

LOCATION SKETCH

		TOTA	AL BIL	L	OF I	MATER	RIAL			
	REMOVAL OF EXISTING STRUCTURE	UNCLASSIFIED STRUCTURE EXCAVATION	BRIDGE APPROACH SLABS	GAL	12 X 53 VANIZED EL PILES	PILE REDRIVES	RIP RAP CLASS I (1'-6" THICK)	GEOTEXTILE FOR DRAINAGE	CONSTRUCTION OF SUBSTRUCTURE	CONSTRUCTION OF SUPERSTRUCTURE
	LUMP SUM	LUMP SUM	LUMP SUM	NO.	LIN.FT.	EACH	TONS	SQ. YARDS	LUMP SUM	LUMP SUM
SUPERSTRUCTURE	LUMP SUM		LUMP SUM						LUMP SUM	LUMP SUM
END BENT 1		LUMP SUM		7	420		140	152		
END BENT 2		LUMP SUM		7	420		120	133		
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	14	840	1	260	285	LUMP SUM	LUMP SUM

HYDROGRAPHIC DATA:

DESIGN DISCHARGE -FREQUENCY OF DESIGN FLOOD -DESIGN HIGH WATER ELEVATION -DRAINAGE AREA -BASIC DISCHARGE (Q 100) -BASIC HIGH WATER ELEVATION -

OVERTOPPING FLOOD DATA:

OVERTOPPING DISCHARGE -FREQUENCY OF OVERTOPPING FLOOD -OVERTOPPING FLOOD ELEVATION -

600 CFS

25 YEAR

4.1 SQ. MI.

1093 CFS

>100 YEAR

900 CFS

3.48

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SESMIC ZONE 1.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR THE DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN ON SHEET 1 IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 25 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

THIS STRUCTURE CONTAINS THE NECESSARY CORROSION PROTECTION FOR A CORROSIVE SITE.

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE END BENT CAPS AND SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR.

ALL BAR SUPPORTS USED IN THE BARRIER RAIL AND END BENT CAPS AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR PILES. SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE TOP 20 FEET OF EACH END BENT PILE IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO. 1 ARE DESIGNED FOR FACTORED RESISTANCE OF 74 TONS PER PILE. DRIVE PILES AT END BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 124 TONS PER PILE.

INSTALL PILES AT END BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN -35 FEET.

PILES AT END BENT NO. 2 ARE DESIGNED FOR FACTORED RESISTANCE OF 74 TONS PER PILE.

DRIVE PILES AT END BENT NO. 2 TO A REQUIRED DRIVING RESISTANCE OF 124 TONS PER PILE. INSTALL PILES AT END BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN -35 FEET.

ADT = 860 FOR YEAR 2009

ROADWAY APPROACH EMBANKMENT SHALL BE WIDENED AS NECESSARY FOR GUARDRAIL INSTALLATION.

FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

NO DECK DRAINS REQUIRED.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

PLANS PREPARED BY:

+MULKEY

PO BOX 33127 RALEIBH, N.C. 27636 (919) 851-1912 (919) 851-1918 (FAX) WWW.MULKEYINC.COM

NG LICENSE NO. C-1021

17BP.1.R.7 PROJECT NO._

CHOWAN

COUNTY

STATION:

12+10.00 -L-

REPLACES BRIDGE NO. 29

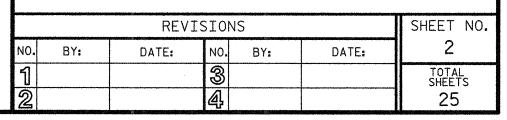
DEPARTMENT OF TRANSPORTATION

LOCATION SKETCH & TOTAL BILL OF MATERIAL

STATE OF NORTH CAROLINA

RALEIGH

30'-10"CLEAR ROADWAY - 90°SKEW



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W.B. ALLEN DATE: 6/12 DRAWN BY : W. A. DAVIS DATE: 7/12 CHECKED BY : ____

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		•								MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	COMMENT NUMBER
		HL-93(Inv)	N/A		1.055		1.75	0.275	1.23	55′	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55'	EL	27	Makanah Ngaga et Shadhad da Rajaka sa
DESIGN	2	HL-93(Opr)	. N/A	***	1.591	400.40	1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N/A		san enge	***	•		
LOAD RATING		HS-20(Inv)	36.000	(2)	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0.523	1.47	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27	
MATINO		HS-20(0pr)	36.000	nem fest	1.9	68.396	1.35	0.275	1.99	55′	EL	27	0.523	1.9	55′	EL	5.4	N/A			40 ASS	#** ***	** ***	
		SNSH	13.500	****	2.776	37.476	1.4	0.275	4.04	55′	EL	27	0.523	4.17	55′	EL	5.4	0.80	0.275	2.78	55′	EL	27	, , , , , , , , , , , , , , , , , , ,
		SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL	27	0.523	3.02	55′	EL	5.4	0.80	0.275	2.15	55′	EL	27	
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3.03	55′	EL	27	0.523	2.83	55′	EL	5.4	0.80	0.275	2.08	55′	EL	27	
		SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55'	EL	27	0.523	2.09	55′	EL	5.4	0.80	0.275	1.38	55′	EL.	27	
) S	SNAGGRS4	34.925	***	1.189	41.527	1.4	0.275	1.73	55′	EL	27	0.523	1.77	55′	EL	5.4	0.80	0.275	1.19	55′	EL	27	
		SNS5A	35.550	•••	1.16	41.255	1.4	0.275	1.69	55′	EL	27	0.523	1.82	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55′	EL	27	0.523	1.68	55′	EL	5.4	0.80	0.275	1.08	55′	EL	27	
LEGAL		SNS7B	42.000	***	1.028	43.175	1.4	0.275	1.5	55′	EL	27	0.523	1.67	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	
LOAD RATING		TNAGRIT3	33.000	***	1.32	43.556	1.4	0.275	1.92	55′	EL	27	0.523	1.98	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27	
117210		TNT4A	33.075	***	1.33	43.979	1.4	0.275	1.94	55′	EL	27	0.523	1.91	55′	EL	5.4	0.80	0.275	1.33	55′	EL	27	
		TNT6A	41.600		1.101	45.811	1.4	0.275	1.6	55'	EL	27	0.523	1.83	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
	121	TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27	ANNERS AND DESCRIPTION OF THE PARTY OF THE P
	j	TNT7B	42,000		1.163	48.848	1.4	0.275	1.69	55'	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
		TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55'	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.5	55′	EL	27	0.523	1.58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	Parallel and control between the control of the con
		TNAGT5B	45.000	(3)	1.013	45.582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27	

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{oc}	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.1.R.7

CHOWAN

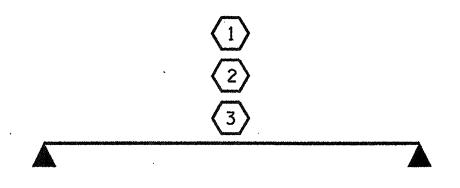
____ COUNTY

STATION: 12+10.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD LRFR SUMMARY FOR 55' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

		REVIS				SHEET NO.
0.	BY:	DATE:	NO.	BY:	DATE:	3.
			3			TOTAL SHEETS
)		-	AL			25



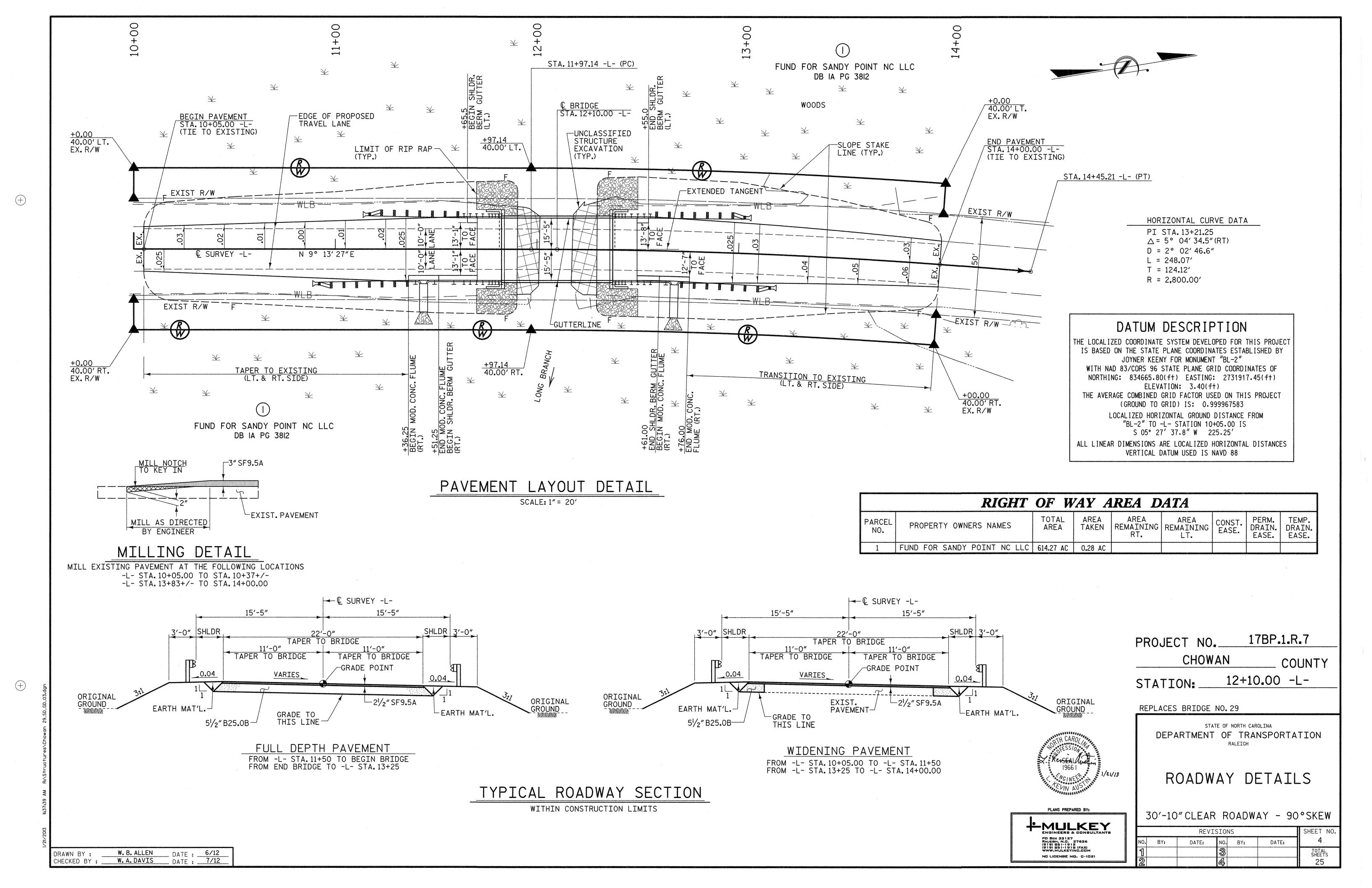
LRFR SUMMARY

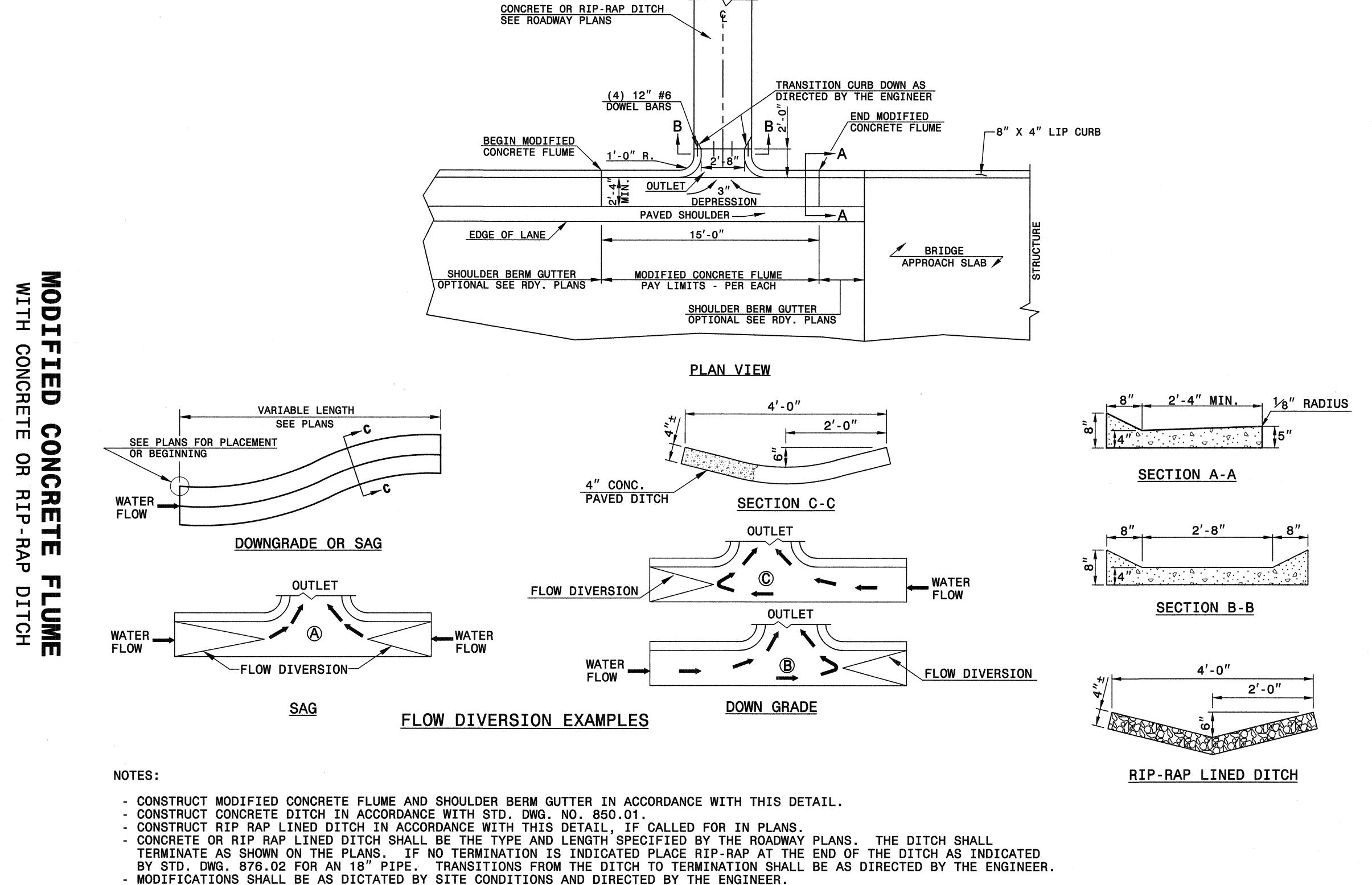
FOR SPAN 'A'

ASSEMBLED BY: N. RUFFIN DATE: 7/27/12 CHECKED BY: M.D. PISO DATE: 8-16-12 DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10

24-0CT-2012 12:08 S:\DPG1\Tim\17BP.I.R.7\nruffin\17BP1R7_sd_CS_01.dgn tcoggins

STD. NO. 21LRFR1_90S_55L





PROJECT NO. 17BP.1.R.7

CHOWAN COUNTY

CHOWAN COUNTY

STATION: 12+10.00 -L
REPLACES BRIDGE NO. 29 SHEET 2 OF 2

REPLACES BRIDGE NO. 29

PLANS PREPARED BY

+MULKEY
ENGINEERS & CONSULTANTS

NC LICENSE NO. C-102

CONCRE

DITCH

AIP

OR O

CONCRETE

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

ROADWAY DETAILS

30'-10"CLEAR ROADWAY - 90°SKEW

REVISIONS

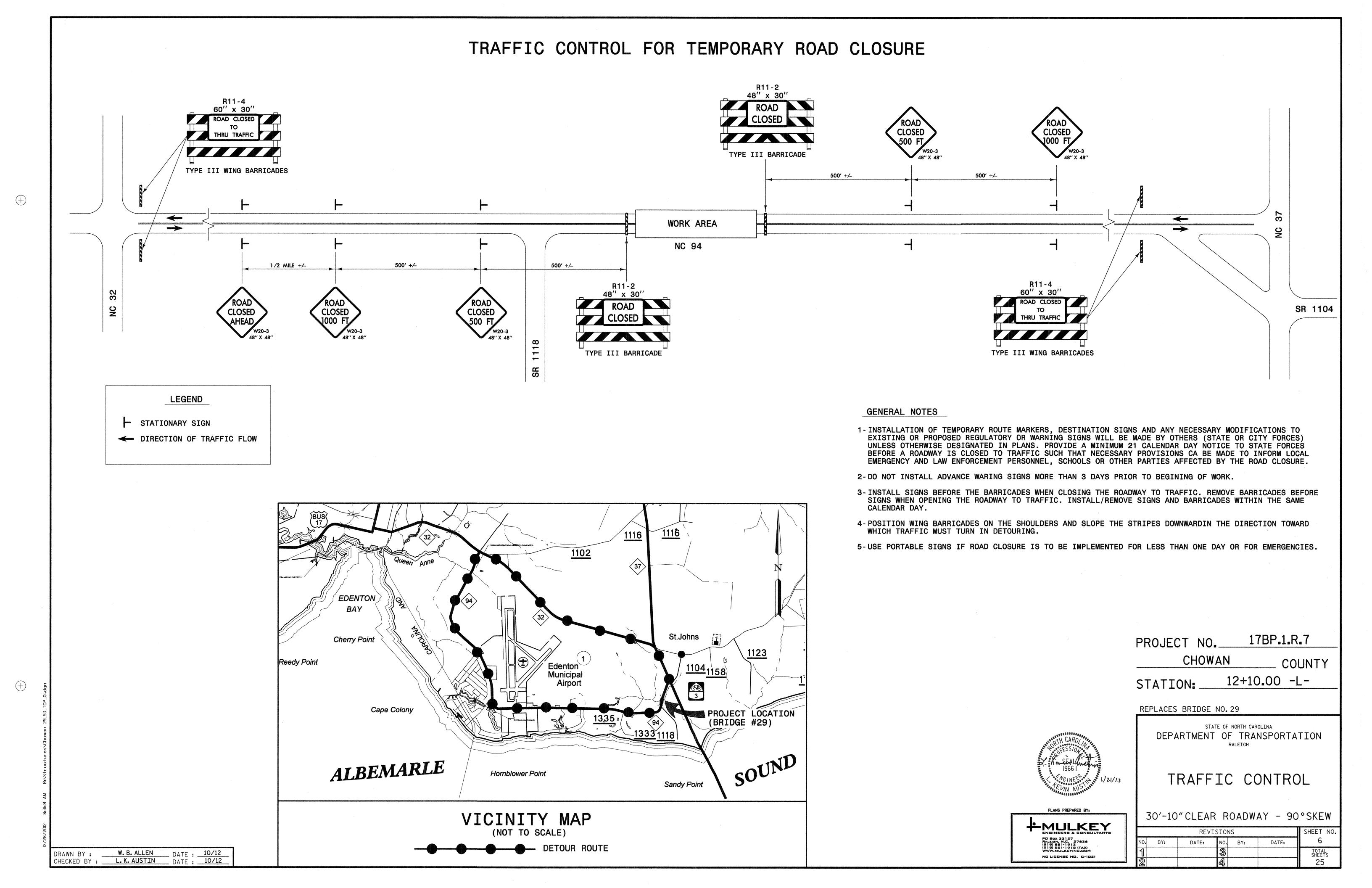
No. BY: DATE: No. BY: DATE: 5

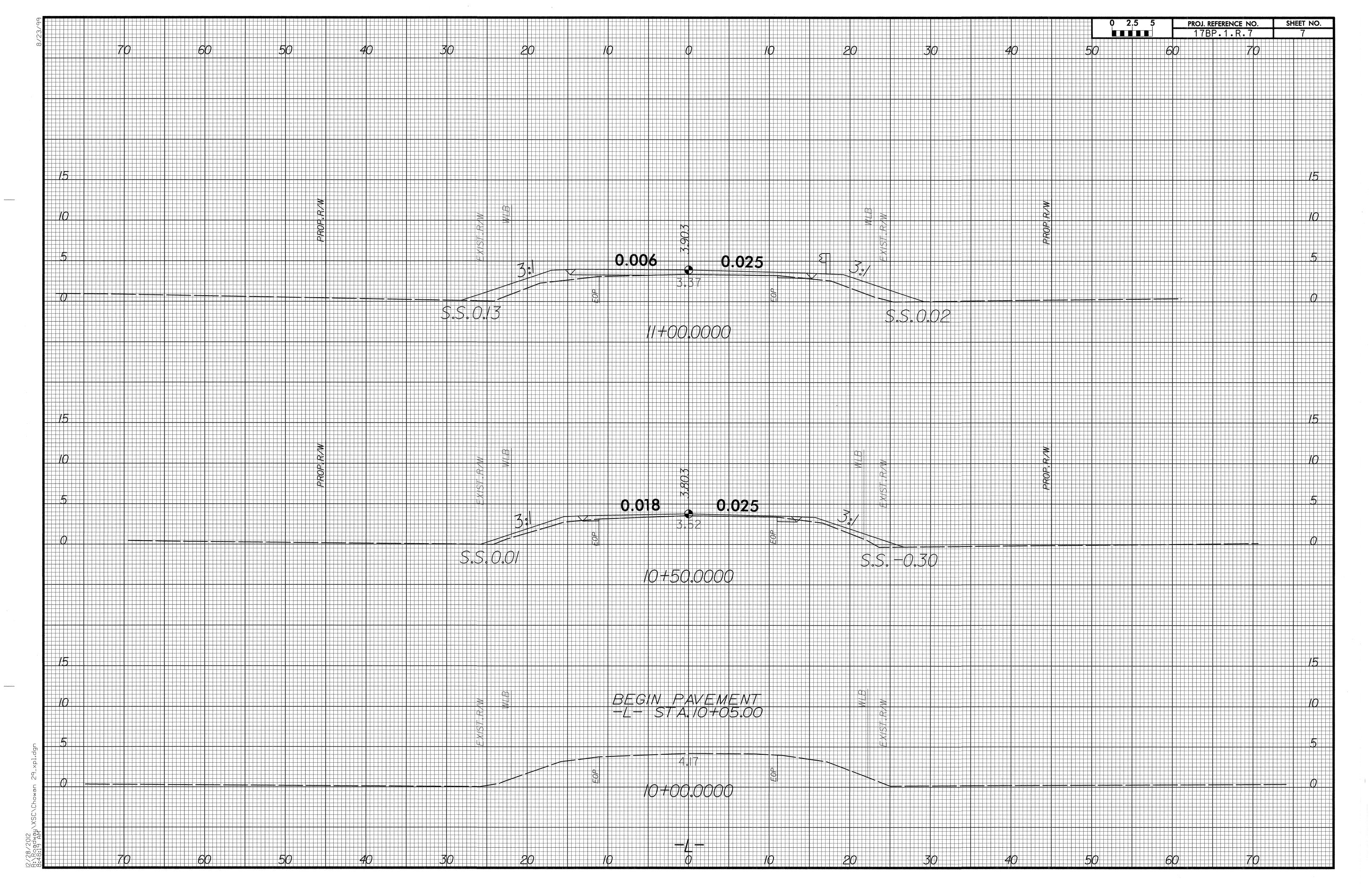
1 3 TOTAL SHEETS
2 4 2 25

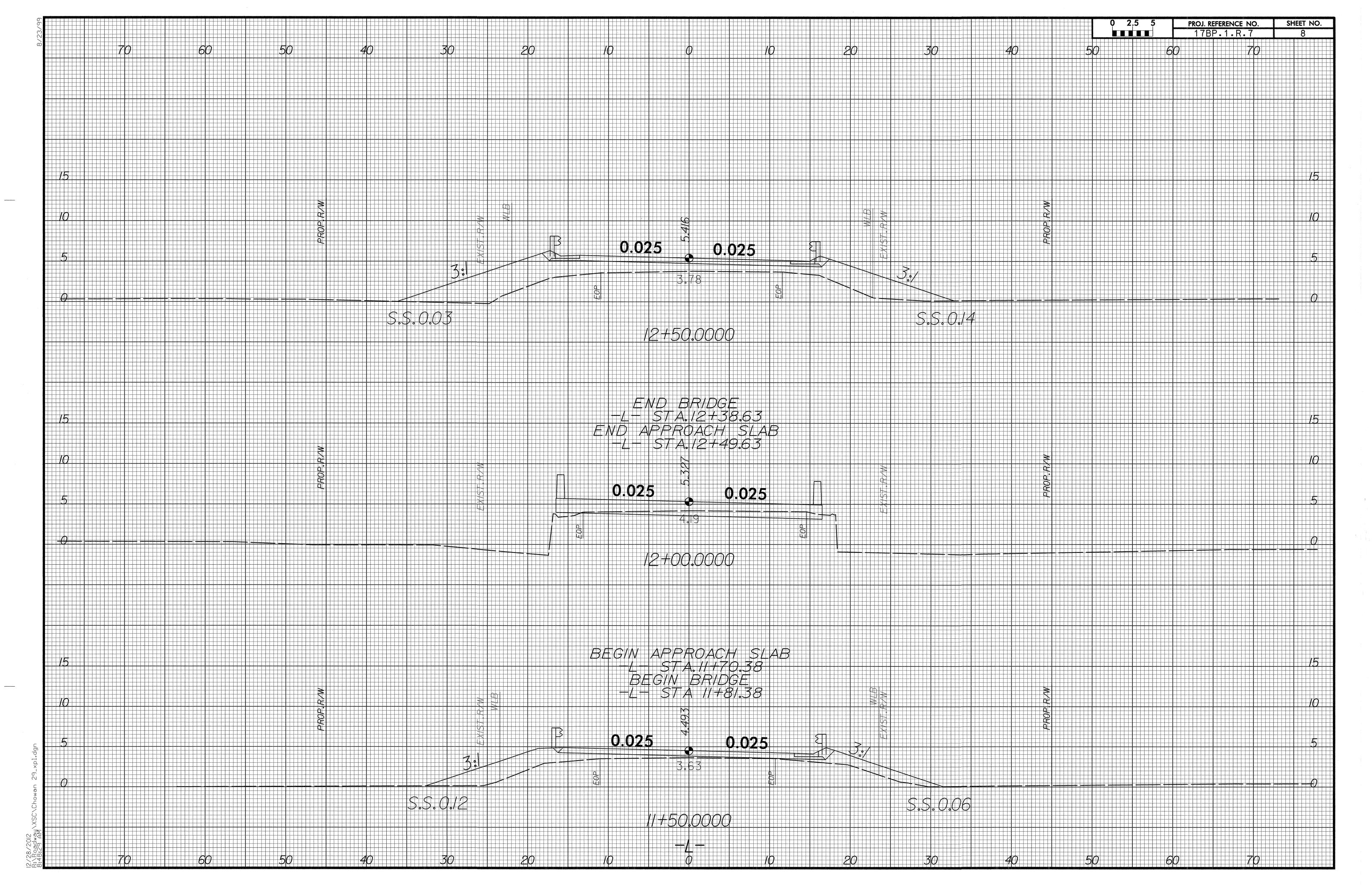
DRAWN BY: W.B.ALLEN DATE: 11/12
CHECKED BY: W.A.DAVIS DATE: 12/12

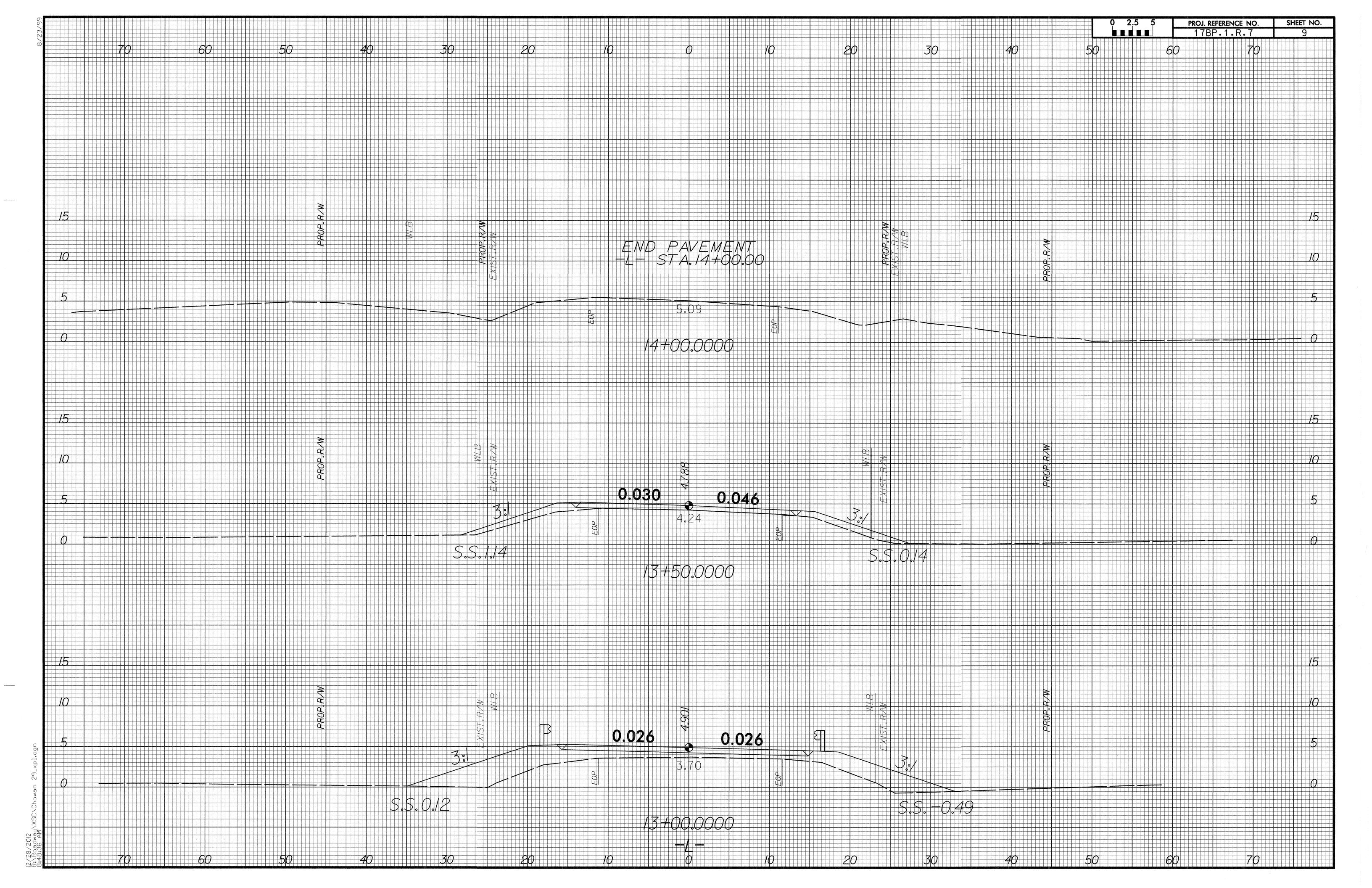
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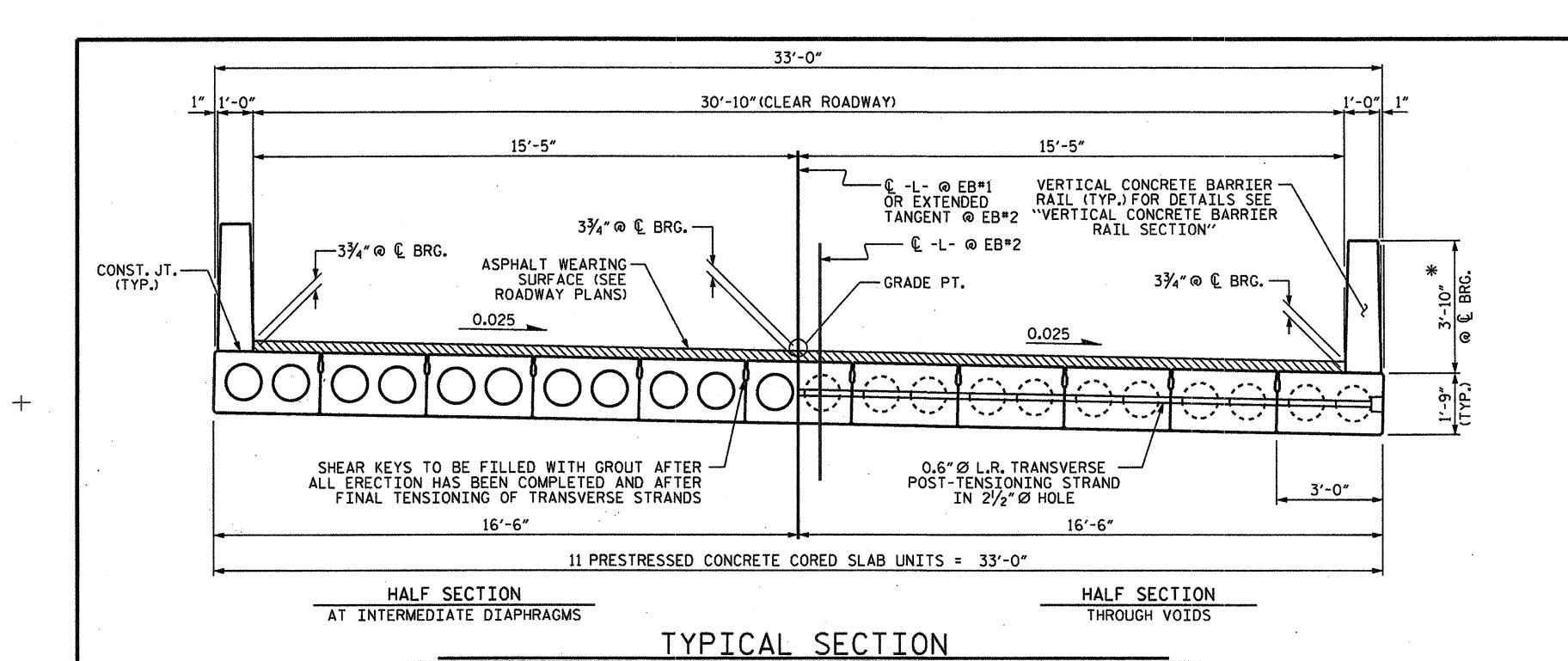
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*- THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL

FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT

THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

0.6" Ø LOW RELAXATION STRAND LAYOUT 3'-0"
10" 1'-4" 10"

*5 S3

*4 'B"
3"
2//2

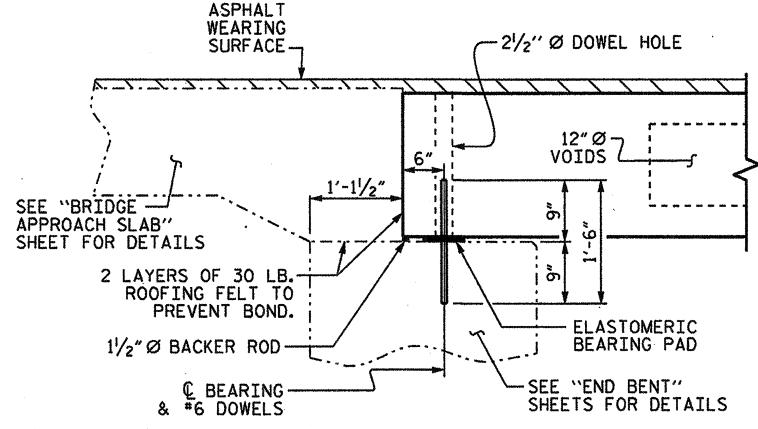
12" Ø VOIDS

3"
3"

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

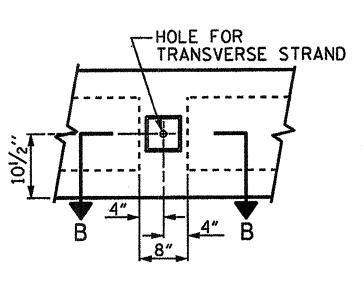
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O" FROM END OF CORED SLAB UNIT SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS.
 THESE STRANDS ARE NOT REQUIRED. IF THE
 FABRICATOR CHOOSES TO INCLUDE THESE STRANDS
 IN THE CORED SLAB UNIT, THE STRANDS SHALL
 BE DEBONDED FOR THE FULL LENGTH OF THE UNIT
 AT NO ADDITIONAL COST. SEE STANDARD
 SPECIFICATIONS. ARTICLE 1078-7.

DEBONDING LEGEND



SECTION AT END BENT

FIXED END



ELEVATION VIEW

© 0.6" Ø L.R. TRANSVERSE
POST-TENSIONING STRAND
SHEATHED WITH A
NON-CORROSIVE PIPE.

STRAND VISE

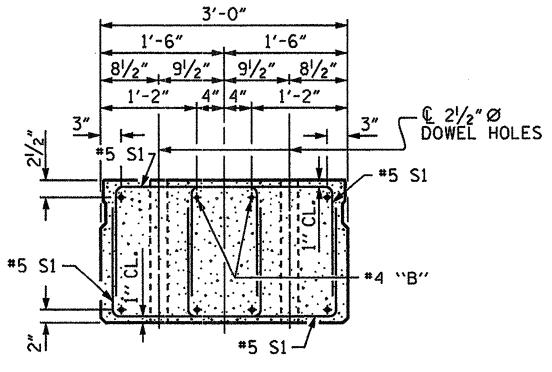
OUTSIDE FACE
OF EXTERIOR
CORED SLAB

OF CORED SLAB

SECTION B-B

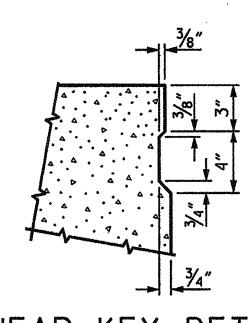
GROUTED RECESS AT END OF POST-TENSIONED STRAND OF CORED SLABS

ASSEMBLED BY: N. RUFFIN DATE: 7/27/12 CHECKED BY: M.D. PISO DATE: 8-16-12 DRAWN BY: DGE 5/09 REV. |2/|| MAA/AAC CHECKED BY: BCH 6/09



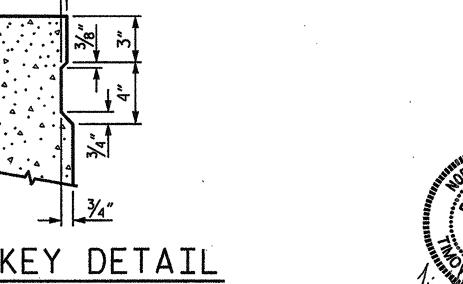
END ELEVATION

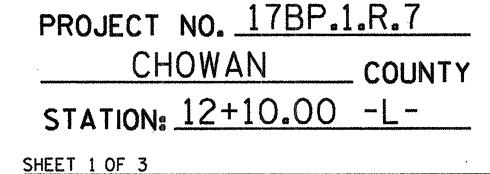
SHOWING PLACEMENT OF DOUBLE STIRRUPS
AND LOCATION OF DOWEL HOLES.
(STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB
UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



SHEAR KEY DETAIL

NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.



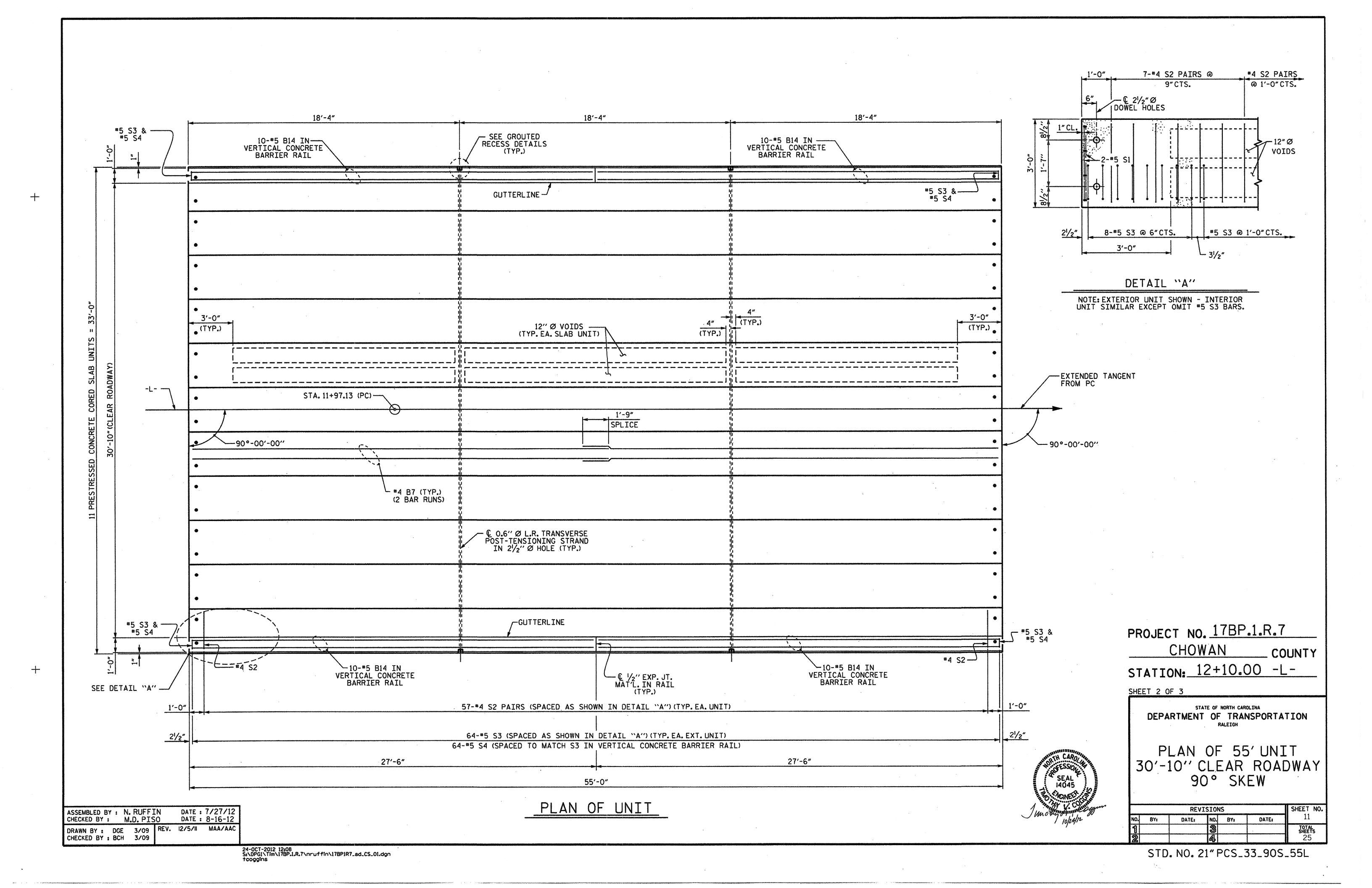


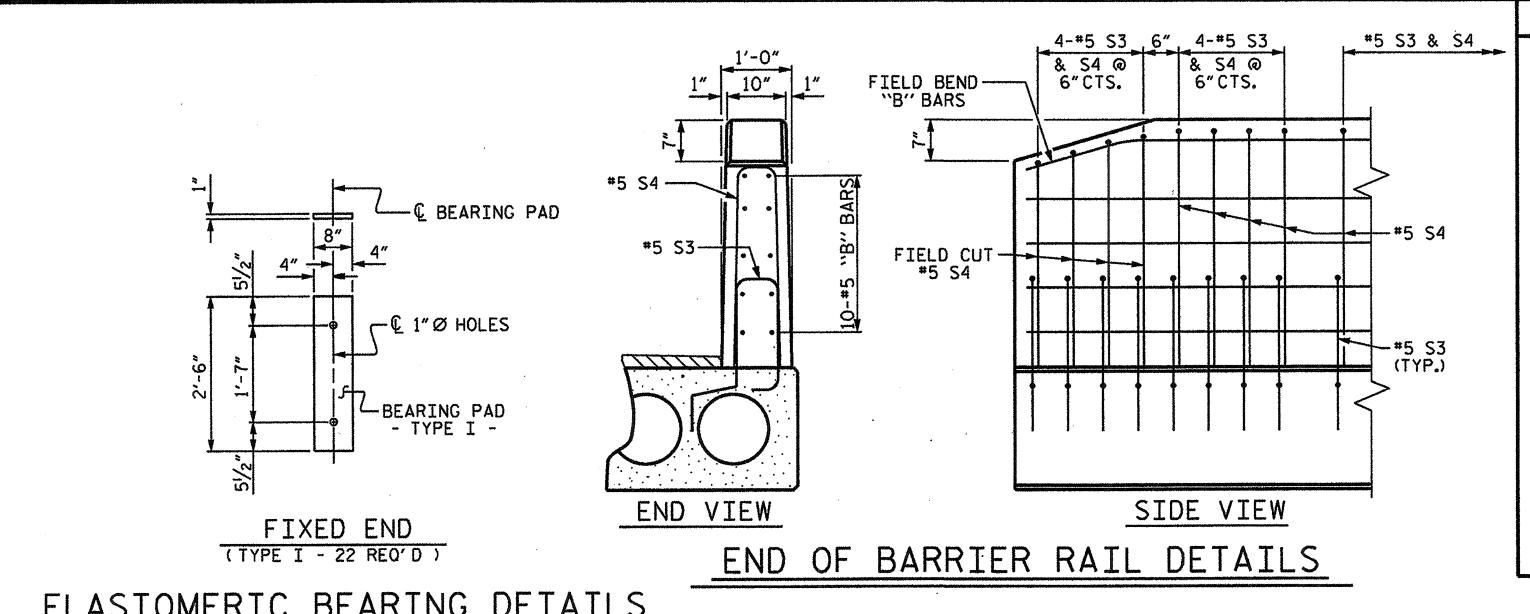
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

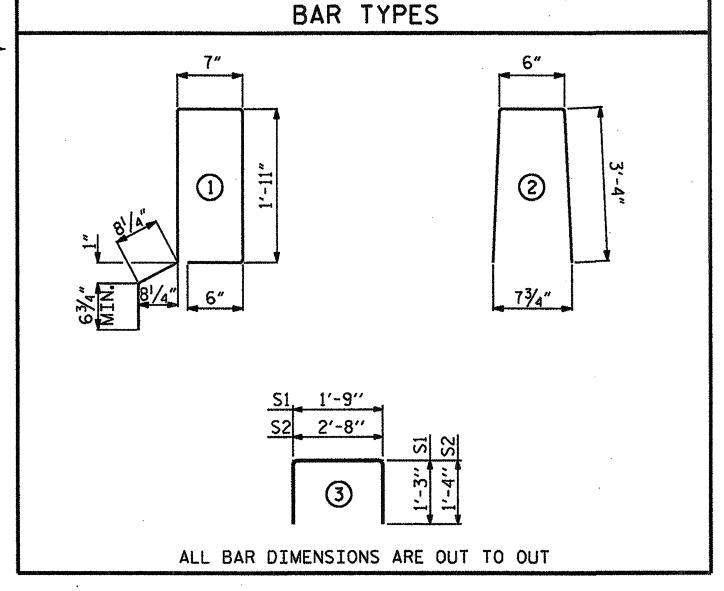
3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
90° SKEW

STD. NO. 21"PCS2_33_90S

24-0CT-2012 12:08 S:\DPG1\Tim\17BP.1.R.7\nruffin\17BP1R7_sd_CS_01.dgn tendins







ELASTOMERIC BEARING DETAILS

2"CL. MIN.

- #5 S4

VERTICAL DIM. VARIES

- #5 S3 (SEE "PLAN OF UNIT" FOR SPACING)

VERTICAL CONCRETE BARRIER RAIL SECTION

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS. GRADE 270 STRANDS 0.6" Ø L.R. 0.217 (SQUARE INCHES) ULTIMATE STRENGTH 58,600 (LBS. PER STRAND) APPLIED PRESTRESS 43,950 (LBS. PER STRAND)

SECTION S-S

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

CHAMFER

@ 1/2"EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS. (NOTE: OMIT EXP. JT. MAT'L. WHEN SLIP FORM IS USED.)—

CHAMFER

CHAMFER

ELEVATION AT EXPANSION JOINTS

CHAMFER

CORE	ED	SLABS	S REQ	UIRE	D
		NUMBER	LENGTH	TOTAL	LENGTH
55' UNI					
EXTERIOR	C.S.	2	55'-0"	110'	-0"
INTERIOR	C.S.	9	55'-0"	495′	-0"
TOTAL			NA CONTRACTOR OF THE CONTRACTOR OF THE	605′	-0"

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0"× 1'-9"
55' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	21/2" 🕴
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	1/4″ ♦
FINAL CAMBER	2¹/₄″ ♦

** INCLUDES FUTURE WEARING SURFACE

	5' UNIT					
*B14	40		1			***************************************
	40	40	#5	STR	27'-1"	1130
* S4	128	128	#5	2	7′-2″	957
* EPOXY COATED REINFO	RCING STEEL			LBS.		2087

	BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT									
				EXTERI	OR UNIT	INTERI(OR UNIT			
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT			
B7	4	#4	STR	28'-3"	75	28′-3″	75			
S1	8	#5	3	4′-3″	35	4'-3"	35			
S2	114	#4	3	5'-4"	406	5'-4"	406			
* S3	64	#5	1	6′-2″	.412					
REINFORCING STEEL L) a	516		516			
REI	XY COATE NFORCING	STEEL			412					
6500 P.S.I. CONCRETE CU. YDS			CU. YDS		7.8		7.8			
0.6″Ø	L.R. STR	ANDS	No) 4	19		19			

GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
30'-10"CLEAR ROADWAY	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
·	SUPERED SECTION	
55' UNITS	11/2"	3'-7¾"

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ % DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

CONCRETE	RELEASE	STRENGTH
UNIT		<u>PSI</u>
55' UNITS		4900

PROJECT NO. 17BP.1.R.7 CHOWAN COUNTY STATION: 12+10.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 1'-9" PRESTRESSED CONCRETE

REVISIONS NO. BY: DATE: BY: DATE:

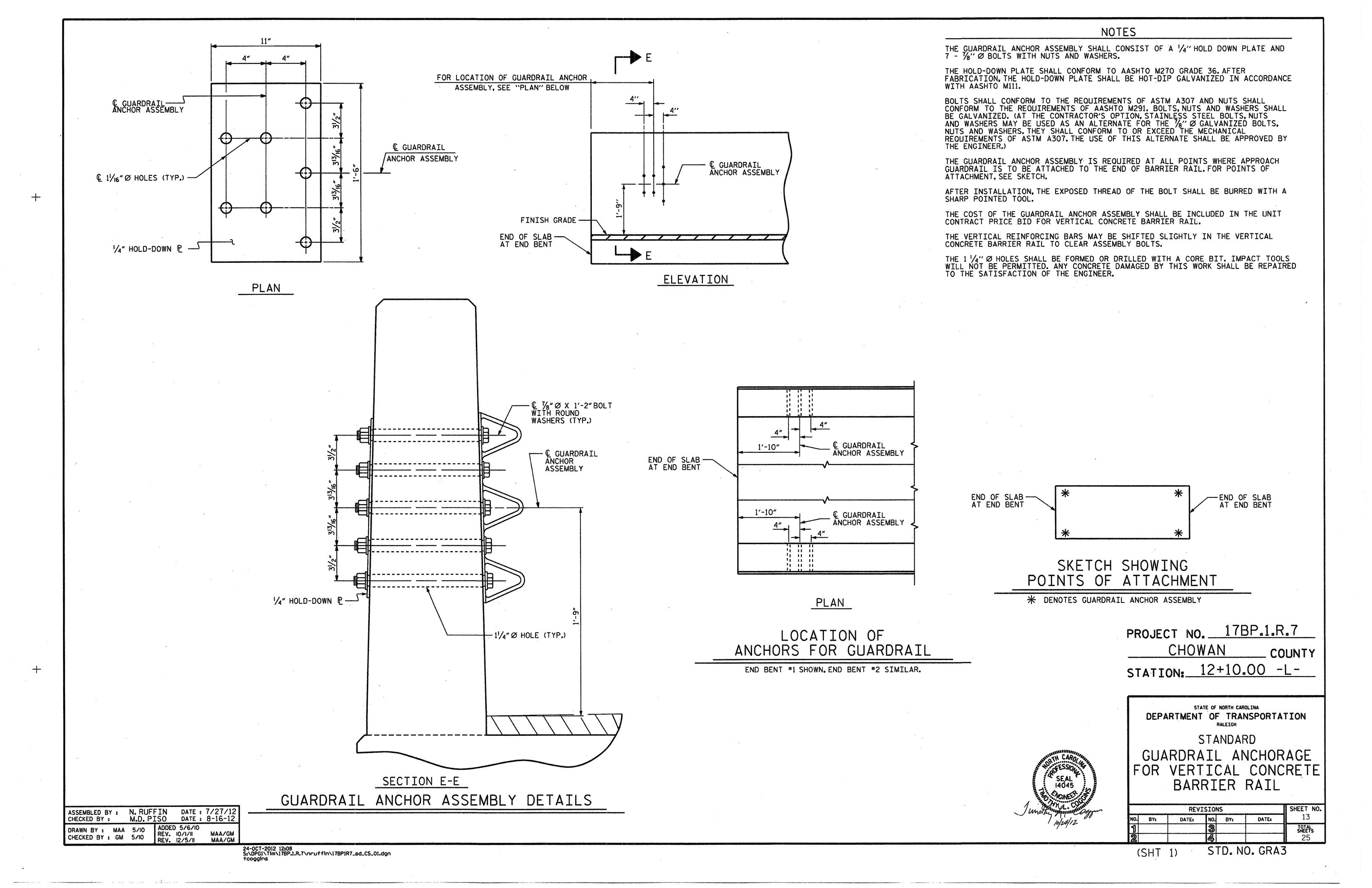
CORED SLAB UNIT 90° SKEW

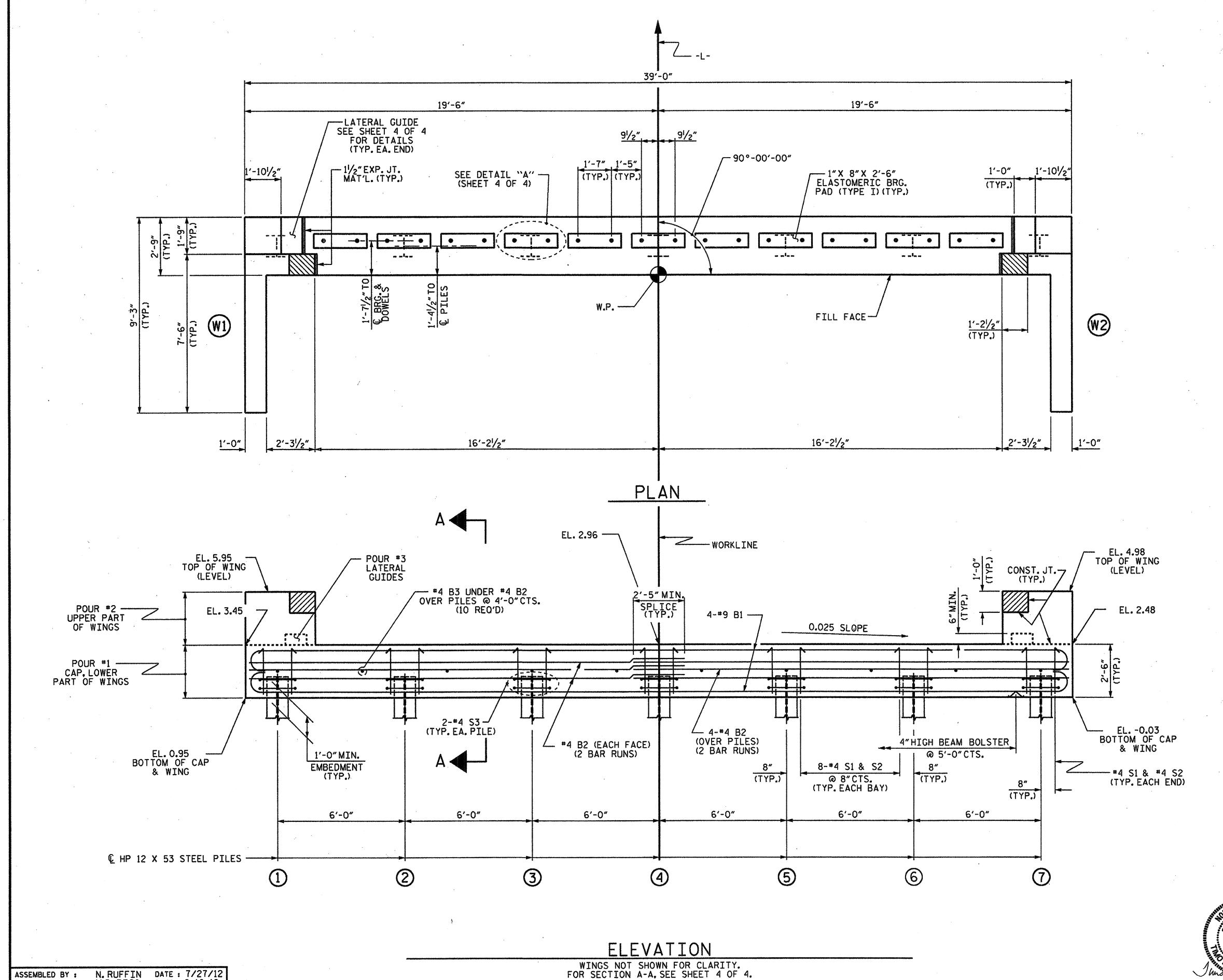
ASSEMBLED BY: N. RUFFIN CHECKED BY: M.D. PISO DATE : 7/31/12 DATE : 8-16-12 DRAWN BY : DGE 5/09 CHECKED BY : BCH 6/09

CONST. JT. -

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STD. NO. 21" PCS3_33_90S





STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

INSTALL THE 4"DIA. DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS. SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE CONCRETE IN THE END BENT CAPS SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB OF CEMENT. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY

GALVANIZE THE TOP 20 FEET OF EACH INTERIOR BENT PILE IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

TOP ELE	OF PILE VATIONS
1	1.93
2	1.78
3	1.63
4	1.48
5	1.33
6	1.18
7	1.03

17BP.1.R.7 PROJECT NO. __ CHOWAN COUNTY

STATION: 12+10.00 -L-

SHEET 1 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No. 1

		REVI	SION	<u> </u>		SHEET NO.
NO,	BY:	DATE:	NO.	BY:	DATE:	14
1			3			TOTAL SHEETS
2			4			25

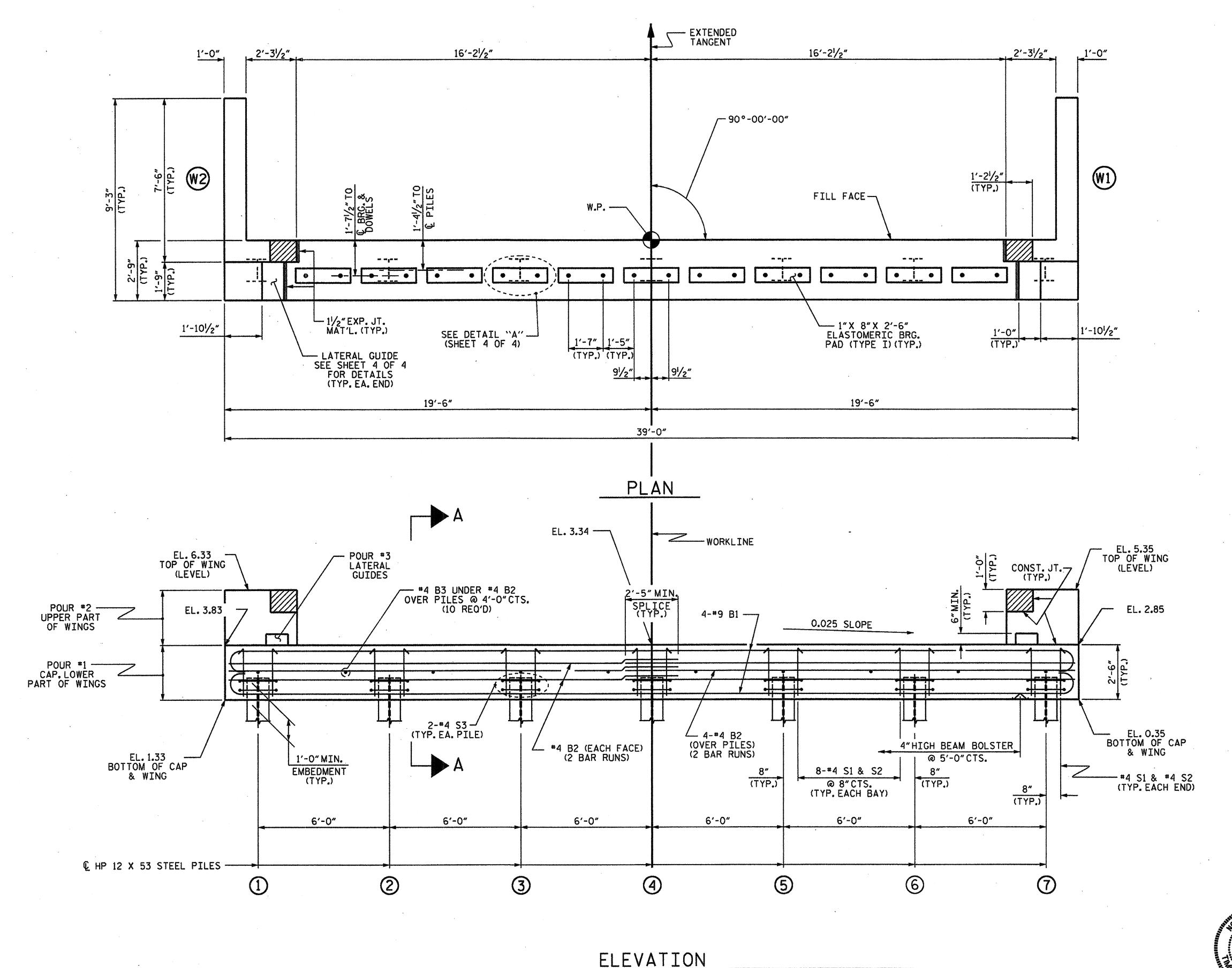
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N. RUFFIN DATE: 7/27/12 M.D. PISO DATE: 8-16-12

ASSEMBLED BY : CHECKED BY :

DRAWN BY : DGE 02/10 CHECKED BY : MKT 02/10

STD. NO. EB_33_90S



WINGS NOT SHOWN FOR CLARITY. FOR SECTION A-A. SEE SHEET 4 OF 4.

NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

INSTALL THE 4"DIA.DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

THE CONCRETE IN THE END BENT CAPS SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS FFLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB OF CEMENT. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

GALVANIZE THE TOP 20 FEET OF EACH INTERIOR BENT PILE IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.

T O F	OF PILE EVATIONS
1	2.30
2	2.15
3	2.00
4	1.85
(5)	1.70
6	1.55
7	1.40

PROJECT NO. 17BP.1.R.7

CHOWAN COUNTY

STATION: 12+10.00 -L-

SHEET 2 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 2

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	15
1			3			TOTAL SHEETS
2			4			25

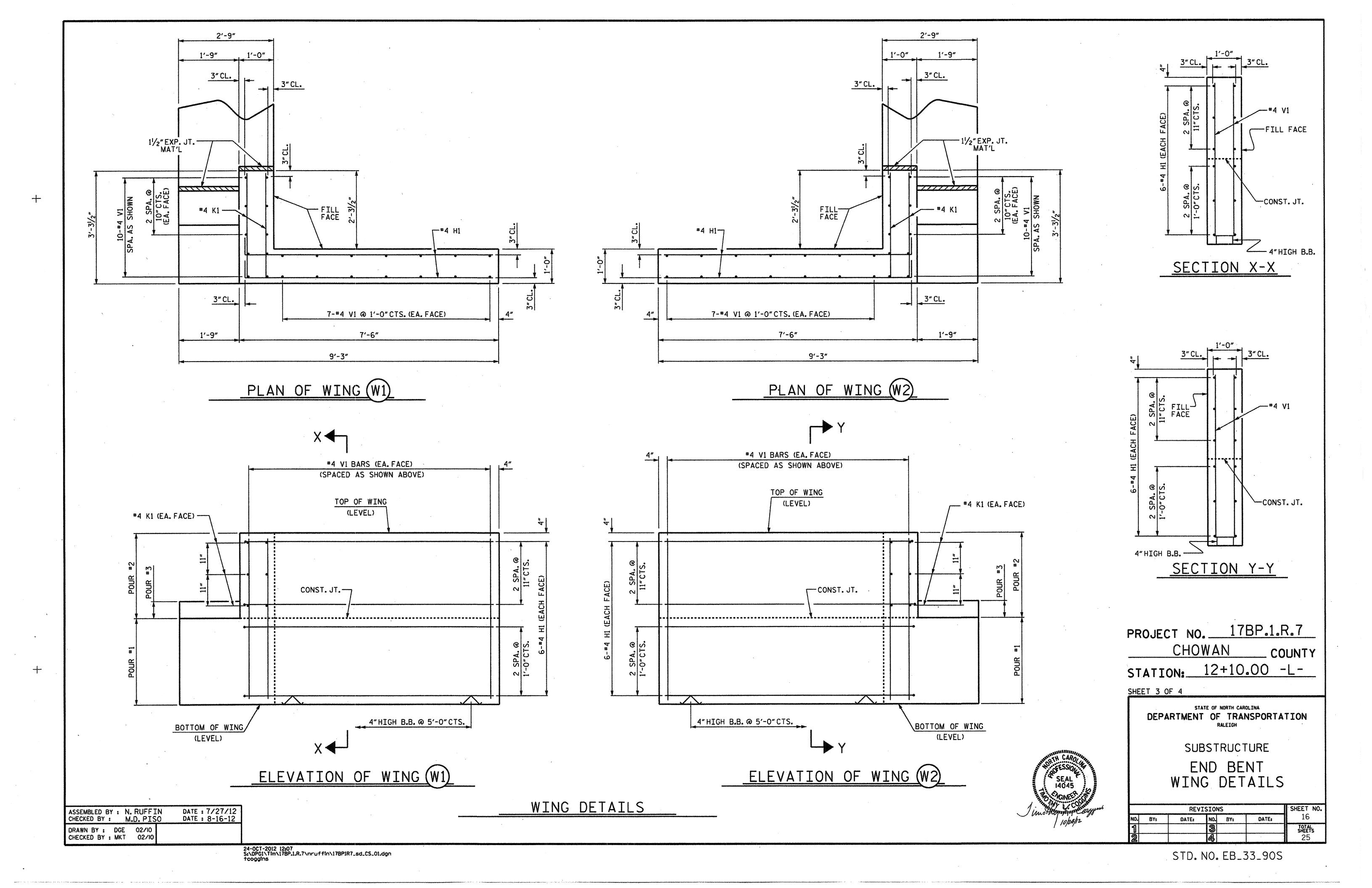
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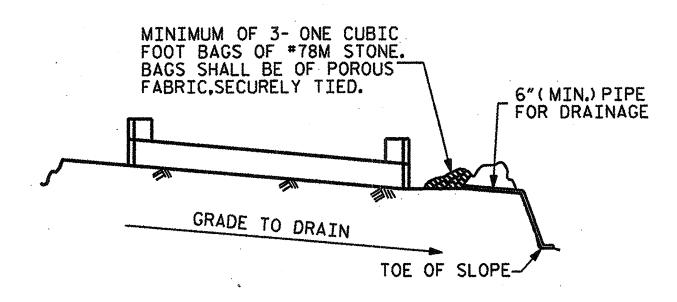
N. RUFFIN DATE: 7/27/12 M.D. PISO DATE: 8-16-12

ASSEMBLED BY : CHECKED BY :

DRAWN BY: DGE 02/10 CHECKED BY: MKT 02/10

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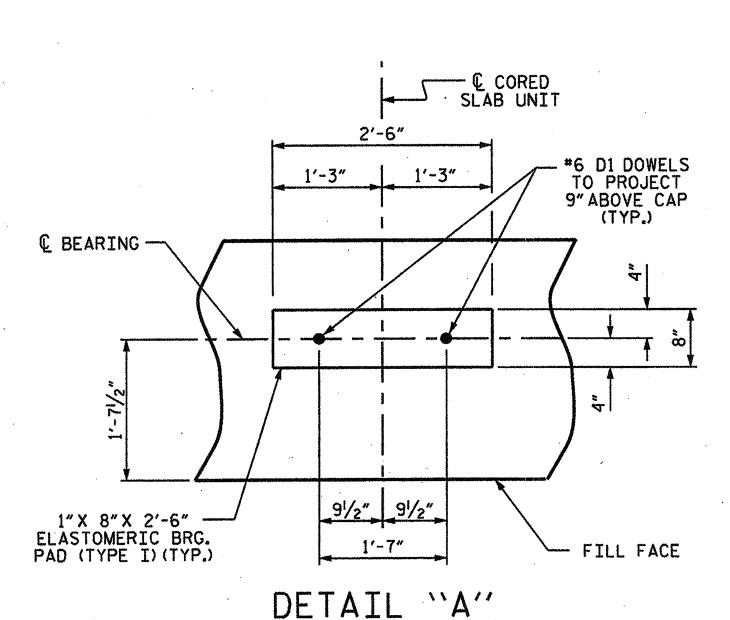


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

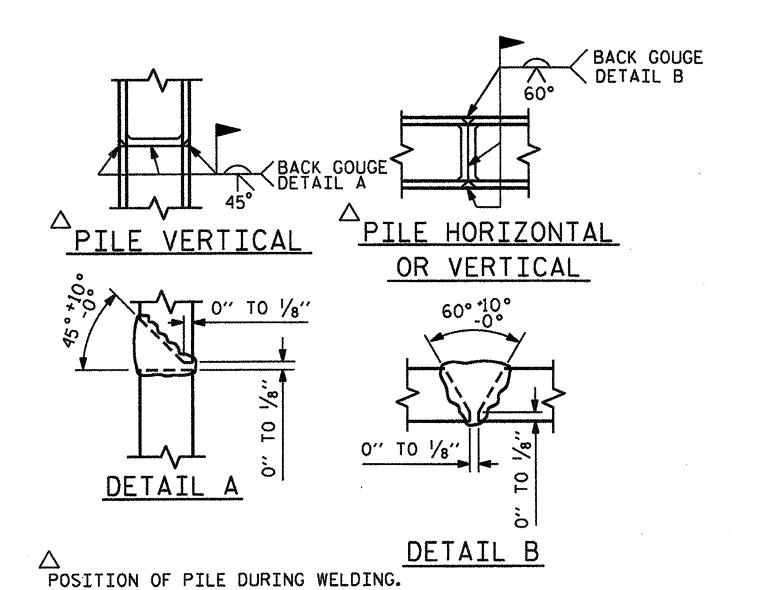
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

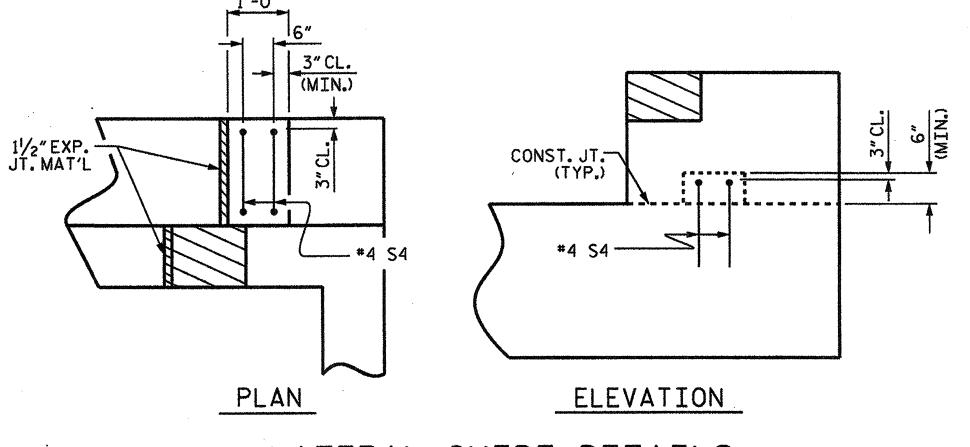
TEMPORARY DRAINAGE AT END BENT



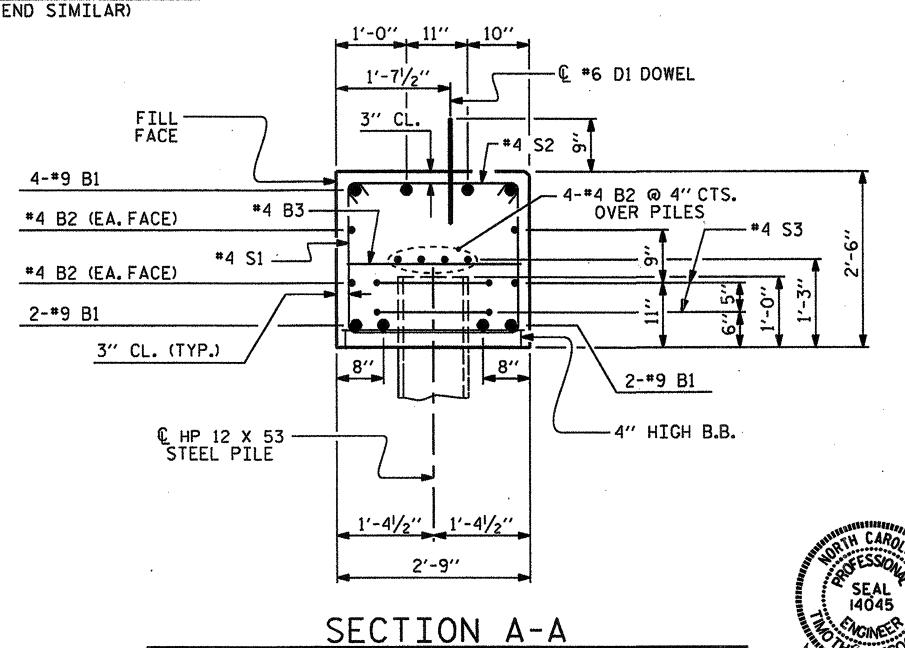
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

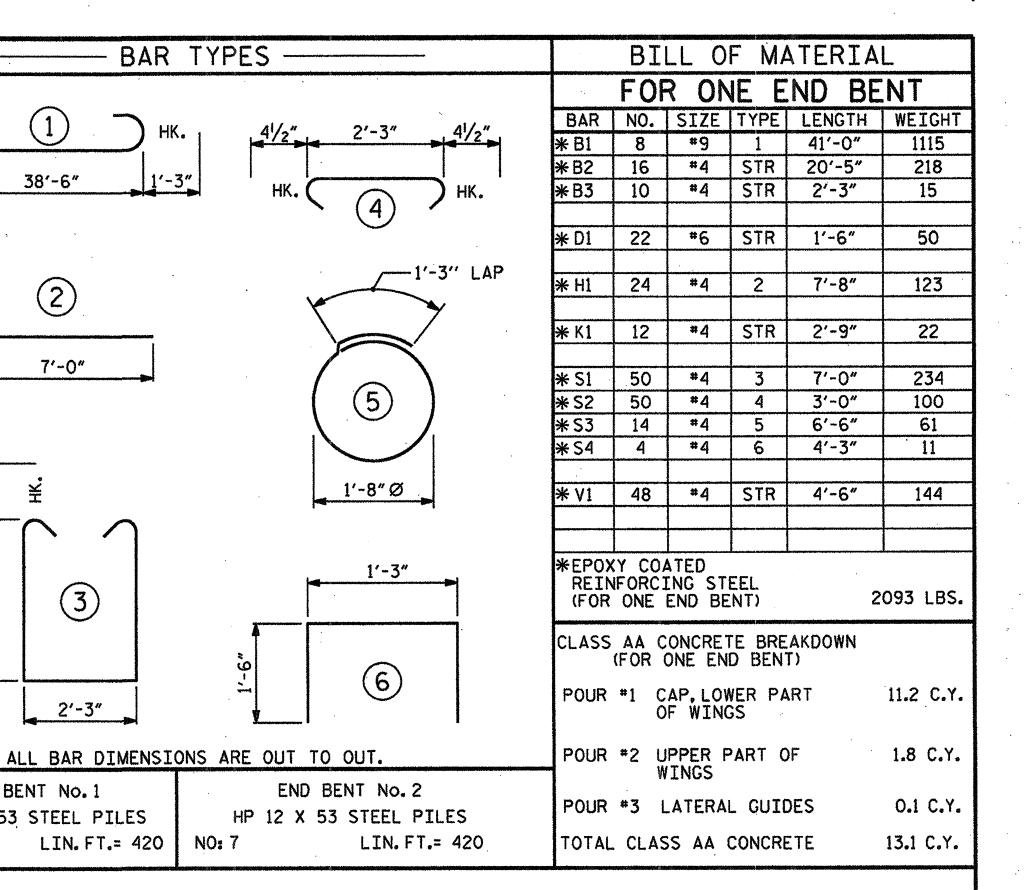


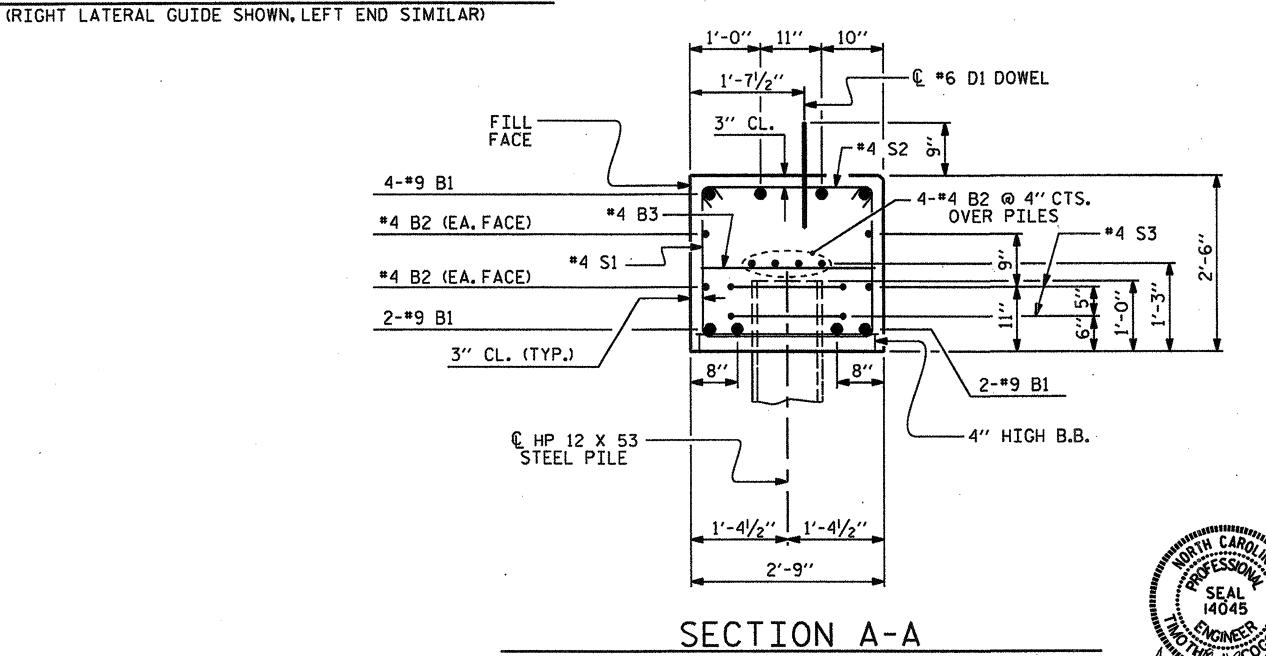
PILE SPLICE DETAILS



LATERAL GUIDE DETAILS







BAR TYPES

7'-0"

2'-3"

LIN. FT.= 420 NO: 7

END BENT No. 1

HP 12 X 53 STEEL PILES

NO: 7

PROJECT NO. 17BP.1.R.7 CHOWAN COUNTY STATION: 12+10.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

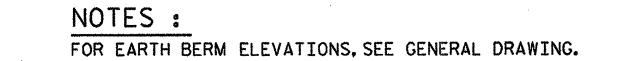
END BENT No.1 & 2 DETAILS

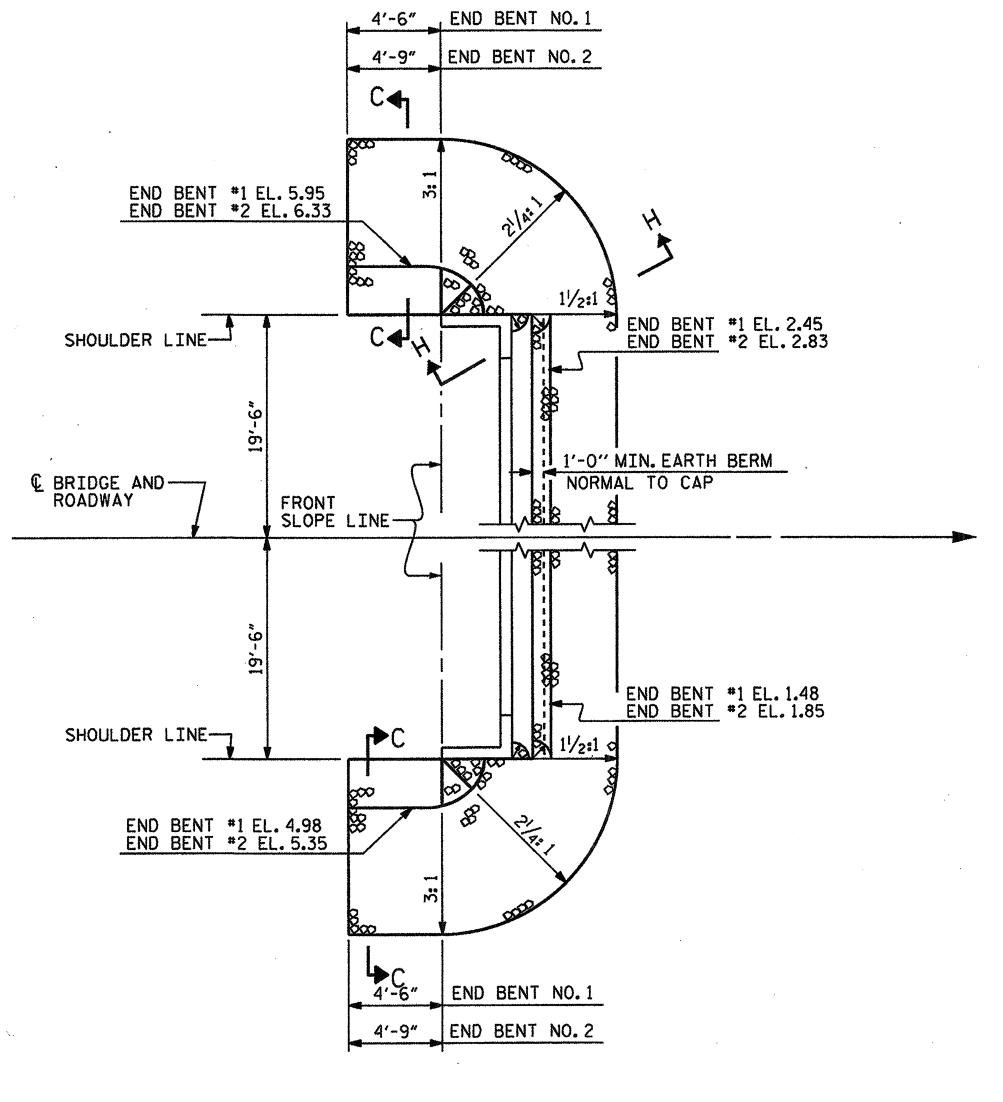
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ASSEMBLED BY: N. RUFFIN CHECKED BY: M.D. PISO DATE : 7/27/12 DATE : 8-16-12 DRAWN BY: DGE 02/10 CHECKED BY: MKT 02/10

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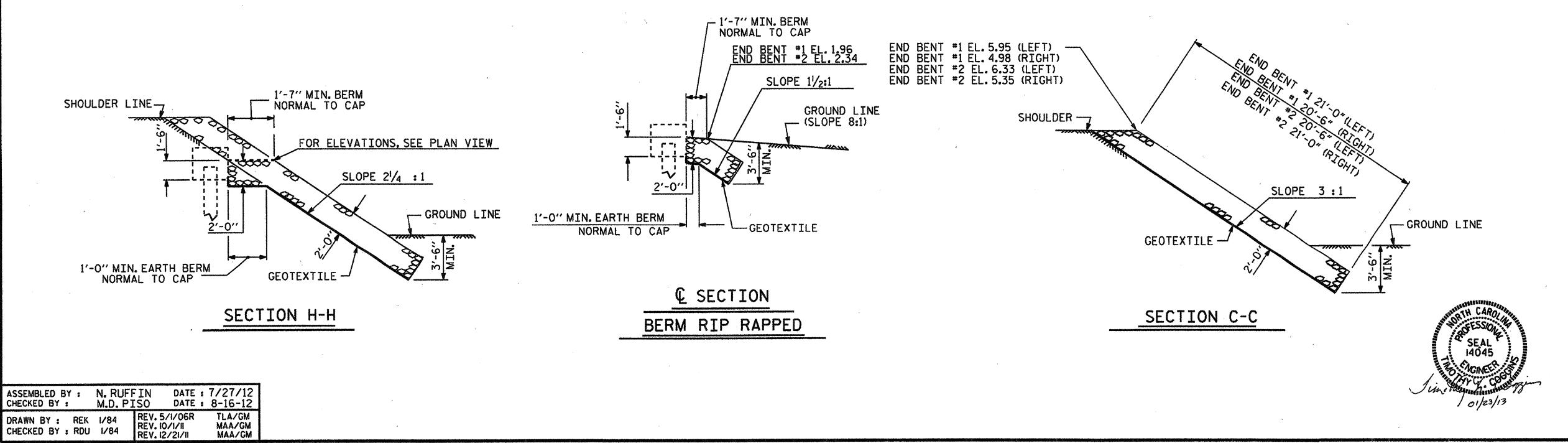
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ESTIMATED QUANTITIES					
BRIDGE @ STA.12+10.00 -L-	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS			
END BENT 1	140	152			
END BENT 2	120	133			

SHOULDER RIP RAP IS HIGHER THAN BERM RIP RAP



PROJECT NO. 17BP.1.R.7

CHOWAN COUNTY

STATION: 12+10.00 -L-

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

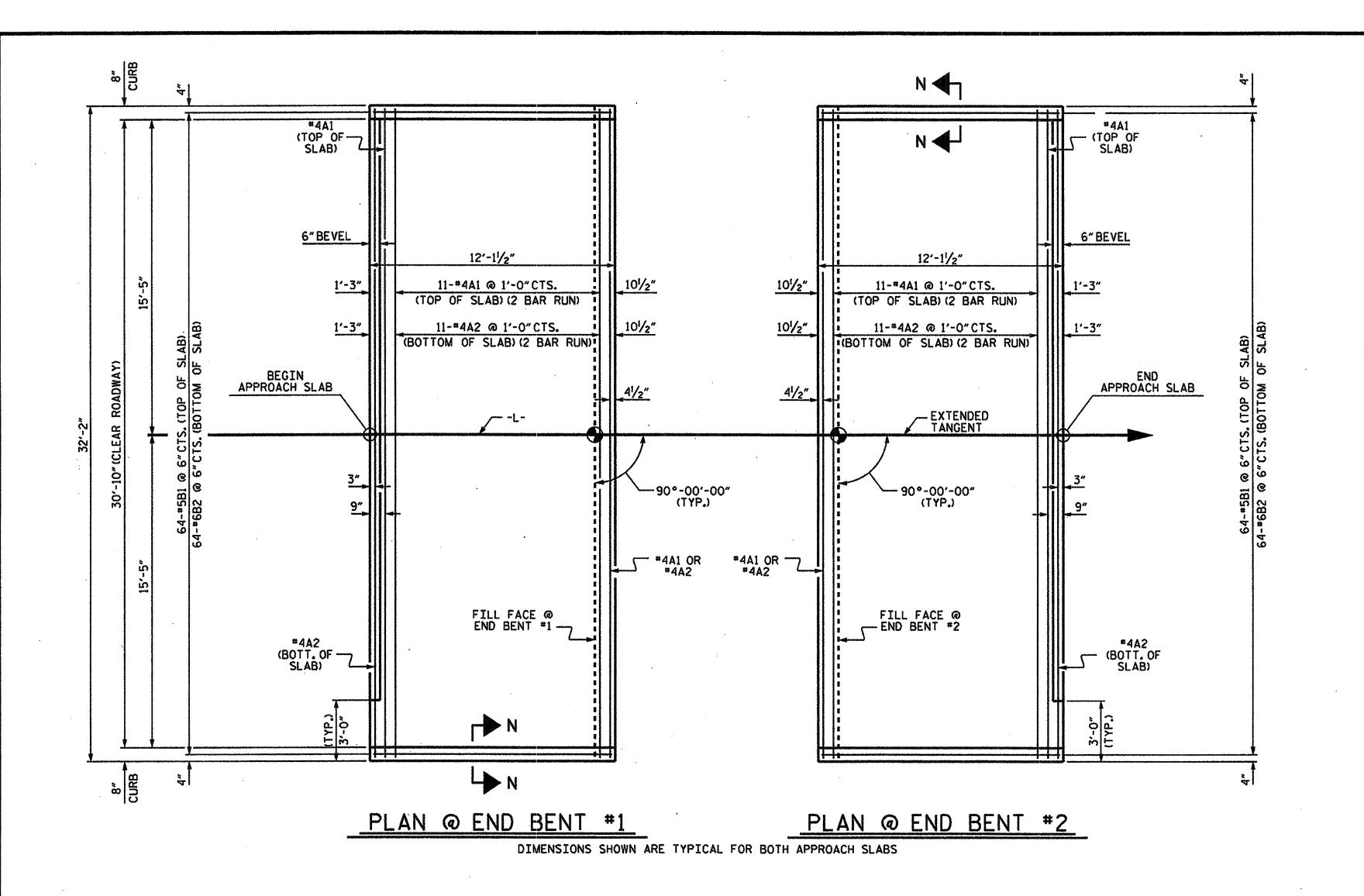
STANDARD

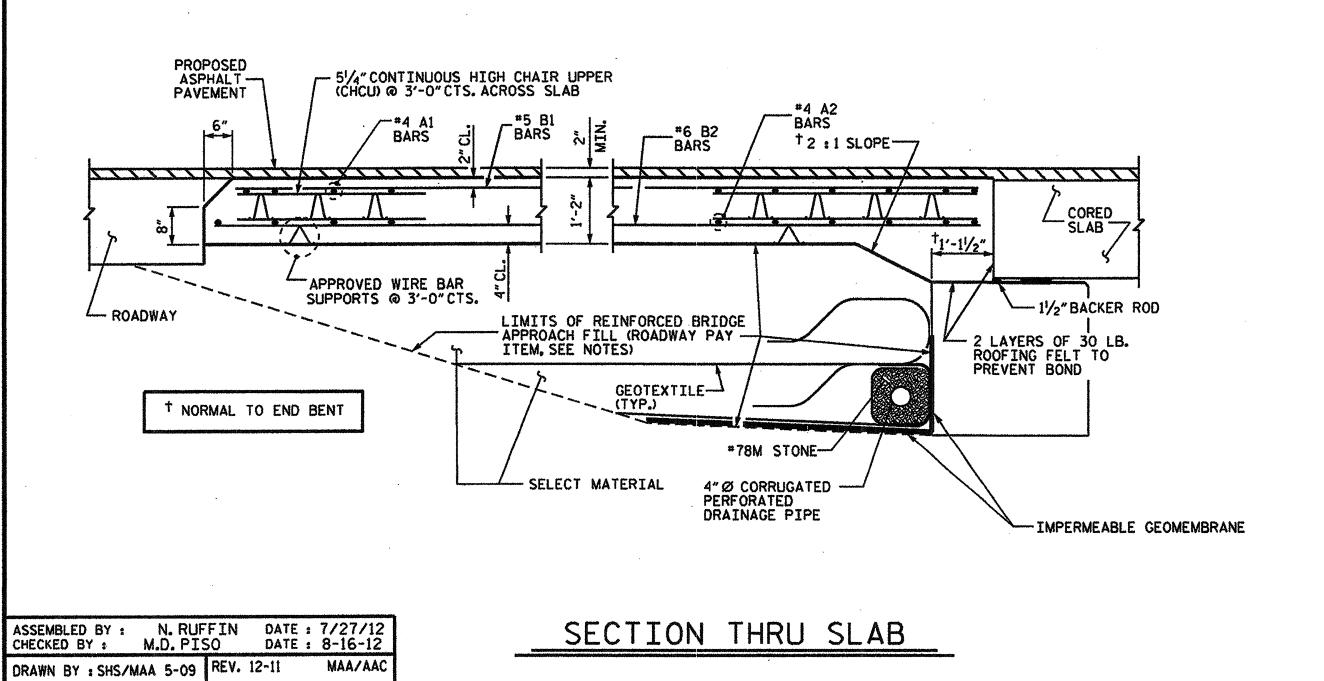
-RIP RAP DETAILS-

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NO.	BY:	DATE:	NO.	BY:	DATE:	18
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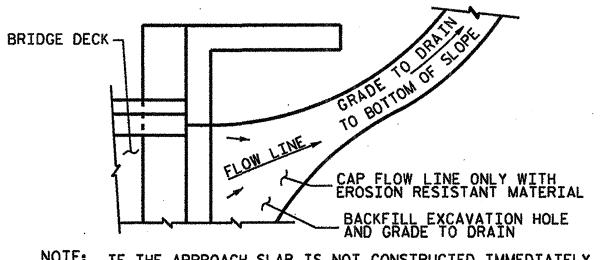


NOTES

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.

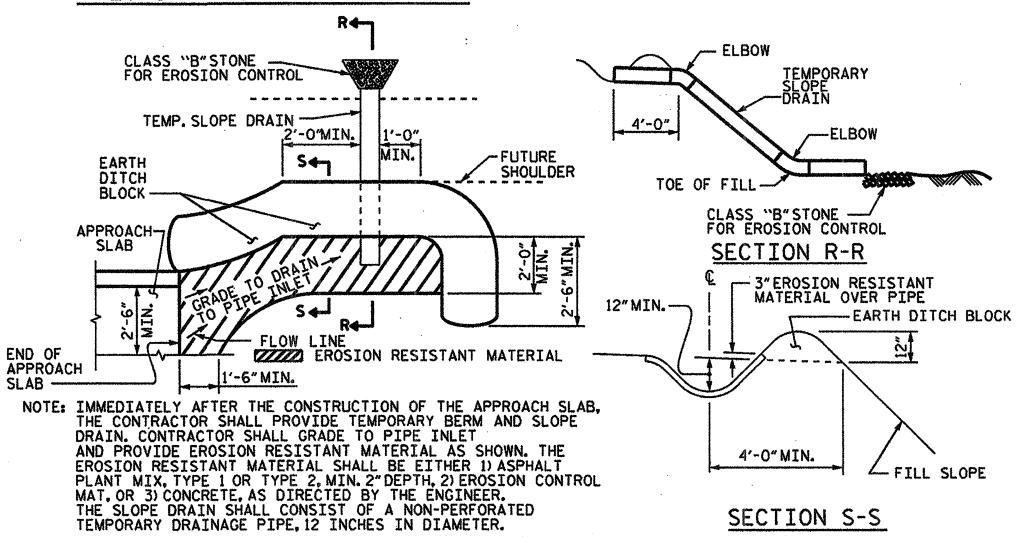
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION. GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL. SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

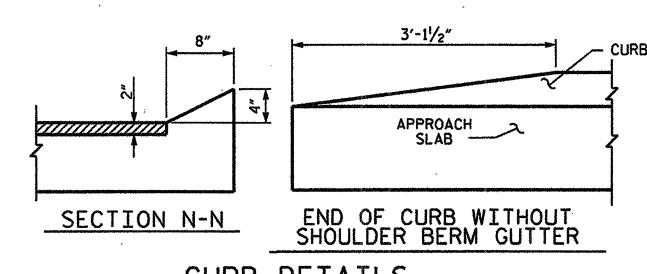
TEMPORARY DRAINAGE DETAIL



PLAN VIEW

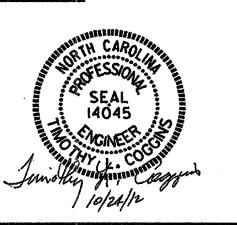
TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



CURB DETAILS

SPL:	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
* 5	2'-6"	2'-2"
#6	3'-10"	2'-7"



PROJECT NO. 17BP.1.R.7

CHOWAN COUNTY

STATION: 12+10.00 -L-

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR NO. SIZE TYPE LENGTH WEIGHT

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

* A1 26 #4 STR 16'-11"

*B1 64 #5 STR 11'-2"

B2 64 #6 STR 11'-8"

A2 | 26 | #4 | STR | 16'-9"

294

291

745

1121

1412

1039

18.6

294

745

1121

1412

1039

18.6

LBS.

LBS.

C. Y.

LBS.

LBS.

C. Y.

* A1 | 26 | #4 | STR | 16'-11" |

A2 26 #4 STR 16'-9"

*B1 64 *5 STR 11'-2"

B2 64 #6 STR 11'-8"

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

* EPOXY COATED REINFORCING STEEL

CLASS AA CONCRETE

* EPOXY COATED REINFORCING STEEL

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
BRIDGE APPROACH SLAB
FOR PRESTRESSED CONCRETE
CORED SLAB UNIT
(SUB-REGIONAL TIER)
90° SKEW

STATE OF NORTH CAROLINA

	REVIS	SHEET NO.			
۲s	DATE:	NO.	BY:	19	
		3			TOTAL SHEETS
		4			25

CHECKED BY : BCH 5-09

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ----- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF - 20,000 LBS. PER SQ. IN. STRUCTURAL STEEL - AASHTO M270 GRADE 36 - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 -- 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS ---- 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER ----

MATERIAL AND WORKMANSHIP:

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

30 LBS. PER CU. FT.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL

STRUCTURAL STEEL:

BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8% SHEAR STUDS FOR THE 3/4% STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3-7/8% STUDS FOR 4-3/4% STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8% STUDS ALONG THE BEAM AS SHOWN FOR 3/4% STUDS BASED ON THE RATIO OF 3-7/8% STUDS FOR 4-3/4% STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0%.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

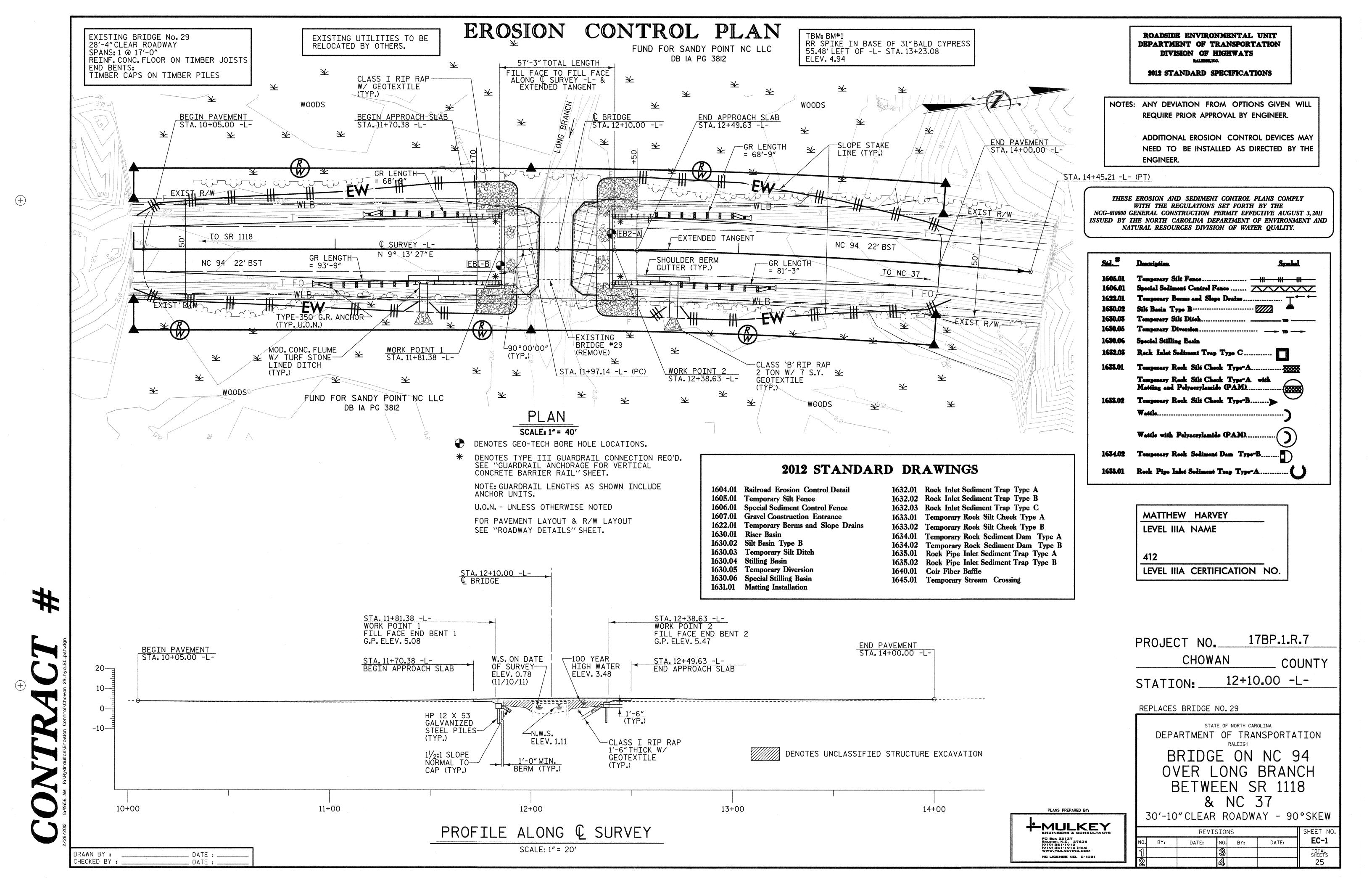
METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

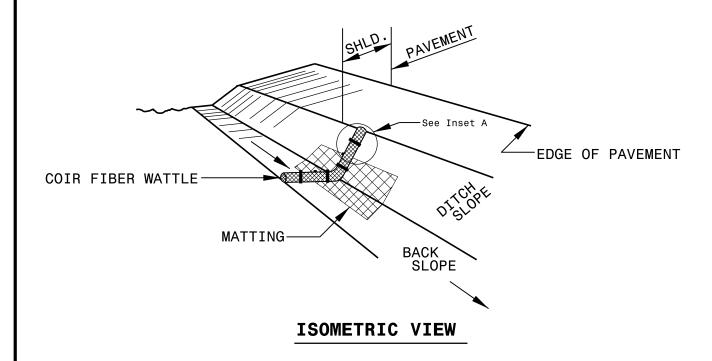
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JANUARY, 1990

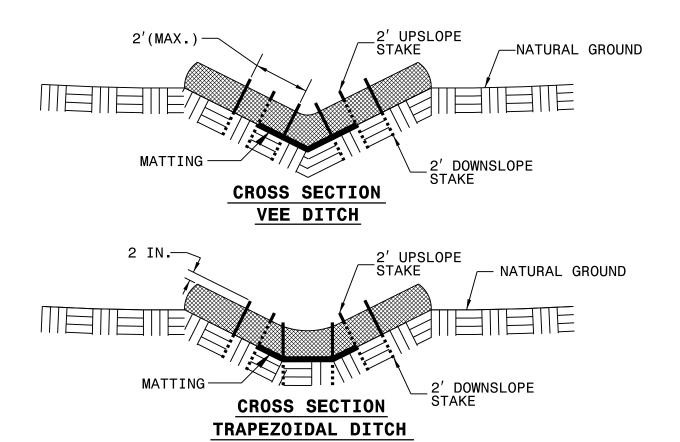
SHEET NO.
20
TOTAL
SHEETS
25



COTR	FTRFR	WATTLE	DFTATI
OOTIL			

PROJECT REFERENCE NO	D. SHEET NO.
17BP.J.R.7	22 OF 25
R/W SHEET N	١٥.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER





NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

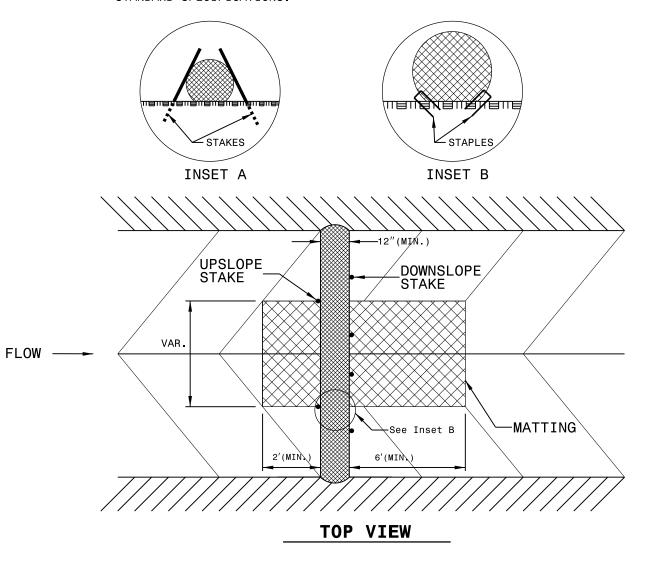
 $\underline{\text{ONLY}}$ INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

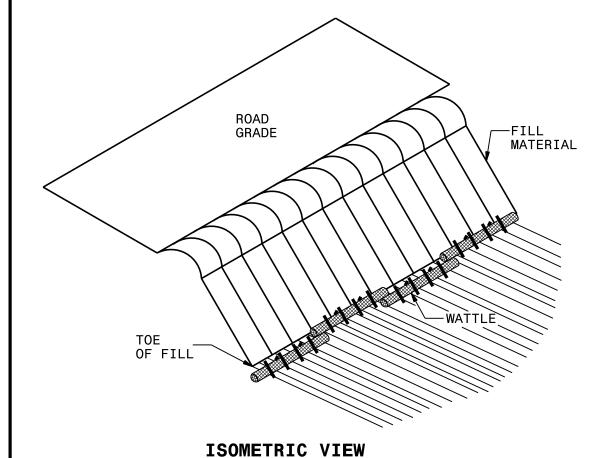
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

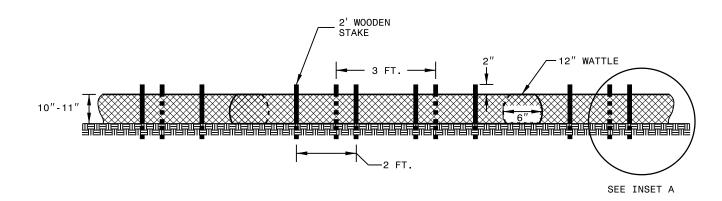
INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.



COIR FIBER WATTLE BARRIER DETAIL

PROJECT REFERENCE NO	D. SHEET NO.
17BP.J.R.7	23 OF 25
R/W SHEET N	10.
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER





FRONT VIEW

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

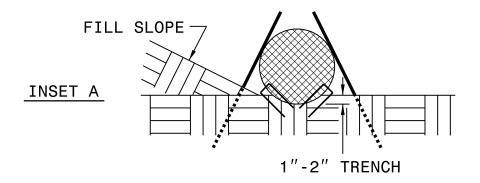
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

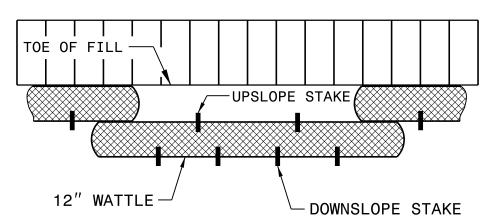
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 20 FT.

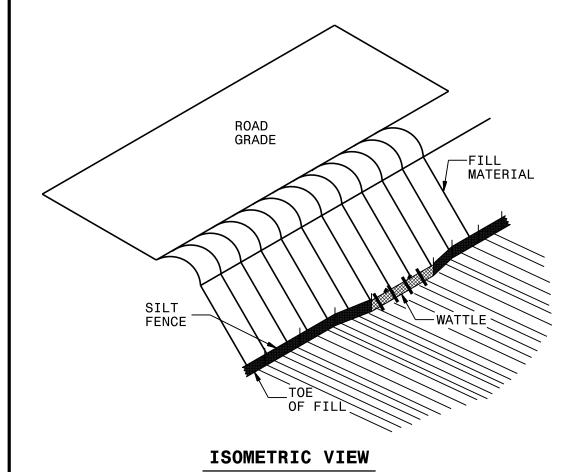


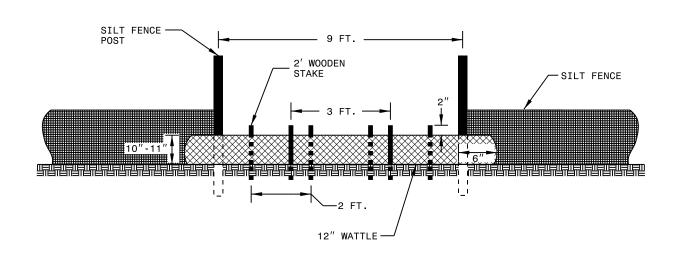


TOP VIEW

SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO	SHEET NO.	Ì		
17BP.I.R.7	17BP.J.R.7			
R/W SHEET N	10.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER		





VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

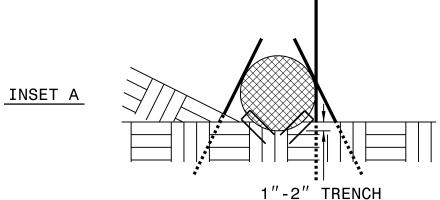
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

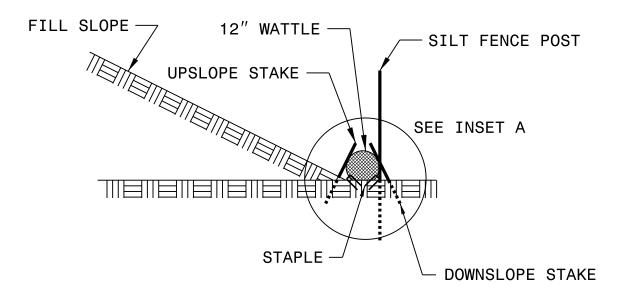
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





SIDE VIEW

PROJECT REFERENCE NO. SHEET NO. 17BP.J.R.7 25 0F 25

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10' OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HOW ZONES.

STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C. STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C. CONCRETE OR RIP-RAP DITCH SEE ROADWAY PLANS (4) 12" #6 DOWEL BARS -8" X 4" LIP CURB BEGIN MODIFIED
CONCRETE FLUME OUTLET/ DEPRESSION PAVED SHOULDER -EDGE OF LANE 15'-0" BRIDGE
APPROACH SLAB SHOULDER BERM GUTTER OPTIONAL SEE RDY. PLANS MODIFIED CONCRETE FLUME PAY LIMITS - PER EACH SHOULDER BERM GUTTER
OPTIONAL SEE RDY. PLANS ENGLISH D FLUME DITCH WITH <u>PLAN VIEW</u> FOR 1∕8″ RADIUS 4'-0" CONCRETE VARIABLE LENGTH MODIFIED CONCRETE FWITH CONCRETE OR RIP-RAP SEE PLANS 2'-0" SEE PLANS FOR PLACEMENT OR BEGINNING DETAIL SECTION A-A 4" CONC CONCRETE PAVED DITCH WATER FLOW SECTION C-C 9 R OUTLET **DOWNGRADE OR SAG** RIP-DRAWING © WATER OUTLET FLOW DIVERSION FLOW RAPSECTION B-B OUTLET (A) WATER FLUME FLOW DITCH FOR WATER $^{\otimes}$ FLOW DIVERSION-4'-0" FLOW DIVERSION 2'-0" **DOWN GRADE** <u>SAG</u> FLOW DIVERSION EXAMPLES NOTES: - CONSTRUCT MODIFIED CONCRETE FLUME AND SHOULDER BERM GUTTER IN ACCORDANCE WITH THIS DETAIL.
- CONSTRUCT CONCRETE DITCH IN ACCORDANCE WITH STD. DWG. NO. 850.01.
- CONSTRUCT RIP RAP LINED DITCH IN ACCORDANCE WITH THIS DETAIL, IF CALLED FOR IN PLANS.
- CONCRETE OR RIP RAP LINED DITCH SHALL BE THE TYPE AND LENGTH SPECIFIED BY THE ROADWAY PLANS. THE DITCH SHALL TERMINATE AS SHOWN ON THE PLANS. IF NO TERMINATION IS INDICATED PLACE RIP-RAP AT THE END OF THE DITCH AS INDICATED BY STD. DWG. 876.02 FOR AN 18" PIPE. TRANSITIONS FROM THE DITCH TO TERMINATION SHALL BE AS DIRECTED BY THE ENGINEER.
- MODIFICATIONS SHALL BE AS DICTATED BY SITE CONDITIONS AND DIRECTED BY THE ENGINEER. RIP-RAP LINED DITCH

PROJECT SERVICES UNIT STANDARDS AND SPECIAL DESIGN Office 919-250-4128 FAX 919-250-4119

SHEET 1 OF 1

MODFLMDTCH

SEE PLATE FOR TITLE

ORIGINAL BY: E.E. Ward DATE	₿:.	Apr.	2002
MODIFIED BY E.E. Ward DATE	8:]	July	2004
CHECKED BY: DATE	B:]		
FILE SPEC.:			

SHEET 1 OF 1

MODFLMDTCH