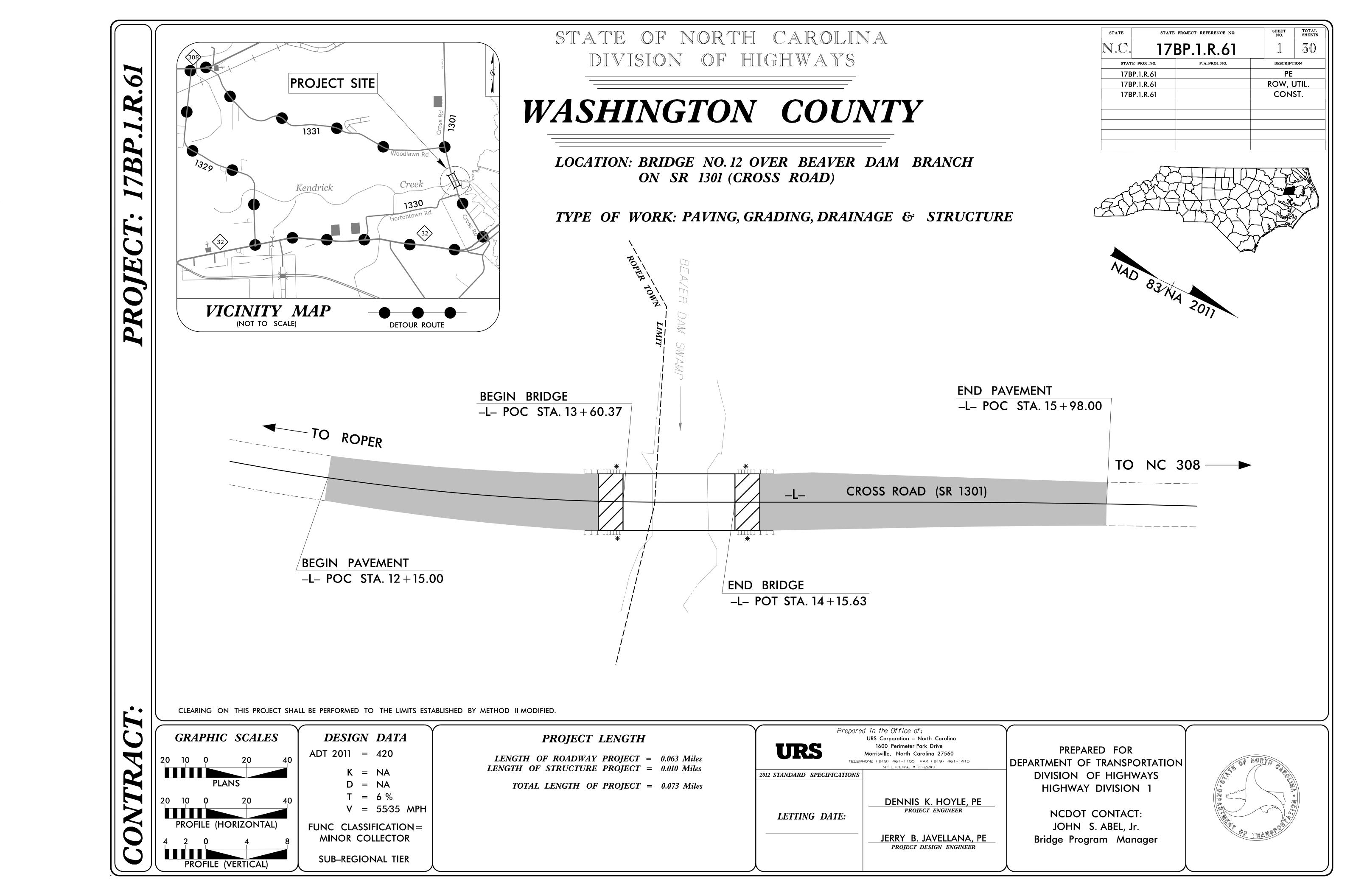
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PROJECT REFERENCE NO.

17BP.I.R.61

URS Corporation - North Carolina 1600 Perimeter Park Drive, Suite 400 Morrisville, NC 27560 PHONE(919)461-1100 FAX(919)461-1415 NC LIC. * C-2243

Prepared by



SHEET NO.

/-A

INDEX OF SHEETS

SHEET NUMBER	TITLE
1	TITLE SHEET
1-A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS
1-B	CONVENTIONAL SYMBOLS
2	TYPICAL SECTIONS, PAVEMENT SCHEDULE, AND WEDGING DETAIL
2-A	DETAIL FOR STRUCTURE ANCHOR UNIT, TYPE III
3-A	SUMMARY OF DRAINAGE QUANTITIES, SUMMARY OF GUARDRAIL,
	EARTHWORK SUMMARY, AND REMOVAL OF ASPHALT PAVEMENT SUMMARY
4	PLAN SHEET & PROFILE
TMP-1	TRAFFIC MANAGEMENT PLAN
EC-1 THRU EC-4	EROSION CONTROL PLANS
X-1 THRU X-4	CROSS-SECTIONS
S-1 THRU S-13	STRUCTURE PLANS
N/A	STRUCTURE STANDARD NOTES

GENERAL NOTES

GENERAL NOTES:	2012 SPECIFICATIONS	EFFECTIVE:	01-17-2012
		REVISED:	10-31-2014
GRADING AND SURFACING (OR RESURFACING AND WIDENING:		

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II (MODIFIED).

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE CENTURY LINK (TELEPHONE), DOMINION NORTH CAROLINA POWER (ELECTRIC), WASHINGTON COUNTY PUBLIC UTILITIES (WATER AND WATER TREATMENT PLANT DISCHARGE LINE).

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

STANDARD DRAWINGS

2012 ROADWAY ENGLISH STANDARD DRAWINGS

EFF. 01-17-2012 REV. 10-30-2012

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch -N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE

DIVISION 2 - EARTHWORK

Method of Clearing - Method II

Guide for Grading Subgrade - Secondary and Local

Method of Obtaining Superelevation - Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation

DIVISION 4 - MAJOR STRUCTURES

422.11 Reinforced Bridge Approach Fills - Sub Regional Tier

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

840.00 Concrete Base Pad for Drainage Structures

Frames and Wide Slot Flat Grates

Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates

Drop Inlet Installation in Shoulder Berm Gutter

840.46 Traffic Bearing Precast Drainage Structure

Guardrail Placement

Guardrail Installation

876.02 Guide for Rip Rap at Pipe Outlets

PROJECT REFERENCE NO.	SHEET NO.
17BP.I.R.61	I−B

*S.U.E. = Subsurface Utility Engineering

CONVENTIONAL PLAN SHEET SYMBOLS

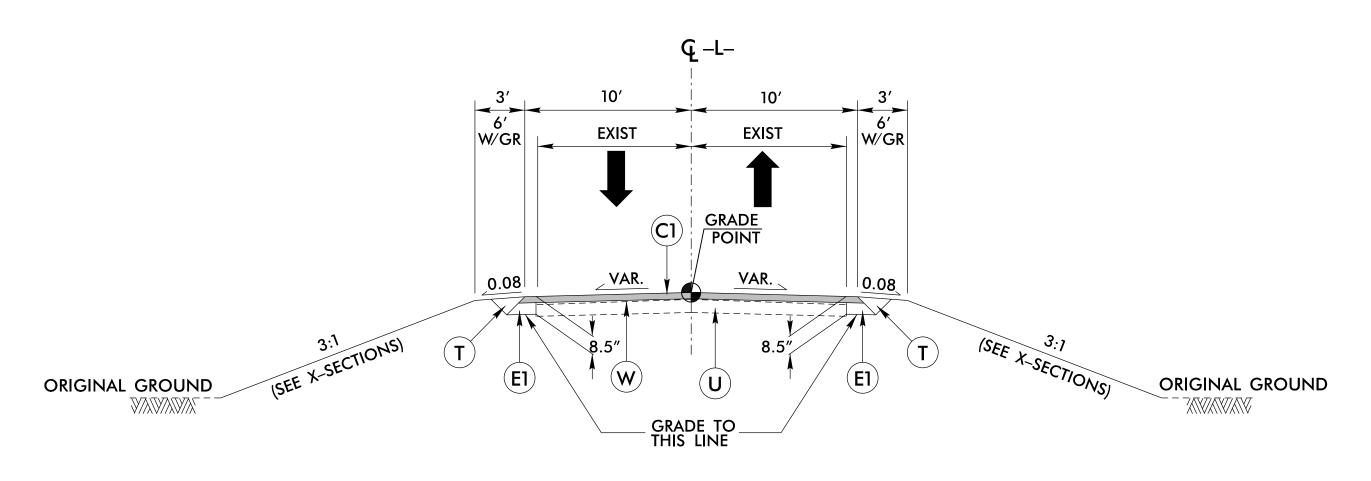
State Line		
County Line		RAILROADS:
Township Line		Standard Gauge —————
City Line		RR Signal Milepost
Reservation Line ————————————————————————————————————		Switch ————————————————————————————————————
Property Line		RR Abandoned
Existing Iron Pin	<u>O</u>	RR Dismantled
Property Corner	×	RIGHT OF WAY:
Property Monument	ECM	Baseline Control Point
Parcel/Sequence Number ————————————————————————————————————		Existing Right of Way Marker —
Existing Fence Line	×××_	Existing Right of Way Line —
Proposed Woven Wire Fence		Proposed Right of Way Line —
Proposed Chain Link Fence		Proposed Right of Way Line with
Proposed Barbed Wire Fence		Iron Pin and Cap Marker Proposed Right of Way Line with
Existing Wetland Boundary		Concrete or Granite R/W Ma
Proposed Wetland Boundary		Proposed Control of Access Line
Existing Endangered Animal Boundary ———		Concrete C/A Marker Existing Control of Access ——
Existing Endangered Plant Boundary		_
Known Soil Contamination: Area or Site		Proposed Control of Access ——
Potential Soil Contamination: Area or Site —		Existing Easement Line ————————————————————————————————————
BUILDINGS AND OTHER CULT	TURE:	Proposed Temporary Construction
Gas Pump Vent or U/G Tank Cap	<u> </u>	Proposed Temporary Drainage E
Sign —	<u>©</u> s	Proposed Permanent Drainage E
Well ———————————————————————————————————	O	Proposed Permanent Drainage /
Small Mine	—	Proposed Permanent Utility Ease
Foundation —	_	Proposed Temporary Utility Ease
Area Outline		Proposed Aerial Utility Easement
Cemetery		Proposed Permanent Easement w
Building —		Iron Pin and Cap Marker
School	_	ROADS AND RELATED
Church		Existing Edge of Pavement
Dam —		Existing Curb
		Proposed Slope Stakes Cut —
HYDROLOGY:		Proposed Slope Stakes Fill ——
Stream or Body of Water ————————————————————————————————————		Proposed Curb Ramp
Hydro, Pool or Reservoir		Existing Metal Guardrail
Jurisdictional Stream		Proposed Guardrail —————
Buffer Zone 1		Existing Cable Guiderail
Buffer Zone 2		Proposed Cable Guiderail
Pisannearing Stream	•	Equality Symbol
Disappearing Stream ————————————————————————————————————		Pavement Removal
Spring ————————————————————————————————————		VEGETATION:
Wetland ————————————————————————————————————		Single Tree
Proposed Lateral, Tail, Head Ditch ————	< ── FLOW	Single Shrub
False Sump ————————————————————————————————————		Hedge —
		Woods Line

RAILROADS:			
Standard Gauge ————	CSX TRANSPORTATION		
RR Signal Milepost	MILEPOST 35	Orchard	· 유 · 유 · 유
Switch —	SWITCH	Vineyard ————————————————————————————————————	Vineyard
RR Abandoned		EXISTING STRUCTURES:	
RR Dismantled		MAJOR:	
RIGHT OF WAY:		Bridge, Tunnel or Box Culvert	CONC
Baseline Control Point	•	Bridge Wing Wall, Head Wall and End Wall –	
Existing Right of Way Marker	\triangle	MINOR:) (
Existing Right of Way Line		Head and End Wall	CONC HW
Proposed Right of Way Line	$\frac{R}{W}$	Pipe Culvert ————	
Proposed Right of Way Line with		Footbridge —	>
Proposed Right of Way Line with Concrete or Granite R/W Marker		Drainage Box: Catch Basin, DI or JB Paved Ditch Gutter	CB
Proposed Control of Access Line with Concrete C/A Marker	(C)	Storm Sewer Manhole	S
Existing Control of Access ——————————————————————————————————		Storm Sewer	s
Proposed Control of Access ——————————————————————————————————	_		
Existing Easement Line	•	UTILITIES:	
Proposed Temporary Construction Easement –	Е	POWER:	I
Proposed Temporary Drainage Easement — -	TDE	Existing Power Pole	•
Proposed Permanent Drainage Easement —	PDE	Proposed Power Pole	Ó
Proposed Permanent Drainage / Utility Easement-	DUE	Existing Joint Use Pole	-← -
Proposed Permanent Utility Easement ———		Proposed Joint Use Pole	-0-
Proposed Temporary Utility Easement ———	TUE	Power Manhole	(P)
Proposed Aerial Utility Easement ————————————————————————————————————	AUE	Power Line Tower	
Proposed Permanent Easement with	^	Power Transformer	
Iron Pin and Cap Marker	•	U/G Power Cable Hand Hole	•
ROADS AND RELATED FEATURES	S:	H-Frame Pole	
Existing Edge of Pavement		Recorded 0/G Fower Line	'
Existing Curb		Designated U/G Power Line (S.U.E.*)	
Proposed Slope Stakes Cut	<u>C</u>	TELEPHONE:	
Proposed Slope Stakes Fill ————	<u>F</u>	Frieting Talambana Dala	
Proposed Curb Ramp		Existing Telephone Pole	-
Existing Metal Guardrail		Proposed Telephone Pole	
Proposed Guardrail ————	<u>T T T T</u>	Telephone Manhole	
Existing Cable Guiderail		Telephone Booth	
Proposed Cable Guiderail		Telephone Pedestal	
Equality Symbol	lacktriangle	Telephone Cell Tower	
Pavement Removal ————		U/G Telephone Cable Hand Hole	
VEGETATION:		Recorded U/G Telephone Cable	
Single Tree	슌	Designated U/G Telephone Cable (S.U.E.*)—	
Single Shrub	\$	Recorded U/G Telephone Conduit	
Hedge ————		Designated U/G Telephone Conduit (S.U.E.*)	
Woods Line		Recorded U/G Fiber Optics Cable	T F0

Orchard —	윤 윤 윤
Vineyard ————————————————————————————————————	Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall -	CONC WW
MINOR:	
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge —————>	
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole ————	<u>(S)</u>
Storm Sewer	
UTILITIES:	
POWER:	
Existing Power Pole ————	•
Proposed Power Pole ————	6
Existing Joint Use Pole	
Proposed Joint Use Pole	-6-
Power Manhole	P
Power Line Tower —	
Power Transformer ———————————————————————————————————	otag
U/G Power Cable Hand Hole	
H-Frame Pole	•—•
Recorded U/G Power Line	P
Designated U/G Power Line (S.U.E.*)	P
TELEPHONE:	
Existing Telephone Pole	-•-
Proposed Telephone Pole	-0-
Telephone Manhole	\bigcirc
Telephone Booth	[3]
Telephone Pedestal	
Telephone Cell Tower	√ L →
U/G Telephone Cable Hand Hole ————	HH
Recorded U/G Telephone Cable ————	Т
Designated U/G Telephone Cable (S.U.E.*)—	
Recorded U/G Telephone Conduit ———	тс
Designated U/G Telephone Conduit (S.U.E.*)	

Designated U/G Fiber Optics Cable (S.U.E.*) -----

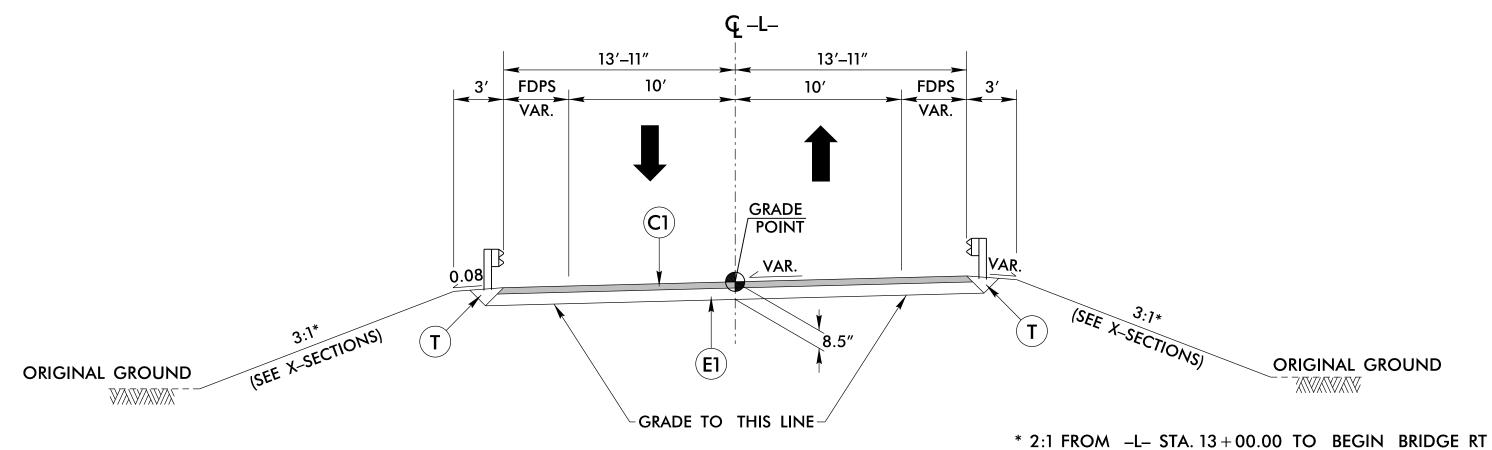
VA/A TED	
WATER:	
Water Manhole	
Water Meter	
Water Valve	_
Water Hydrant —	- \$
Recorded U/G Water Line ————————————————————————————————————	
Designated U/G Water Line (S.U.E.*)	— — — w — — — —
Above Ground Water Line	A/G Water
T\/.	
TV:	N /
TV Satellite Dish	
TV Pedestal	
TV Tower	\bigotimes
U/G TV Cable Hand Hole	H _H
Recorded U/G TV Cable	
Designated U/G TV Cable (S.U.E.*)	
Recorded U/G Fiber Optic Cable ————	
Designated U/G Fiber Optic Cable (S.U.E.*)—	- — — TV F0— — —
GAS:	
Gas Valve	\Diamond
Gas Meter —	
Recorded U/G Gas Line	•
Designated U/G Gas Line (S.U.E.*)	
Above Ground Gas Line ————————————————————————————————————	
SANITARY SEWER:	
Sanitary Sewer Manhole	
Sanitary Sewer Cleanout —————	\oplus
U/G Sanitary Sewer Line —————	ss
Above Ground Sanitary Sewer ————	A/G Sanitary Sewer
Recorded SS Forced Main Line	FSS
Designated SS Forced Main Line (S.U.E.*) —	— — — FSS— — — —
AAICCELLANICOLIC	
MISCELLANEOUS:	
Utility Pole ————————————————————————————————————	•
Utility Located Object ————————————————————————————————————	
Utility Traffic Signal Box ———————————————————————————————————	
Utility Unknown U/G Line ————————————————————————————————————	
U/G Tank; Water, Gas, Oil ———————————————————————————————————	
Underground Storage Tank, Approx. Loc. ——	
A/G Tank; Water, Gas, Oil ———————————————————————————————————	
Geoenvironmental Boring ————————————————————————————————————	lacktriangle
U/G Test Hole (S.U.E.*)	
Abandoned According to Utility Records —	AATUR
End of Information ————————————————————————————————————	E.O.I.



TYPICAL SECTION NO. 1

USE TYPICAL SECTION NO. 1

-L- STA. 12+15.00 TO -L- STA. 13+00.00
-L- STA. 15+00.00 TO -L- STA. 15+98.00



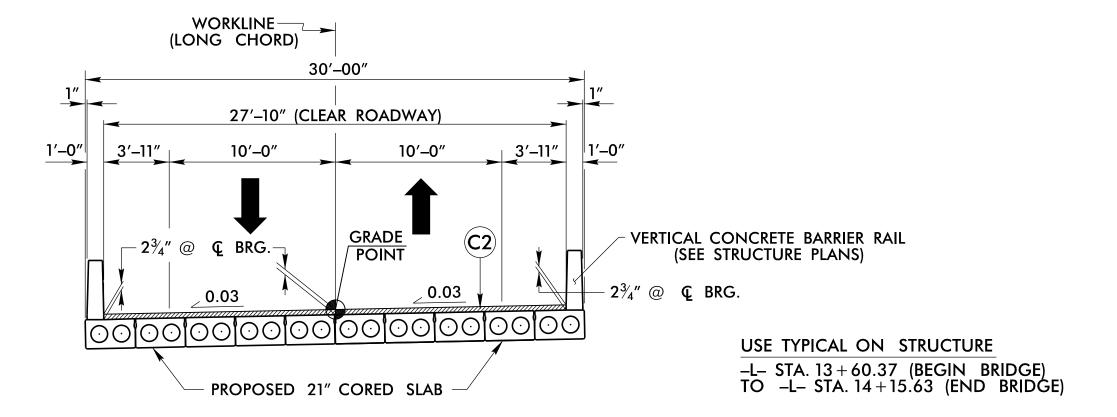
TYPICAL SECTION NO. 2

* 2:1 FROM END BRIDGE TO _L_ STA. 14 + 50.00 LT & RT

2:1 FROM END BRIDGE 10 -L- 31A. 14+30.00 LT & RT

USE TYPICAL SECTION NO. 2

-L- STA. 13 + 00.00 TO -L- STA. 13 + 60.37 (BEGIN BRIDGE) -L- STA. 14 + 15.63 (END BRIDGE) TO -L- STA. 15 + 00.00



TYPICAL SECTION ON STRUCTURE (SEE STRUCTURE PLANS)

	PAVEMENT SCHEDULE
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 1.0" IN DEPTH OR GREATER THAN 1.5" IN DEPTH.
E1	PROP. APPROX. 5.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 627 LBS. PER SQ. YD.
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH.
Т	EARTH MATERIAL.
U	EXISTING PAVEMENT.
V	TIE-IN MILLING.
W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL)

PROJECT REFERENCE NO.

17BP.I.R.61

2

WASHINGTON COUNTY BRIDGE NO.930012

ROADWAY DESIGN
ENGINEER
8/19/2016
SEAL
039831

SEAL
039831

Docusigned by:

Jerry Javellana
1A000F025A194D7...

Prepared by

URS Corporation - North Carolina

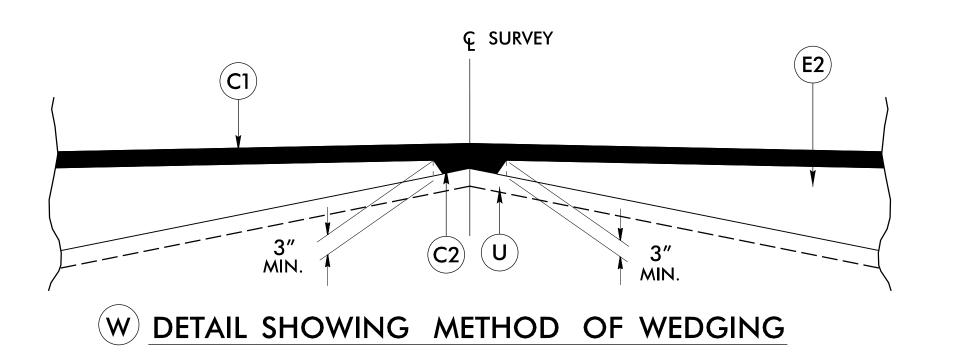
1600 Perimeter Park Drive, Suite 400

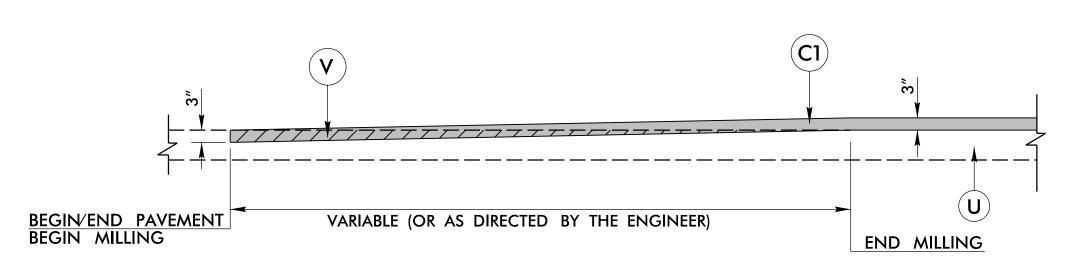
Morrisville, NC 27560

PHONE(919)461-1100 FAX(919)461-1415

NC LIC.* C-2243

NOTE: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.

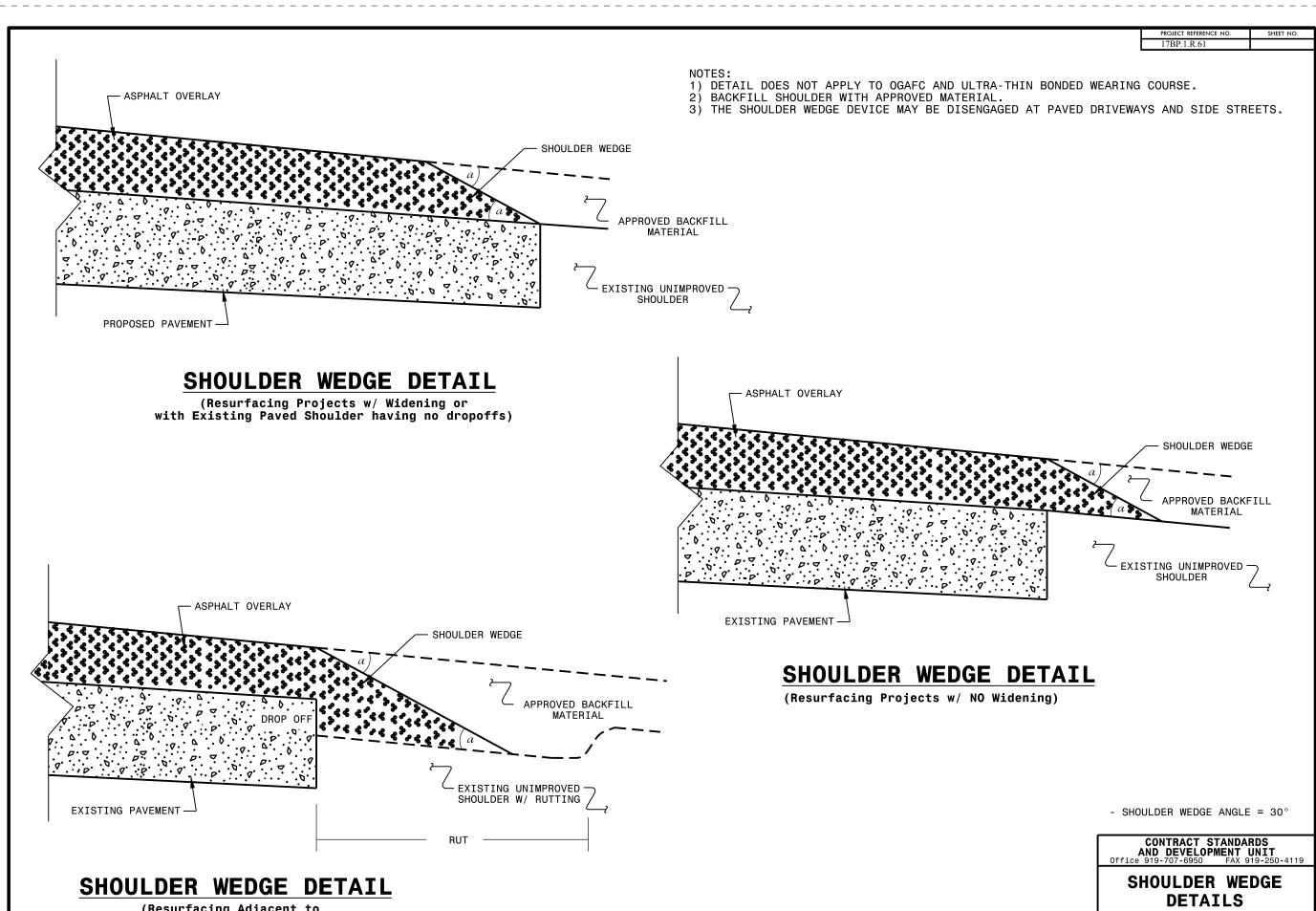




V TIE-IN MILLING DETAIL

NOTE: END MILLING WHERE PROPOSED GRADE
IS 3" ABOVE EXISTING PAVEMENT

Javellana



(Resurfacing Adjacent to Rutted Shoulder)

ORIGINAL	BY:_	T.SPELL	_DATE:	7-19-11
MODIFIED	BY:		DATE:	10/16/12
CHECKED B	3Y :		DATE:	
ETLE CDEC				adaka 1 dan

PROJECT REFERENCE NO. SHEET NO. 17BP.1.R.61 2-A

NORTH CAROLINA DEPT, OF TRANSPORTATION SYAWHOIH OF HIGHWAYS .D.N , HDIBLAR GUARDRAIL ANCHOR UNIT, TYPE III STRUCTURE ANCHOR UNITS STATE OF ENGLISH DETAIL DRAWING FOR BEAM BLOCK BEAM POST "9-,L **JARIABLE** THRIE THRIE OFFSET " pt7 | "8/27 "417 "8°87 34" DIA POST AND OFFSET BLOCK (SECTION WILL REQUIRE BOLT HOLE DRILLING IN IE BEAM OFFSET BLOCK E POST. 3,-2,, SECTION OF BEAM POST WTR SECTION ELEVATION VIEW ,,0-,9 5, - 6^{3/9},, 3,-2,, SECTION OF WTR BEAM POST 8 WTR OPT S ,,0-,9 7,-9,, SECTION OF THRIE BEAM POST 7 1" DIA. HOLES (TYP. FOR ANCHOR BOLTS 78"x 118" FOR UNION 315/ 213/6/ 313/6/ ,,0-,9 10" 10" 50,, THRIE "8-'r THRIE-BEAM SECTION SECTION OF 1 "p\E "8\I "p\E ۷, - 0 STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C. ENGLISH DETAIL DRAWING FOR STRUCTURE ANCHOR UNITS GUARDRAIL ANCHOR UNIT, TYPE III

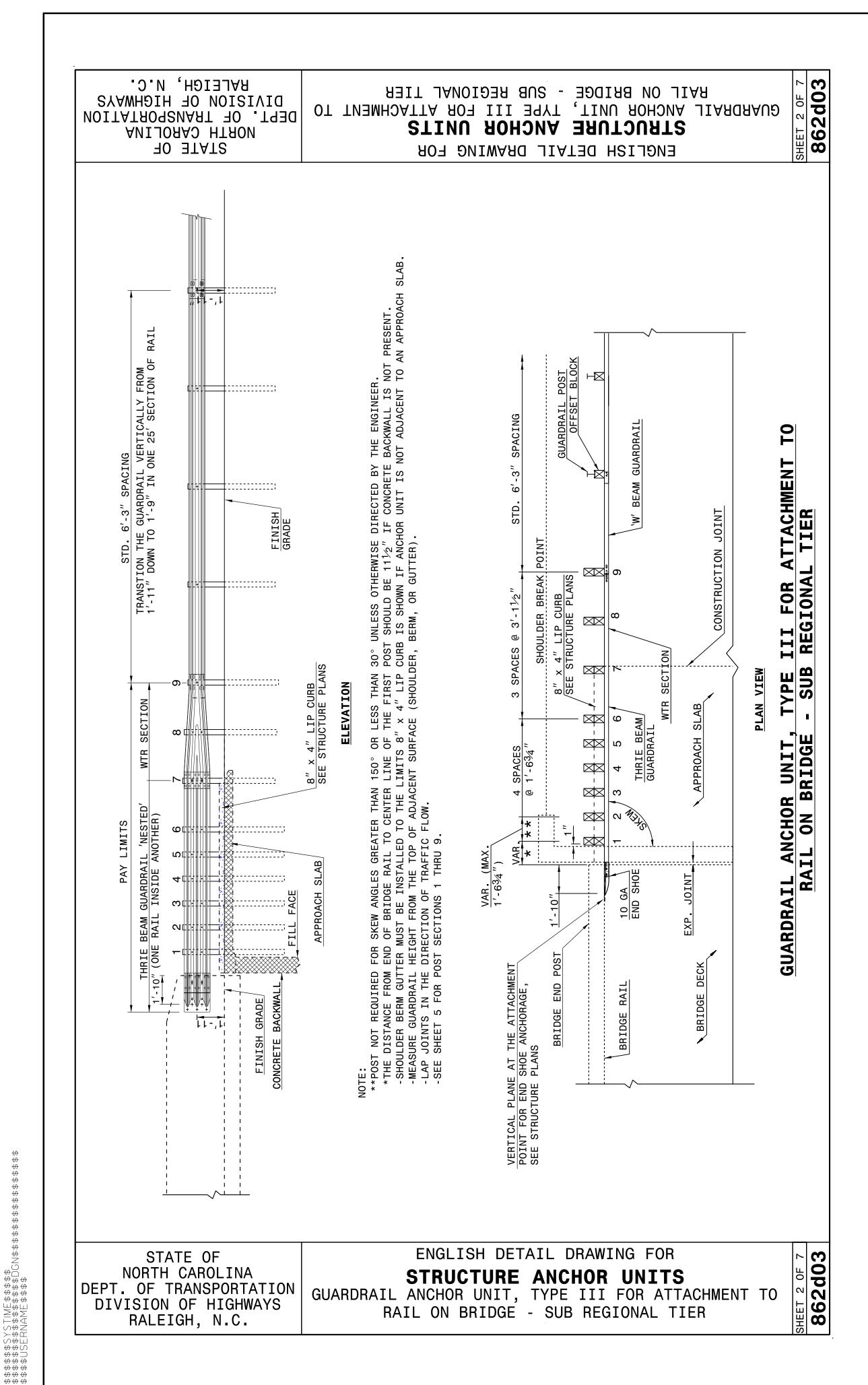
SEAL
022966

Docusigned by:
Joll Howerton
873F3D17DCDC45F...

CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J HOWERTON	DATE: 06-22-12
ONIGINAL BIL <u>G HOME HIS</u>	DAIL: <u> </u>
MODIFIED BY:	DATE:
MODILIED DI"	DATE
CHECKED BY:	DATE:
UNEUNED DI	DATE:
FILE SPEC.:	
FILE SPEU	



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO. 17BP.1.R.61 3-A

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

LINE & STATION SIZE THICKNESS OR GAUGE	OFFSET	OW SEQU	D (RCP, CSP	DRAINAGE PIPE P, CAAP, HDPE, or PVC) NOT USE CAAP NOT USE HDPE	C.S. PIPE 12" 15" 18" 24" 30" 54" 790 90 90 60 60	R.C. PIPE CLASS III 12" 15" 18" 24" 30" 60"	R.C. PIPE CLASS IV	ENDWALLS STD. 838.01 OR STD. 838.11 (UNLESS NOTED OTHERWISE)	MASONRY DRAINAGE STRUCTURE	THRU 5' THRU 10' AND ABOVE THRU 10' AND ABOVE	STD. 852.04 OR STD. 852.06 CONCRETE TRANSITIONAL SECTION	STD. 840.14 OR STD. 840.15 FRAME AND GRATES STD. 840.16 I. TYPE "A" STD. 840.17 OR 840.26 I. TYPE "B" STD. 840.18 OR 840.27	I. (W.S. FLAT) FRAME W/2 GRATES STD. 840.20	C. & BRICK PIPE PLUG, C.Y. STD. 840.71	ABBREVIATIONS C.A.A. CORRUGATED ALUMINIUM ALC.B. CATCH BASIN C.S. CORRUGATED STEEL D.I. DROP INLET G.D.I. GRATED DROP INLET H.D.P.E HIGH DENSITY POLYETHYLENE J.B. JUNCTION BOX M.H. MANHOLE N.S. NARROW SLOT P.V.C. POLYVINYL CHLORIDE R.C. REINFORCED CONCRETE T.B.D.I. TRAFFIC BEARING DROP INLET T.B.J.B. TRAFFIC BEARING JUNCTION W.S. WIDE SLOT
		TOP EL INVERT % MIN		0 0 0 0 0			15" SID	».	Y CY	O S O L L S O S O N O N O N O N O N O N O O N O	E F G		G.D.	O CY	REMARKS
14 + 50.00	12 LT	0401 4.75 1.58 0.34				20				1			1 1		TB 2GI
TOTAL						20				1			1 1		

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE 350

GUARDRAIL SUMMARY

110 - 110	11-CATITO IMITACI P	TITE SS	<u> </u>																					
SURVEY LINE	BEG. STA.	END STA.	LOCATION		LENGTH		WARRANT POINT		"N" DIST.	_T TOTAL	FLARE	FLARE LENGTH		w		ANCHORS				IMPACT ATTENUATOR SINGLE TYPE 350 FACED	REMOVE	REMOVE AND		
				STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	FROM SHOUL. FOL WIDTH APF	APPROACH END		APPROACH END	TRAILING END	XI MOD		GRAU 350 TL–3	M-350 XIII CAT-1	VI BIC	AT-1	ATTENUATOR SINGLE FACED GUARDRAIL	GUARDRAIL	STOCKPILE EXISTING GUARDRAIL	REMARKS
-L-	12 + 78.02	13 + 60.25	LT	82.23′			BRIDGE		VAR.	VAR.		50.00′		1.00′		1	1							
-L-	12 + 80.54	13 + 60.75	RT	80.21′				BRIDGE	VAR.	VAR.	50.00′		1.00′			1	1							
-L-	14 + 15.50	14 + 96.75	LT	81.25′				BRIDGE	VAR.	VAR.	50.00′		1.00′			1	1							
-L-	14+15.50	14 + 96.77	RT	81.27′			BRIDGE		VAR.	VAR.		50.00′		1.00′		1	1							
			TOTAL	324.96′												4	4							
		DEDUCT	TON FOR ANCHORS	-275.00′												DEDU	ICTION F	FOR ANCHORS:						
			PROJECT TOTAL	49.96′											(GRAU 350 TL-3) 4 @ 50' = 200'									
			SAY	62.50′														(TYPE IIII) 4 @ 18.75' =	75′					
		ADDITIONAL GUARDRA	AIL POST = 2 EACH											TOTAL DEDUCTIONS = 275'										

SUMMARY OF EARTHWORK

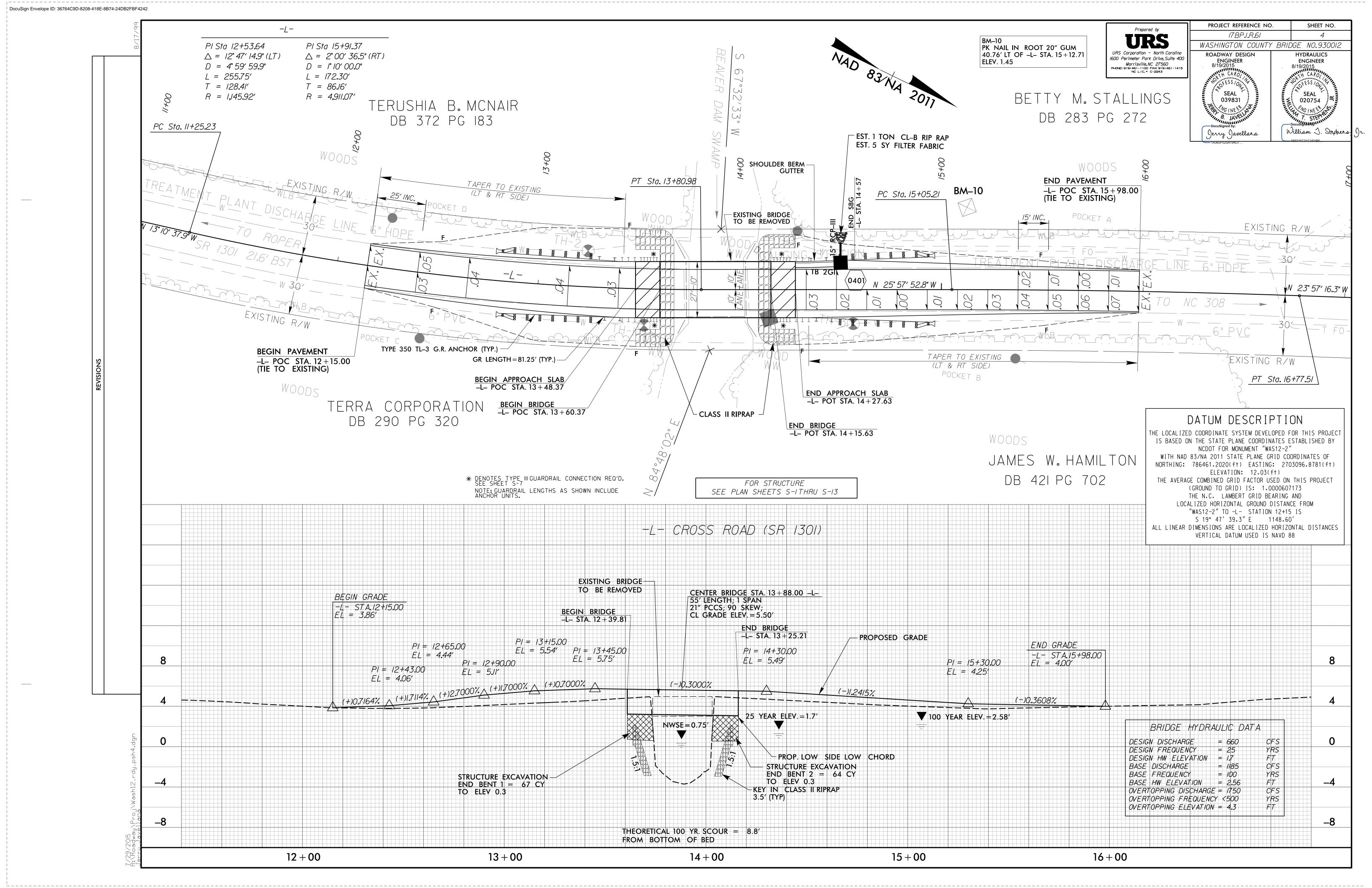
IN CUBIC YARDS

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT + %	BORROW	WASTE
-L-					
12+15.00 TO 13+60.37 (BEGIN BRIDGE)	2		240	238	
14+15.63 (END BRIDGE) TO 15+98.00	5		144	139	
SUB-TOTAL	7		384	377	
LOSS DUE TO CLEARING & GRUBBING					
PROJECT TOTAL	7		384	377	
5% TO REPLACE TOP SOIL ON BORROW PIT				19	
GRAND TOTAL	7			396	
SAY	10			400	

REMOVAL OF EXISTING ASPHALT PAVEMENT

LINE	BEGIN STATION	END STATION	LOCATION	SQ. YD.
-L-	13 + 00.00	13 + 60.50	CL	142.52
-L-	14+15.00	15 + 00.00	CL	199.95
			TOTAL	342.47
			SAY	350

NOTE: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading".



TRAFFIC MANAGEMENT FOR TEMPORARY ROAD CLOSURE R11-2 48" x 30" ROAD CLOSED CLOSED CLOSED **CLOSED** 1000 FT AHEAD R11-4 60" x 30" ROAD CLOSED TYPE III BARRICADE(S) 9 500′+/-500′+/-1/2 MILE +/-TYPE III BARRICADE **7B** SR 1301 CROSS RD SR 1301 CROSS RD. **WORK AREA** BRIDGE #12 R11-4 # 60" x 30" ROAD CLOSED 500′+/-1/2 MILE +/-500′+/-500′+/-**WBS** ,* TYPE III BARRICADE CLOSED CLOSED CLOSED **CLOSED** 500 FT 1000 FT AHEAD, ROAD TYPE III BARRICADE(S) CLOSED CLOSED AHEAD LEGEND DIRECTION OF TRAFFIC FLOW NEXT LEFT SP-4L NEXT RIGHT SP-4R 42" X 12" STATIONARY SIGN BARRICADE (TYPE III) INSTALL 500'+/- PRIOR TO EXISTING INTERSECTION Prepared by GENERAL NOTES 1 - INSTALLATION OF TEMPORARY ROUTE MARKERS, DESTINATION SIGNS AND ANY NECESSARY MODIFICATIONS TO EXISTING OR PROPOSED REGULATORY OR WARNING SIGNS WILL BE URS Corporation - North Carolina MADE BY OTHERS (STATE OR CITY FORCES) UNLESS OTHERWISE DESIGNATED IN PLANS. 1600 Perimeter Park Drive, Suite 400 Morrisville,NC 27560 PROVIDE A MINIMUM 21 CALENDAR DAY NOTICE TO STATE FORCES BEFORE A ROADWAY IS PHONE(919)461-1100 FAX(919)461-1415 NC LIC. # C-2243 CLOSED TO TRAFFIC SUCH THAT NECESSARY PROVISIONS CAN BE MADE TO INFORM LOCAL ROADWAY STANDARD DRAWINGS EMERGENCY AND LAW ENFORCEMENT PERSONNEL, SCHOOLS OR ANY OTHER PARTIES THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD AFFECTED BY THE ROAD CLOSURE. DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF PROJECT: 17BP.1.R.61

TRANSPORTATION - RALEIGH N.C., ARE CONSIDERED A PART OF THE PLANS:

<u>STD. NO.</u>	<u>TITLE</u>
1101.03	TEMPORARY ROAD CLOSURES
1101.04	TEMPORARY SHOULDER CLOSURES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1135.01	CONES
1145.01	BARRICADES
904.10	ORIENTATION OF GROUND MOUNTED SIGNS

- 2 INSTALL SIGNS BEFORE THE BARRICADES WHEN CLOSING THE ROADWAY TO TRAFFIC. REMOVE BARRICADES BEFORE SIGNS WHEN OPENING THE ROADWAY TO TRAFFIC. INSTALL/REMOVE SIGNS AND BARRICADES WITHIN THE SAME CALENDAR DAY.
- 3 POSITION WING BARRICADES ON THE SHOULDERS AND SLOPE THE STRIPES DOWNWARD IN THE DIRECTION TOWARD WHICH TRAFFIC MUST TURN IN DETOURING.
- 4 USE ADDITIONAL TYPE III BARRICADES IN STAGGERED LOCATIONS SUPPLEMENTED WITH SIGN R11-4 "ROAD CLOSED TO THRU TRAFFIC" IN THE EVENT THAT TRAFFIC MUST BE MAINTAINED BEYOND THE DETOUR POINT.
- 5 SEE STANDARD SPECIFICATION 1089-1 FOR WORK ZONE SIGNS.
- 6 SEE STANDARD SPECIFICATION 1089-2 FOR WORK ZONE SIGN SUPPORTS.

WASHINGTON _ COUNTY

STATION: 13+88.00 -L-

REPLACES BRIDGE NO. 930012 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

TRAFFIC MANAGEMENT PLAN

039831 27'-10" CLEAR ROADWAY - 90° SKEW

SEAL

		P	EVISIO	NIS		SHEET NO.				
	REVISIONS									
NO.	BY:	DATE:	NO.	BY:	DATE:	<i>TMP-1</i>				
1			1			TOTAL SHEETS				
9			9			1				

DATE: 12/22/14 DRAWN BY: ____LHJ_ CHECKED BY: _____JBJ___ DATE: <u>07/17/15</u>

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

PROJECT: 17BP.I.R.61 <u> WASHINGTON</u> _ COUNTY STATION: 13+88.00 -L-REPLACES BRIDGE NO. 930012

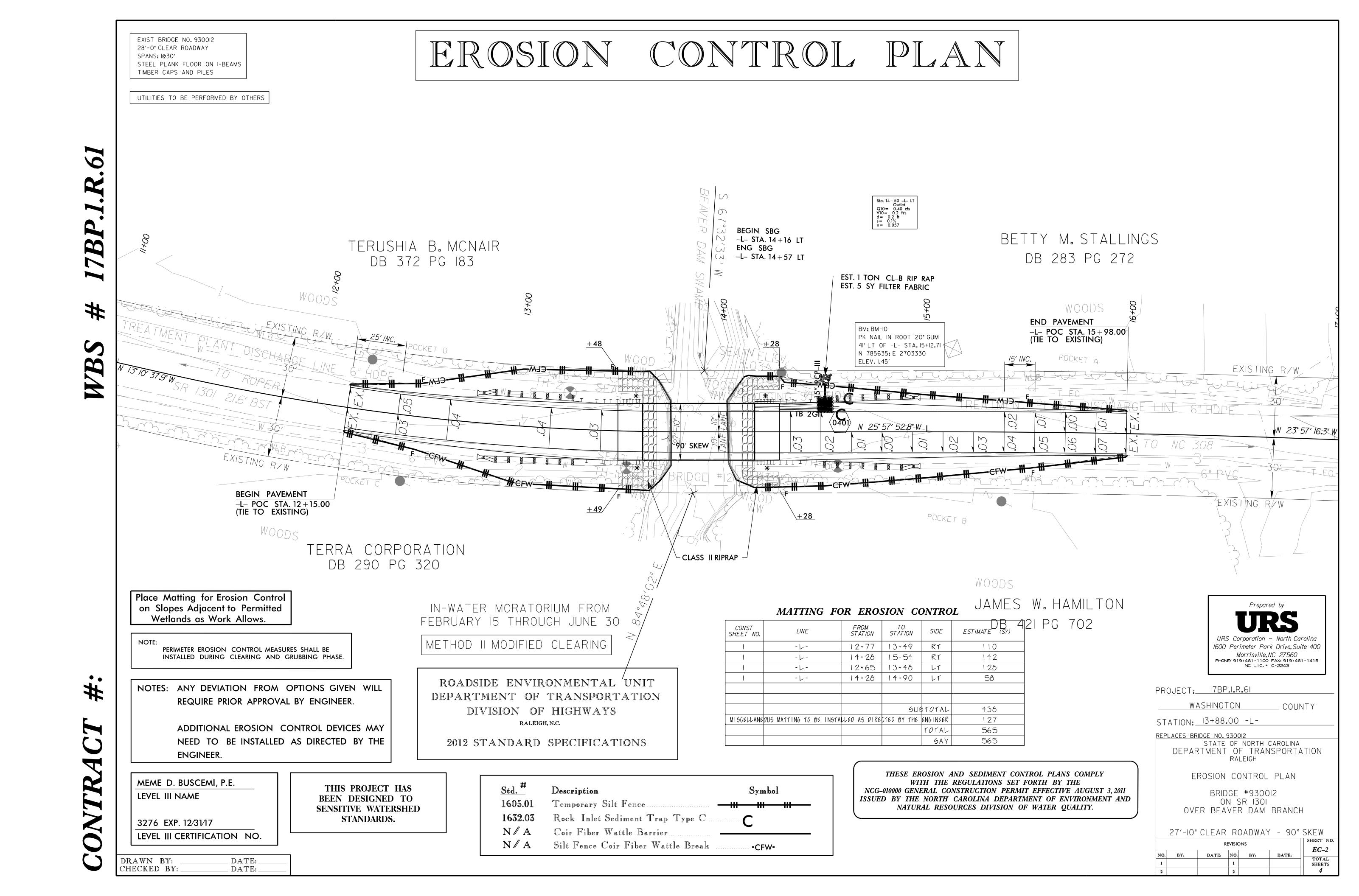
STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION RALEIGH

> EROSION CONTROL PLAN BRIDGE ON SR 1301 OVER BEAVER DAM BRANCH

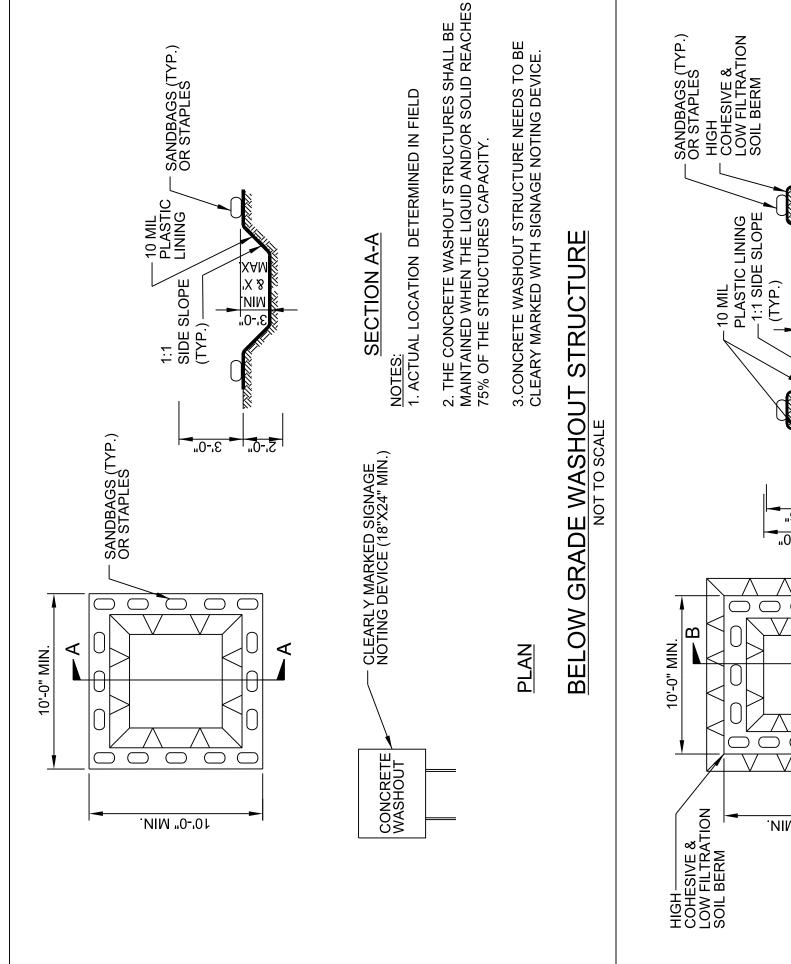
27'-10" CLEAR ROADWAY - 90° SKEW

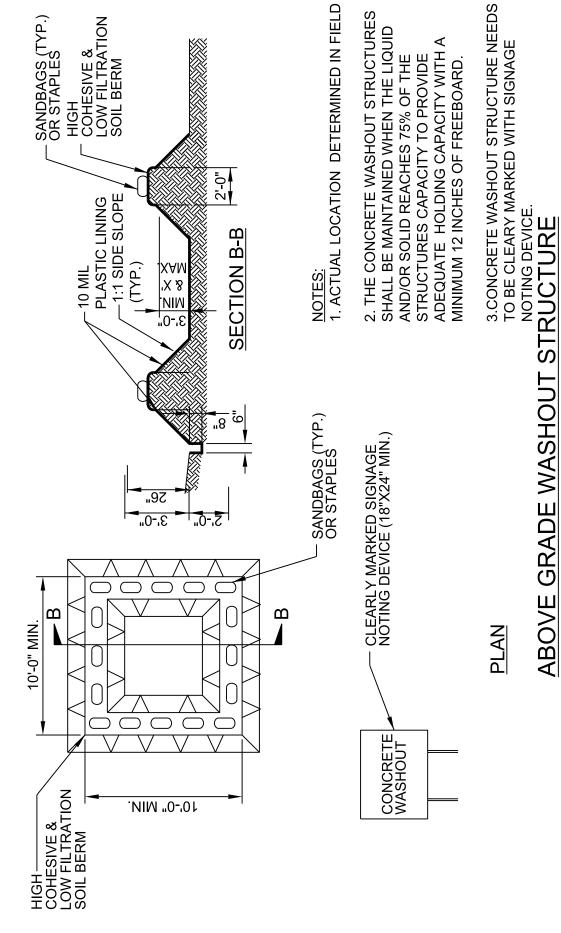
NO. BY: DATE: NO. BY: DATE: TOTAL SHEETS



WITH LINER, NO GRAVEL APPROACH

CONCRETE WASHOUT STRUCTURE WITH LINER





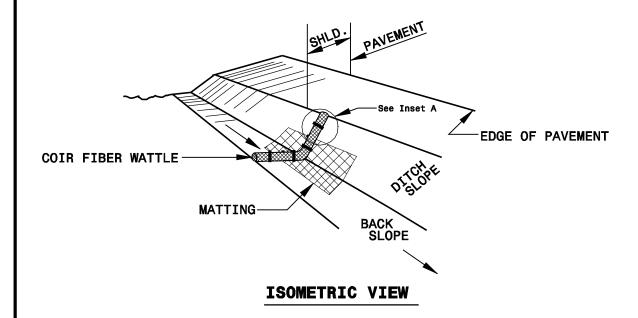
PRELIMINARY DESIGN NOT FOR CONSTRUCTION

ABOVE GRADE WASHOUT

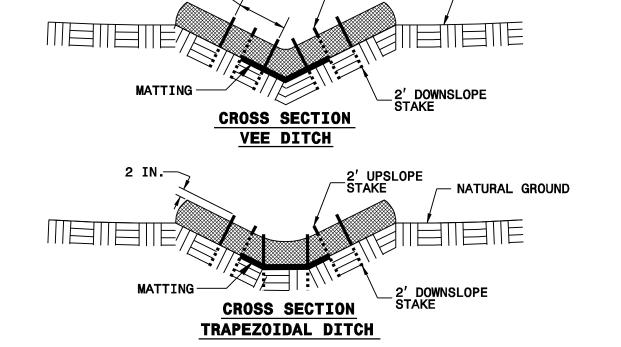
COIR FIBER WATTLE DETAIL

-NATURAL GROUND

PROJECT REFERENCE NO	D. SHEET NO	Э.
17BP.1.R.61		
R/W SHEET I	10.	
ROADWAY DESIGN ENGINEER	HYDRÁULICS ENGINEER	



2'(MAX.)-



_2' UPSLOPE STAKE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

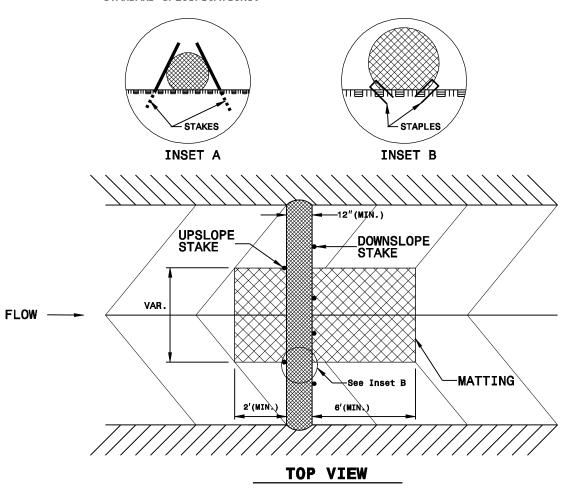
ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.



SILT FENCE COIR FIBER WATTLE BREAK DETAIL

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

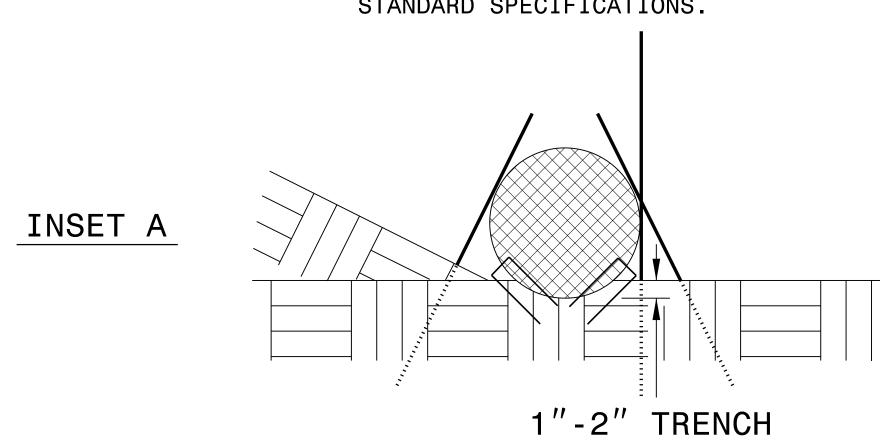
WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

SILT FENCE POST

DOWNSLOPE STAKE

SEE INSET A

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.



12" WATTLE -

STAPLE -

UPSLOPE STAKE

SILT FENCE 9 FT. 2' WOODEN STAKE SILT FENCE 2" 4 FT. 12" WATTLE

VIEW FROM SLOPE

-FILL

WATTLE

MATERIAL

ROAD

SILT

FENCE

GRADE

OF FILL

ISOMETRIC VIEW

SIDE VIEW

Prepared by

URS Corporation - North Carolina
1600 Perimeter Park Drive, Suite 400

Morrisville, NC 27560

PHONE(919)461-1100 FAX(919)461-1415
NC LIC. # C-2243

PROJECT: 17BP.I.R.61

WASHINGTON COUNTY

STATION: 13+88.00 -L-

REPLACES BRIDGE NO. 930012

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

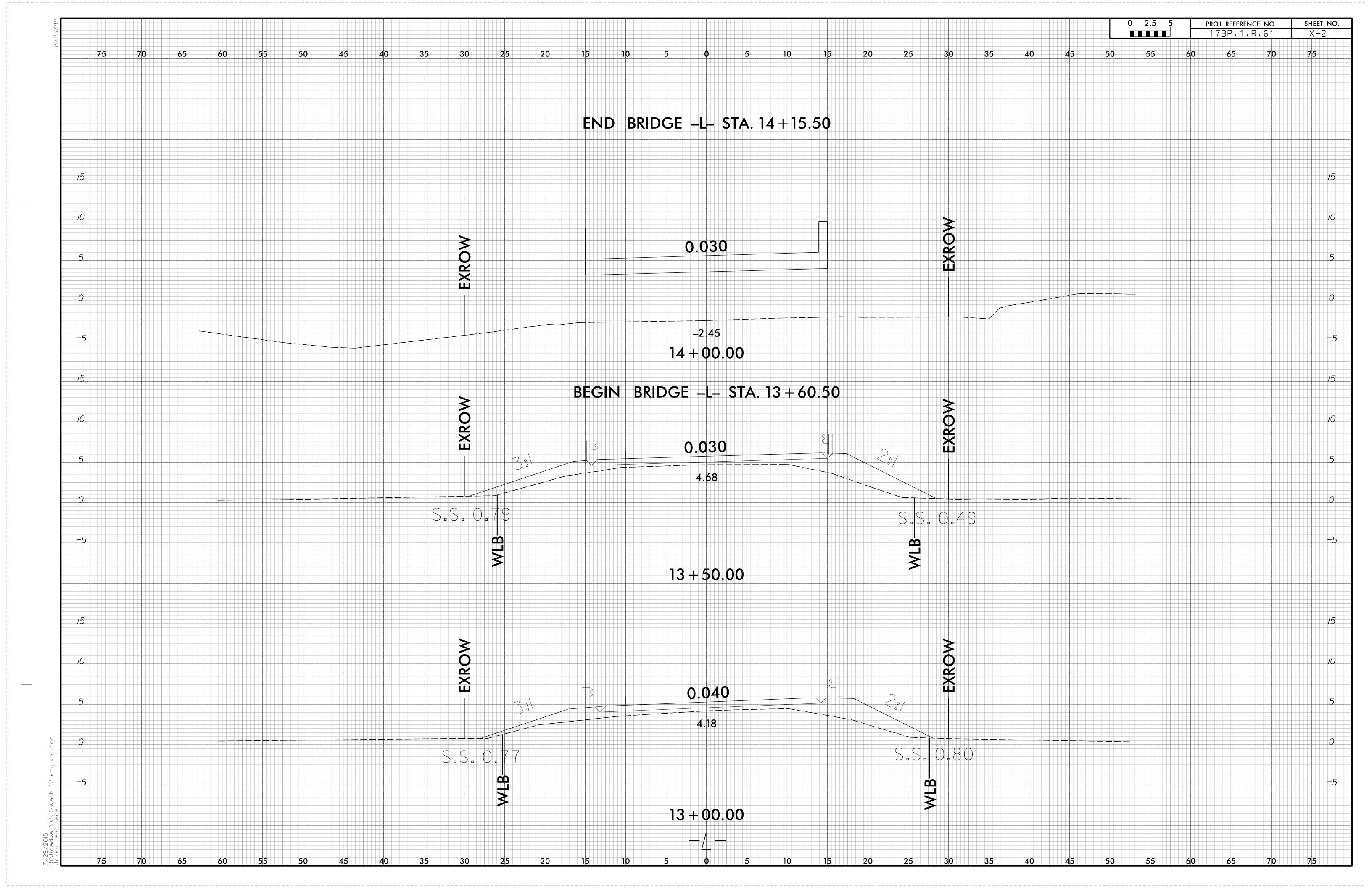
RALEIGH

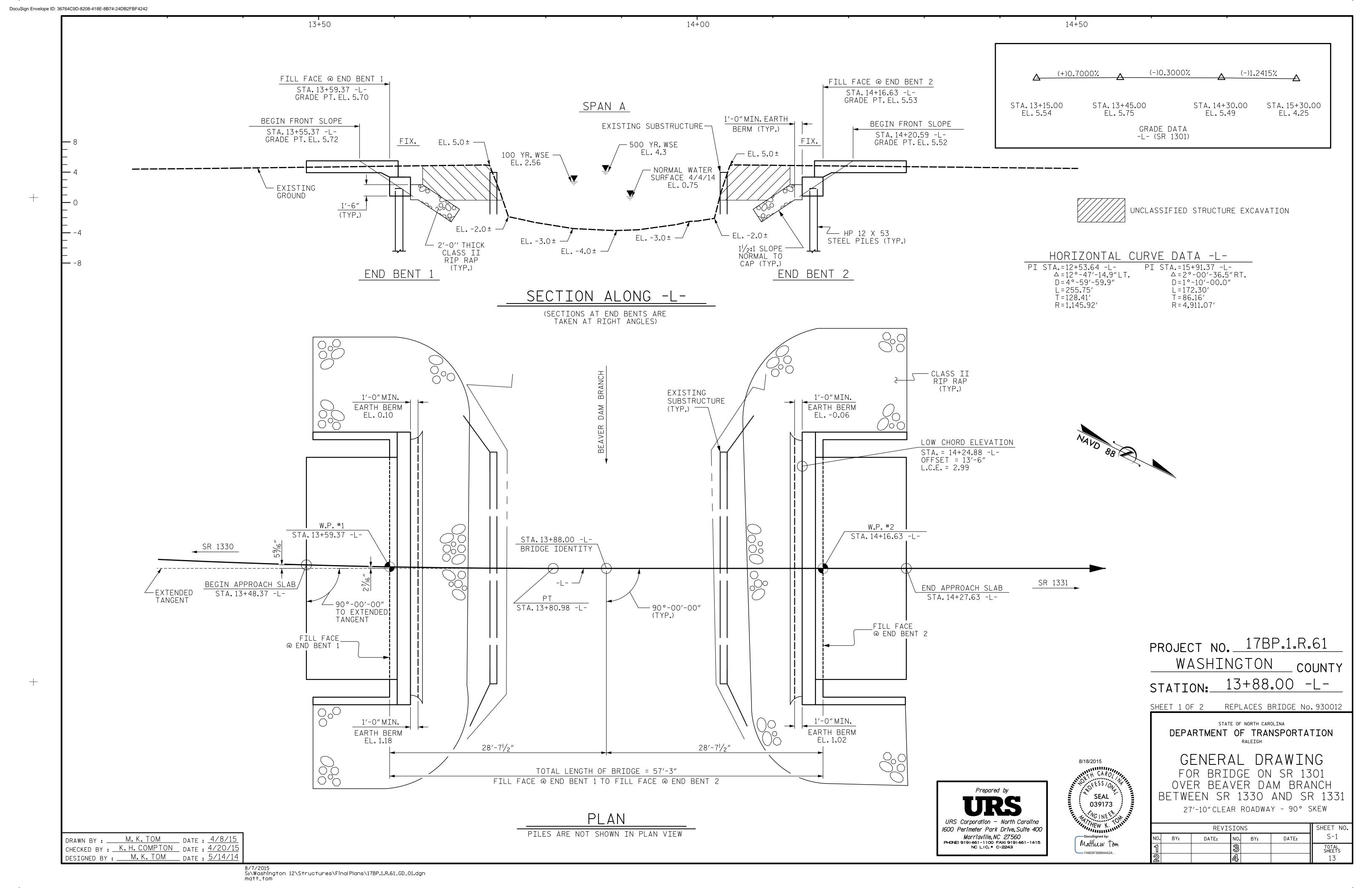
EROSION CONTROL PLAN
BRIDGE #930012

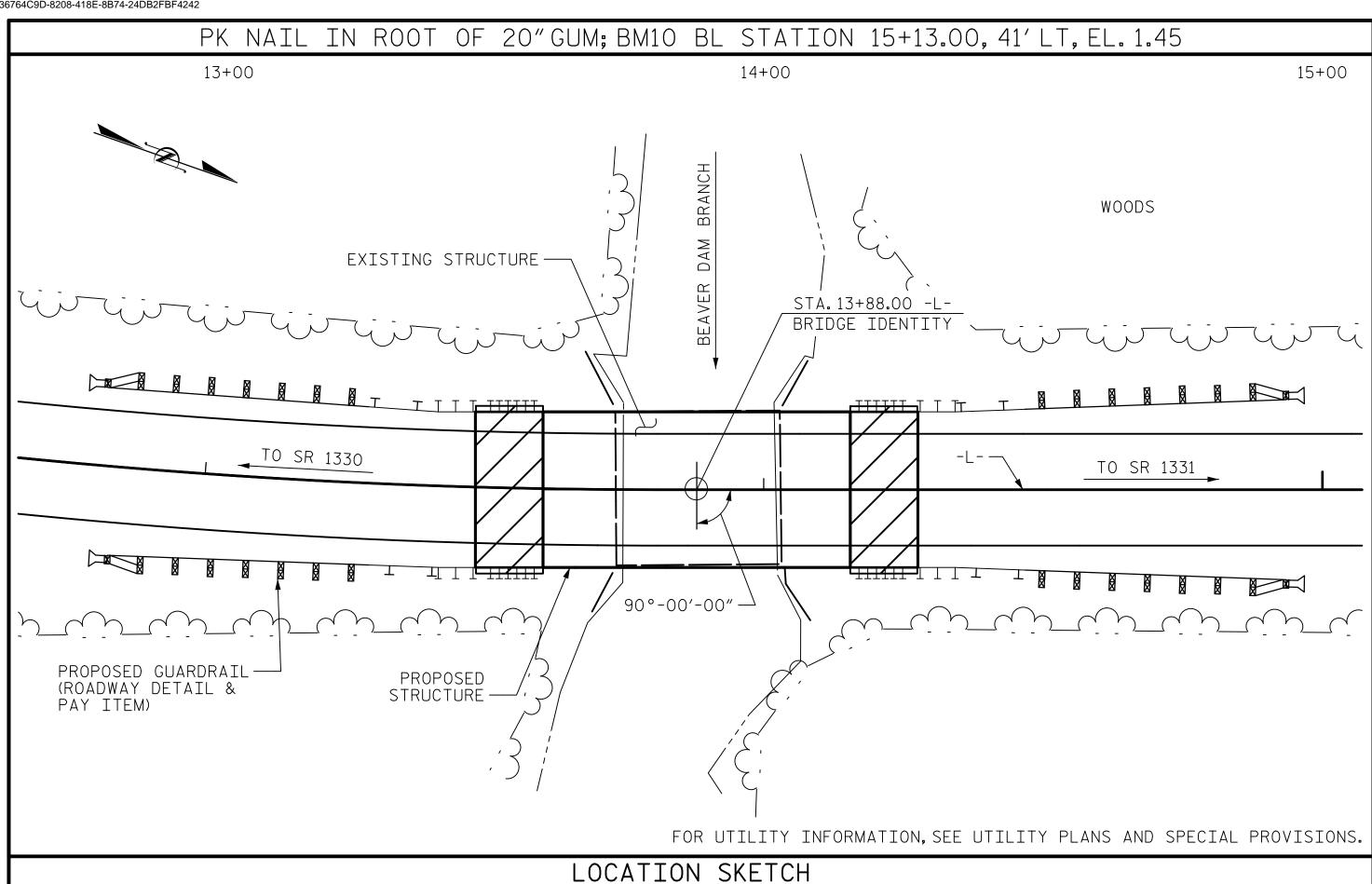
OVER BEAVER DAM BRANCH

| 27'-10" CLEAR ROADWAY - 90° SKEW | SHEET NO | EC-4 | TOTAL | SHEETS

DRAWN BY: _____ DATE: ____ CHECKED BY: ____ DATE: ___







NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASTHO LRFD BRIDGE DESIGN SPECIFICATIONS FOR SEISMIC ZONE 1.

THIS STRUCTURE CONTAINS THE NECESSARY CORROSION PROTECTION REQUIRED FOR A CORROSIVE SITE.

FOR UTILITY INFORMATION. SEE UTILITY PLANS AND SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURE, SEE SPECIAL PROVISIONS.

MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCATED FOR A DISTANCE OF 18 FT EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICAITONS.

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE COLUMNS, BENT CAPS, PILE CAPS, AND FOOTINGS, AND SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICAITONS.

ALL BAR SUPPORTS USED IN THE BARRIER RAIL.BENT CAPS.END BENTS CAPS AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICAITONS.

THE CONCRETE IN THE END BENT CAPS SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB OF CEMENT. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF 1 SPAN TOTALING 30'-0", WITH STEEL BEAMS ON TIMBER SUBSTRUCTURE, AND 28'-0' CLEAR ROADWAY TO BE REMOVED). THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECASSARY DURING THE LIFE OF THE PROJECT.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES.

FOR CALCIUM NITRITE CORROSION INHIBITOR. SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENTS No.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT END BENTS No. 1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 145 TONS PER PILE.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 15 TO 25 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENTS No.1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING, FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS (AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION).

HYDRAULIC DATA

DESIGN DISCHARGE=	660	CFS.
FREQUENCY OF DESIGN FLOOD =	25 Y	R.
DESIGN HIGH WATER ELEVATION=	1.7 F	Т.
DRAINAGE AREA =	5.17	SQ.MI.
BASE FLOOD DISCHARGE (Q100)=	1185	CFS.
BASE HIGH WATER ELEVATION =	2.56	FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	= 1750 CFS
FREQUENCY OF OVERTOPPING FLOOD_	= < 500(+) YR.
OVERTOPPING FLOOD ELEVATION	= 4.3 FT.

^{*}OVERTOPPING STA.12+51.00 -L- ELEV.3.9

PROJECT NO. __17BP.1.R.61 WASHINGTON _ COUNTY STATION: 13+88.00 -L-

SHEET 2 OF 2 REPLACES BRIDGE No. 930012

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING FOR BRIDGE ON SR 1301 OVER BEAVER DAM BRANCH BETWEEN SR 1330 AND SR 1331

27'-10" CLEAR ROADWAY - 90° SKEW

		SHEET NO.					
0.	BY:	DATE:	NO.	BY:	DATE:	S-2	
			8			TOTAL SHEETS	
2			4			13	
							,

	TOTAL BILL OF MATERIAL											
	REMOVAL OF EXISTING STRUCTURE AT STA 13+88.00 -L-	UNCLASSIFIED STRUCTURE EXCAVATION		IP 12 X 53 TEEL PILES	RIP RIP CLASS II (2'-0"THICK)	GEOTEXTILE FOR DRAINAGE	CONSTRUCTION OF SUBSTRUCTURE	CONSTRUCTION OF SUPERSTRUCTURE	BRIDGE APPROACH SLAB			
	LUMP SUM	LUMP SUM	No. LIN.FT.		TON	SQ. YDS.	LUMP SUM	LUMP SUM	LUMP SUM			
SUPERSTRUCTURE												
END BENT 1			5	375	68	75						
END BENT 2			5	375	68	75						
TOTAL	LUMP SUM	LUMP SUM	10	750	136	150	LUMP SUM	LUMP SUM	LUMP SUM			

Prepared by URS URS Corporation - North Carolina 1600 Perimeter Park Drive, Suite 400 Morrisville, NC 27560 PHONE(919) 461-1100 FAX(919) 461-1415 NC LIC. # C-2243



M.K. TOM DATE: 4/8/15 DRAWN BY : ___ CHECKED BY: K. H. COMPTON DATE: 4/20/15 DESIGNED BY: M.K. TOM DATE: 5/14/14

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

								STRENGTH I LIMIT STATE						SE	SERVICE III LIMIT STATE									
									MOMENT					SHEAR				MOMENT						
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.055		1.75	0.275	1.23	55′	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55′	EL	27	
DESIGN		HL-93(0pr)	N/A		1.591		1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0 . 523	1.47	55′	EL	5.4	0.80	0 . 275	1.32	55′	EL	27	
		HS-20(0pr)	36.000		1.9	68.396	1.35	0.275	1.99	55′	EL	27	0 . 523	1.9	55′	EL	5.4	N/A						<u> </u>
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55′	EL	27	0.523	4.17	55′	EL	5.4	0.80	0.275	2.78	55′	EL	27	<u> </u>
		SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL	27	0 . 523	3.02	55′	EL	5.4	0.80	0 . 275	2.15	55′	EL	27	<u> </u>
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3 . 03	55′	EL	27	0 . 523	2.83	55′	EL	5.4	0.80	0 . 275	2.08	55′	EL	27	
	 	SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55′	EL	27	0 . 523	2.09	55′	EL	5.4	0.80	0 . 275	1.38	55′	EL	27	
	S	SNAGGRS4	34.925		1.189	41.527	1.4	0.275	1.73	55′	EL	27	0.523	1.77	55′	EL	5.4	0.80	0.275	1.19	55′	EL	27	
		SNS5A	35 . 550		1.16	41.255	1.4	0.275	1.69	55′	EL	27	0.523	1.82	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55′	EL	27	0.523	1.68	55′	EL	5.4	0.80	0.275	1.08	55′	EL	27	
LEGAL		SNS7B	42.000		1.028	43.175	1.4	0.275	1 . 5	55′	EL	27	0 . 523	1.67	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	
LOAD RATING		TNAGRIT3	33.000		1.32	43 . 556	1.4	0.275	1.92	55′	EL	27	0.523	1.98	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27	
NATINO		TNT4A	33.075		1.33	43.979	1.4	0.275	1.94	55′	EL	27	0.523	1.91	55′	EL	5.4	0.80	0.275	1.33	55′	EL	27	
		TNT6A	41.600		1.101	45 . 811	1.4	0.275	1.6	55′	EL	27	0.523	1.83	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
	ST	TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27	
	=	TNT7B	42.000		1.163	48.848	1.4	0.275	1.69	55′	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
		TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55′	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1 . 5	55′	EL	27	0.523	1 . 58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	
		TNAGT5B	45.000	3	1.013	45 . 582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27	

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

1.

2.

J.

4.

(#) CONTROLLING LOAD RATING

- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
- ** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

1 2 3

LRFR SUMMARY

FOR SPAN A

SEAL 22005

NGINEER HERITAGE THE PASCHERING THE PAS

DocuSigned by:

A. Keith Paschal
F8B6AD6DB2FC48F...

4/2/2015

PROJECT NO. 17BP.1.R.61

WASHINGTON COUNTY

STATION: 13+88.00 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

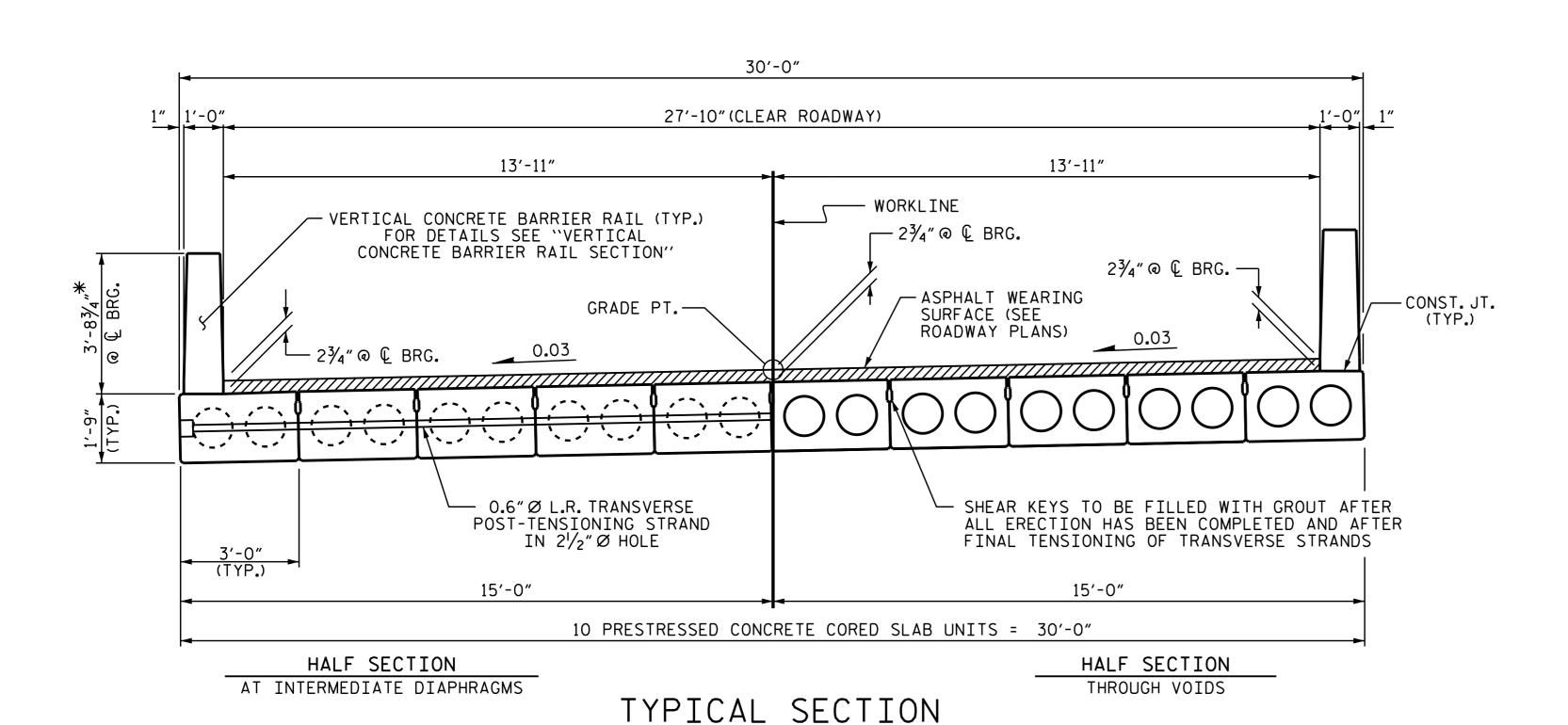
LRFR SUMMARY FOR
55' CORED SLAB UNIT
90° SKEW

(NON-INTERSTATE TRAFFIC)

REVISIONS						
BY:	DATE:	NO.	BY:	DATE:	5	
		3			T S	

ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15 CHECKED BY: P. N. HOLDER DATE: 3-6-15

DRAWN BY: CVC 6/IO CHECKED BY: DNS 6/IO



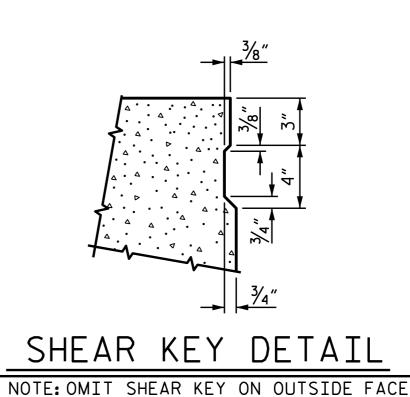
*-THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL

FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

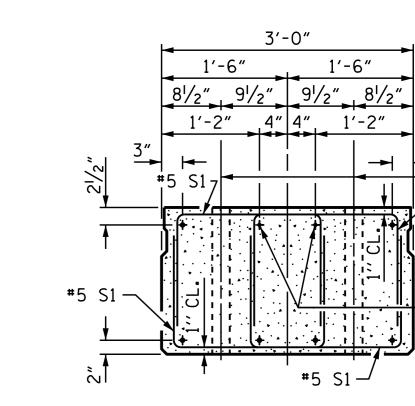
0.6" Ø LOW RELAXATION STRAND LAYOUT

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND



OF EXTERIOR CORED SLABS.



3%" CL.

END ELEVATION

3'-0''

1'-4''

-**#**5 S3

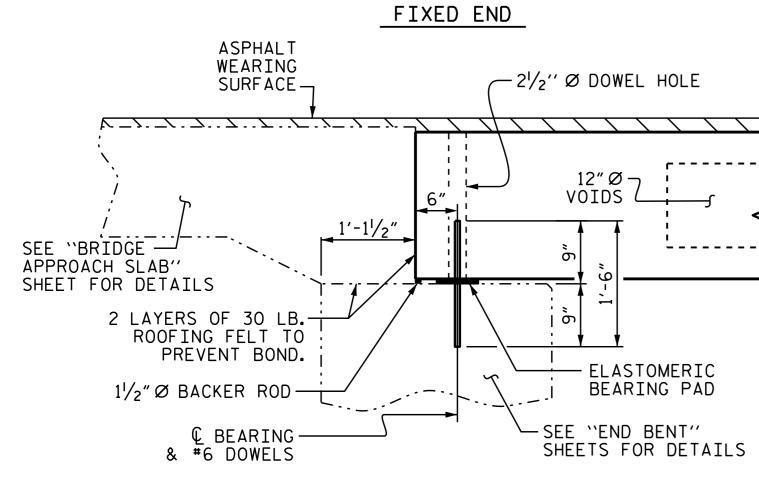
12" Ø VOIDS→

EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

10′′

SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



SECTION AT END BENT

ELEVATION VIEW

Q 0.6" Ø L.R. TRANSVERSE
POST-TENSIONING STRAND
SHEATHED WITH A
NON-CORROSIVE PIPE.

STRAND VISE

OUTSIDE FACE
OF EXTERIOR
CORED SLAB

OF CORED SLAB

SECTION B-B

GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

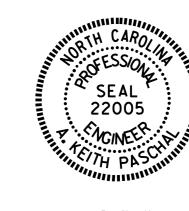
PERMITTED THREADED INSERT

CAST IN OUTSIDE FACE OF

EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY

THREADED INSERT DETAIL

CONTRACTOR. -



DocuSigned by:

A. Keith Parchal
F8B6AD6DB2FC48F...

4/2/2015

PROJECT NO. 17BP.1.R.61

WASHINGTON COUNTY

STATION: 13+88.00 -L-

C 21/2" Ø DOWEL HOLES

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

3'-0'' X 1'-9''

PRESTRESSED CONCRETE

CORED SLAB UNIT

90° SKEW

REVISIONS

Y: DATE: NO. BY: DATE: S-4

TOTAL SHEETS

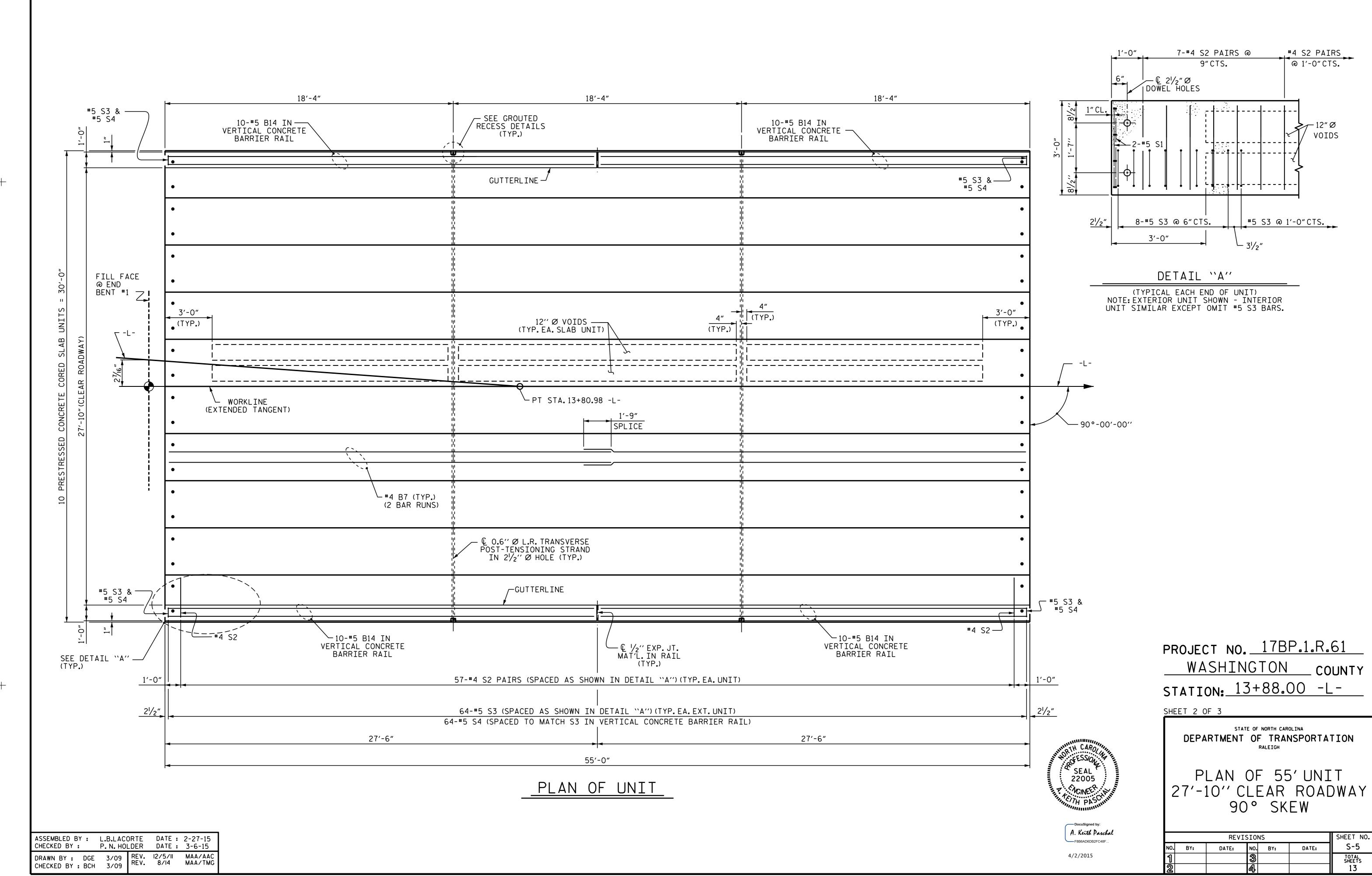
13

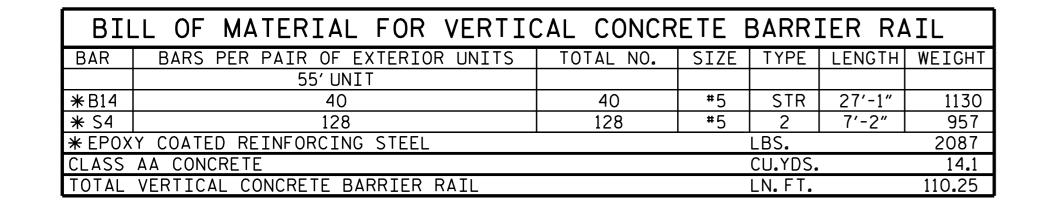
ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15
CHECKED BY: P.N. HOLDER DATE: 3-6-15

DRAWN BY: DGE 5/09
CHECKED BY: BCH 6/09

REV. 8/14

MAA/TMG

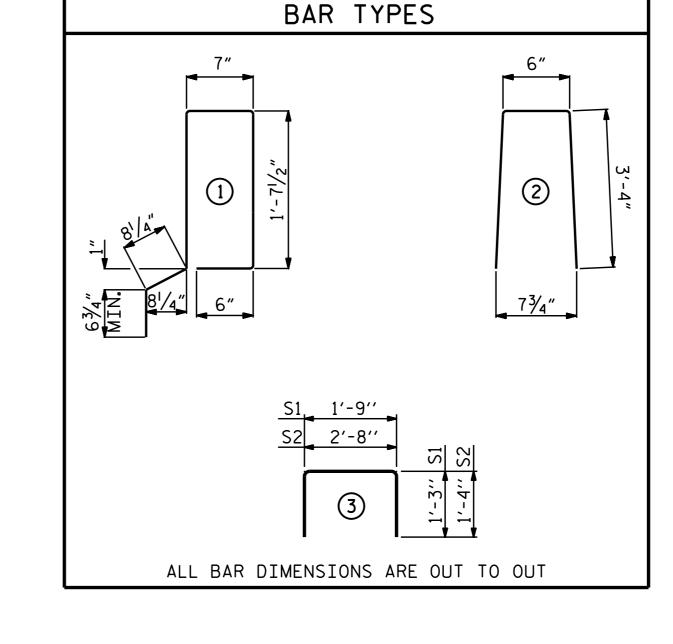




GUTTERLINE ASPI	HALT THICKNESS & R	RAIL HEIGHT
	ASPHALT OVERLAY THICKNES	S RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
55' UNIT	25/8″	3′-85⁄8″

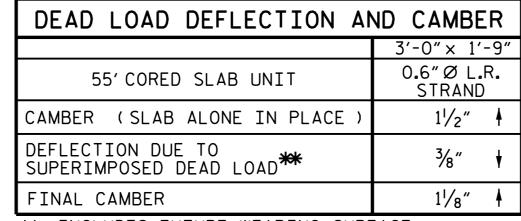
CORED SLABS REQUIRED							
	NUMBER	LENGTH	TOTAL LENGTH				
55' UNIT							
EXTERIOR C.S.	2	55'-0"	110'-0"				
INTERIOR C.S.	8	55'-0"	440'-0"				
TOTAL			550′-0″				

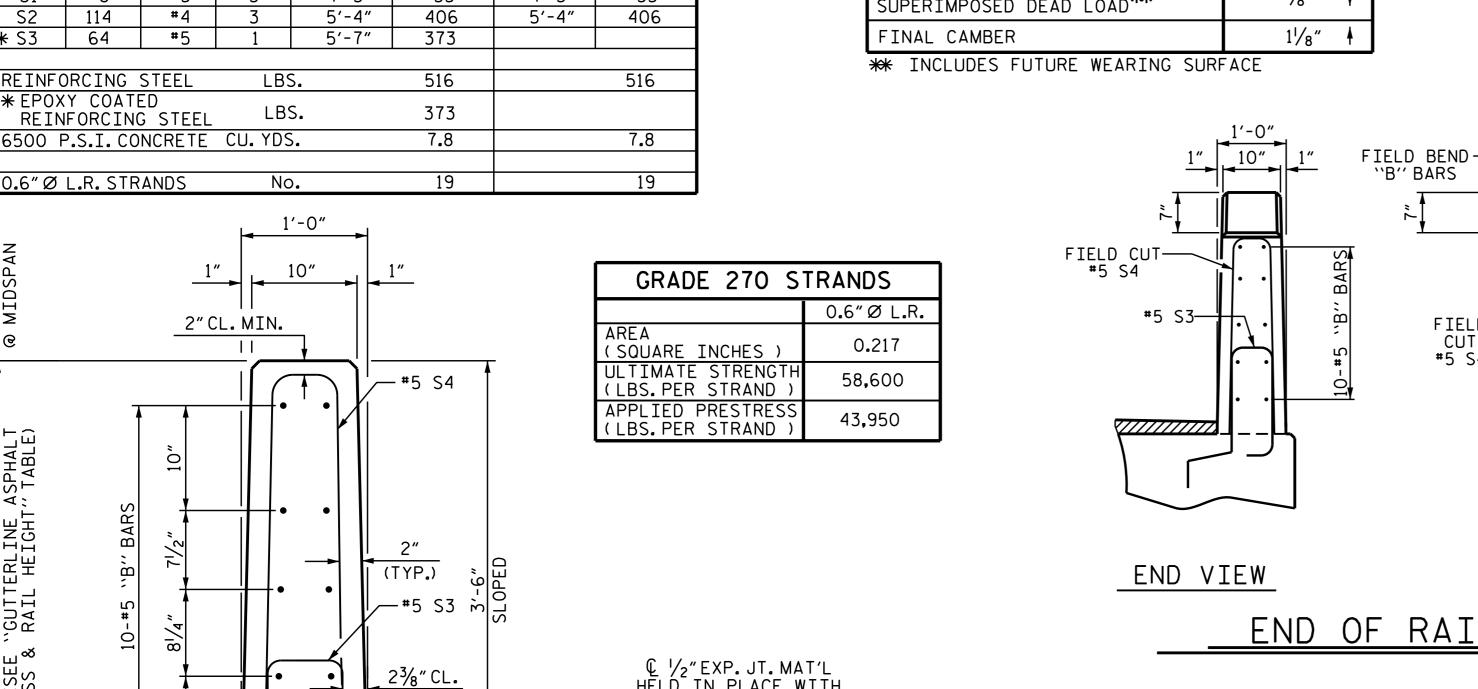
CONCRETE R	LEASE STRENGTH
UNIT	PSI
55' UNIT	4900

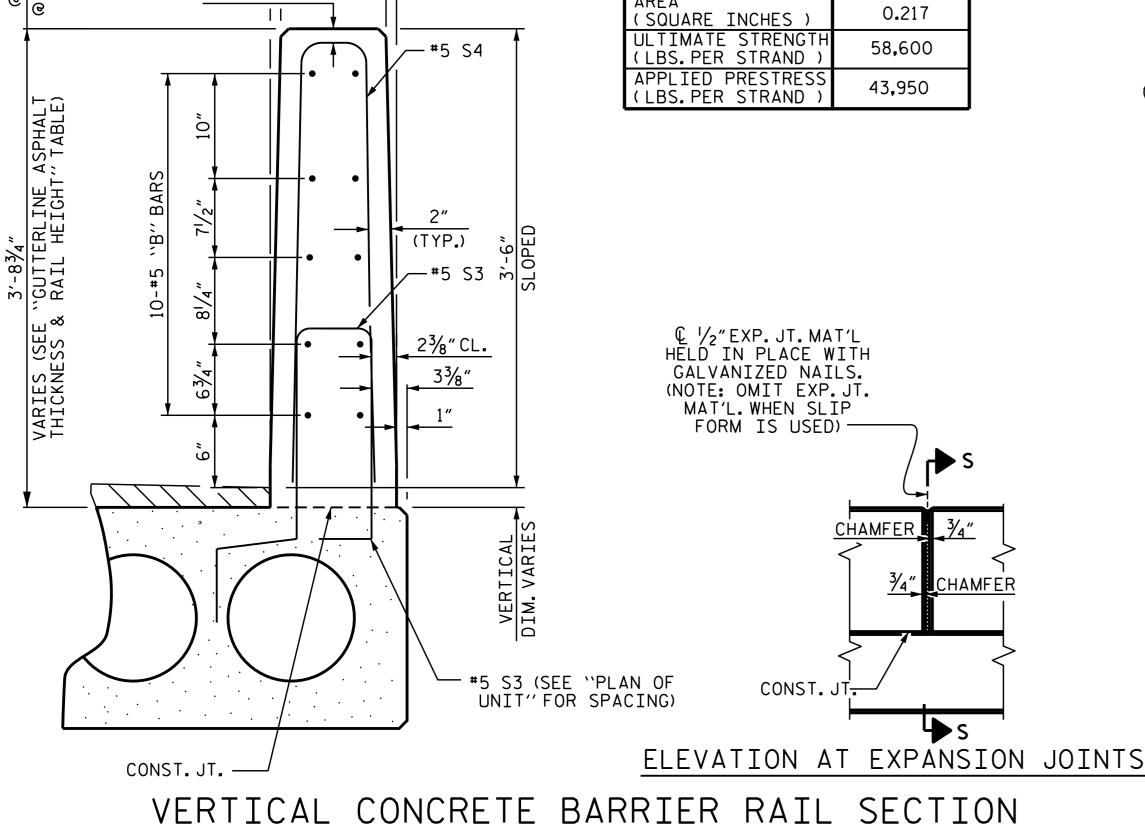


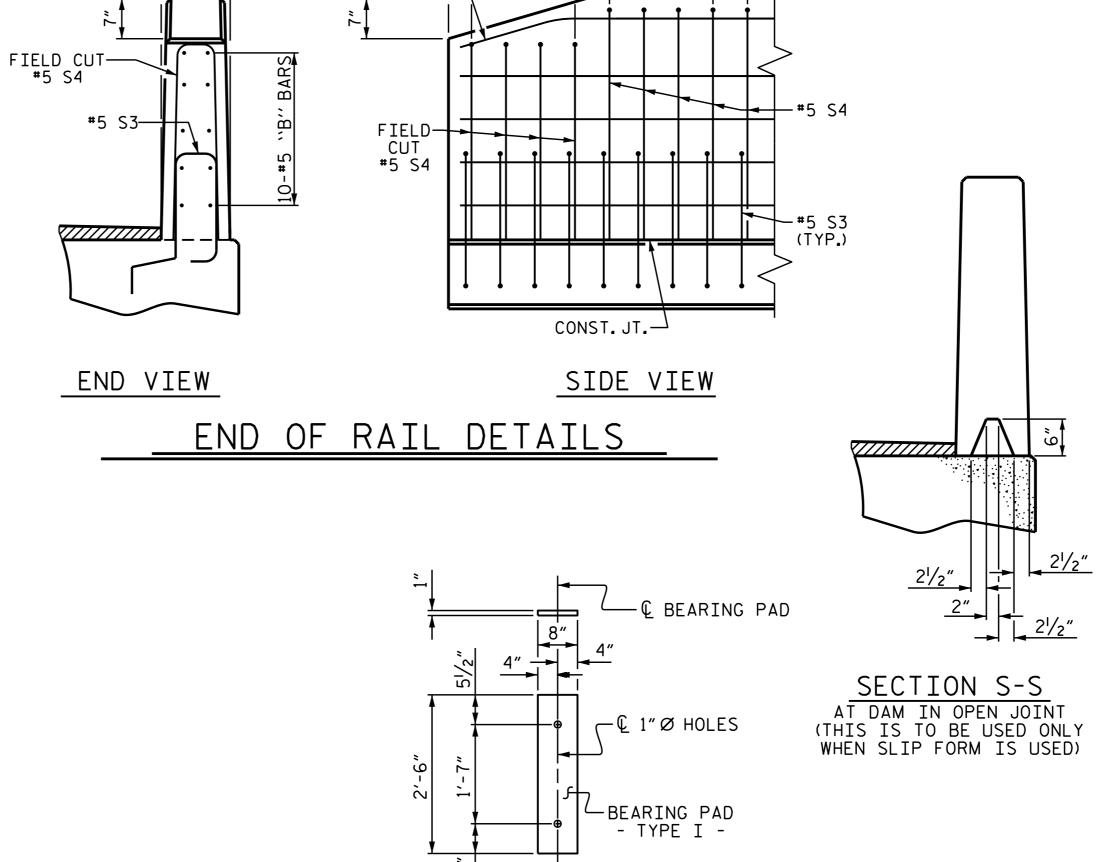
#5 S3 & S4

BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT								
				EXTERI	OR UNIT	INTERI	OR UNIT	
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
В7	4	#4	STR	28'-3"	75	28'-3"	75	
S1	8	# 5	3	4'-3"	35	4'-3"	35	
S2	114	#4	3	5′-4″	406	5′-4″	406	
* S3	64	# 5	1	5′-7"	373			
REINFO	REINFORCING STEEL LBS. 516 516						516	
* EPOXY COATED REINFORCING STEEL LBS. 373								
6500 P.S.I. CONCRETE CU. YDS. 7.8 7.8						7.8		
0 6" Ø	O 6" Ø L R STRANDS NO. 19 19							









2'-0"

& S4 @

6"CTS.

(FIELD CUT

4-#5 S3 ,6", 4-#5 S3

& S4 @

6"CTS.

ELASTOMERIC BEARING DETAILS

FIXED END

(TYPE I - 20 REQ'D)

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

> PROJECT NO. <u>17BP.1.R.61</u> WASHINGTON COUNTY STATION: 13+88.00 -L-

SHEET 3 OF 3



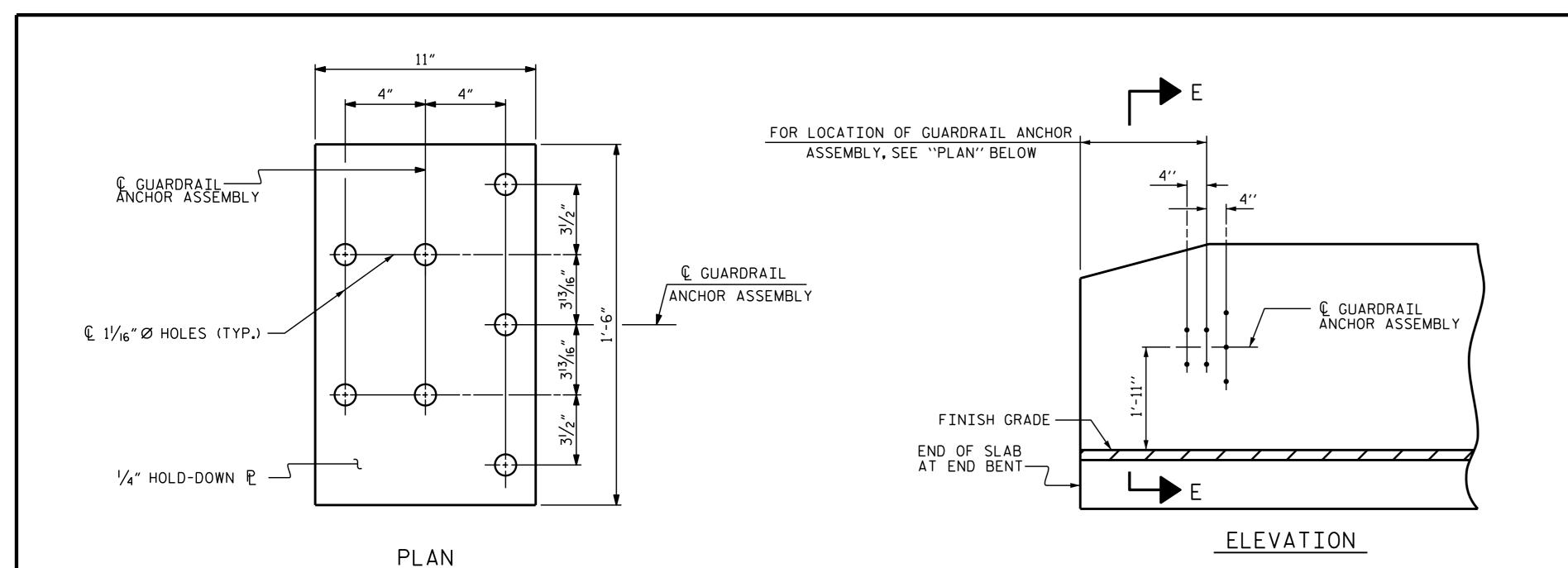
A. Keith Paschal

4/2/2015

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

SHEET NO. REVISIONS S-6 DATE: DATE:

ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15 CHECKED BY: P. N. HOLDER DATE: 3-6-15 DRAWN BY: DGE 5/09 REV. II/I4 MAA/TMG CHECKED BY : BCH 6/09



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

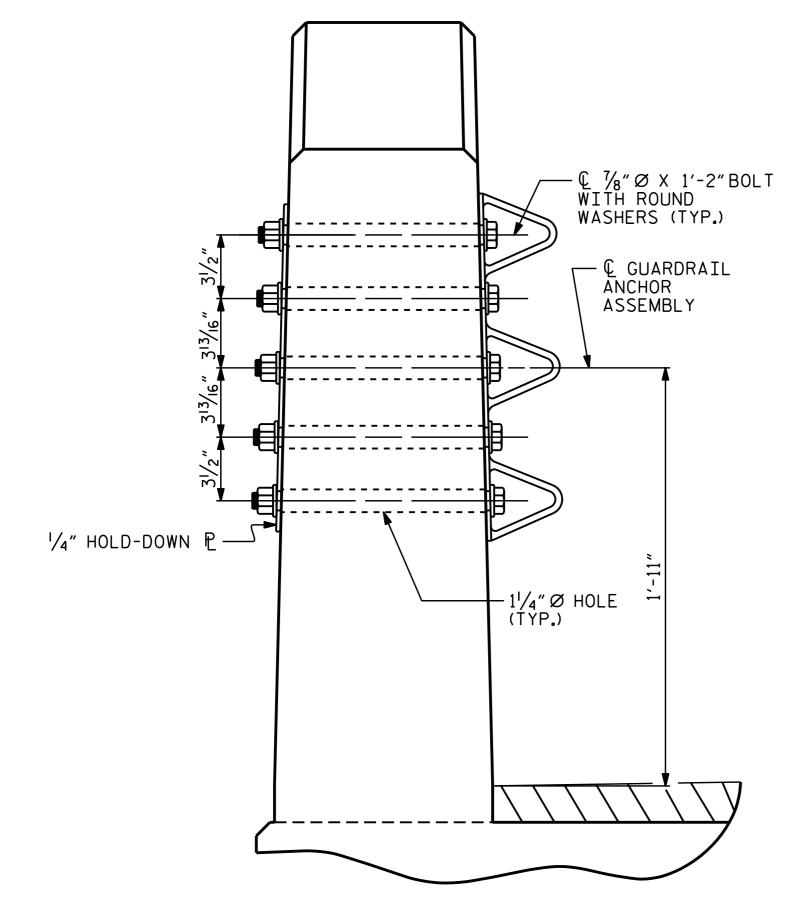
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15 CHECKED BY: P. N. HOLDER DATE: 3-6-15

MAA/GM MAA/GM MAA/TMG

DRAWN BY: MAA 5/10 REV. 12/5/II REV. 6/13 REV. 1/15

END OF SLAB
AT END BENT

1'-10"

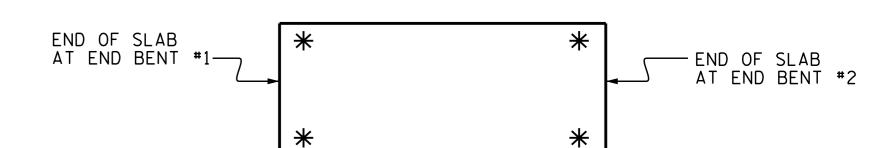
Q GUARDRAIL
ANCHOR ASSEMBLY

4"
ANCHOR ASSEMBLY

PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.1.R.61

WASHINGTON COUNTY

STATION: 13+88.00 -L-



DocuSigned by:

A. Keith Paschal
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4/2/2015

DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE

DETAILS

FOR VERTICAL CONCRETE

BARRIER RAIL

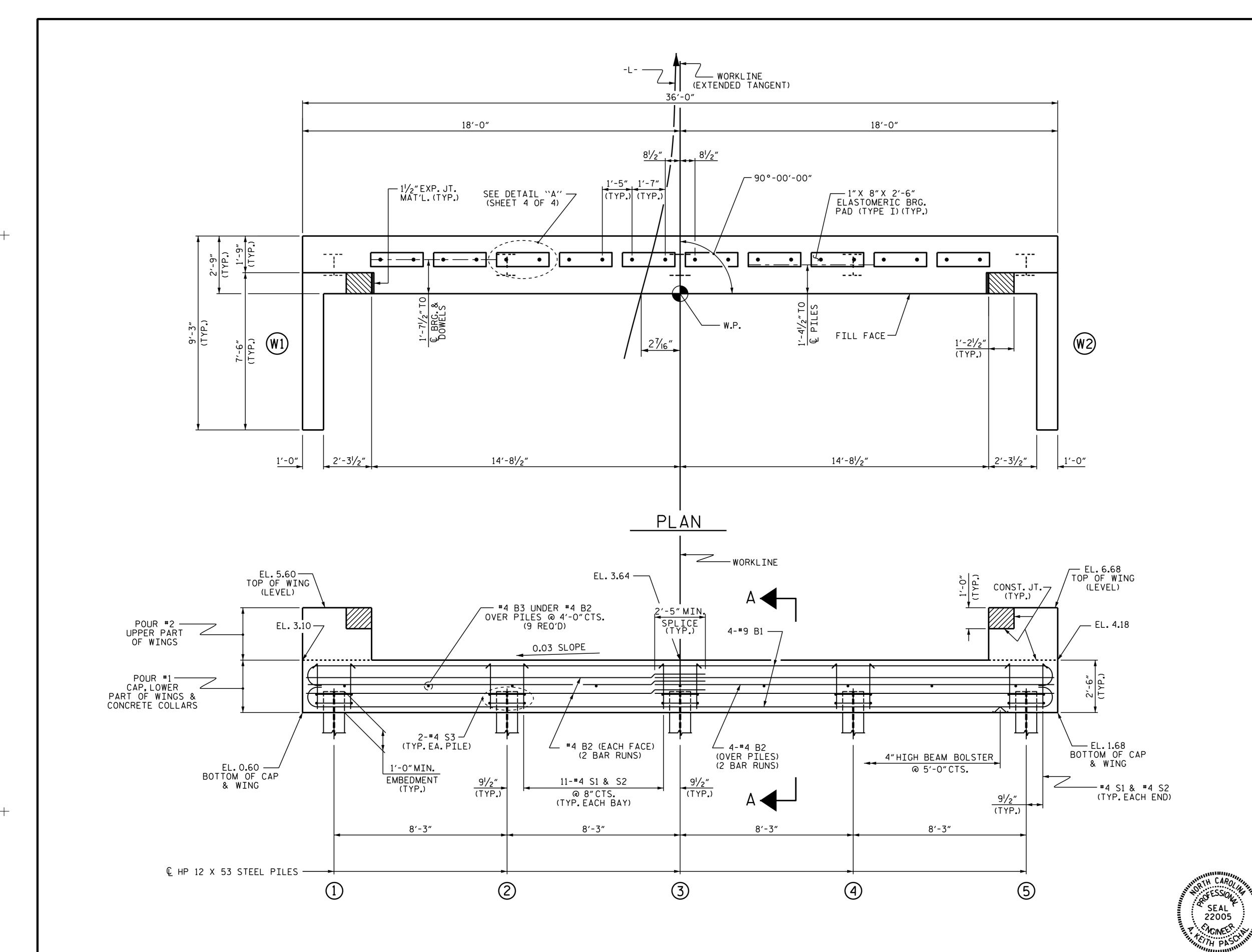
REVISIONS
SHEET NO.

BY: DATE: NO. BY: DATE:

S-7

TOTAL SHEETS
13

02-APR-2015 14:56 S:\DPG1\Division1\17BP.1.R.61\17BP.1.R.61.dgn (SHT 1) STD. NO. GRA3



ELEVATION

WINGS NOT SHOWN FOR CLARITY.
FOR SECTION A-A, SEE SHEET 4 OF 4.
CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.
SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 4 OF 4.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 4"DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

TOP ELE	OF PILE VATIONS
1	1.66′
2	1.91′
3	2.16′
4	2.41′
5	2.66′

PROJECT NO. 17BP.1.R.61

WASHINGTON COUNTY

STATION: 13+88.00 -L-

SHEET 1 OF 4

A. Keith Paschal

4/2/2015

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 1

		REVIS	SION	NS		SHEET NO
NO.	BY:	DATE:	NO.	BY:	DATE:	S-8
1			3			TOTAL SHEETS
୭୮						 13

ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15 CHECKED BY: P. N. HOLDER DATE: 3-6-15

MAA/TMG

DRAWN BY: DGE OI/IO
CHECKED BY: MKT OI/IO
REV. II/I4



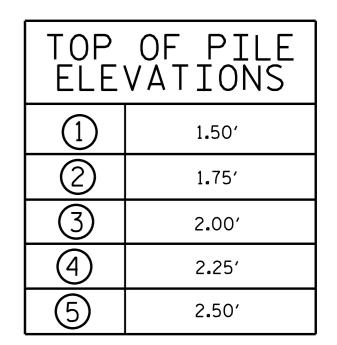
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 4"DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.



PROJECT NO. 17BP.1.R.61 WASHINGTON COUNTY STATION: 13+88.00 -L-

SHEET 2 OF 4

A. Keith Paschal

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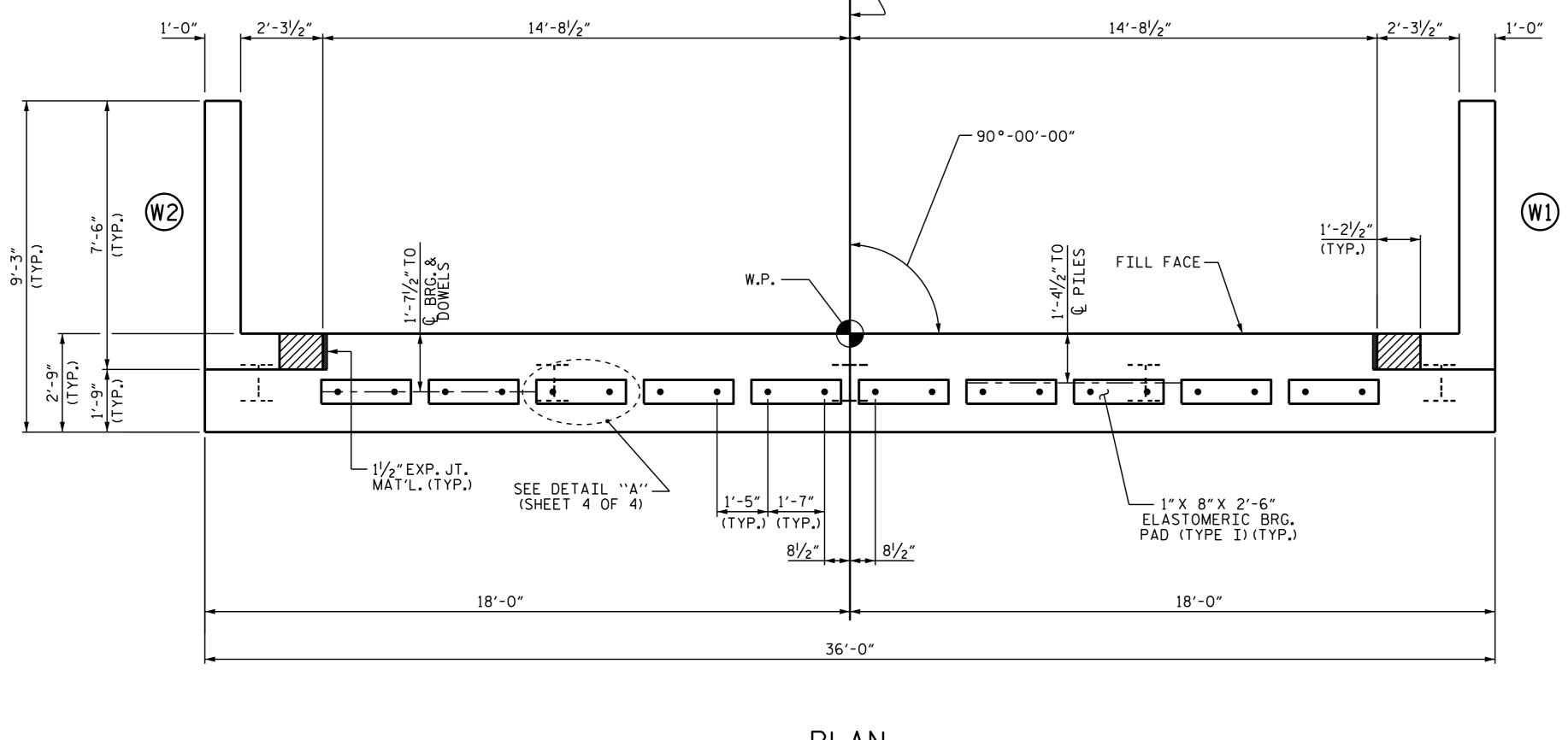
4/2/2015

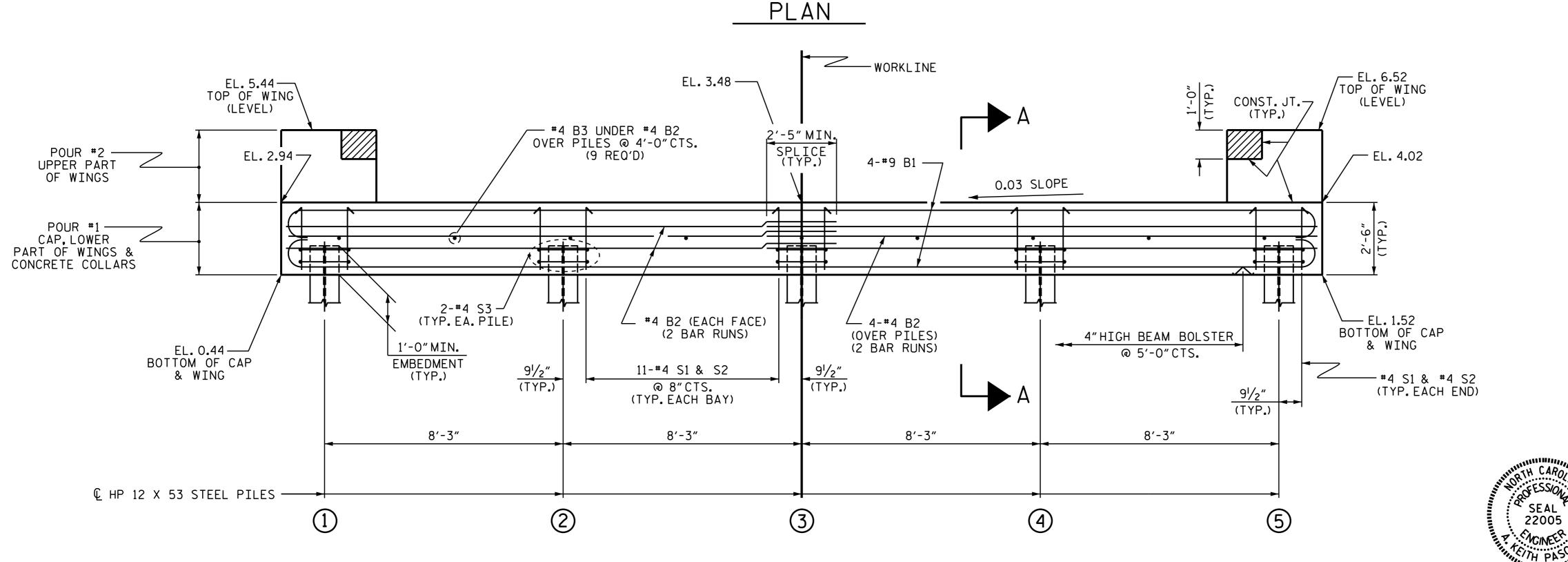
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

END BENT No. 2

	SHEET N					
0.	BY:	DATE:	NO.	BY:	DATE:	S-9
][3			TOTAL SHEETS
2						1 17





ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15 CHECKED BY: P.N. HOLDER DATE: 3-6-15 DRAWN BY: DGE OI/IO
CHECKED BY: MKT OI/IO
REV. II/I4 MAA/TMG ELEVATION

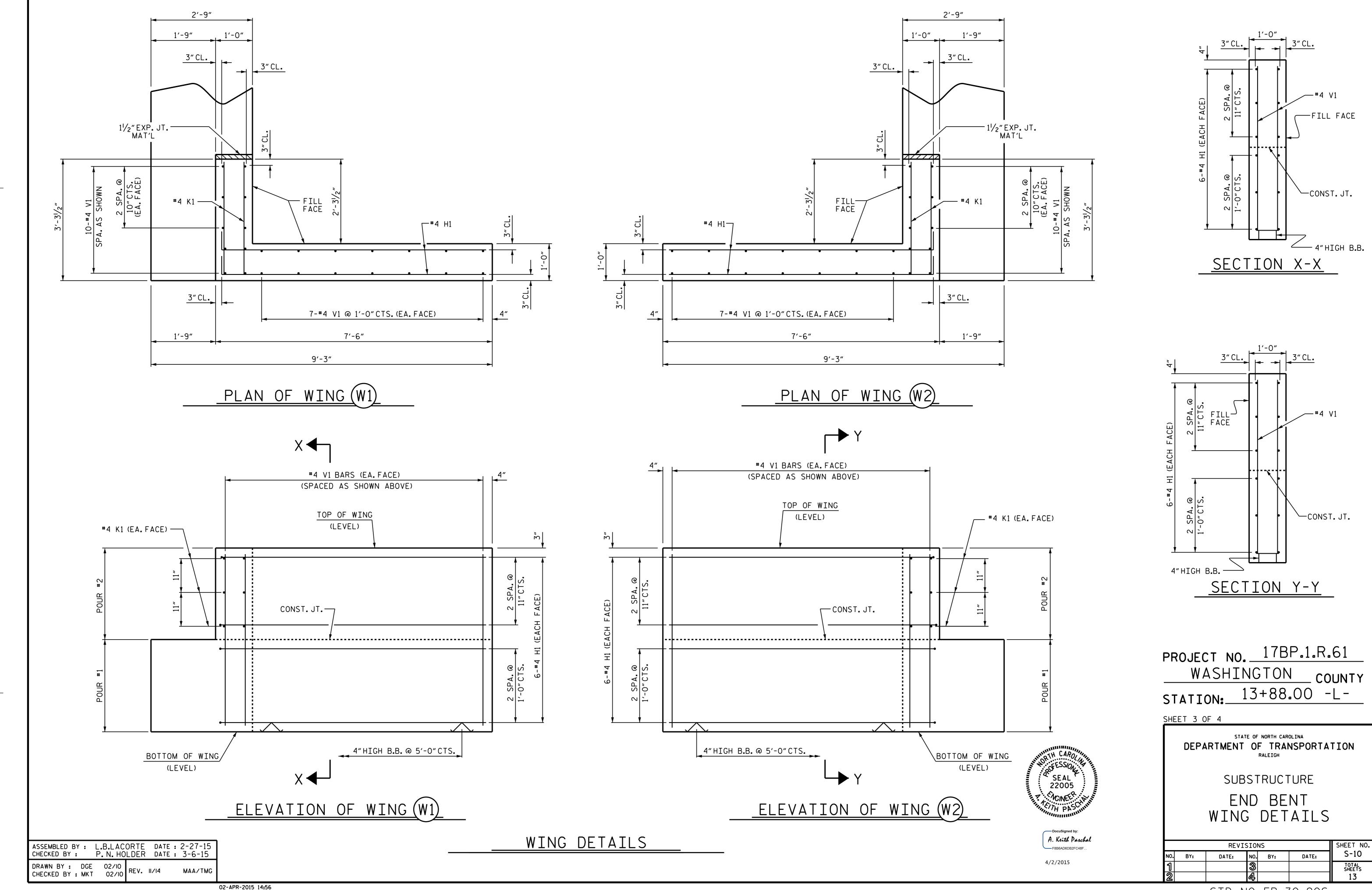
WINGS NOT SHOWN FOR CLARITY.

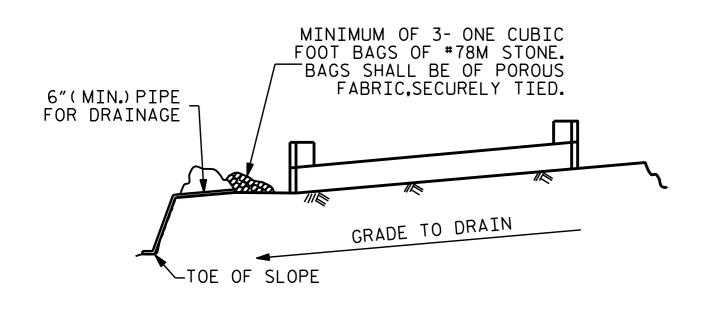
FOR SECTION A-A, SEE SHEET 4 OF 4.

CONCRETE COLLARS FOR STEEL PILES NOT SHOWN IN PLAN AND ELEVATION VIEWS FOR CLARITY.

SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL", SHEET 4 OF 4.

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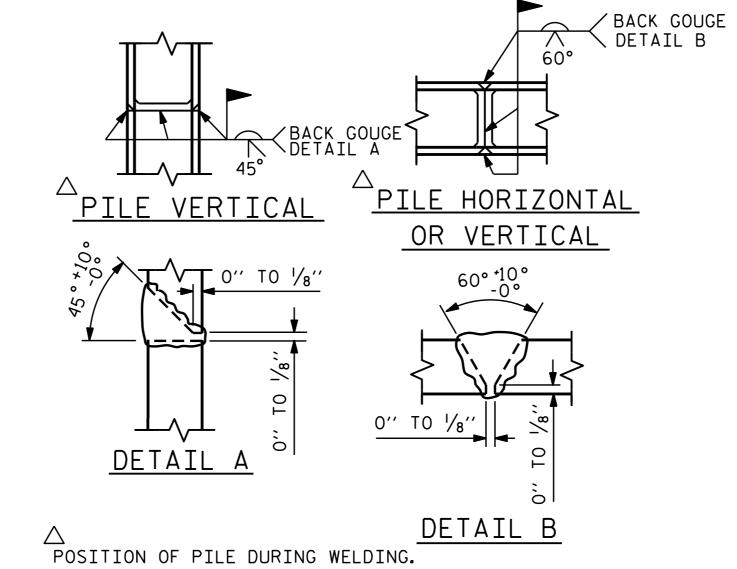


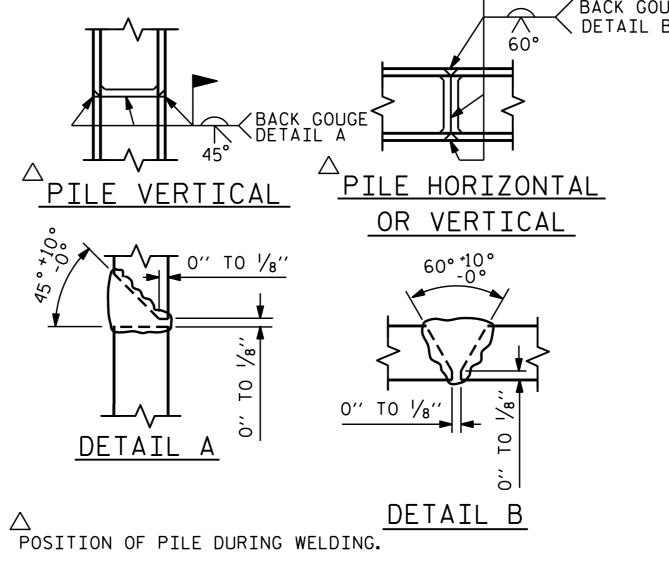
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

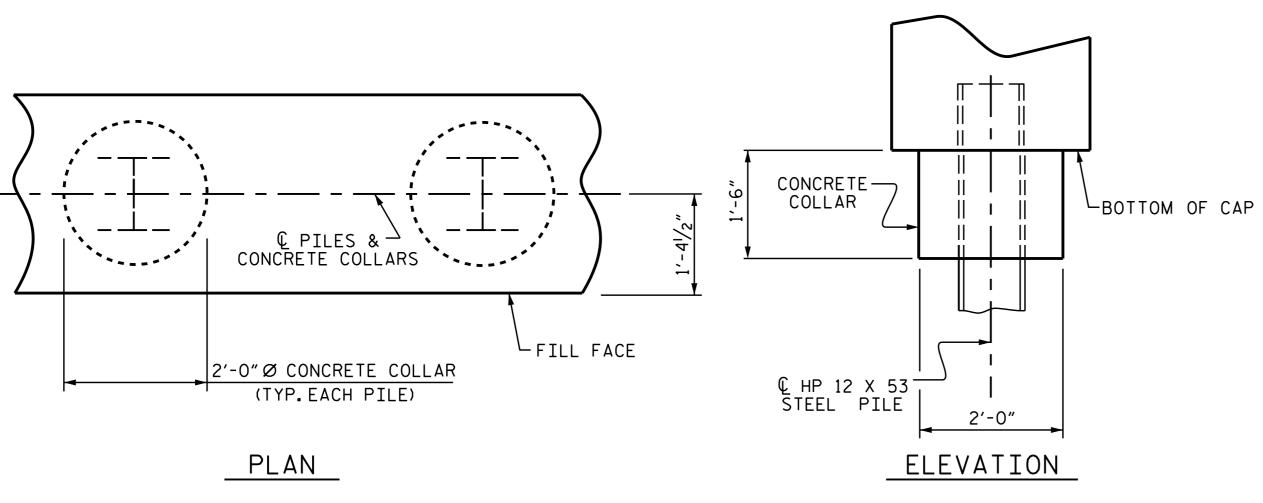
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



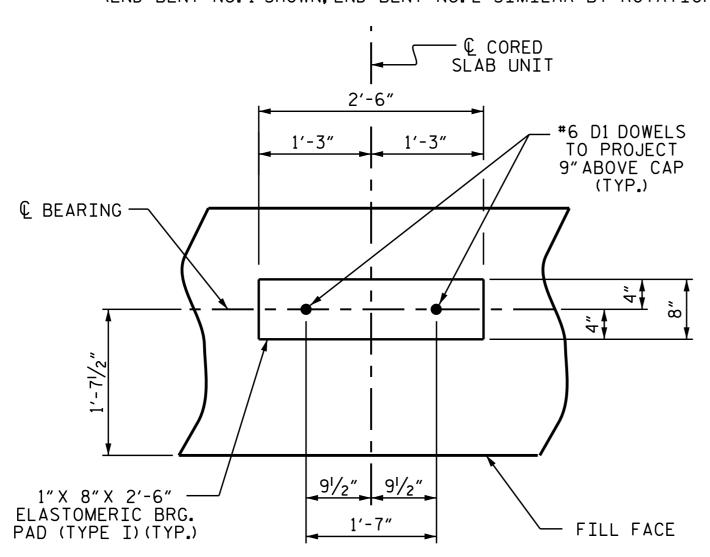


PILE SPLICE DETAILS



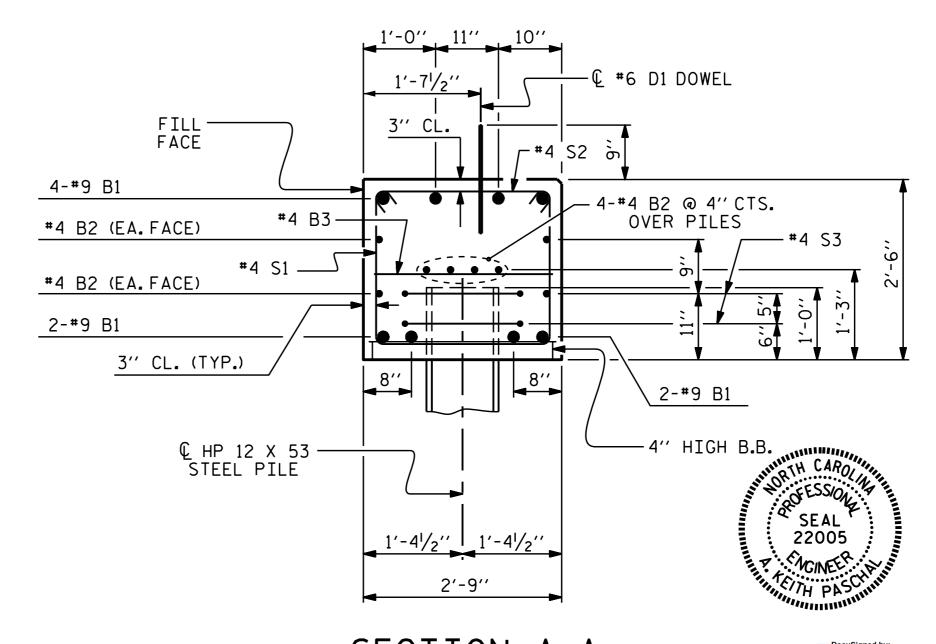
CORROSION PROTECTION FOR STEEL PILES DETAIL

(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)



DETAIL "A"

(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



BAR TYPES

2'-3"

LIN.FT.= 375'

NO.: 2

END BENT No. 1

HP 12 X 53 STEEL PILES

NO: 5

PILE REDRIVES

SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL." PROJECT NO. 17BP.1.R.61 WASHINGTON ___ COUNTY STATION: 13+88.00 -L-

BILL OF MATERIAL

FOR ONE END BENT

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

7′-8″

6'-11"

3'-0"

6'-6"

16 #4 STR 18'-11"

9 #4 STR 2'-3"

|*D1 | 20 | #6 | STR | 1'-6"

*K1 12 #4 STR 2'-9"

46 #4 4

* EPOXY COATED REINFORCING

STEEL (FOR ONE END BENT)

(FOR ONE END BENT)

POUR #1 CAP, LOWER PART

POUR #2 UPPER PART OF

LIN. FT.= 375' TOTAL CLASS AA CONCRETE

NO.: 2

WINGS

CLASS AA CONCRETE BREAKDOWN

OF WINGS & COLLARS

*H1 | 24 | #4 | 2

*S1 46 #4 3

*****S3 | 10 | *****4 | 5

*****B3

*****S2

7'-0"

2'-3"

(4)

(5)

1'-8" Ø

END BENT No. 2

HP 12 X 53 STEEL PILES

ALL BAR DIMENSIONS ARE OUT TO OUT.

NO: 5

PILE REDRIVES

A. Keith Paschal

4/2/2015

1030

203

14

45

123

22

213

92

43

150

1935 LBS.

11.2 C.Y.

1.8 C.Y.

13.0 C.Y.

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

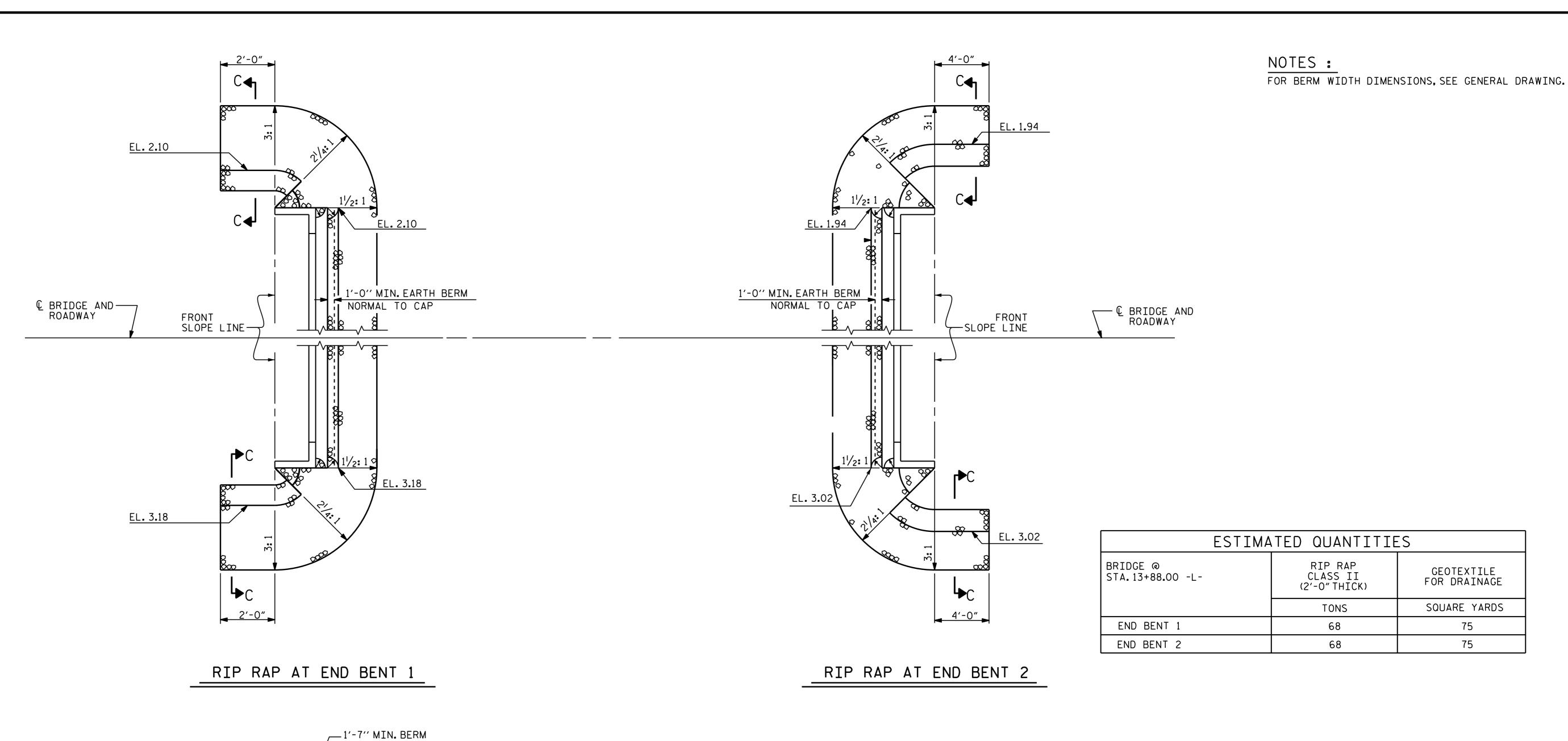
END BENT No.1 & 2 DETAILS

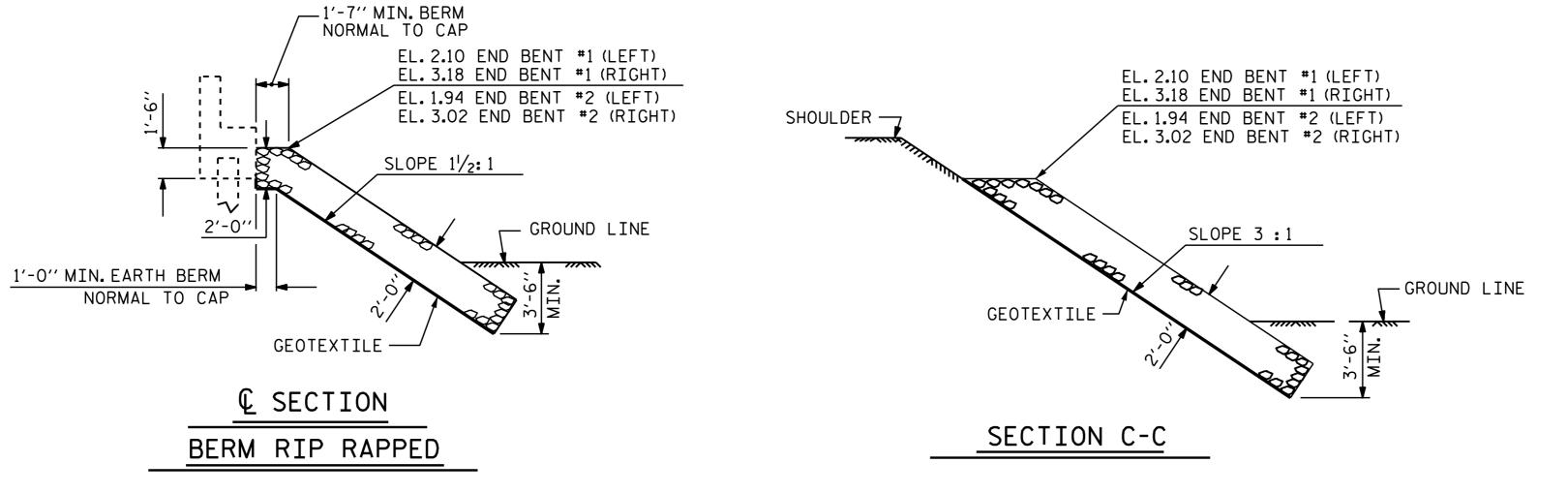
	SHEET NO					
١0.	BY:	DATE:	NO.	BY:	DATE:	S-11
1			3			TOTAL SHEETS
2			4			13

DRAWN BY: DGE 12/09
CHECKED BY: MKT 01/10
REV. 11/14 MAA/TMG

ASSEMBLED BY: L.B.LACORTE DATE: 2-27-15 CHECKED BY: P. N. HOLDER DATE: 3-6-15

02-APR-2015 14:56 S:\DPG1\Division1\17BP.1.R.61\17BP.1.R.61.dgn





PROJECT NO. 17BP.1.R.61

WASHINGTON COUNTY

STATION: 13+88.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

---RIP RAP DETAILS---

	SHEET NO					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			TOTAL SHEETS
2			4			13

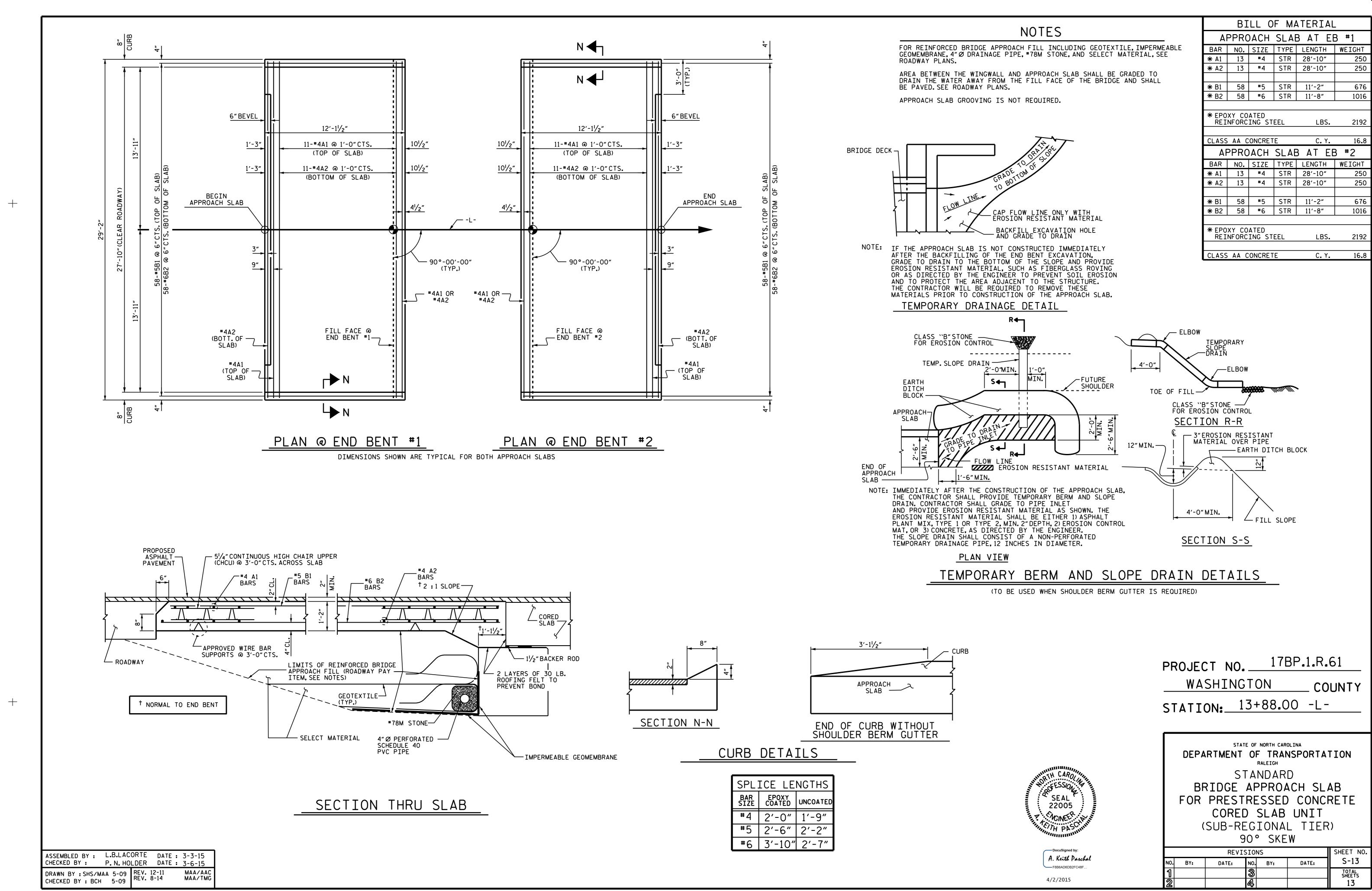
ASSEMBLED BY: L.B. LACORTE DATE: 3-4-15 CHECKED BY: P. N. HOLDER DATE: 3-6-15

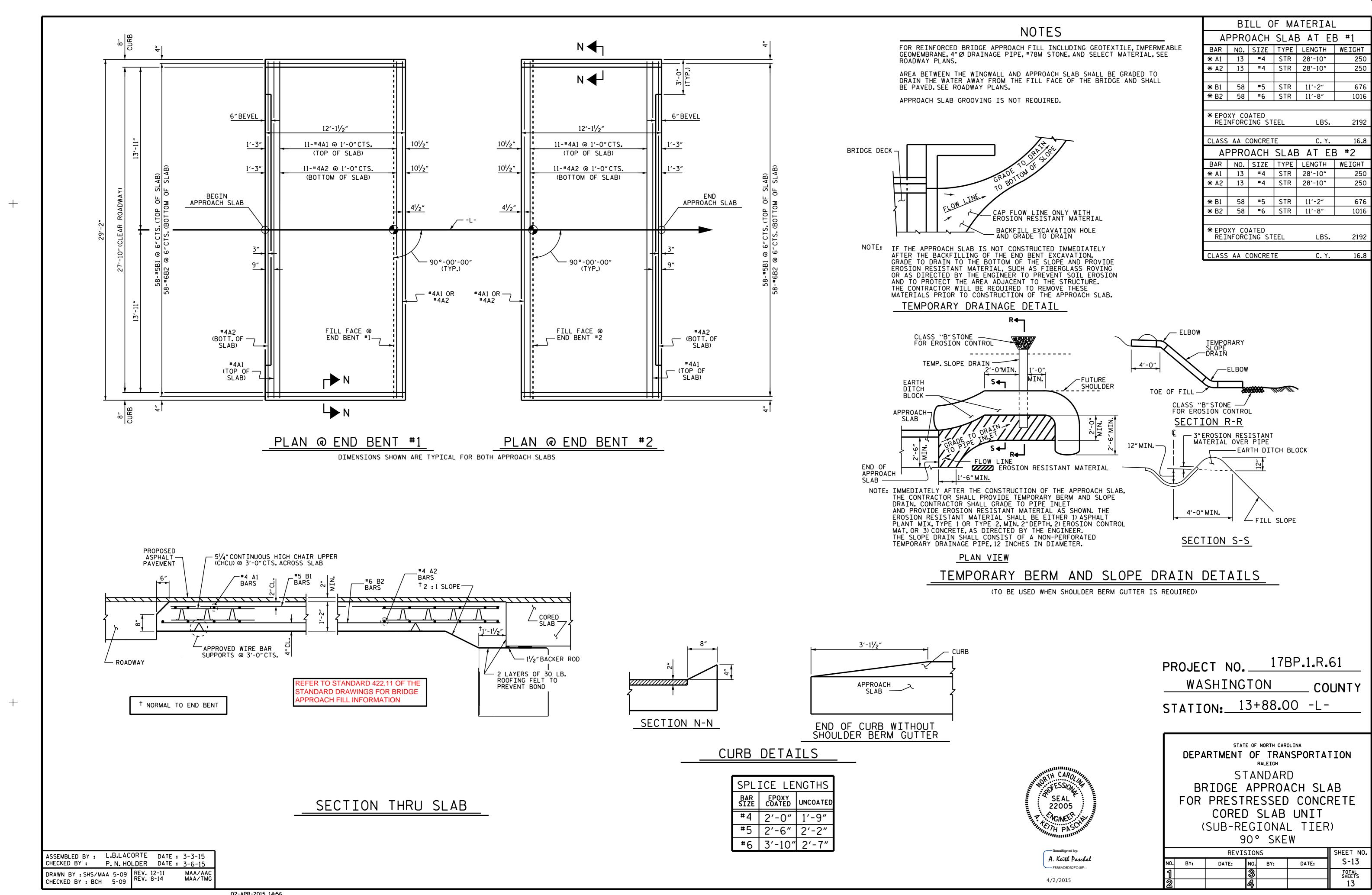
DRAWN BY: REK I/84 REV. 5/I/06R REV. 10/I/II MAA/GM REV. 12/21/II MAA/GM

A. Keith Paschal F8B6AD6DB2FC48F... 4/2/2015

SEAL 22005

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STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS.PER SQ.IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER ----

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH - - - - -

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.
ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH