATE

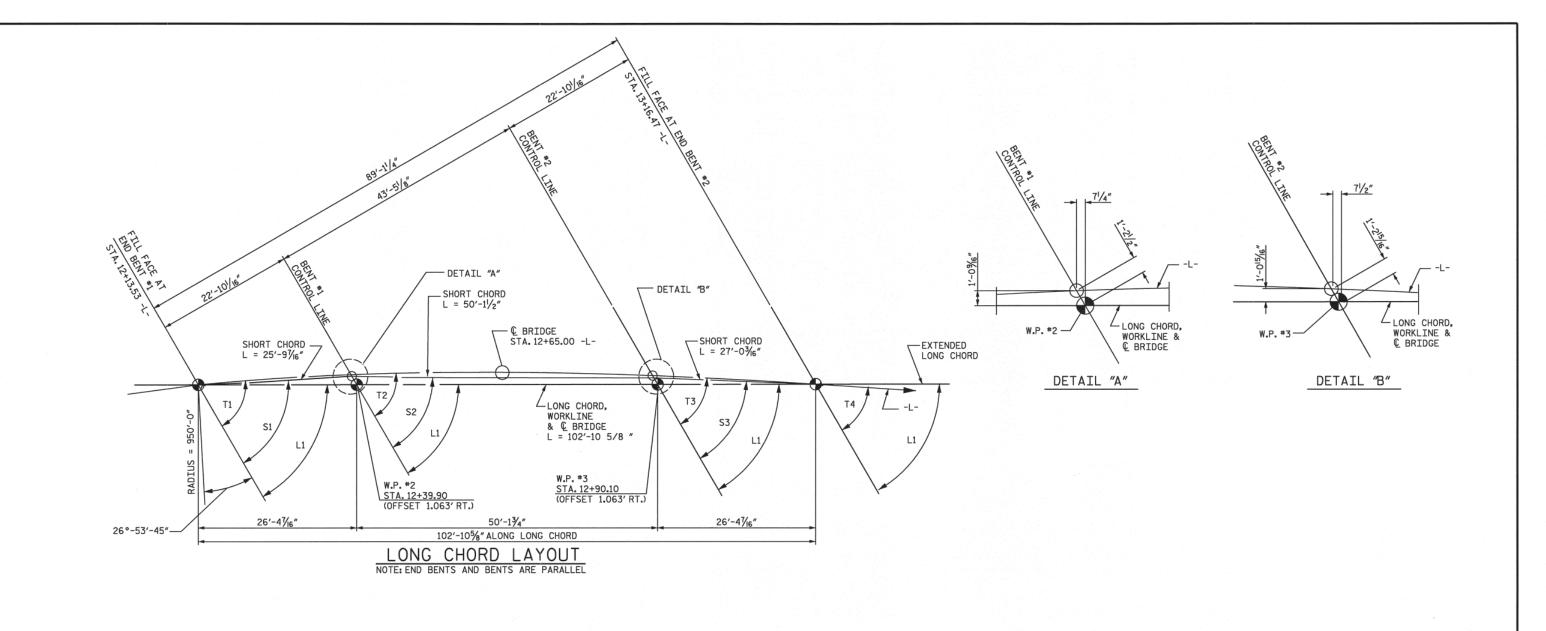
22072



559 Jones Franklin Rd. Suite 164 Raleigh, N.C. 27606 Bus: 919 851 8077

TRANSPORTATION PLANNING/DESIGN — BRIDGE/STRUCTURE DESIGN CIVIL/SITE DESIGN — GIS/GPS — CONSTRUCTION OBSERVATION

CHECKED BY : ________ DATE : ______ 6/12



HORIZONTAL CURVE DATA -L-

PI Sta 12+42.51 \triangle = 26° 10′ 42.3″(RT)

D = 6° 01′ 52.1″ L = 434.05′

T = 220.88'

R = 950.00'

			ANGLES	887.6		
	LONG CHORD	S	HORT CHORD	TANGENT TO CURVE		
L1	60°-00′-00″	S1	62°-19'-35"	T1	63°-06′-15″	
7		S2	60°-02′-13″	T2	61°-32′-57″	
2		S3	57°-42′-38″	Т3	58°-31′-31"	
				T4	56°-53′-45″	

559 Jones Franklin Rd. Suite 164 Raleigh, N.C. 27606 Bus: 919 851 8077 Fax: 919 851 8107

TRANSPORTATION PLANNING/DESIGN - BRIDGE/STRUCTURE DESIGN CIVIL/SITE DESIGN - GIS/GPS - CONSTRUCTION OBSERVATION

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH GENERAL DRAWING

PROJECT NO. 17BP.1.R.18

STATION: 12+65.00 -L-

COUNTY

REPLACES BRIDGE NO. 43

BERTIE

FOR BRIDGE ON SR 1260 OVER CHISKA CREEK BETWEEN SR 1257 AND NC 308

SHEET NO. S-3 REVISIONS NO. BY: DATE: NO. BY: DATE TOTAL SHEETS 20



DRAWN BY : J. PENDERGRAFT DATE : 6/12
CHECKED BY : J. DILWORTH DATE : 6/12

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIM	IIT SI	TATE				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93(Inv)	N/A	(1)	1.208		1.75	0.257	2.83	25′	EL	11.923	0.659	1.21	25′	EL	1.192	0.80	0.257	2.60	25′	EL	11.923	
DESIGN		HL-93(0pr)	N/A		1.565		1.35	0.257	3.66	25′	EL	11.923	0.659	1.57	25′	EL	1.192	N/A						
LOAD RATING		HS-20(Inv)	36.000	<u> </u>	1.402	50.457	1.75	0.257	4.17	25′	EL	11.923	0.659	1.4	25′	EL	1.192	0.80	0.257	3.85	25′	EL	11.923	
NATINO		HS-20(0pr)	36.000		1.817	65.407	1.35	0.257	5.41	25′	EL	11.923	0.659	1.82	25′	EL	1.192	N/A						
	-	SNSH	13.500		3.24	43.746	1.4	0.257	7.59	25′	EL	11.923	0.659	3.24	25′	EL	1.192	0.80	0.257	5.59	25′	EL	11.923	
		SNGARBS2	20.000		2.6	51.994	1.4	0.257	7.1	25′	EL	11.923	0.659	2.6	25′	EL	1.192	0.80	0.257	5.24	25′	EL	11.923	
		SNAGRIS2	22.000		2.548	56.063	1.4	0.257	7.59	25′	EL	11.923	0.659	2.55	25′	EL	1.192	0.80	0.257	5.59	25′	EL	11.923	
		SNCOTTS3	27.250		1.645	44.82	1.4	0.257	3.98	25′	EL	11.923	0.659	1.64	25'	EL	1.192	0.80	0.257	2.93	25′	EL	11.923	
	S	SNAGGRS4	34.925		1.585	55.347	1.4	0.257	3.96	25′	EL	11.923	0.659	1.58	25'	EL	1.192	0.80	0.257	2.92	25′	EL	11.923	
		SNS5A	35.550		1.655	58.841	1.4	0.257	3.85	25′	EL	11.923	0.659	1.66	25′	EL	1.192	0.80	0.257	2.82	25′	EL	11.923	
		SNS6A	39.950		1,588	63.45	1.4	0.257	3.6	25′	EL	11.923	0.659	1.59	25′	EL	1.192	0.80	0.257	2.66	25′	EL	11.923	
LEGAL		SNS7B	42.000		1.599	67.158	1.4	0.257	3.6	25'	EL	11.923	0.659	1.6	25'	EL	1.192	0.80	0.257	2.64	25′	EL	11.923	
LOAD RATING		TNAGRIT3	33.000		1.948	64.275	1.4	0.257	5.09	25′	EL	11.923	0.659	1.95	25′	EL	1.192	0.80	0.257	3.75	25′	EL	11.923	
		TNT4A	33.075		1.764	58.347	1.4	0.257	4.4	25′	EL	11.923	0.659	1.76	25′	EL	1.192	0.80	0.257	3.25	25′	EL	11.923	
		TNT6A	41.600		1.662	69.142	1.4	0.257	4.13	25′	EL	11.923	0.659	1.66	25′	EL	1.192	0.80	0.257	3.05	25'	EL	11.923	
	TST	TNT7A	42,000		1.657	69.603	1.4	0.257	4.28	25′	EL	11.923	0.659	1.66	25′	EL	1.192	0.80	0.257	3.15	25′	EL	11.923	
	-	TNT7B	42,000		1.598	67.097	1.4	0.257	3.85	25′	EL	11.923	0.659	1.6	25′	EL	1.192	0.80	0.257	2.84	25′	EL	11.923	
		TNAGRIT4	43.000		1.595	68.603	1.4	0.257	4.14	25′	EL	11.923	0.659	1.6	25′	EL	1.192	0.80	0.257	3.04	25′	EL	11.923	
		TNAGT5A	45.000		1.625	73.143	1.4	0.257	4.14	25′	EL	11.923	0.659	1.63	25′	EL	1.192	0.80	0.257	3.04	25′	EL	11.923	
		TNAGT5B	45.000	⟨3⟩	1.476	66.434	1.4	0.257	4.08	25′	EL	9.538	0.659	1.48	25′	EL	1.192	0.80	0.257	3.02	25′	EL	9.538	

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	γ_{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

LRFR SUMMARY

FOR SPAN 'A' OR 'C'

PROJECT NO. ___17BP.1.R.18 BERTIE ____ COUNTY STATION: 12+65.00 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH STANDARD

LRFR SUMMARY FOR 25'CORED SLAB UNIT 60° SKEW & 120° SKEW

(NON-INTERSTATE TRAFFIC)

	REVISIONS										
BY:	DATE:	S-4									
		3			TOTAL SHEETS						
		4			20						



ASSEMBLED BY: N.RUFFIN DATE: 7/10/12 CHECKED BY: B.L.GREEN DATE: 7/10/12 DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	NGTH	I LIN	MIT ST	ГАТЕ				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (†+)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.35		1.75	0.25	1.74	50′	EL	24.423	0.656	1.35	50′	EL	9.769	0.80	0.25	1.59	50′	EL	24.423	
DESIGN		HL-93(0pr)	N/A		1.75		1.35	0.25	2.25	50′	EL	24.423	0.656	1.75	50'	EL	9.769	N/A						
LOAD RATING		HS-20(Inv)	36.000	<u>\(2\)</u>	1.586	57.108	1.75	0.25	2.15	50′	EL	24.423	0.656	1.59	50′	EL	9.769	0.80	0.25	1.97	50′	EL	24.423	
		HS-20(0pr)	36.000		2.056	74.028	1.35	0.25	2.79	50′	EL	24.423	0.656	2.06	50′	EL	9.769	N/A						
		SNSH	13.500		4.009	54.117	1.4	0.25	5.47	50′	EL	24.423	0.656	4.31	50′	EL	9.769	0.80	0.25	4.01	50′	EL	24.423	
		SNGARBS2	20.000		3.168	63.352	1.4	0.25	4.32	50′	EL	24.423	0.656	3.19	50′	EL	9.769	0.80	0.25	3.17	501	EL	24.423	
		SNAGRIS2	22.000		3.009	66.192	1.4	0.25	4.18	50′	EL	19.538	0.656	3.01	50′	EL	9.769	0.80	0.25	3.07	50′	EL	24.423	
		SNCOTTS3	27.250		2	54.493	1.4	0.25	2.73	50′	EL	24.423	0.656	2.16	50'	EL	9.769	0.80	0.25	2.00	50′	EL	24.423	
	SV	SNAGGRS4	34.925		1.739	60.742	1.4	0.25	2.37	50′	EL	24.423	0.656	1.88	50′	EL	9.769	0.80	0.25	1.74	50′	EL	24.423	
		SNS5A	35,550		1.696	60,292	1.4	0.25	2.31	50′	EL	24.423	0.656	1.96	50′	EL	9.769	0.80	0.25	1.70	50'	EL	24.423	
		SNS6A	39.950		1.586	63.364	1.4	0.25	2.16	50′	EL	24.423	0.656	1.82	50′	EL	9.769	0.80	0.25	1.59	50′	EL	24.423	
LEGAL		SNS7B	42,000		1.512	63.487	1.4	0.25	2.06	50′	EL	24.423	0.656	1.85	50′	EL	9.769	0.80	0.25	1.51	50′	EL	24.423	
LOAD RATING		TNAGRIT3	33.000		1.943	64.127	1.4	0.25	2.65	50′	EL	24.423	0.656	2.14	50′	EL	9.769	0.80	0.25	1.94	50′	EL	24.423	
		TNT4A	33.075		1.96	64.837	1.4	0.25	2.67	50′	EL	24.423	0.656	2.04	50′	EL	9.769	0.80	0.25	1.96	50′	EL	24.423	
		TNT6A	41.600		1.633	67.938	1.4	0.25	2.23	50′	EL	24.423	0.656	2	50′	EL	9.769	0.80	0.25	1.63	50′	EL	24.423	
	TST	TNT7A	42.000		1.658	69.634	1.4	0.25	2.26	50′	EL	24.423	0.656	1.86	50′	EL	9.769	0.80	0.25	1.66	50′	EL	24.423	
	-	TNT7B	42.000		1.728	72.595	1.4	0.25	2.36	50′	EL	24.423	0.656	1.76	50′	EL	9.769	0.80	0.25	1.73	50′	EL	24.423	
		TNAGRIT4	43.000		1.64	70.537	1.4	0.25	2.24	50′	EL	24.423	0.656	1.69	50'	EL	9.769	0.80	0.25	1.64	50′	EL	24.423	
		TNAGT5A	45.000		1.532	68.95	1.4	0.25	2.09	50′	EL	24.423	0.656	1.75	50′	EL	9.769	0.80	0.25	1.53	50′	EL	24.423	
		TNAGT5B	45.000	3	1.501	67.548	1.4	0.25	2.05	50′	EL	24.423	0.656	1.6	50′	EL	9.769	0.80	0.25	1.50	50′	EL	24.423	

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\rm DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2.

3.

4.

CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.1.R.18
BERTIE COUNTY
STATION: 12+65.00 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

LRFR SUMMARY FOR 50'CORED SLAB UNIT 60°SKEW & 120°SKEW (non-interstate traffic)

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
1			3			TOTAL SHEETS
2			4			20

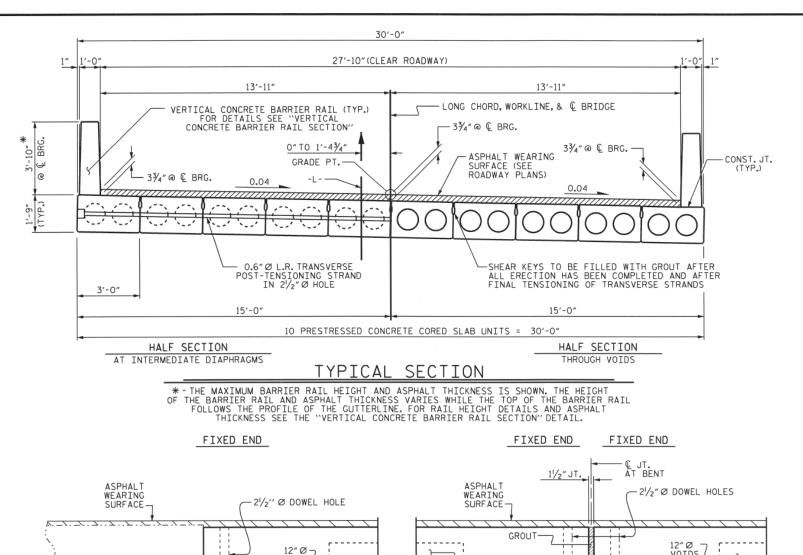


LRFR SUMMARY
FOR SPAN 'B'

ASSEMBLED BY: N, RUFFIN DATE: 7/19/12
CHECKED BY: T,L. COGGINS DATE: 7/19/12
DRAWN BY: CVC 6/10
CHECKED BY: DNS 6/10

20-JUL-2012 10:43 S:\DPG\\Tim\\17BP.I.R.18\\nruffln\\17BP.I.R.18_sd_CS_01.dgn

STD. NO. 21LRFR1_60&120S_50L



VOIDS

SECTION AT END BENT

OUTSIDE FACE OF EXTERIOR CORED SLAB

GROUTED RECESS AT END OF

POST-TENSIONED STRAND OF CORED SLABS

€ 0.6" Ø L.R. TRANSVERSE POST-TENSIONING STRAND SHEATHED WITH A

NON-CORROSIVE PIPE.

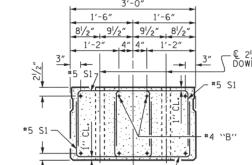
- ELASTOMERIC BEARING PAD

SEE "END BENT" SHEETS FOR DETAILS

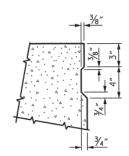
SECTION B-B

VOIDS VOIDS ELASTOMERIC BEARING PAD 2"Ø BACKER ROD --- ELASTOMERIC BEARING PAD © BEARING & #6 DOWELS -SEE "BENT" SHEETS FOR DETAILS

SECTION AT BENT



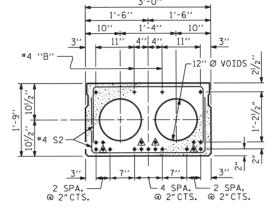
SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



SHEAR KEY DETAIL NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

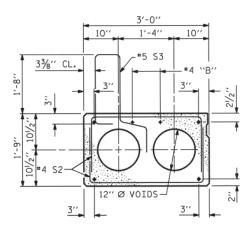
1'-6" 1'-6" 10" 11" _ 4" 4" -12" Ø VOIDS 💸 4 SPA. 2 SPA. @ 2"CTS. @ 2"CTS. 2 SPA.

INTERIOR SLAB SECTION (25' UNIT) (9 STRANDS REQUIRED



INTERIOR SLAB SECTION (50' UNIT) (19 STRANDS REQUIRED

0.6" Ø LOW RELAXATION STRAND LAYOUT



EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-0"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS.
 THESE STRANDS ARE NOT REQUIRED. IF THE
 FABRICATOR CHOOSES TO INCLUDE THESE STRANDS
 IN THE CORED SLAB UNIT, THE STRANDS SHALL
 BE DEBONDED FOR THE FULL LENGTH OF THE UNIT
 AT NO ADDITIONAL COST. SEE STANDARD
 SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

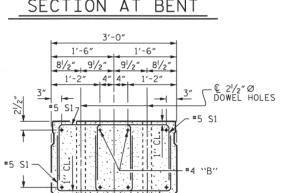
PROJECT NO. <u>17BP.1.R.18</u> BERTIE COUNTY STATION: 12+65.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 1'-9"

PRESTRESSED CONCRETE CORED SLAB UNIT 60° SKEW

_		SHEET NO.				
Ū	BY:	DATE:	NO.	BY:	DATE:	S-6
T			3			TOTAL SHEETS
2			4			20



END ELEVATION

DATE: 7/3/12 DATE: 7/10/12 B.L. GREEN DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09

ELEVATION VIEW

SEE "BRIDGE

SHEET FOR DETAILS

2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND.

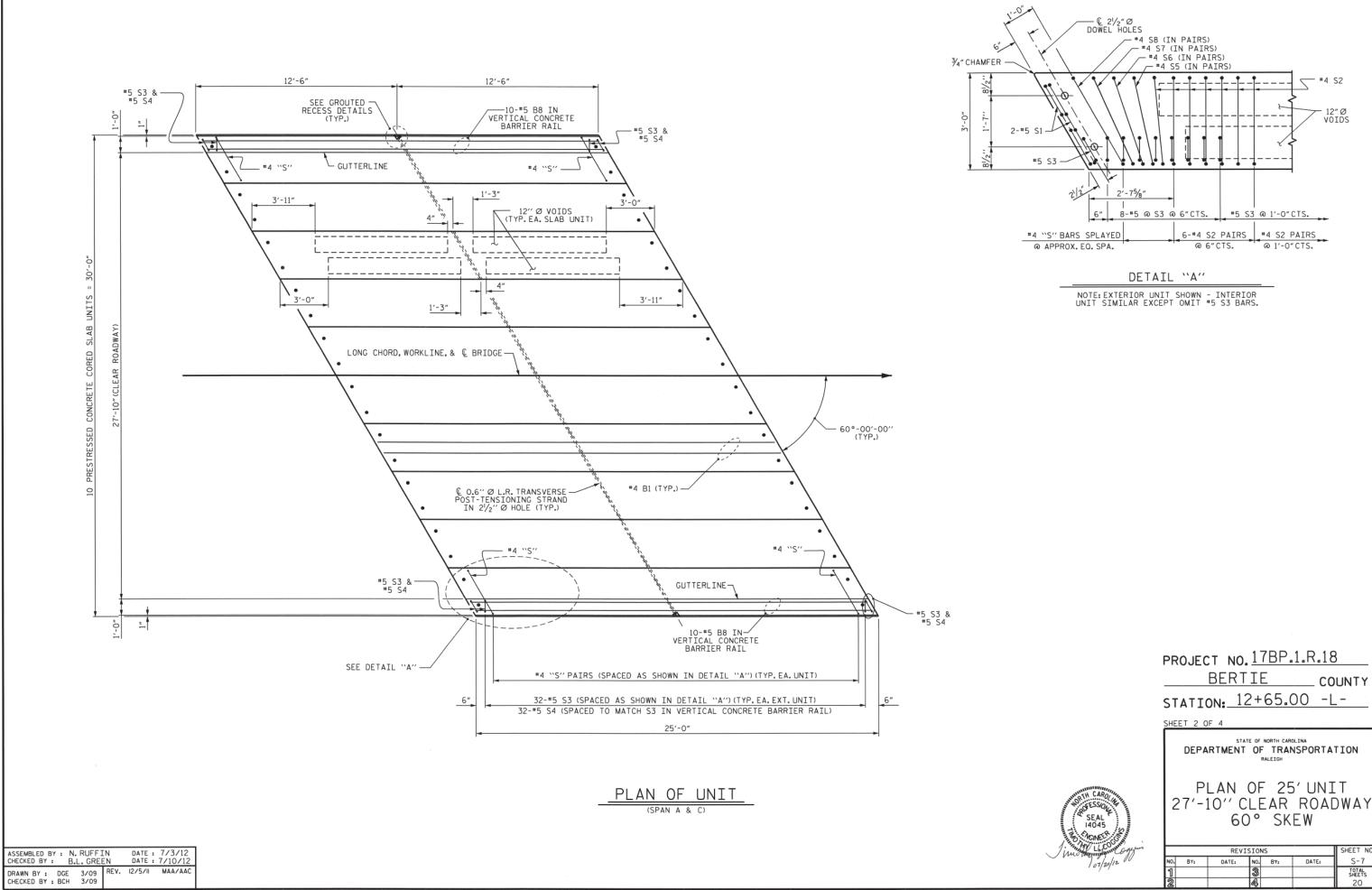
11/2" Ø BACKER ROD

© BEARING & #6 DOWELS

TRANSVERSE STRAND

20-JUL-2012 10:43 S:\DPG|\Tim\17BP.1.R.18\nruffin\17BP.1.R.18_sd_CS_01.dgn

STD. NO. 21" PCS2_30_60S



DATE: NO. BY: DATE: TOTAL SHEETS 20

STATE OF NORTH CAROLINA

PLAN OF 25' UNIT

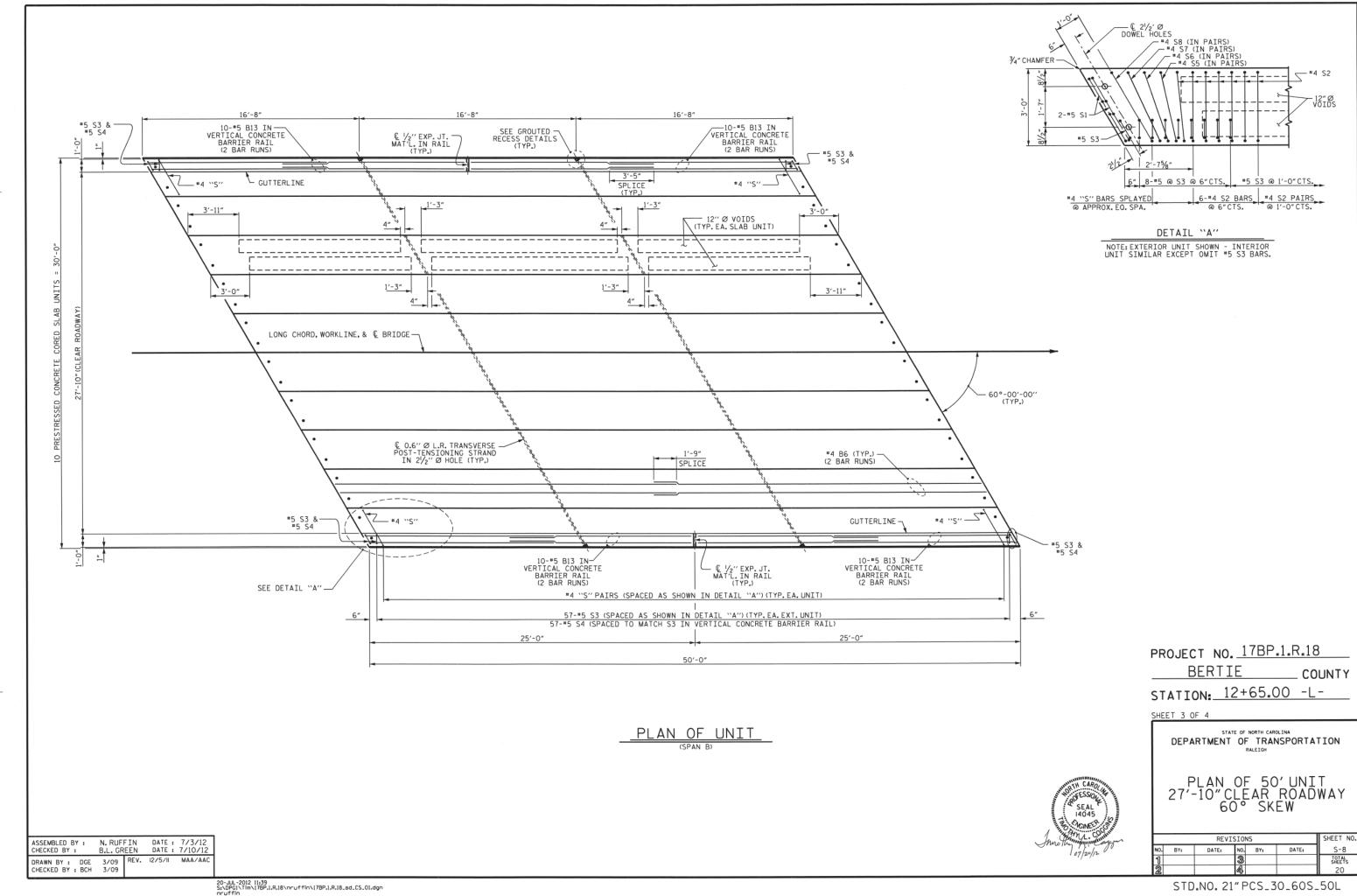
60° SKEW

REVISIONS

BERTIE

_ COUNTY

#5 S3 @ 1'-0"CTS.



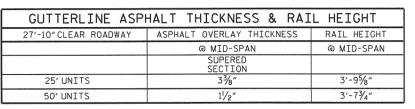
** IN	CLUDES	FUTURE	WEARING	SURFACE
-------	--------	--------	---------	---------

BI	LL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	RIER R	AIL			
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT			
	25' UNIT								
* B8	20	40	#5	STR	24'-6"	1022			
* S4	68	136	#5	2	7'-2"	1017			
* EPOX	Y COATED REINFORCING STEEL			LBS.		2039			
CLASS									
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		100.29			

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
50' CORED SLAB UNIT	0.6"Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2 [!] / ₂ " 🕴
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	1/4″ ♦
FINAL CAMBER	21/4″ ∤

**	INCLUDES	FUTURE	WEARING	SURFACE

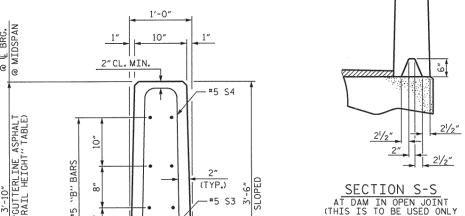
BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL									
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT			
	50' UNIT								
* B13	80	80	#5	STR	14'-3"	1189			
* S4	118	118	#5	2	7'-2"	882			
* EPOXY COATED REINFORCING STEEL LBS.									
CLASS	AA CONCRETE			CU.YDS.		13.1			
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN. FT.		100.29			



CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
25' UNIT			
EXTERIOR C.S.	4	25'-0"	100'-0"
INTERIOR C.S.	16	25'-0"	400'-0"
TOTAL	20		500'-0"

COR				UIRED
		NUMBER	LENGTH	TOTAL LENGTH
50' UNI	T			
EXTERIOR	C.S.	2	50'-0"	100'-0"
INTERIOR	C.S.	8	50'-0"	400'-0"
TOTAL		10		500′-0"

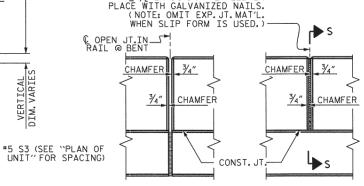
GRADE 270 S	TRANDS
	0.6" Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950



-#5 S3

2¾"CL. 33/8"

SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED) © ½″EXP.JT.MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP.JT.MAT'L. WHEN SLIP FORM IS USED.)-



ELEVATION AT EXPANSION JOINTS

VERTICAL CONCRETE BARRIER RAIL SECTION

ASSEMBLED BY: N. RUFF CHECKED BY: B.L. GRE		: 7/3/12 : 7/10/12
DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09	REV. 12/11	MAA/AAC

CONST. JT.

VARIES (SEE Y

+

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

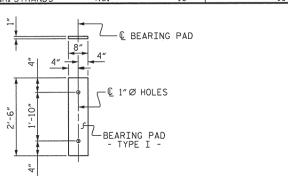
BAR TYPES 2 1 S7 2'-11" S6 2'-10" S5_2'-9'' S2 2'-8'' S1 2'-0" 3 ALL BAR DIMENSIONS ARE OUT TO OUT

25' CORED SLAB UNIT										
EXTERIOR UNIT INTERIOR UNIT										
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT			
B1	2	#4	STR	24'-7"	33	24'-7"	33			
S1	8	#5	3	4'-6"	38	4'-6"	38			
S2	52	#4	3	5′-4″	185	5'-4"	185			
★ S3	34	#5	1	6'-2"	219					
S5	4	#4	3	5′-5″	14	5′-5″	14			
S6	4	#4	3	5′-6″	15	5′-6″	15			
S7	4	#4	3	5'-7"	15	5′-7″	15			
S8	4	#4	3	5'-9"	15	5′-9″	15			
REINFO	ORCING	STEEL	LBS	5.	315		315			
* EPOXY COATED REINFORCING STEEL LBS. 219										
5000 P.S.I. CONCRETE CU. YDS				ò.	3.8		3.8			
0.6" Ø	L.R. STR	ANDS	No).	9		9			

BILL OF MATERIAL FOR ONE

	BILL OF MATERIAL FOR ONE 50' CORED SLAB UNIT											
	EXTERIOR UNIT INTERIOR UNIT											
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT					
B6	4	#4	STR	25'-9"	69	25'-9"	69					
S1	8	#5	3	4'-6"	38	4'-6"	38					
S2	102	#4	3	5'-4"	363	5'-4"	363					
* S3	59	#5	1	6'-2"	379							
S5	4	#4	- 3	5′-5″	14	5′-5″	14					
S6	4	#4	3	5′-6″	15	5′-6″	15					
S7	4	#4	3	5'-7"	15	5′-7″	15					
S8	4	#4	3	5'-9"	15	5′-9″	15					
REINFO	ORCING	STEEL	LB:	S.	529		529					
	* EPOXY COATED											
REINFORCING STEEL LBS. 379												
6500 P.S.I. CONCRETE CU. YDS. 7.3 7.3												
0.6" Ø	L.R. STR	ANDS	No).	19		19					

DILL OF MATERIAL FOR ONE



FIXED END (TYPE I - 60 REQ'D)

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{l}\!/_{2}^{m}\varnothing$ dowel holes at fixed ends of slab sections shall be filled with non-shrink grout.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, \(\frac{1}{2}\)' IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

CONCRETE	RELEA	4SE	STRENGTH
UNIT			PSI
25' UNITS			4000
50' UNITS			4900

PROJECT NO. <u>17BP.1.R.18</u> BERTIE STATION: 12+65.00 -L-

SHEET 4 OF 4

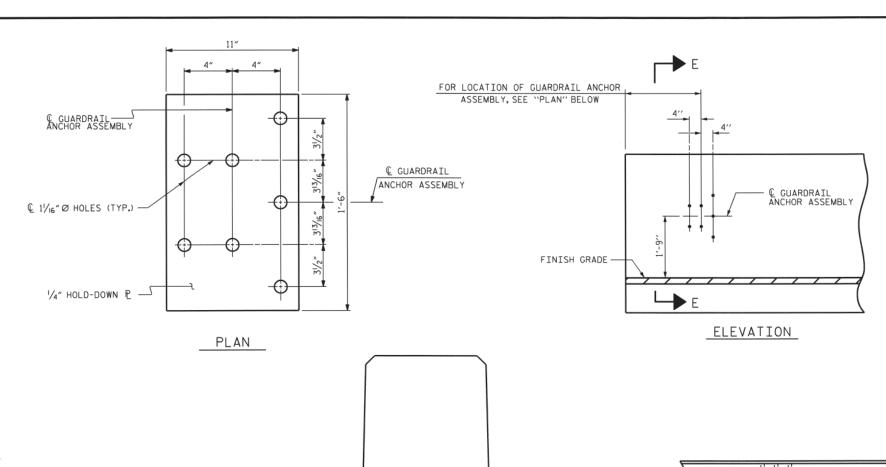
SEAL 14045

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
60° SKEW

		SHEET NO.				
o.	BY:	DATE:	NO.	BY:	DATE:	S-9
] [3			TOTAL SHEETS
2			4			20

31-JUL-2012 11:11 S:\DPG1\Tim\178P.1.R.18\nruffin\178P.1.R.18_sd_CS_01.dgn

STD. NO. 21' PCS3_30_60S



© 78" Ø X 1'-2" BOLT WITH ROUND WASHERS (TYP.)

> - © GUARDRAIL ANCHOR

ASSEMBLY

-11/4" Ø HOLE (TYP.)

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/\!\!/_4$ HOLD DOWN PLATE AND 7 - $1/\!\!/_8$ W Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE $\frac{7}{6}$? \emptyset GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

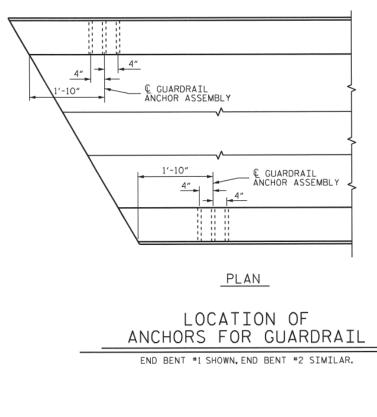
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" \varnothing HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.





SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.1.R.18
BERTIE COUNTY
STATION: 12+65.00 -L-



STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION

STANDARD
GUARDRAIL ANCHORAGE
FOR VERTICAL CONCRETE
BARRIER RAIL

		SHEET NO.				
0.	BY:	DATE:	NO.	BY:	DATE:	S-10
1			3			TOTAL SHEETS
2			4			20

DRAWN BY: MAA 5/10 ADDED 5/6/10 REV. 10/1/11 MAA/GM REV. 12/5/11 MAA/GM

M. Ruffin DATE: 7/3/12 B.L. GREEN DATE: 7/10/12

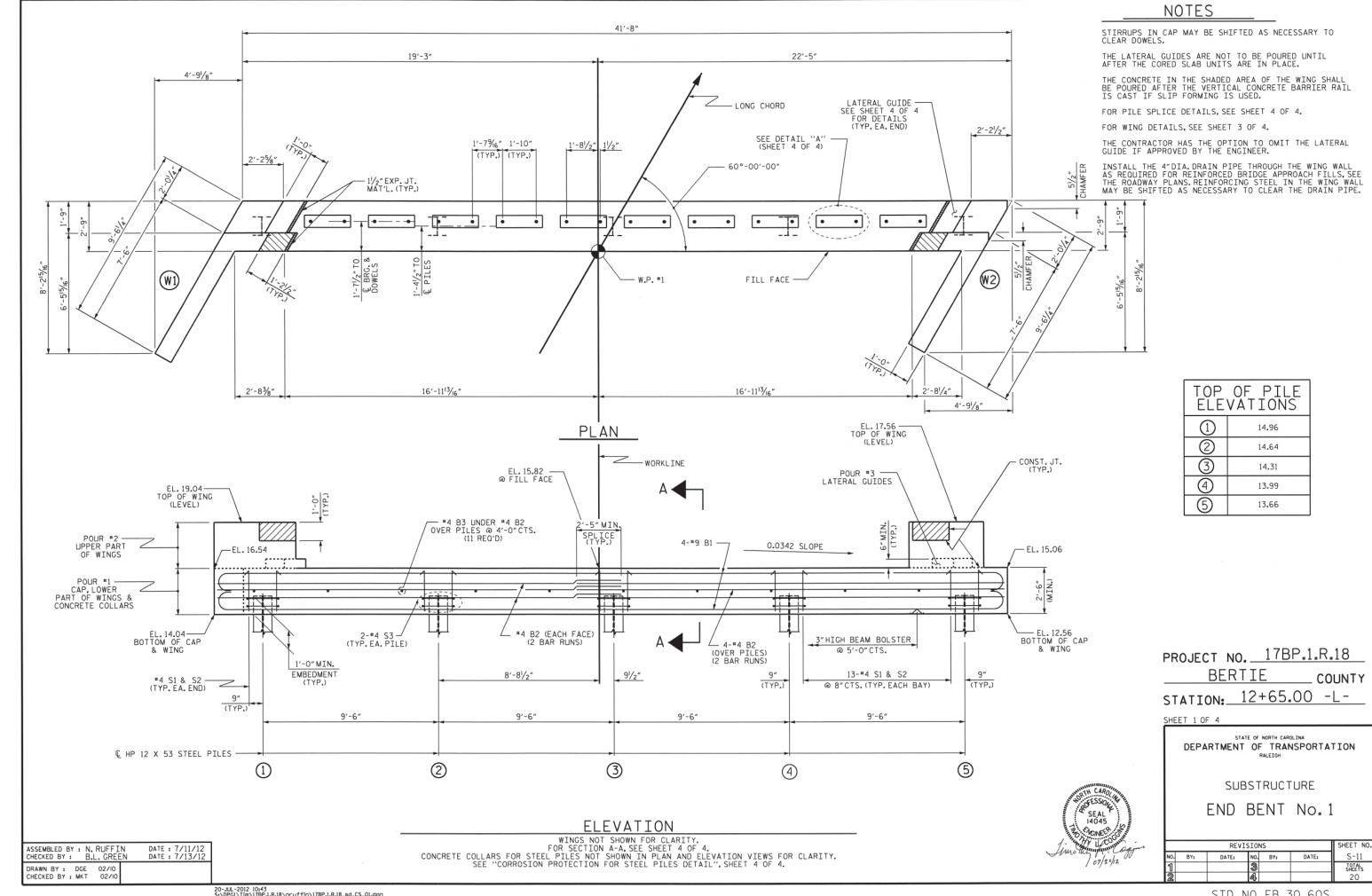
ASSEMBLED BY : CHECKED BY :

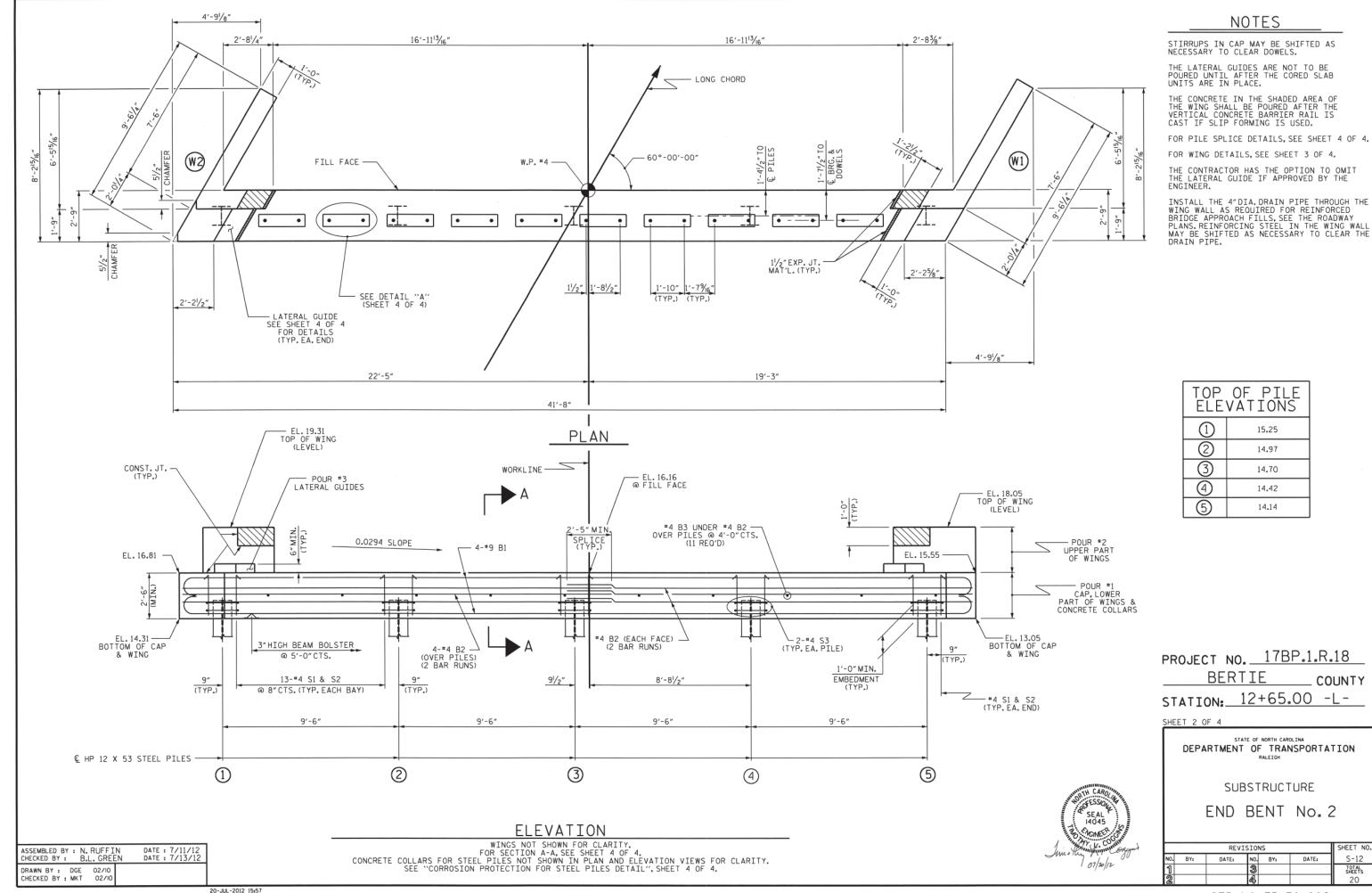
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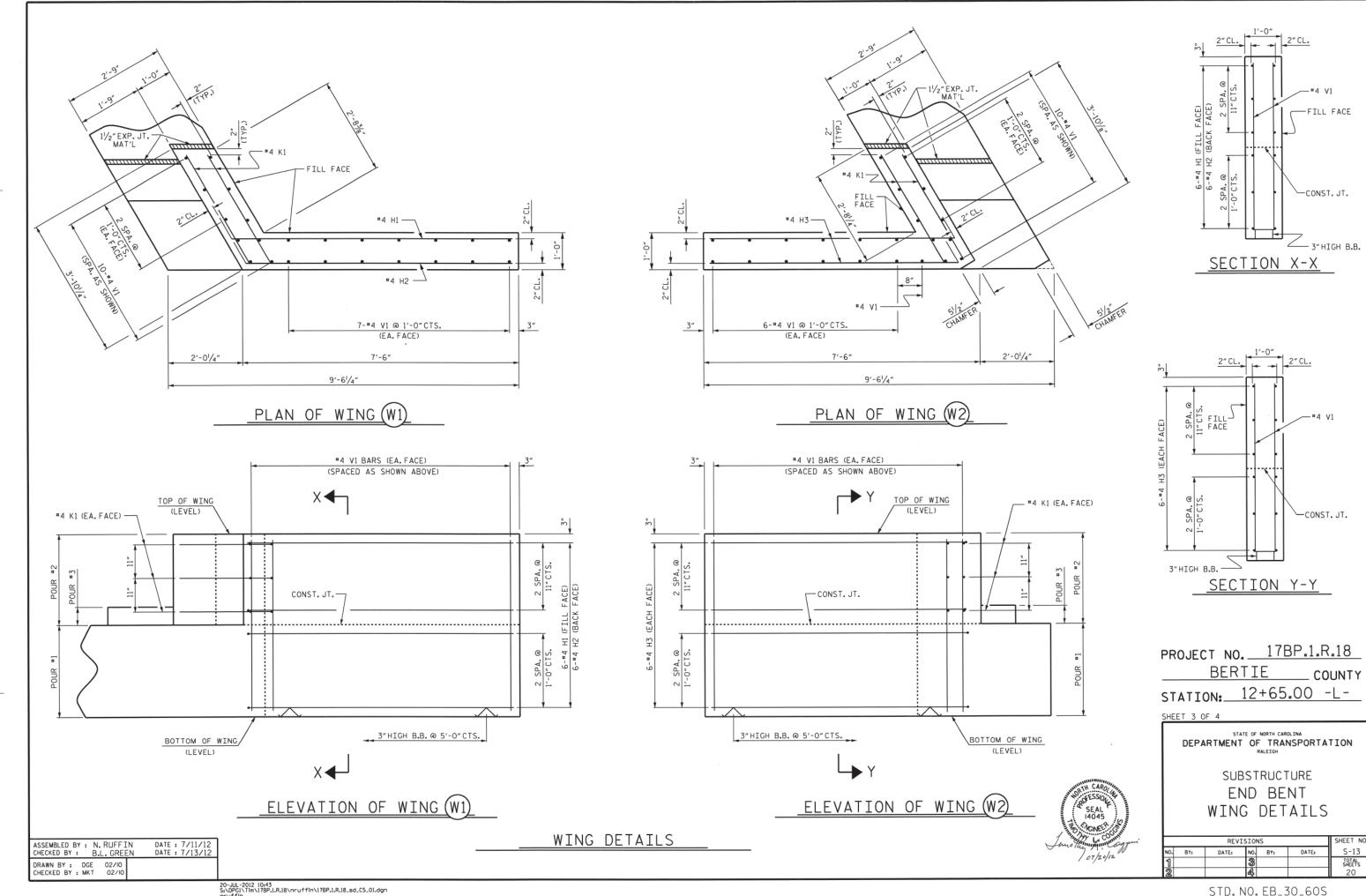
SECTION E-E

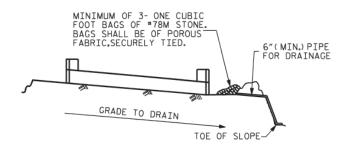
GUARDRAIL ANCHOR ASSEMBLY DETAILS

1/4" HOLD-DOWN P







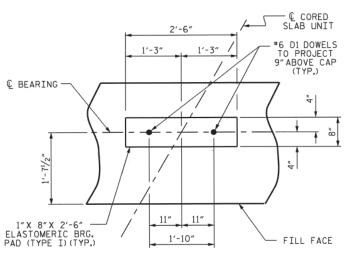


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION, PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

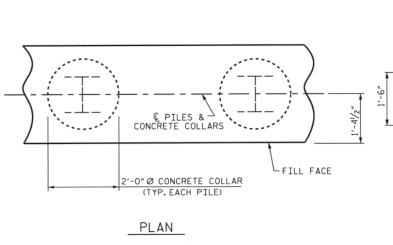
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



DETAIL "A"

(END BENT No.1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

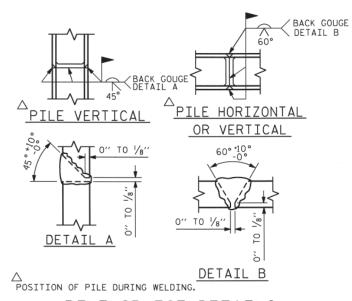
COLLAR

© HP 12 X 53 STEEL PILE

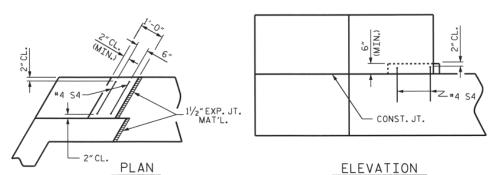
ELEVATION

ASSEMBLED BY: N. RUFFIN DATE: 7/11/12
CHECKED BY: B.L. GREEN DATE: 7/13/12

DRAWN BY: DGE 02/10
CHECKED BY: MKT 02/10



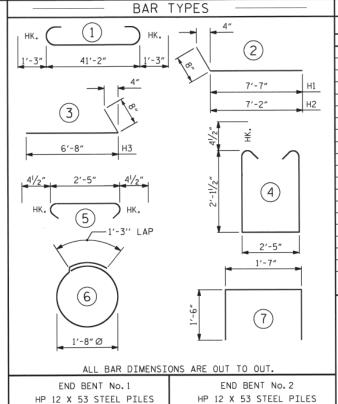
PILE SPLICE DETAILS



LATERAL GUIDE DETAILS

(END BENT No.1, LEFT LATERAL GUIDE SHOWN, RIGHT END SIMILAR)
(END BENT No. 2 SIMILAR BY ROTATION)

-BOTTOM OF CAP



NO: 5

PILE REDRIVES

LIN. FT.= 245

EA. 2

NO: 5

PILE REDRIVES

FOR ONE END BENT BAR NO. SIZE TYPE LENGTH WEIGHT B1 8 #9 1 43'-8" B2 16 #4 STR 21'-11" #4 STR B3 #6 STR H2 6 #4 2 7'-10" 31 #4 59 12 #4 STR 3'-3" K1 12 26 S2 54 #4 5 3'-2" 114 #4 6 6′-6" 43 10 #4 4'-7" 47 #4 STR 147 4'-8" REINFORCING STEEL (FOR ONE END BENT) 2218 LBS.

BILL OF MATERIAL

CLASS A CONCRETE BREAKDOWN
(FOR ONE END BENT)

POUR #1 CAP, LOWER PART
OF WINGS & COLLARS

POUR #2 UPPER PART OF

1.9 C.Y.

WINGS
POUR *3 LATERAL GUIDES 0.1 C.Y.

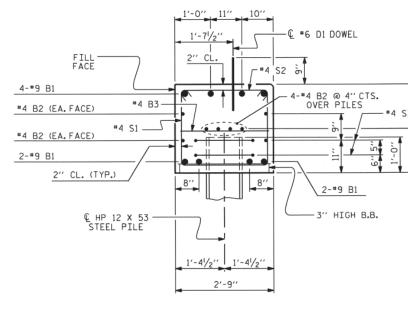
TOTAL CLASS A CONCRETE 14.7 C.Y.

TOTAL CLASS A CONCRETE

REFER TO SPECIAL PROVISIONS FOR GALVANIZATION OF STEEL PILES

LIN. FT.= 265

EA. 2



<u>SECTION A-A</u>

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.") PROJECT NO. 17BP.1.R.18
BERTIE COUNTY
STATION: 12+65.00 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No.1 & 2
DETAILS

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-14
1			3			TOTAL SHEETS
2			4			20

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

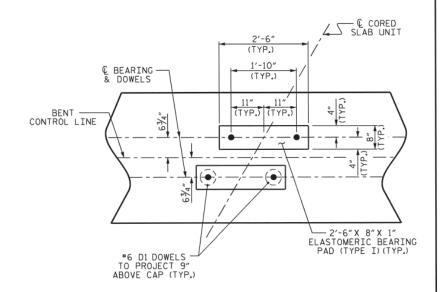
THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

★ INVERT ALTERNATE STIRRUPS.

ALVANIZE THE TOP OF EACH INTERIOR BENT PILE - <u>MINIMUM OF 26 FEET, CALVANIZE IN ACCORDANCE</u> LITH SECTION 1076 OF THE STANDARD SPECIFICATIONS

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

REFER TO SPECIAL PROVISIONS FOR GALVANIZATION OF STEEL PILES



DETAIL "A"

(DIMENSIONS ARE TYPICAL EACH BEARING)

TOP OF PILE ELEVATIONS

① 14.94
② 14.74
③ 14.54
④ 14.34
⑤ 13.93
⑦ 13.73

PROJECT NO. 17BP.1.R.18
BERTIE COUNTY

STATION: 12+65.00 -L-

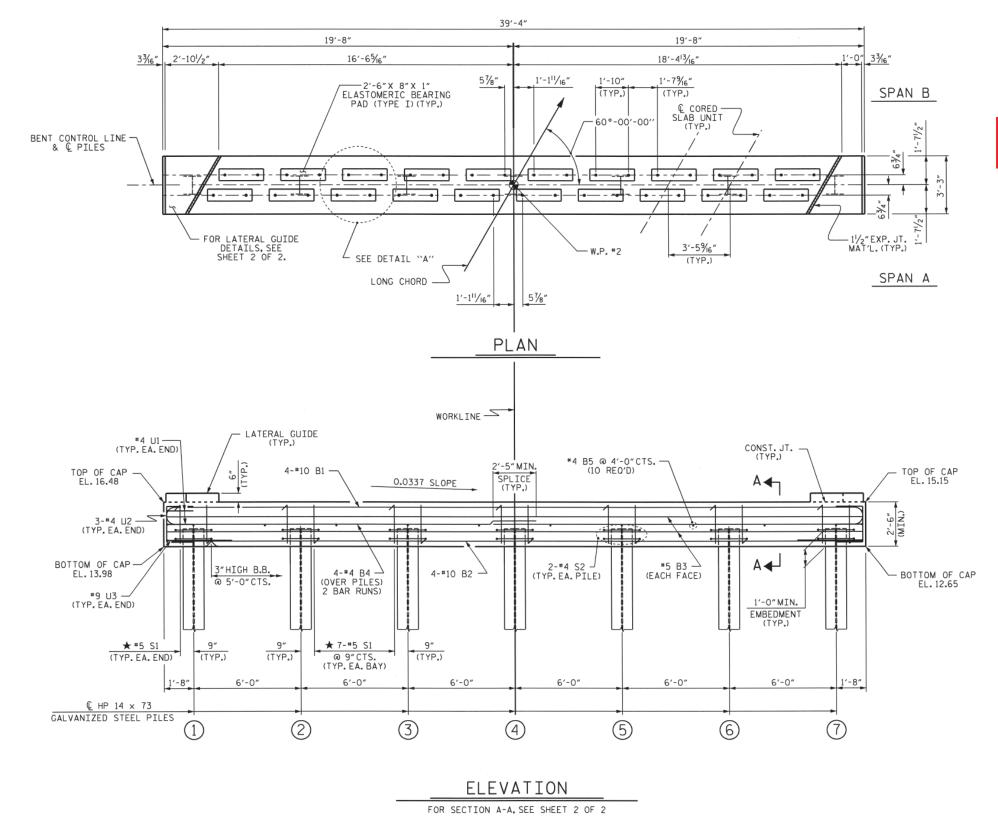
SHEET 1 OF 2

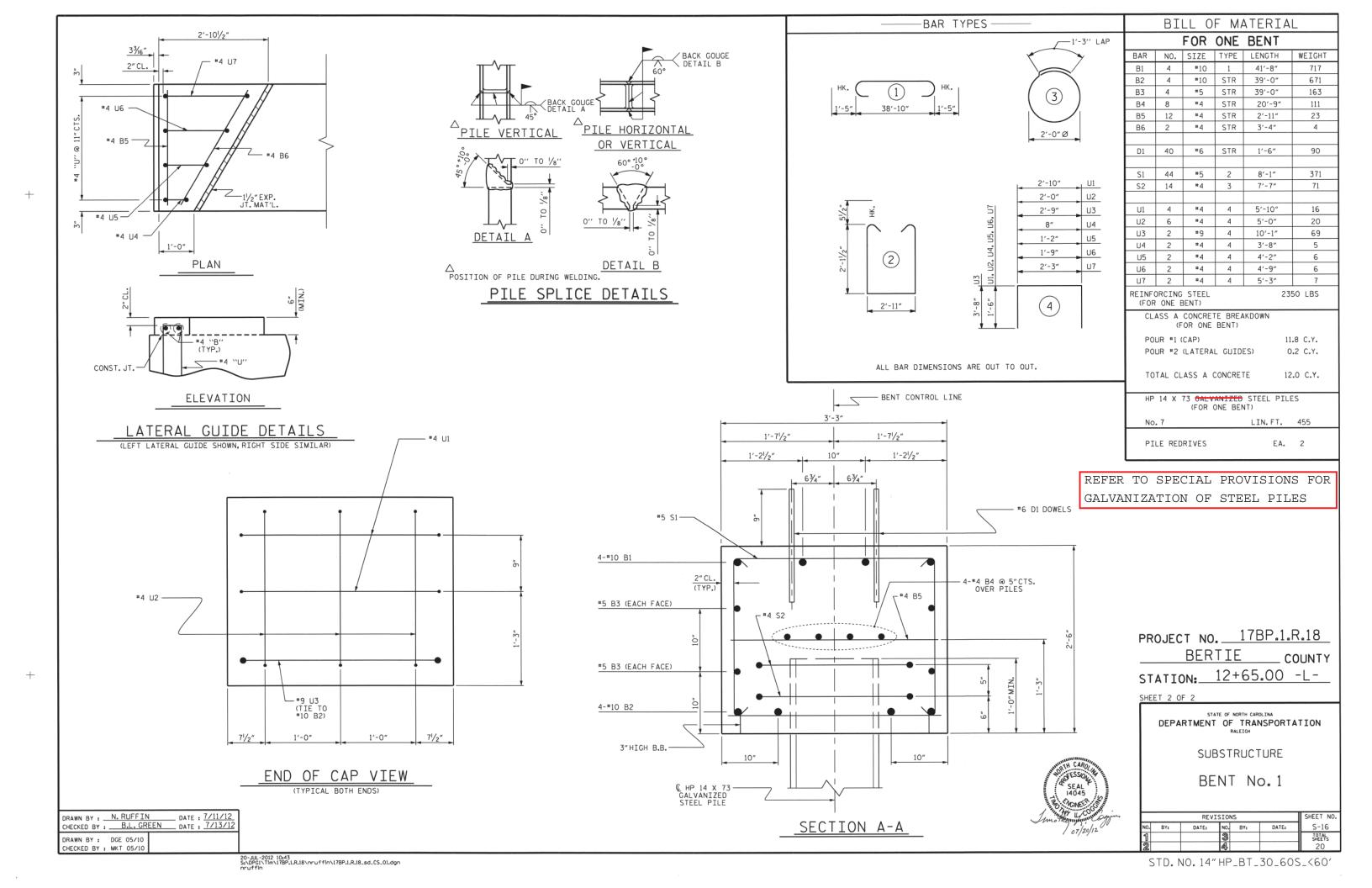
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

BENT No. 1

		SHEET NO.				
10.	BY:	DATE:	NO.	BY:	DATE:	S-15
1			3			TOTAL SHEETS
2			4			20







STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

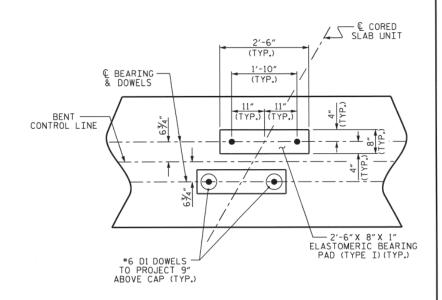
THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

★ INVERT ALTERNATE STIRRUPS.

ALVANIZE THE TOP OF EACH INTERIOR BENT PILE WINIMUM OF 26 FEET. GALVANIZE IN ACCORDANCE HITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

REFER TO SPECIAL PROVISIONS FOR GALVANIZATION OF STEEL PILES



DETAIL "A"

(DIMENSIONS ARE TYPICAL EACH BEARING)

TOP OF PILE ELEVATIONS

1 15.07

2 14.88

3 14.69

4 14.50

5 14.31

6 14.12

7 13.93

PROJECT NO. 17BP.1.R.18

BERTIE COUNTY
STATION: 12+65.00 -L-

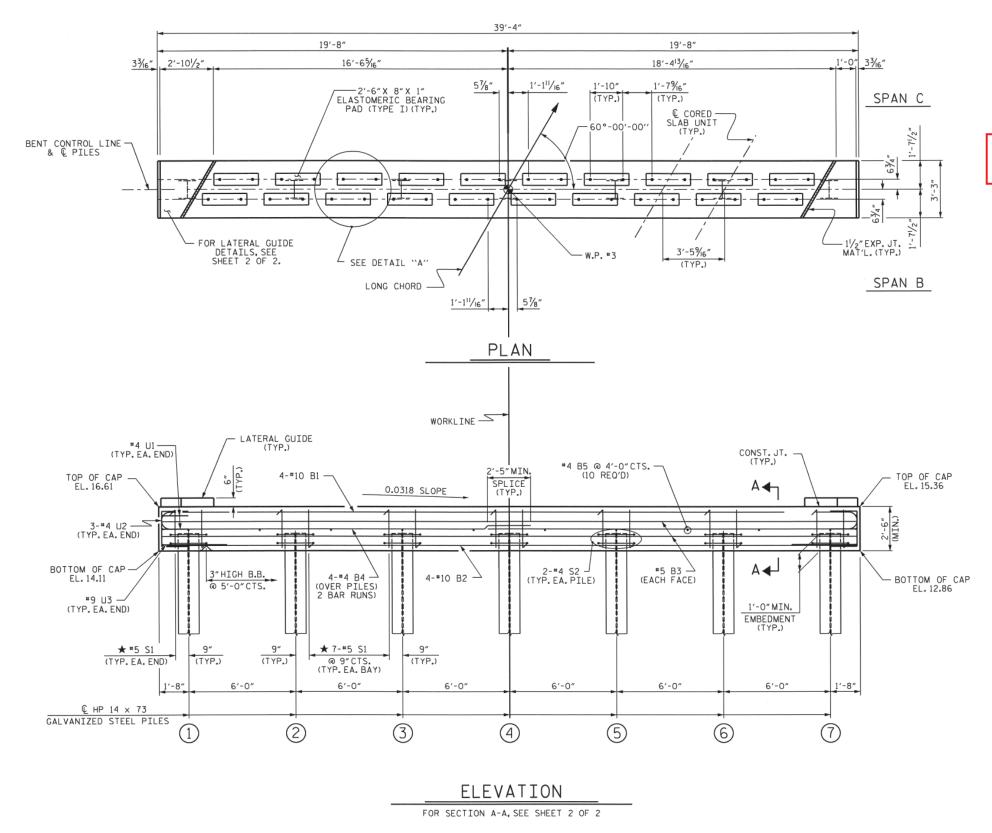
SHEET 1 OF 2

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

BENT No. 2

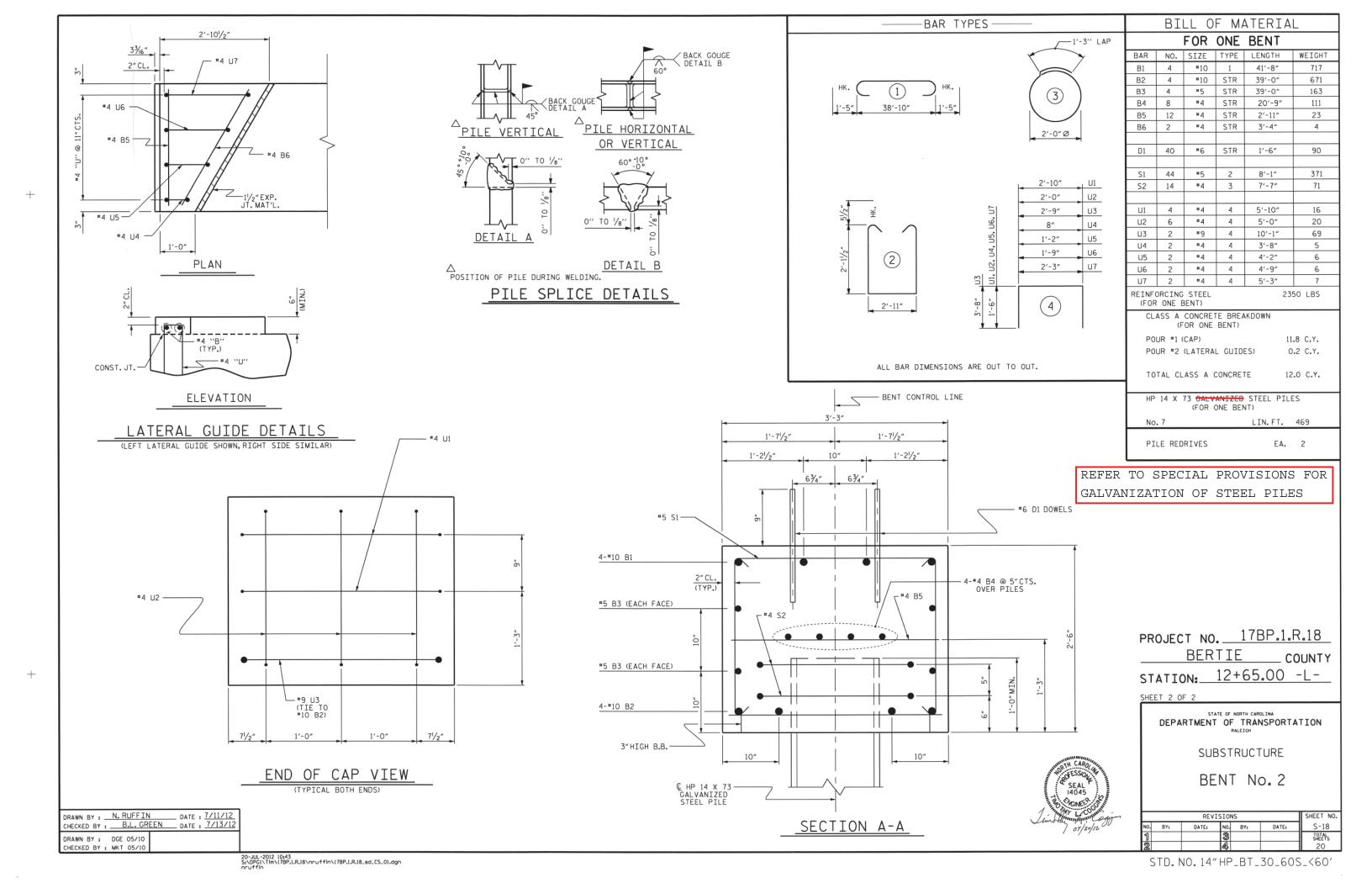
		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-17
1			3			TOTAL SHEETS
2			4			20



ASSEMBLED BY: N. RUFFIN DATE: 7/11/12 CHECKED BY: B.L. GREEN DATE: 7/13/12

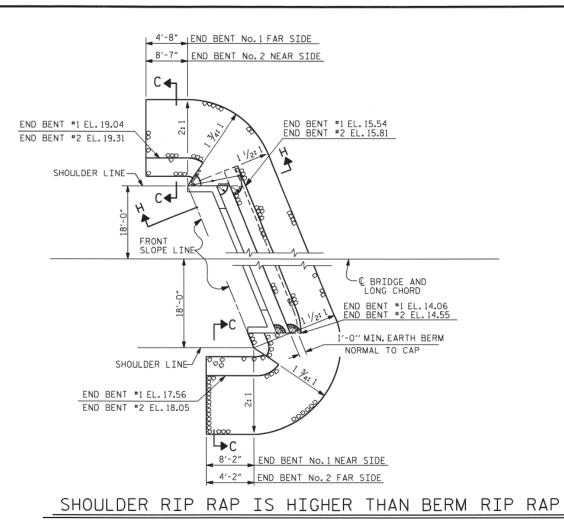
DRAWN BY: DGE 05/10 CHECKED BY: MKT 05/10

20-JUL-2012 16:06 St\DPG\\TIm\\17BP.I.R.18\nruffIn\17BP.I.R.18_sd_CS_0I.dgn tcoggins STD. NO. 14" HP_BT_30_60S_<60'

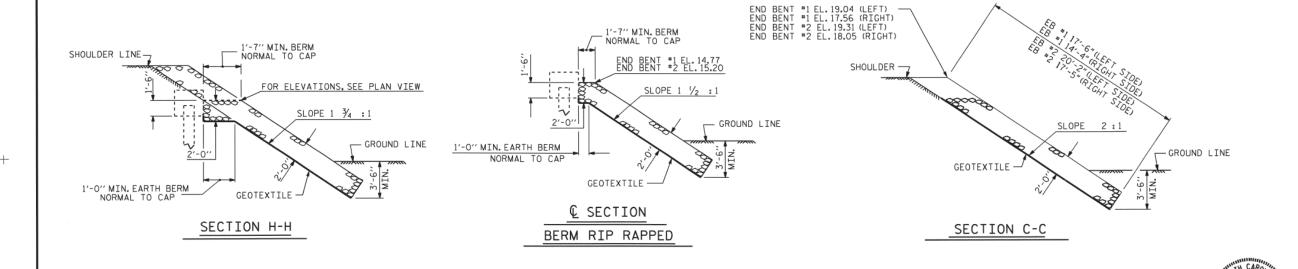


NOTES:

FOR EARTH BERM ELEVATIONS, SEE GENERAL DRAWING.



ESTIMATED QUANTITIES							
BRIDGE @ STA.12+65.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE					
	TONS	SQUARE YARDS					
END BENT 1	90	96					
END BENT 2	115	124					



PROJECT NO. 17BP.1.R.18
BERTIE COUNTY
STATION: 12+65.00 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

-RIP RAP DETAILS-

		SHEET NO.							
NO.	BY:	DATE:	NO.	BY:	DATE:	S-19			
1			3			TOTAL SHEETS			
2			4			20			

ASSEMBLED BY: N. RUFFIN DATE: 7/12/12
CHECKED BY: B.L. GREEN DATE: 7/17/12

DRAWN BY: REK 1/84 REV. 5/1/06R TLA/GM REV. 12/21/II
CHECKED BY: RDU 1/84 REV. 12/21/II

MAA/GM MAA/GM

20-JUL-2012 10:43 5:\PFGI\Tim\178P.1.R.18\nruffin\178P.1.R.18_sd_CS_01.dgn pruffin

