

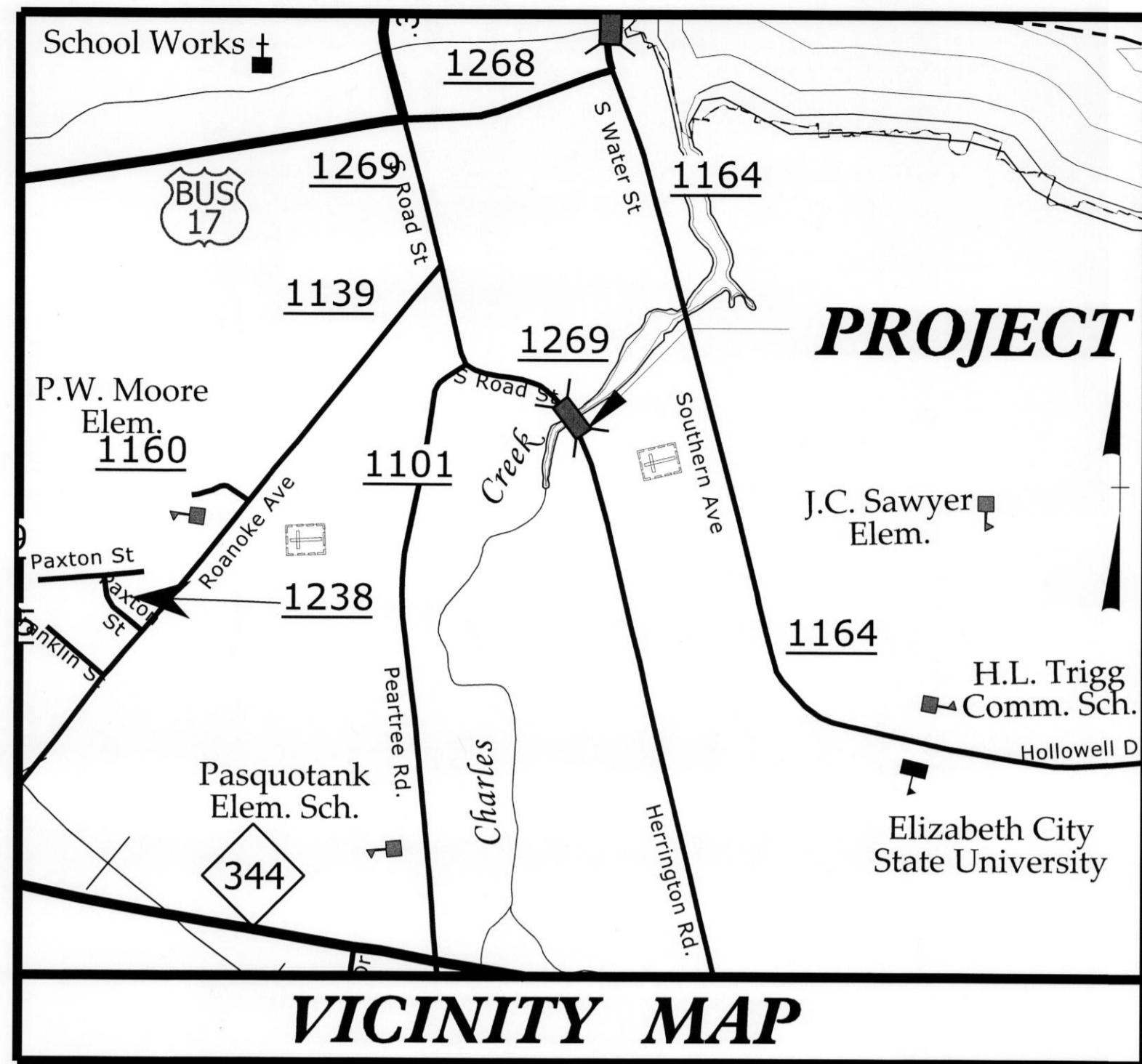
09-08-13

See Sheet 1-A For Index of Sheets
See Sheet 1-B For Conventional Symbols

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BD-5101N	1	
STATE PROJ. NO.	F.A. PROJ. NO.	DESCRIPTION	
45347.1.15	BRSTP-1269(2)	P.E.	

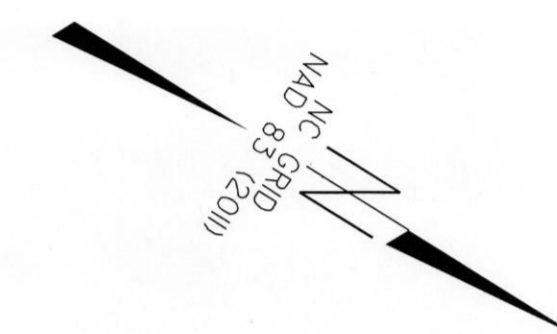
TIP PROJECT: BD-5101N



PASQUOTANK COUNTY

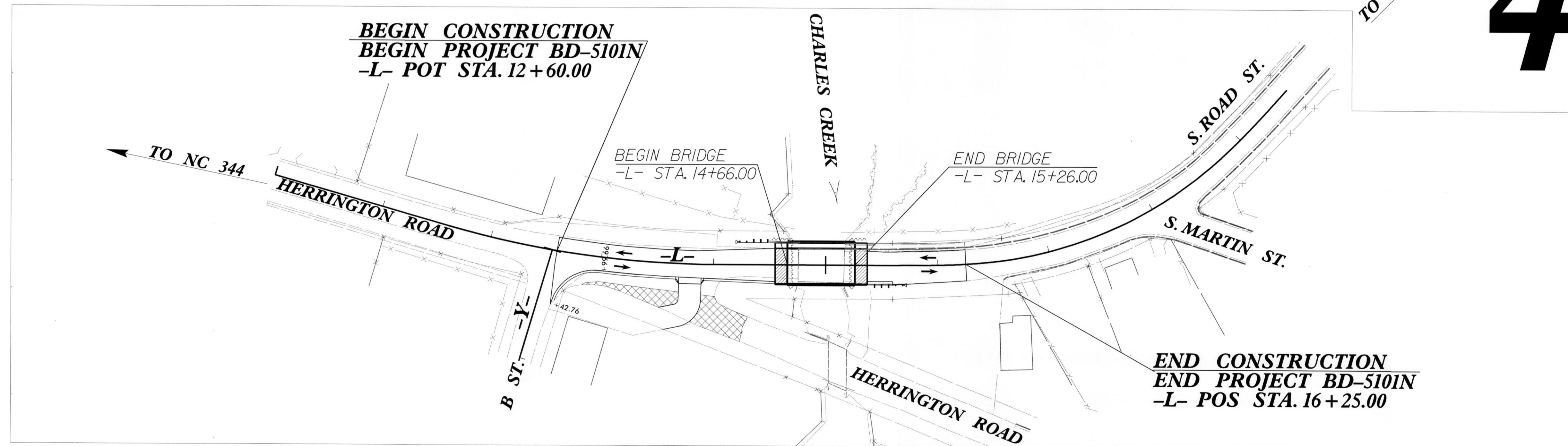
LOCATION: BRIDGE NO. 18 OVER CHARLES CREEK ON NC 34

TYPE OF WORK: GRADING, DRAINAGE, PAVING,
AND STRUCTURES

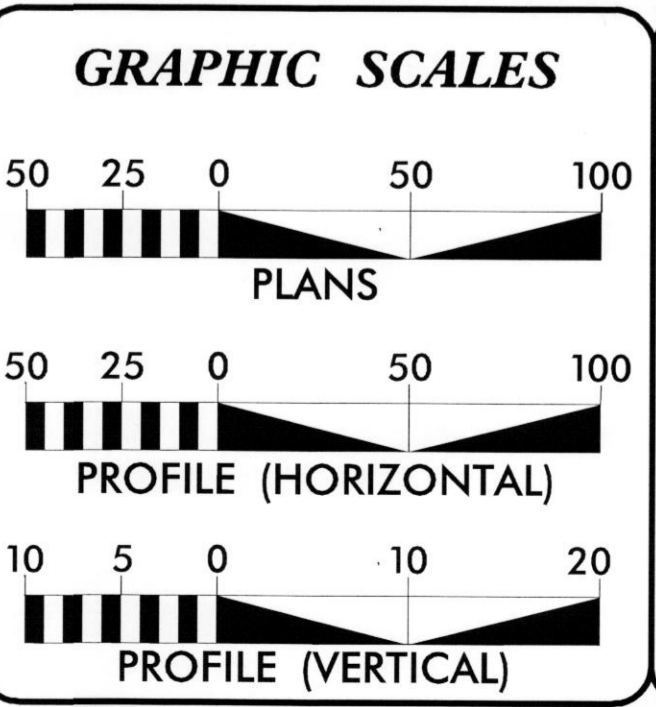


TO BUSINESS 17
4

CONTRACT: DA00198



CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II MODIFIED. FIVE FEET BEYOND SLOPE LIMITS TO BE HAND CLEARED. THIS PROJECT IS WITHIN ELIZABETH CITY MUNICIPAL BOUNDARIES.



PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT BD-5101N	=	0.058 MILES
LENGTH STRUCTURE TIP PROJECT BD-5101N	=	0.011 MILES
TOTAL LENGTH TIP PROJECT BD-5101N	=	0.069 MILES

Prepared in the Office of:
CDM Smith
5400 Glenwood Avenue, Suite 300, Raleigh, NC 27612

2012 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: AUGUST 1, 2013

LETTING DATE: JUNE 18, 2014

DOUGLAS B. SAUNDERS, P.E.
PROJECT ENGINEER

DAVID J. CLOGDO, PE
PROJECT DESIGN ENGINEER

HYDRAULICS ENGINEER

Melanie Q.C. Nguyen
SIGNATURE: MELANIE Q.C. NGUYEN
5/20/14

ROADWAY DESIGN ENGINEER

Douglas B. Saunders
SIGNATURE: DOUGLAS B. SAUNDERS
5/20/14

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA

5/20/2014 11:08:51 AM
N:\Proj\BD-5101N\RDY_TSH_01.dgn
USER: TAYLORJB

INDEX OF SHEETS

SHEET NUMBER	TITLE
1	TITLE SHEET
1-A	INDEX OF SHEETS, GENERAL NOTES AND LIST OF STANDARDS
1-B	CONVENTIONAL SYMBOLS
2	TYPICAL SECTIONS AND PAVEMENT SCHEDULE
2-A THRU 2-B	STRUCTURE ANCHOR UNITS DETAIL
3-A	SUMMARY OF DRAINAGE QUANTITIES, SUMMARY OF GUARDRAIL, EARTHWORK SUMMARY
4	PLAN AND PROFILE SHEET
TMP-1	TRAFFIC MANAGEMENT PLAN
EC-1 THRU EC-3	EROSION CONTROL SHEETS
X-1	CROSS SECTION SUMMARY SHEET
X-2 THRU X-4	CROSS SECTIONS
S-1 THRU S-24	STRUCTURE PLANS

GENERAL NOTES

2012 SPECIFICATIONS
EFFECTIVE: 01-17-2012
REVISED: 07-30-2012

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 AND/OR STD. NO. 225.05 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01 AND/OR STD. NO. 560.02

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

DRIVEWAYS:

DRIVEWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 848.02 USING 3' RADIUS OR RADII AS SHOWN ON THE PLANS. LOCATIONS OF DRIVES WILL BE AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.

STREET TURNOUT:

STREET RETURNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 848.04 USING THE RADII NOTED ON PLANS.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATION AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

CENTURYLINK - TELEPHONE

CITY OF ELIZABETH CITY - WATER

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

STANDARD DRAWINGS

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

STD. NO.	TITLE
DIVISION 2 - EARTHWORK	
200.02	Method of Clearing - Method II
225.02	Guide for Grading Subgrade - Secondary and Local
225.04	Method of Obtaining Super-elevation - Two Lane Pavement
DIVISION 3 - PIPE CULVERTS	
300.01	Method of Pipe Installation
DIVISION 4 - MAJOR STRUCTURES	
422.11	Reinforced Bridge Approach Fills - Sub Regional Tier
DIVISION 5 - SURGRADE, BASES AND SHOULDERS	
560.01	Method of Shoulder Construction - High Side of Super-elevated Curve - Method I
DIVISION 6 - ASPHALT BASES AND PAVEMENTS	
654.01	Pavement Repairs
DIVISION 8 - INCIDENTALS	
840.01	Brick Catch Basin - 12" thru 54" Pipe
840.02	Concrete Catch Basin - 12" thru 54" Pipe
840.03	Frame, Grates and Hood - for Use on Standard Catch Basin
840.36	Traffic Bearing Grated Drop Inlet - for Steel (840.37) Double Frame and Grates
840.37	Steel Grate and Frame
840.46	Traffic Bearing Precast Drainage Structure
840.66	Drainage Structure Steps
846.01	Concrete Curb, Gutter and Curb & Gutter
846.04	Drop Inlet Installation in Shoulder Berm Gutter
848.01	Concrete Sidewalk
848.03	Driveway Turnout - Drop Curb Type
848.04	Street Turnout
862.01	Guardrail Placement
862.02	Guardrail Installation
862.03	Structure Anchor Units (Beg. March 2013 Letting use detail in lieu of Standard)
DIVISION 11 - WORK ZONE TRAFFIC CONTROL	
1101.01	Work Zone Advance Warning Signs
1101.02	Temporary Lane Closures
1101.03	Temporary Road Closures
1101.11	Traffic Control Design Tables
1110.01	Stationary Work Zone Signs
1130.01	Drum
1135.01	Cones
1145.01	Barricades
1150.01	Flagging Devices

EFF. 01-17-2012
REV. 10-30-2012

PROJECT REFERENCE NO. BD-5101N	SHEET NO. 1-A
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

Note: Not to Scale

*S.U.E. = Subsurface Utility Engineering

CONVENTIONAL PLAN SHEET SYMBOLS

BOUNDARIES AND PROPERTY:

State Line	-----
County Line	-----
Township Line	-----
City Line	-----
Reservation Line	-----
Property Line	-----
Existing Iron Pin	○ EIP
Property Corner	----- X
Property Monument	□ ECM
Parcel/Sequence Number	⑫③
Existing Fence Line	-X-X-X-
Proposed Woven Wire Fence	○
Proposed Chain Link Fence	□
Proposed Barbed Wire Fence	◇
Existing Wetland Boundary	--- WLB ---
Proposed Wetland Boundary	--- WLB ---
Existing Endangered Animal Boundary	--- EAB ---
Existing Endangered Plant Boundary	--- EPB ---
Known Soil Contamination: Area or Site	☠ ☠
Potential Soil Contamination: Area or Site	☠ ?

BUILDINGS AND OTHER CULTURE:

Gas Pump Vent or U/G Tank Cap	○
Sign	○ S
Well	○ W
Small Mine	✕
Foundation	□
Area Outline	□
Cemetery	□ †
Building	□
School	□
Church	□
Dam	□

HYDROLOGY:

Stream or Body of Water	-----
Hydro, Pool or Reservoir	□
Jurisdictional Stream	--- JS ---
Buffer Zone 1	--- BZ 1 ---
Buffer Zone 2	--- BZ 2 ---
Flow Arrow	←
Disappearing Stream	→
Spring	○
Wetland	---
Proposed Lateral, Tail, Head Ditch	--- FLOW ---
False Sump	▽

RAILROADS:

Standard Gauge	-----
RR Signal Milepost	○ CSX TRANSPORTATION MILEPOST 35
Switch	□ SWITCH
RR Abandoned	-----
RR Dismantled	-----

RIGHT OF WAY:

Baseline Control Point	△
Existing Right of Way Marker	△
Existing Right of Way Line	-----
Proposed Right of Way Line	○ R W
Proposed Right of Way Line with Iron Pin and Cap Marker	○ R W ▲
Proposed Right of Way Line with Concrete or Granite RW Marker	▲ R W
Proposed Control of Access Line with Concrete CA Marker	▲ C A
Existing Control of Access	○ C A
Proposed Control of Access	○ C A
Existing Easement Line	--- E ---
Proposed Temporary Construction Easement	--- E ---
Proposed Temporary Drainage Easement	--- TDE ---
Proposed Permanent Drainage Easement	--- PDE ---
Proposed Permanent Drainage / Utility Easement	--- DUE ---
Proposed Permanent Utility Easement	--- PUE ---
Proposed Temporary Utility Easement	--- TUE ---
Proposed Aerial Utility Easement	--- AUE ---
Proposed Permanent Easement with Iron Pin and Cap Marker	▲

ROADS AND RELATED FEATURES:

Existing Edge of Pavement	-----
Existing Curb	-----
Proposed Slope Stakes Cut	--- C ---
Proposed Slope Stakes Fill	--- F ---
Proposed Curb Ramp	○ CR
Existing Metal Guardrail	--- T T T T ---
Proposed Guardrail	--- T T T T ---
Existing Cable Guiderail	--- T T T T ---
Proposed Cable Guiderail	--- T T T T ---
Equality Symbol	⊕
Pavement Removal	□

VEGETATION:

Single Tree	○
Single Shrub	○
Hedge	-----
Woods Line	-----

Orchard	○ ○ ○ ○
Vineyard	□ Vineyard

EXISTING STRUCTURES:

MAJOR:	
Bridge, Tunnel or Box Culvert	--- CONC ---
Bridge Wing Wall, Head Wall and End Wall	--- CONC WW ---
MINOR:	
Head and End Wall	--- CONC HW ---
Pipe Culvert	---
Footbridge	---
Drainage Box: Catch Basin, DI or JB	□ CB
Paved Ditch Gutter	---
Storm Sewer Manhole	○ S
Storm Sewer	--- S ---

UTILITIES:

POWER:	
Existing Power Pole	●
Proposed Power Pole	○
Existing Joint Use Pole	●
Proposed Joint Use Pole	○
Power Manhole	○ P
Power Line Tower	□
Power Transformer	□
U/G Power Cable Hand Hole	□
H-Frame Pole	●
Recorded U/G Power Line	--- P ---
Designated U/G Power Line (S.U.E.*)	--- P ---

TELEPHONE:

Existing Telephone Pole	●
Proposed Telephone Pole	○
Telephone Manhole	○ T
Telephone Booth	□
Telephone Pedestal	□
Telephone Cell Tower	●
U/G Telephone Cable Hand Hole	□
Recorded U/G Telephone Cable	--- T ---
Designated U/G Telephone Cable (S.U.E.*)	--- T ---
Recorded U/G Telephone Conduit	--- TC ---
Designated U/G Telephone Conduit (S.U.E.*)	--- TC ---
Recorded U/G Fiber Optics Cable	--- T FO ---
Designated U/G Fiber Optics Cable (S.U.E.*)	--- T FO ---

WATER:

Water Manhole	○ W
Water Meter	○
Water Valve	⊗
Water Hydrant	○
Recorded U/G Water Line	--- W ---
Designated U/G Water Line (S.U.E.*)	--- W ---
Above Ground Water Line	--- A/G Water ---

TV:

TV Satellite Dish	○
TV Pedestal	□
TV Tower	⊗
U/G TV Cable Hand Hole	□
Recorded U/G TV Cable	--- TV ---
Designated U/G TV Cable (S.U.E.*)	--- TV ---
Recorded U/G Fiber Optic Cable	--- TV FO ---
Designated U/G Fiber Optic Cable (S.U.E.*)	--- TV FO ---

GAS:

Gas Valve	◇
Gas Meter	○
Recorded U/G Gas Line	--- G ---
Designated U/G Gas Line (S.U.E.*)	--- G ---
Above Ground Gas Line	--- A/G Gas ---

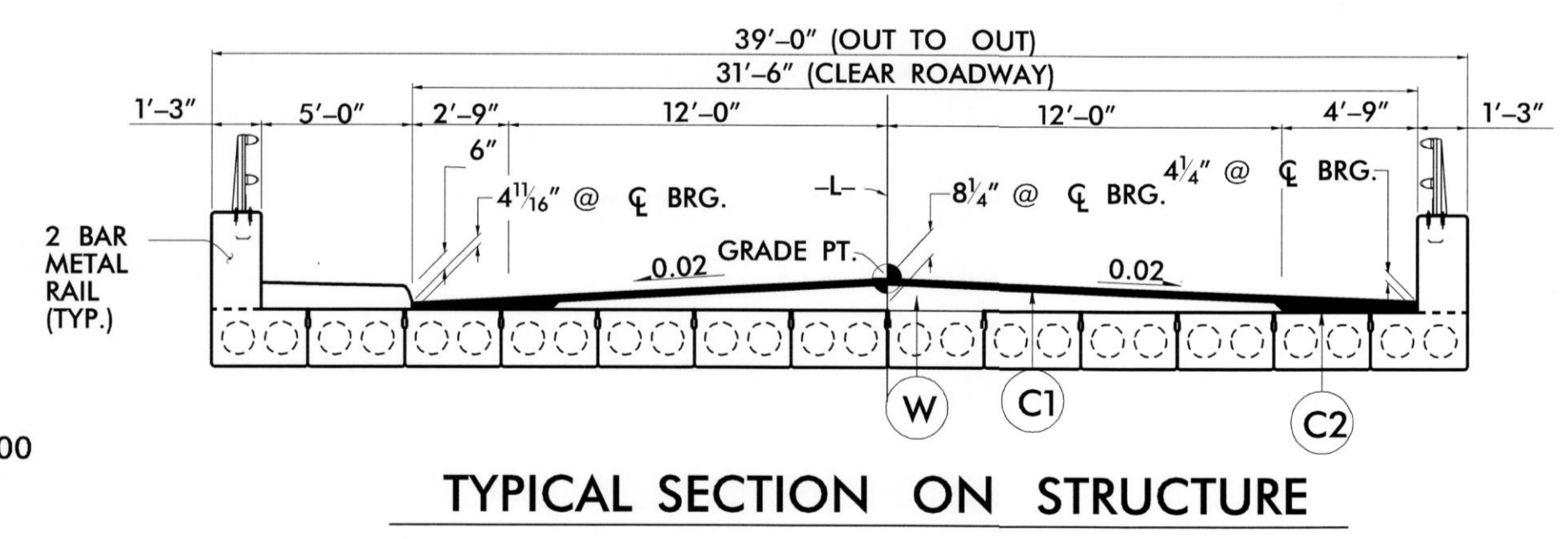
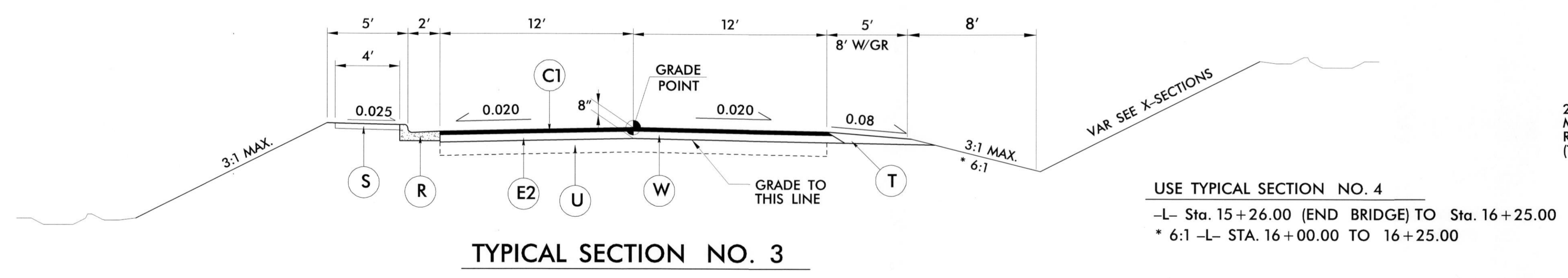
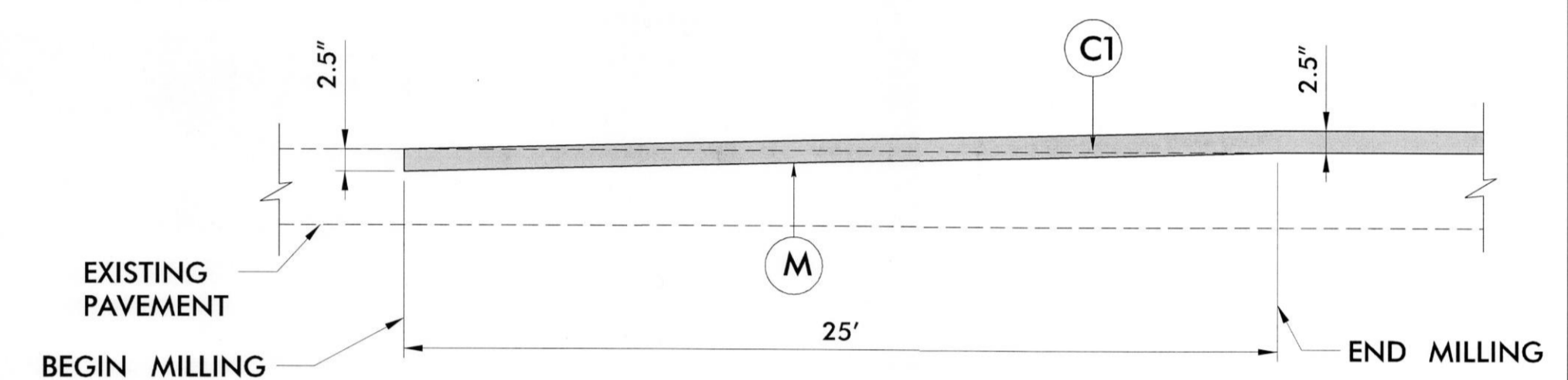
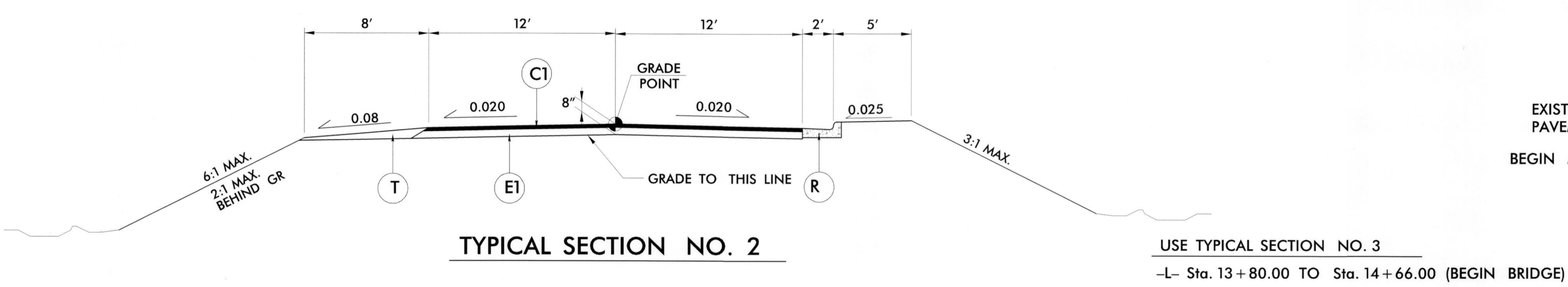
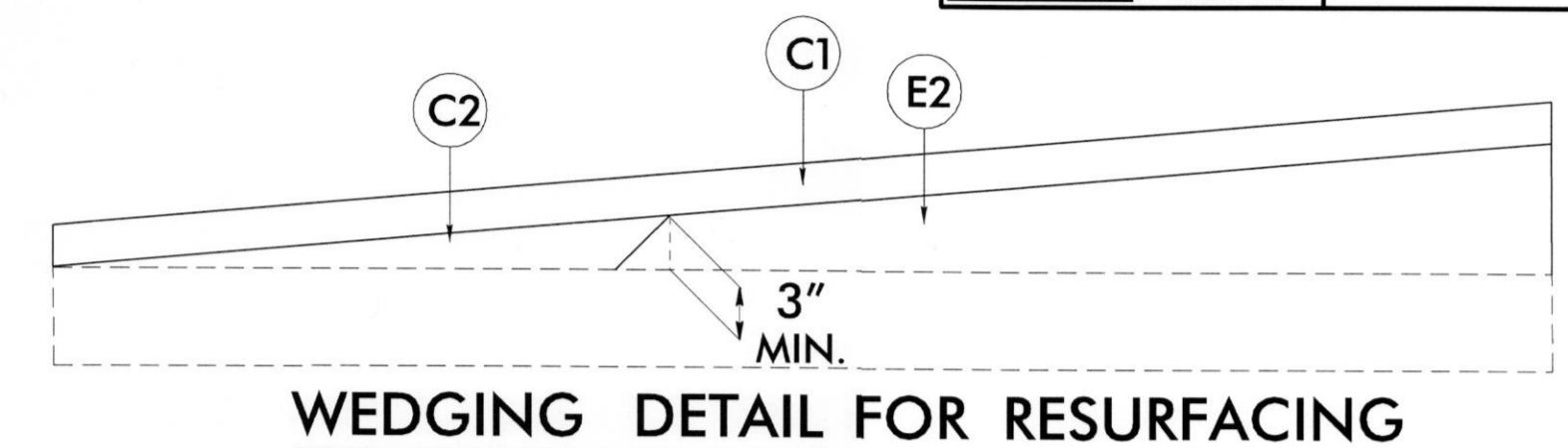
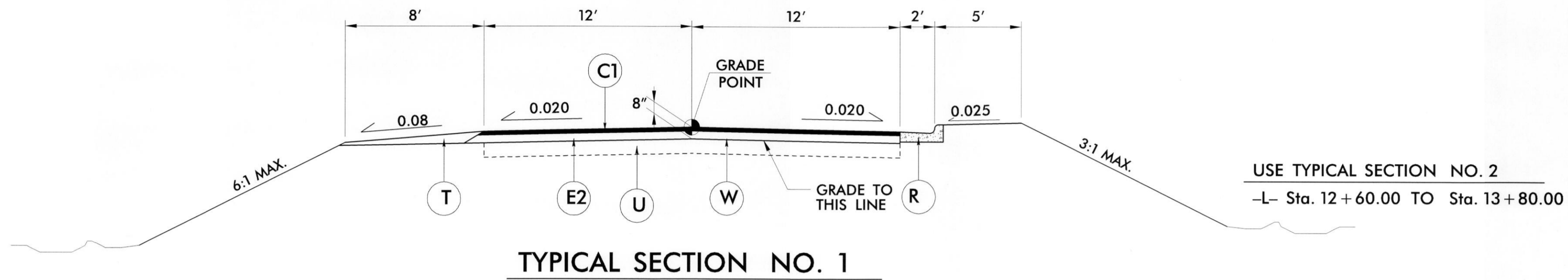
SANITARY SEWER:

Sanitary Sewer Manhole	⊕
Sanitary Sewer Cleanout	⊕
U/G Sanitary Sewer Line	--- SS ---
Above Ground Sanitary Sewer	--- A/G Sanitary Sewer ---
Recorded SS Forced Main Line	--- FSS ---
Designated SS Forced Main Line (S.U.E.*)	--- FSS ---

MISCELLANEOUS:

Utility Pole	●
Utility Pole with Base	□
Utility Located Object	○
Utility Traffic Signal Box	□
Utility Unknown U/G Line	--- ZUTL ---
U/G Tank; Water, Gas, Oil	□
Underground Storage Tank, Approx. Loc.	□ UST
A/G Tank; Water, Gas, Oil	□
Geoenvironmental Boring	●
U/G Test Hole (S.U.E.*)	●
Abandoned According to Utility Records	AATUR
End of Information	E.O.I.

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PAVEMENT SCHEDULE

ITEM	DESCRIPTION	ITEM	DESCRIPTION	ITEM	DESCRIPTION
C1	PROPOSED APPROXIMATE 2.5 IN. ASPHALT SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 137.50 LBS/SY.	S	4 IN. CONCRETE SIDEWALK		
C2	PROP. VAR. DEPTH ASPHALT CONC. SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 110 LBS/SY/IN. IN LIFTS NOT LESS THAN 1.0 IN. NOR GREATER THAN 1.5 IN.	T	EARTH MATERIAL		
E1	PROP. APPROX. 5.5 IN. ASPHALT BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 627 LBS/SY.	M	MILLING (SEE MILLING DETAIL ABOVE)		
E2	PROP. VAR. DEPTH ASPHALT BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS/SY/IN IN LIFTS NOT LESS THAN 3 IN. NOR GREATER THAN 5.5 IN.	U	EXISTING PAVEMENT		
R	2'-6" CURB & GUTTER	W	WEDGING (SEE WEDGE DETAIL ABOVE)		

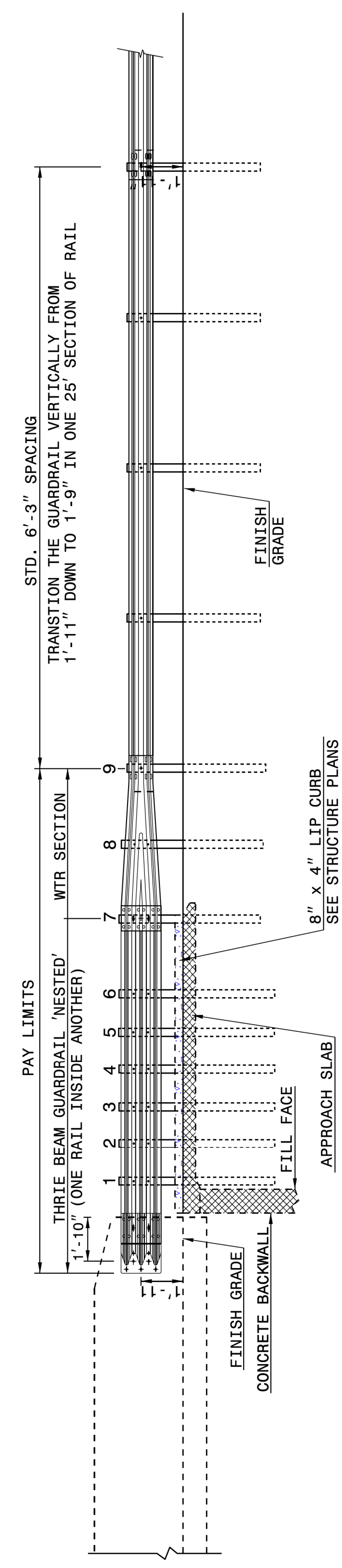
NOTES :
PAVEMENT EDGES ARE 1:1 UNLESS OTHERWISE NOTED.

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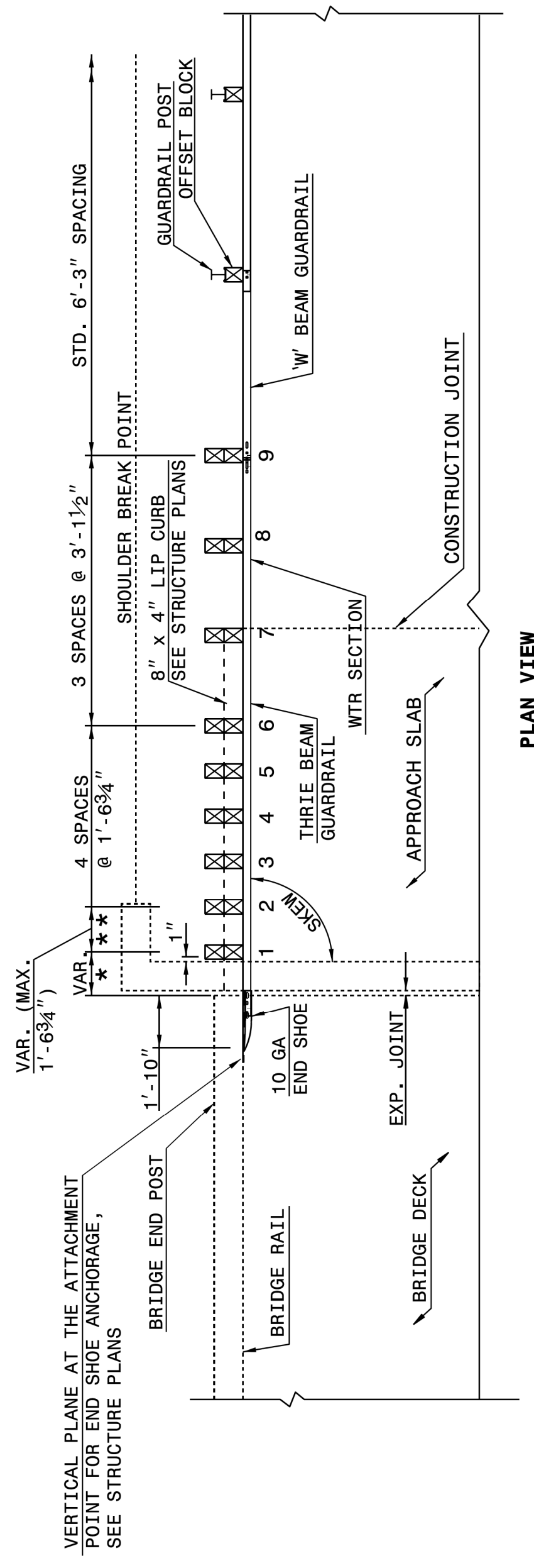
STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO
RAIL ON BRIDGE - SUB REGIONAL TIER

SHEET 2 OF 7
862d03



NOTE:
 **POST NOT REQUIRED FOR SKEW ANGLES GREATER THAN 150° OR LESS THAN 30° UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
 *THE DISTANCE FROM END OF BRIDGE RAIL TO CENTER LINE OF THE FIRST POST SHOULD BE 11½" IF CONCRETE BACKWALL IS NOT PRESENT.
 -SHOULDER BERM GUTTER MUST BE INSTALLED TO THE LIMITS 8" x 4" LIP CURB IS SHOWN IF ANCHOR UNIT IS NOT ADJACENT TO AN APPROACH SLAB.
 -MEASURE GUARDRAIL HEIGHT FROM THE TOP OF ADJACENT SURFACE (SHOULDER, BERM, OR GUTTER).
 -LAP JOINTS IN THE DIRECTION OF TRAFFIC FLOW.
 -SEE SHEET 5 FOR POST SECTIONS 1 THRU 9.



**GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO
RAIL ON BRIDGE - SUB REGIONAL TIER**

STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
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RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III FOR ATTACHMENT TO
RAIL ON BRIDGE - SUB REGIONAL TIER

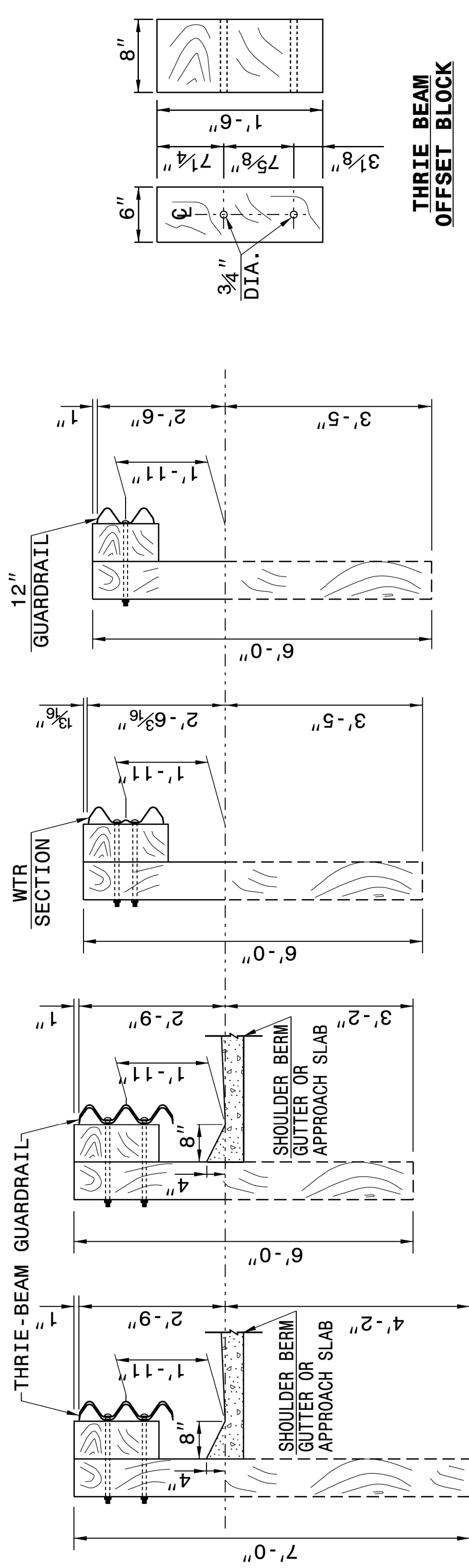
SHEET 2 OF 7
862d03

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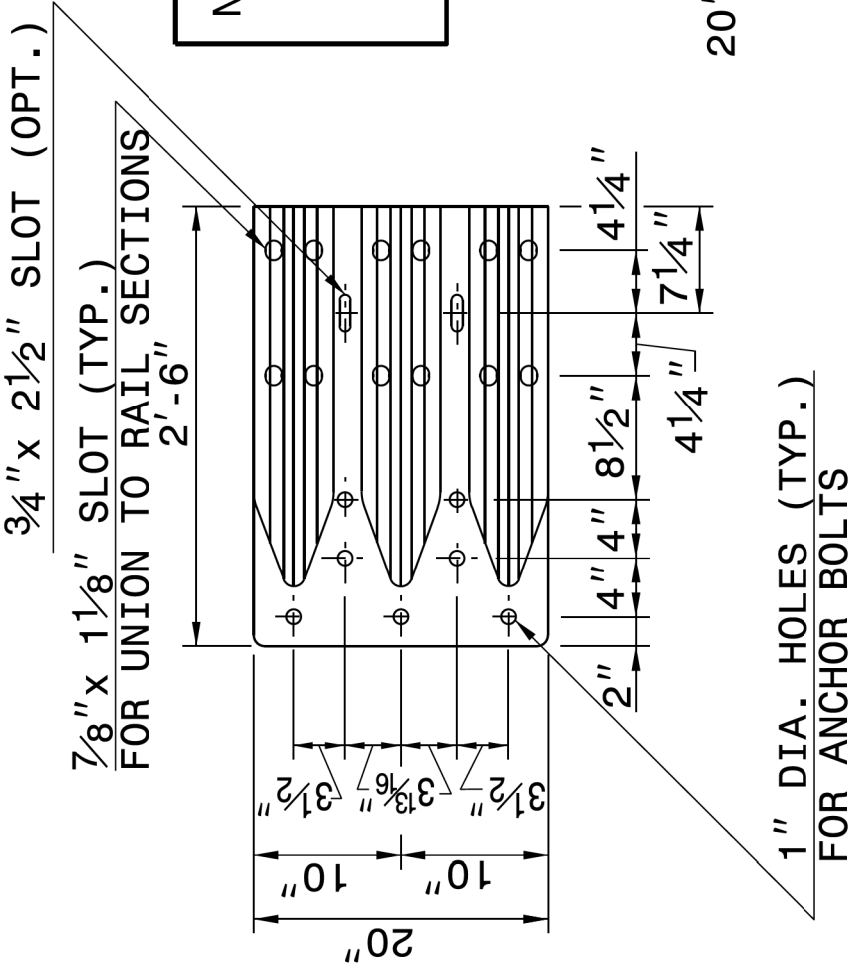
STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III

SHEET 3 OF 7
862d03



NOTE: THE MID POST AND OFFSET BLOCK OF THE WTR SECTION WILL REQUIRE SPECIAL BOLT HOLE DRILLING IN THE THRIE BEAM OFFSET BLOCK AND LINE POST.



STATE OF NORTH CAROLINA
DEPT. OF TRANSPORTATION
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RALEIGH, N.C.

ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNITS
GUARDRAIL ANCHOR UNIT, TYPE III

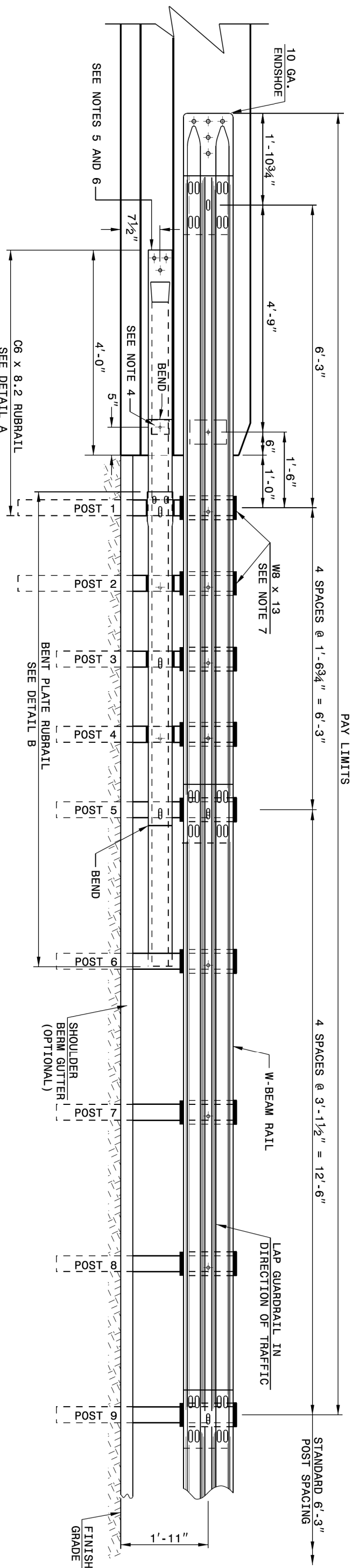
SHEET 3 OF 7
862d03

**CONTRACT STANDARDS
AND DEVELOPMENT UNIT**
Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

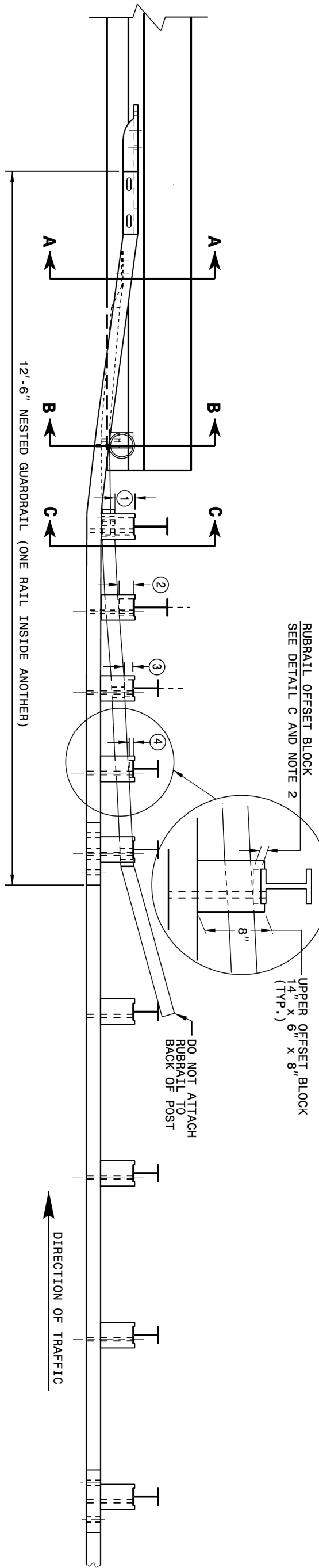
ORIGINAL BY: J HOWERTON	DATE: 06-22-12
MODIFIED BY:	DATE:
CHECKED BY:	DATE:
FILE SPEC.:	

STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.



ELEVATION

- GENERAL NOTES:
- POSTS 1 THROUGH 5 REQUIRE AN ADDITIONAL HOLE TO ATTACH LOWER BLOCKOUTS AND/OR RUBRAIL.
 - RUBRAIL BLOCKOUTS LOCATED ON POSTS 1 THROUGH 4 ARE OFFSET DRILLED AND SECURED WITH 3/8" BUTTONEHEAD BOLTS (SEE CHART FOR BOLT WITH A 5/8" x 3 1/2" BUTTONEHEAD BOLT RUBRAIL IS FLARED TO BACK OF POST 6 AND NOT SECURED TO POST 5
 - STEEL SPACER TUBE IS A SCHEDULE 40 GALVANIZED PIPE 6" INSIDE DIAMETER x 9' LONG. ATTACH TUBE TO GUARDRAIL ONLY WITH 5/8" x 1 1/4" LONG BUTTONEHEAD BOLT AND RECTANGULAR PLATE WASHER.
 - SEE DETAIL D FOR SLOPED RUBRAIL BLOCKOUT. BLOCKOUT IS ATTACHED TO RAIL ELEMENT ONLY. USE 3/8" x 3" LAG BOLT WITH FLAT WASHER.
 - SEE DETAIL E FOR SHOULDER BERM GUTTER OR SHOULDER BERM ON BRIDGE RAIL.
 - ANCHORAGE: (a) AT EXISTING BRIDGE RAIL AND NEW OR EXISTING BARRIERS, ANCHOR RUBRAIL USING THREE 5/8" x 6" CHEMICALLY ANCHORED BOLTS WITH WASHERS. MAXIMUM PROJECTION FOR BOLTS IS 1 1/2". (b) AT EXISTING BRIDGE RAIL AND NEW OR EXISTING BARRIERS, ANCHOR THE W-BEAM END SHOE USING A 4 BOLT HOLD DOWN PLATE (SEE STD. DWG. 862.04). (c) AT NEW BRIDGE RAIL, ANCHOR THE W-BEAM END SHOE AND RUBRAIL AS DETAILED ON THE STRUCTURE PLANS.
 - POSTS 1 AND 2 ARE W8 x 13, 7'-6" LONG. ALL OTHER POSTS IN THE ANCHOR UNIT ARE W8 x 8.5.

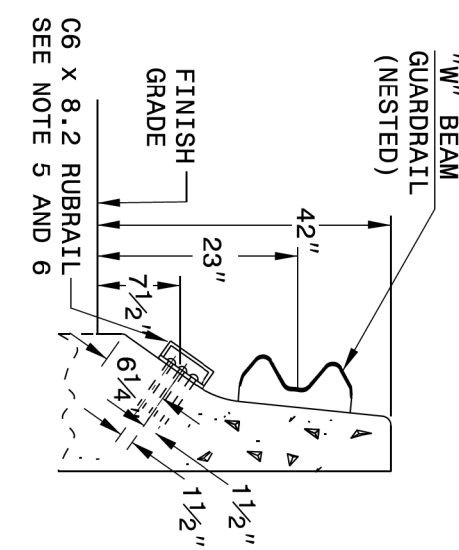


PLAN

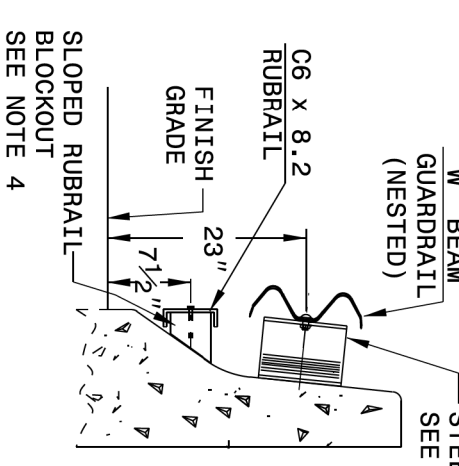
ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNIT
GUARDRAIL ANCHOR UNIT TYPE B-77
FOR F-SHAPE BARRIER

SHEET 4 OF 7
862D03

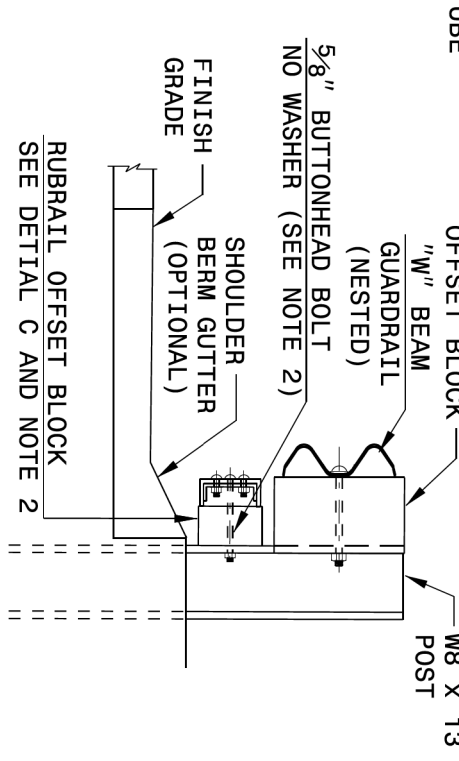
STATE OF
NORTH CAROLINA
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RALEIGH, N.C.



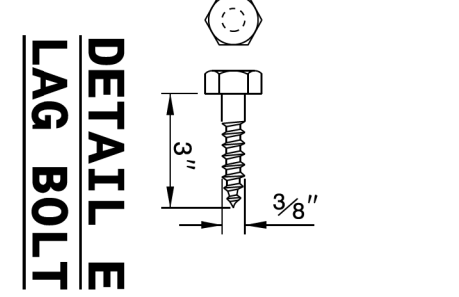
SECTION A-A



SECTION B-B



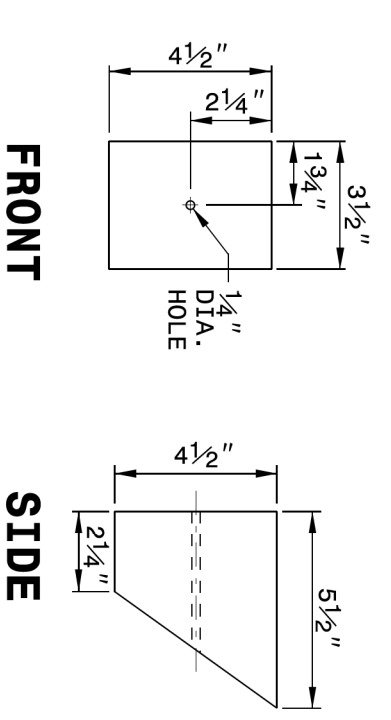
SECTION C-C



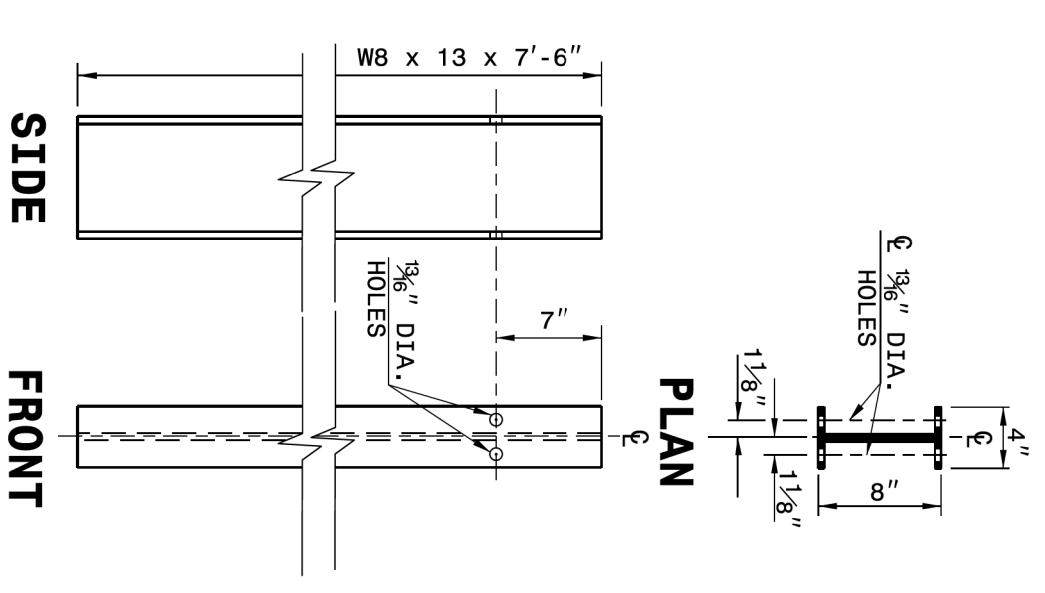
DETAIL E
LAG BOLT

POST	THICKNESS	BOLT LENGTH
(1)	4 1/4"	9"
(2)	3 1/4"	5"
(3)	2"	6"
(4)	1"	3"

* BOLTS FOR POSTS 2 AND 4 ARE USED TO ATTACH TO BLOCK.



DETAIL D
SLOPED RUBRAIL BLOCKOUT

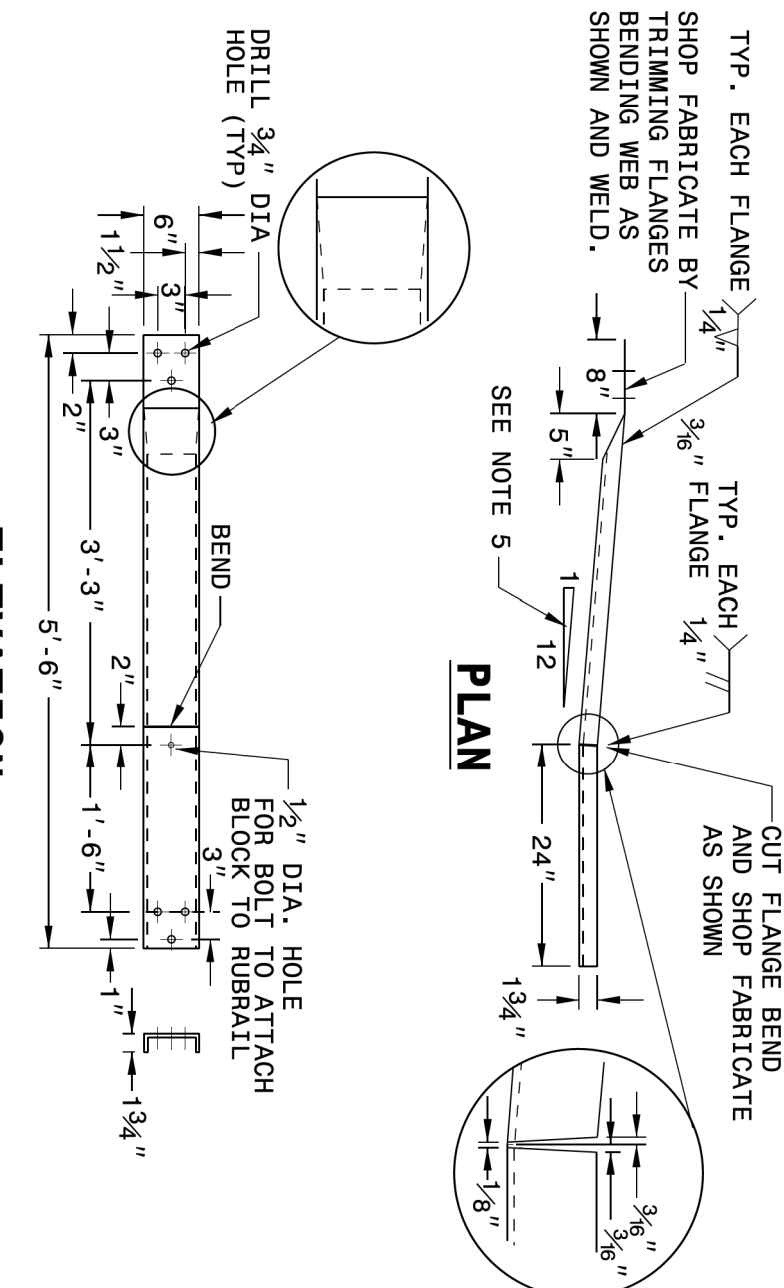


DETAIL F
W8 X 13 X 7'-6"

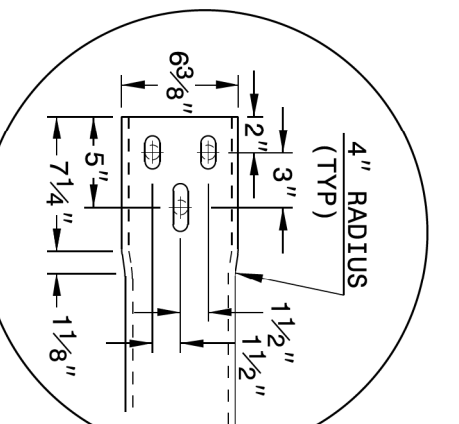
STATE OF
NORTH CAROLINA
DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, N.C.

SHEET 4 OF 7
862D03

ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNIT
FOR F-SHAPE BARRIER

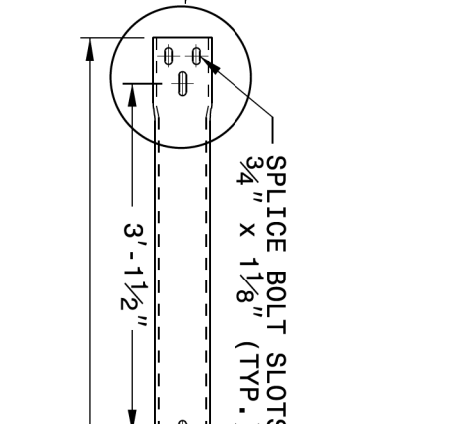


DETAIL C
RUBRAIL BLOCKOUT



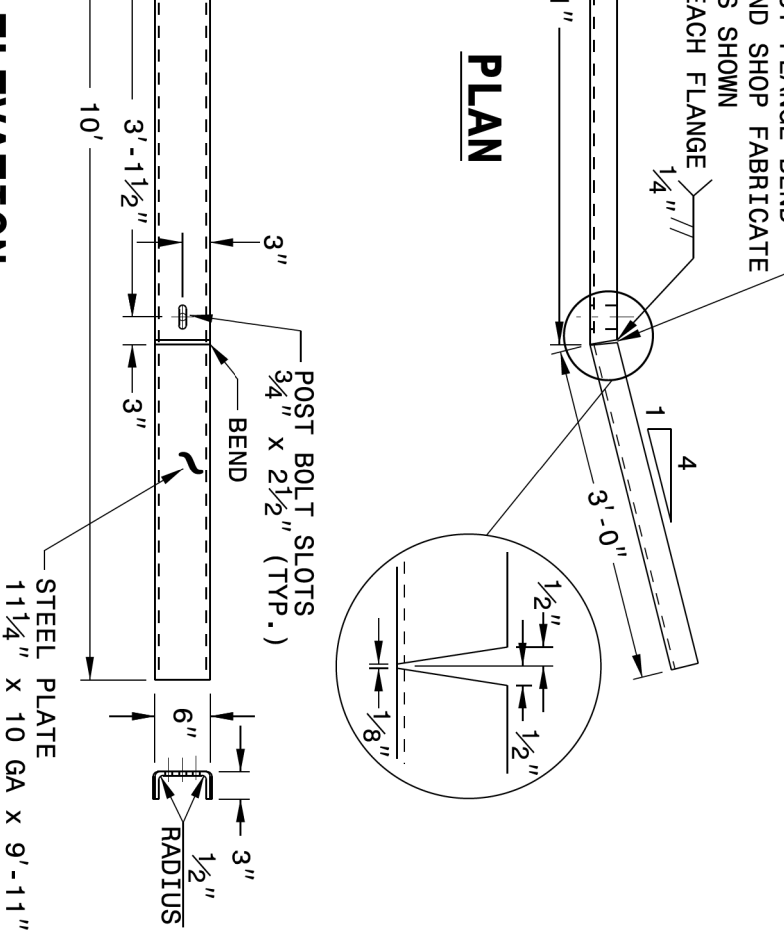
ELEVATION

DETAIL A
C6 X 8.2 RUBRAIL



ELEVATION

DETAIL B
BENT PLATE RUBRAIL



PLAN

ENGLISH DETAIL DRAWING FOR
STRUCTURE ANCHOR UNIT
GUARDRAIL ANCHOR UNIT TYPE B-77
FOR F-SHAPE BARRIER

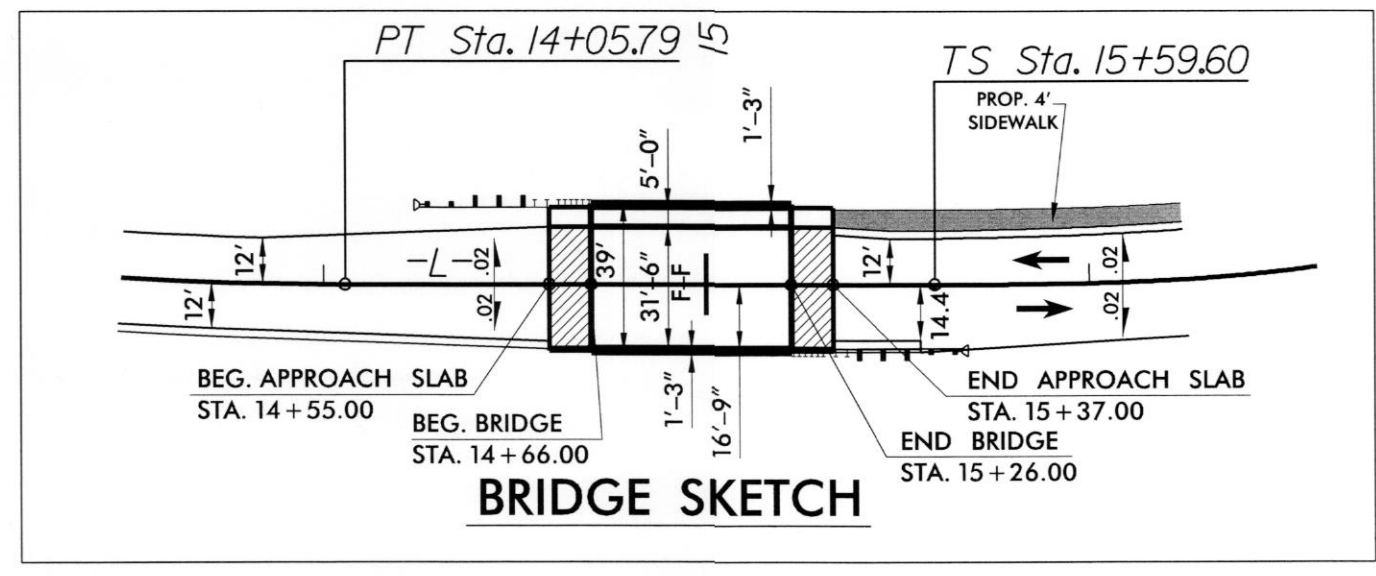
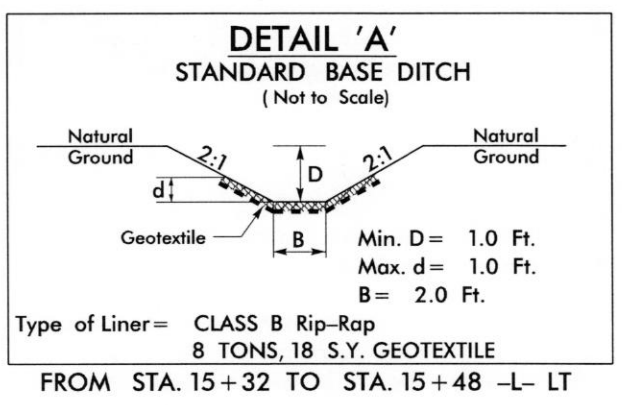
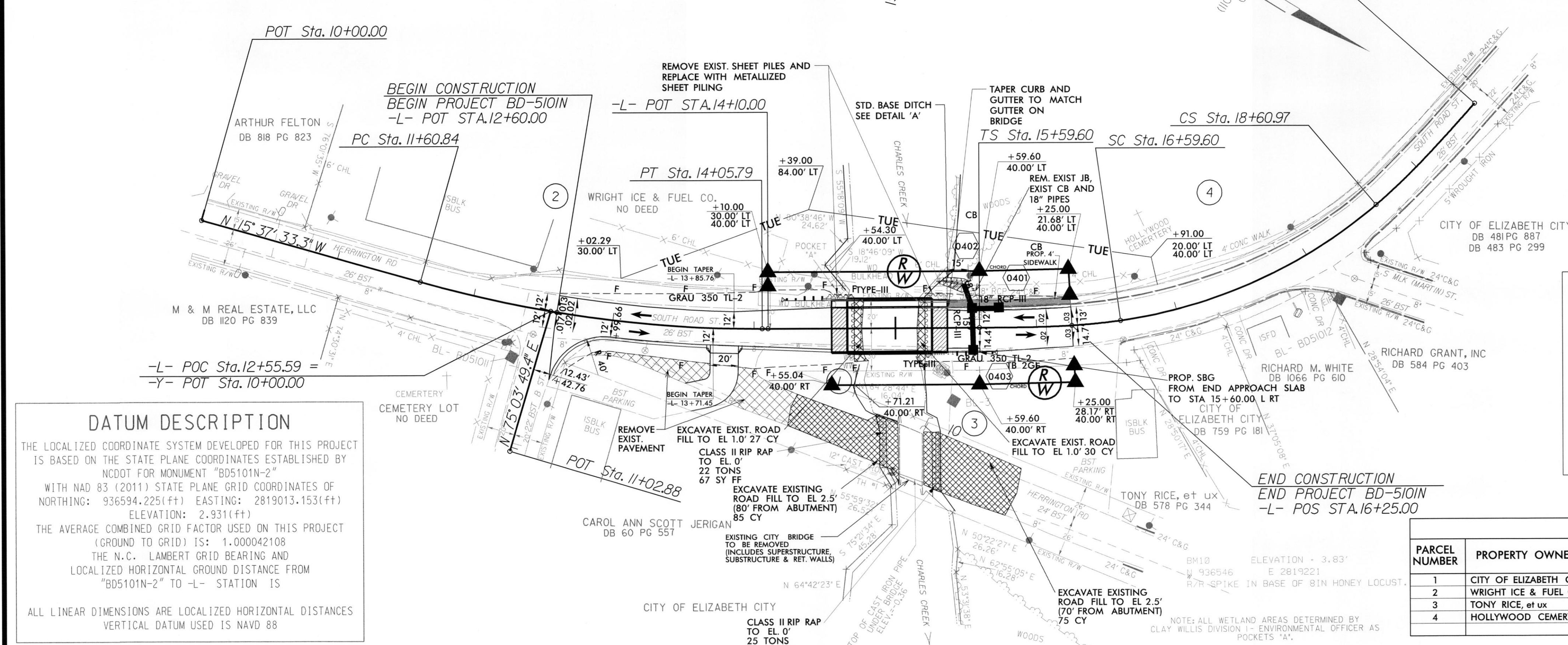
SHEET 5 OF 7
862D03

CONTRACT STANDARDS AND DEVELOPMENT UNIT
Office 919-707-6950 FAX 919-250-4119

SEE TITLE BLOCK

ORIGINAL BY: J HOWERTON DATE: 06-22-12
MODIFIED BY: DATE:
CHECKED BY: DATE:
FILE SPEC.: DATE:

PI Sta 12+84.11 Δ = 15°55'20.0" (LT) D = 6'30"00.0" L = 244.96' T = 123.27' R = 881.47'
 PI Sta 16+26.34 Δs = 8°00'00.0" Ls = 100.00' Ts = 66.73' ST = 33.40'
 PI Sta 18+94.36 Δs = 8°00'00.0" Ls = 100.00' Ts = 66.73' ST = 33.40'
 PI Sta 17+63.03 Δ = 32°13'06.0" (LT) D = 16'00"00.0" L = 201.36' T = 103.42' R = 358.10'



DATUM DESCRIPTION
 THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "BD5101N-2" WITH NAD 83 (2011) STATE PLANE GRID COORDINATES OF NORTHING: 936594.225(FT) EASTING: 2819013.153(FT) ELEVATION: 2.931(FT)
 THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 1.000042108
 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL DISTANCE FROM "BD5101N-2" TO -L- STATION IS
 ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES VERTICAL DATUM USED IS NAVD 88

RIGHT OF WAY AREA DATA

PARCEL NUMBER	PROPERTY OWNERS NAMES	TOTAL AREA	AREA TAKEN	AREA REMAINING	CONST. EASEMENT	PERMANENT DRAINAGE EASEMENT	TEMPORARY UTILITY EASEMENT
1	CITY OF ELIZABETH CITY	1,963.89 SF	297.62 SF	1,666.27 SF			
2	WRIGHT ICE & FUEL CO.	1,649 Ac.	485.03 SF	1,638 Ac.			4,428.69 SF
3	TONY RICE, et ux	0.453 Ac.	1,614.60 SF	0.416 Ac.			
4	HOLLYWOOD CEMETERY	21,724 Ac.	3,156.77 SF	21,651 Ac.			6,413.64 SF

BRIDGE HYDRAULIC DATA

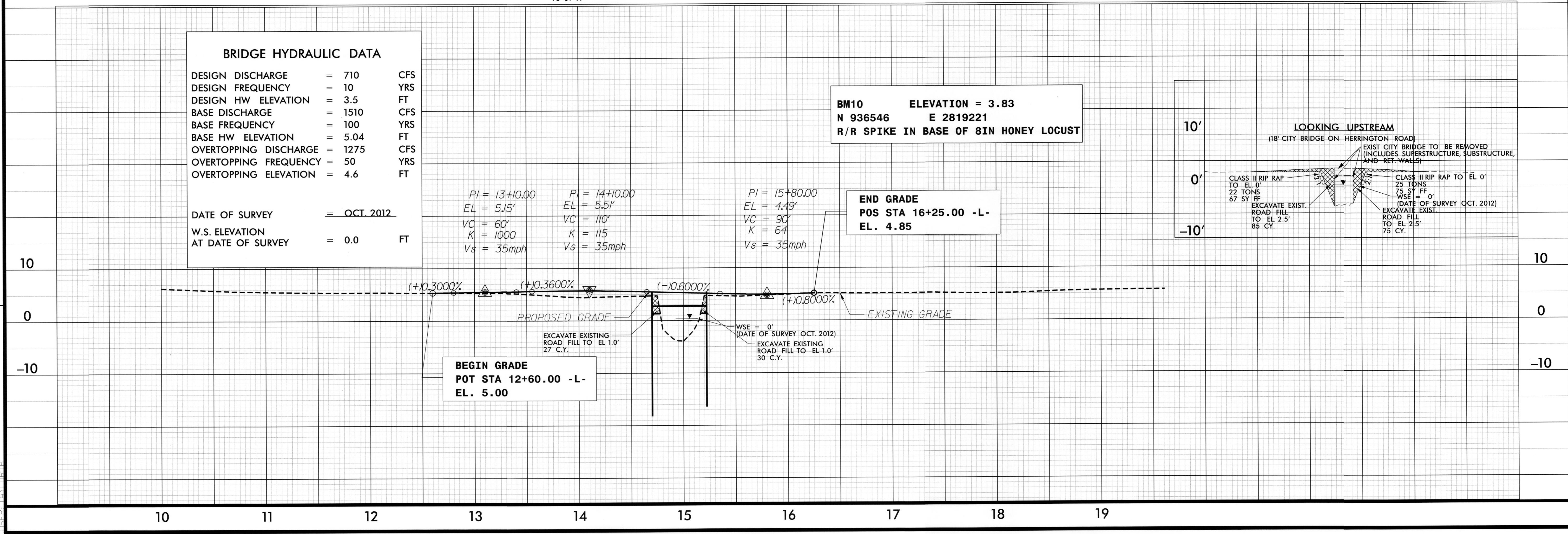
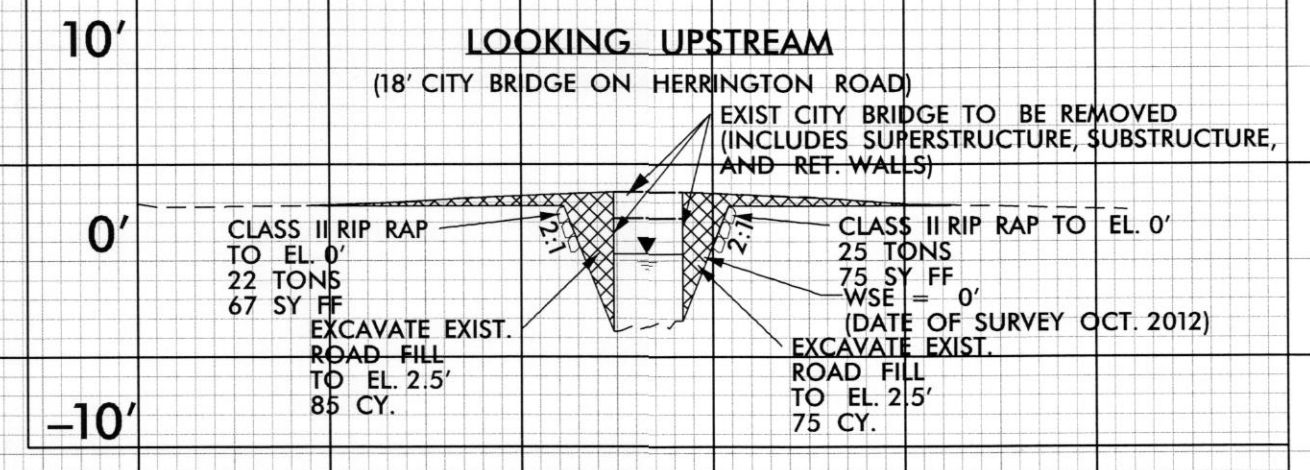
DESIGN DISCHARGE	= 710	CFS
DESIGN FREQUENCY	= 10	YRS
DESIGN HW ELEVATION	= 3.5	FT
BASE DISCHARGE	= 1510	CFS
BASE FREQUENCY	= 100	YRS
BASE HW ELEVATION	= 5.04	FT
OVERTOPPING DISCHARGE	= 1275	CFS
OVERTOPPING FREQUENCY	= 50	YRS
OVERTOPPING ELEVATION	= 4.6	FT
DATE OF SURVEY	= OCT. 2012	
W.S. ELEVATION AT DATE OF SURVEY	= 0.0	FT

PI = 13+10.00 EL = 5.15' VC = 60' K = 1000 Vs = 35mph
 PI = 14+10.00 EL = 5.51' VC = 110' K = 115 Vs = 35mph
 PI = 15+80.00 EL = 4.49' VC = 90' K = 64 Vs = 35mph

BM10 ELEVATION = 3.83
 N 936546 E 2819221
 R/R SPIKE IN BASE OF 8IN HONEY LOCUST

END GRADE
 POS STA 16+25.00 -L-
 EL. 4.85

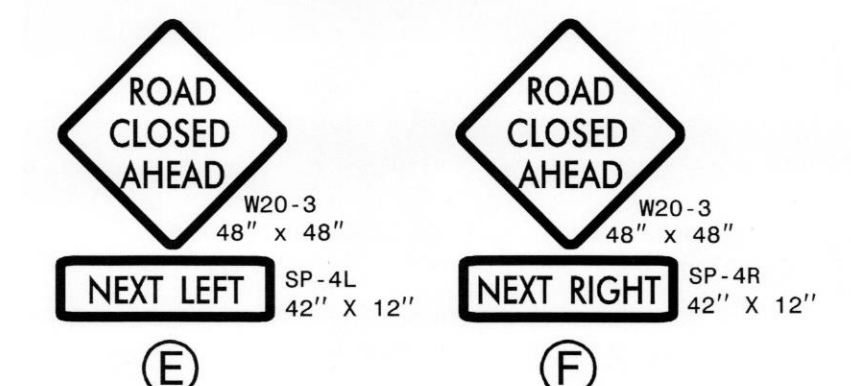
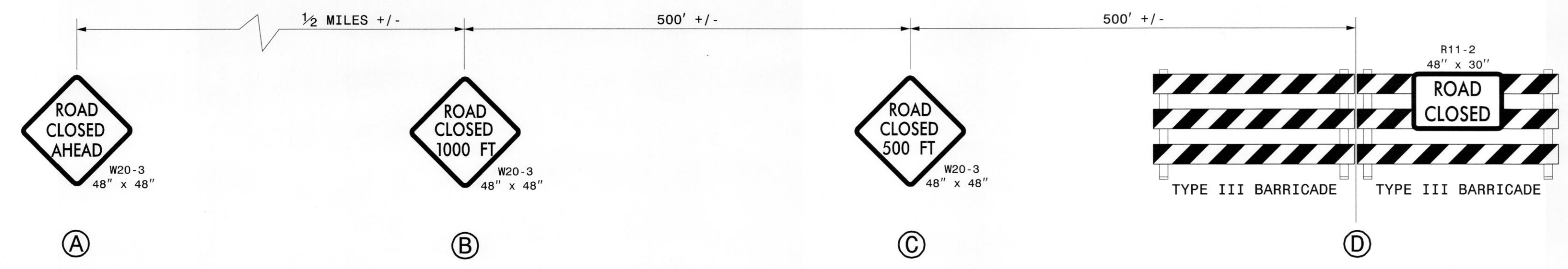
BEGIN GRADE
 POT STA 12+60.00 -L-
 EL. 5.00



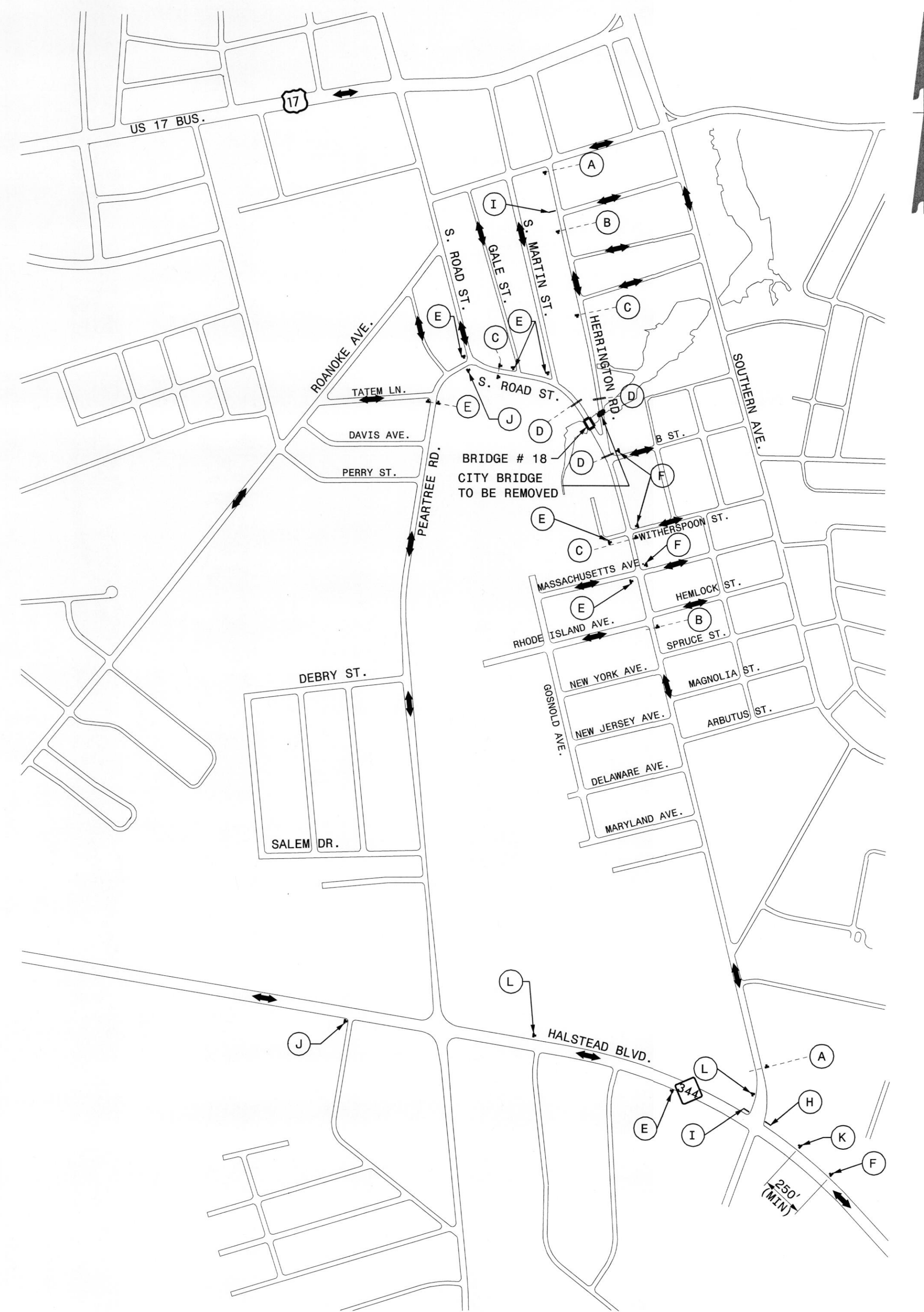
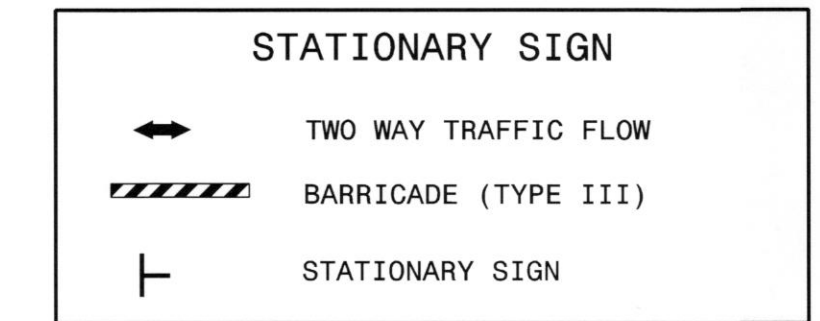
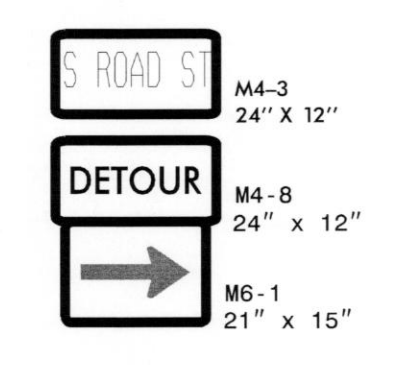
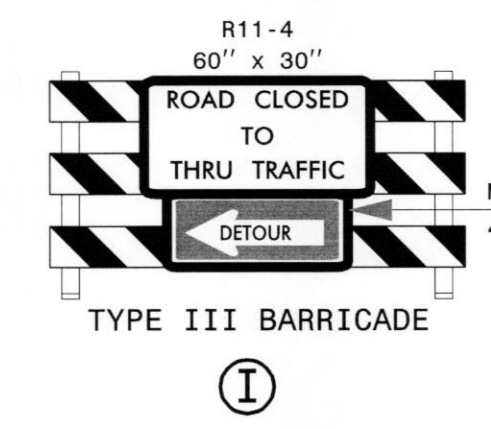
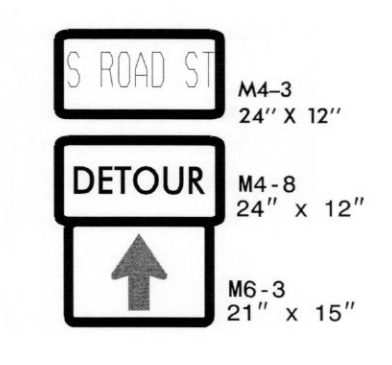
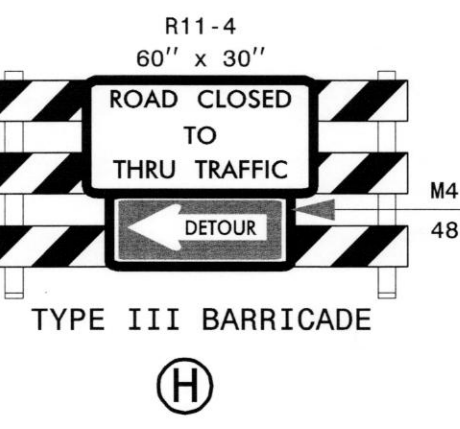
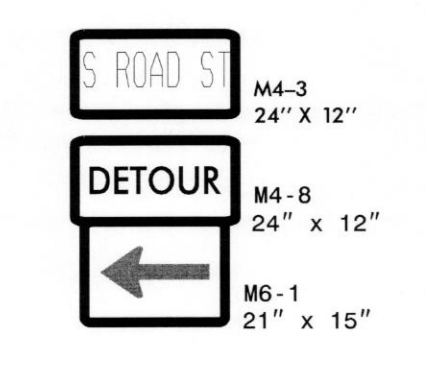
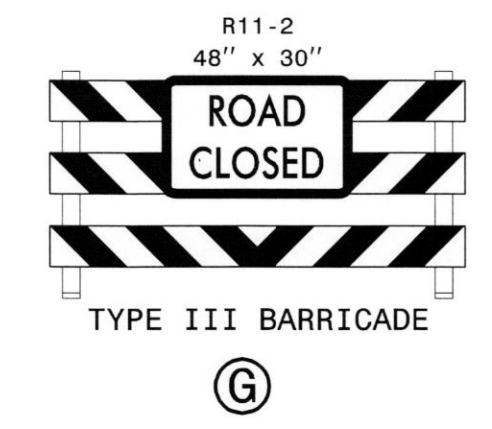
REVISIONS

03/19/14 - RW REVISION: ADDED TUE TO PARCELS 2 AND 4. DIC

5/20/2014 IUC26.AM, PSH_04.dgn



INSTALL 500'+/- IN ADVANCE OF EXISTING INTERSECTIONS UNLESS SHOWN OTHERWISE



ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	TITLE
1101.01	WORK ZONE ADVANCE WARNING SIGNS
1101.03	TEMPORARY ROAD CLOSURES
1101.04	TEMPORARY SHOULDER CLOSURES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1135.01	CONES
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO-LANE AND MULTI-LANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	RAISED PAVEMENT MARKERS - INSTALLATION SPACING
1251.01	RAISED PAVEMENT MARKERS - PERMANENT AND TEMPORARY
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING
1262.01	GUARDRAIL END DELINEATION

GENERAL NOTES

1. INSTALLATION OF TEMPORARY ROUTE MARKERS, DESTINATION SIGNS, AND ANY NECESSARY MODIFICATIONS TO EXISTING OR PROPOSED REGULATORY OR WARNING SIGNS WILL BE MADE BY OTHERS (STATE FORCES) UNLESS OTHERWISE DESIGNATED IN PLANS. PROVIDE A MINIMUM 21 CALENDAR DAY NOTICE TO STATE FORCES BEFORE A ROADWAY IS CLOSED TO TRAFFIC SUCH THAT THE NECESSARY PROVISIONS CAN BE MADE TO INFORM LOCAL EMERGENCY AND LAW ENFORCEMENT PERSONNEL, SCHOOLS, OR ANY OTHER PARTIES AFFECTED BY THE ROAD CLOSURE.
2. INSTALL SIGNS BEFORE THE BARRICADES WHEN CLOSING THE ROADWAY TO TRAFFIC. REMOVE BARRICADES BEFORE SIGNS WHEN OPENING THE ROADWAY TO TRAFFIC. INSTALL / REMOVE SIGNS AND BARRICADES WITHIN THE SAME CALENDAR DAY.
3. POSITION WING BARRICADES ON THE SHOULDERS AND SLOPE THE STRIPES DOWNWARD IN THE DIRECTION TOWARD WHICH TRAFFIC MUST TURN IN DETOURING.
4. USE ADDITIONAL TYPE III BARRICADES IN STAGGERED LOCATIONS SUPPLEMENTED WITH SIGN R11-4 "ROAD CLOSED TO THRU TRAFFIC" IN THE EVENT THAT TRAFFIC MUST BE MAINTAINED BEYOND THE DETOUR POINT.
5. SEE STANDARD SPECIFICATION 1089-1 FOR WORK ZONE SIGNS.
6. SEE STANDARD SPECIFICATION 1089-1 FOR WORK ZONE SIGN SUPPORTS.

5/20/2014 2:56:31 PM
...TCF\BD-5101N\TCF_PSH_01.dgn
USER: TAYLOR,JB

CDM Smith
Camp Dresser & McKee
5400 Glenwood Avenue
Suite 300
Raleigh, NC 27612-3228
NC COA No. F-0412

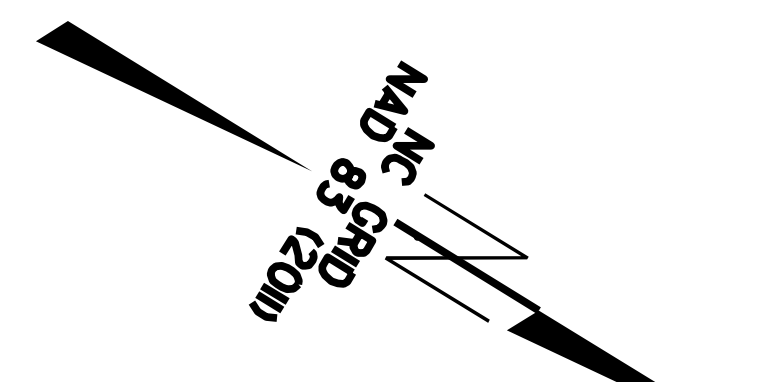
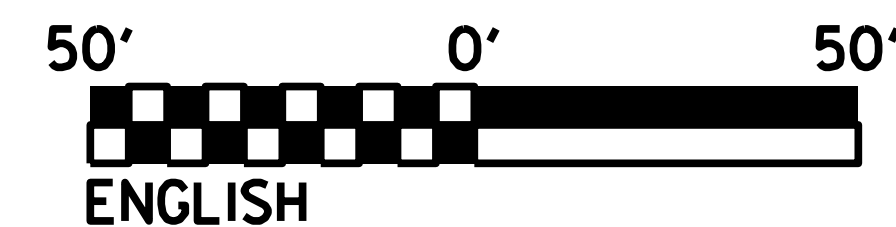
APPROVED: *Douglas B. Saunders* DATE: 5/20/14
SEAL
NORTH CAROLINA
PROFESSIONAL
ENGINEER
DOUGLAS B. SAUNDERS
14160

DIVISION OF HIGHWAYS
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
WORK ZONE TRAFFIC CONTROL

**ROAD CLOSURE
HERRINGTON ROAD
AND SOUTH ROAD STREET**

EROSION CONTROL PLAN

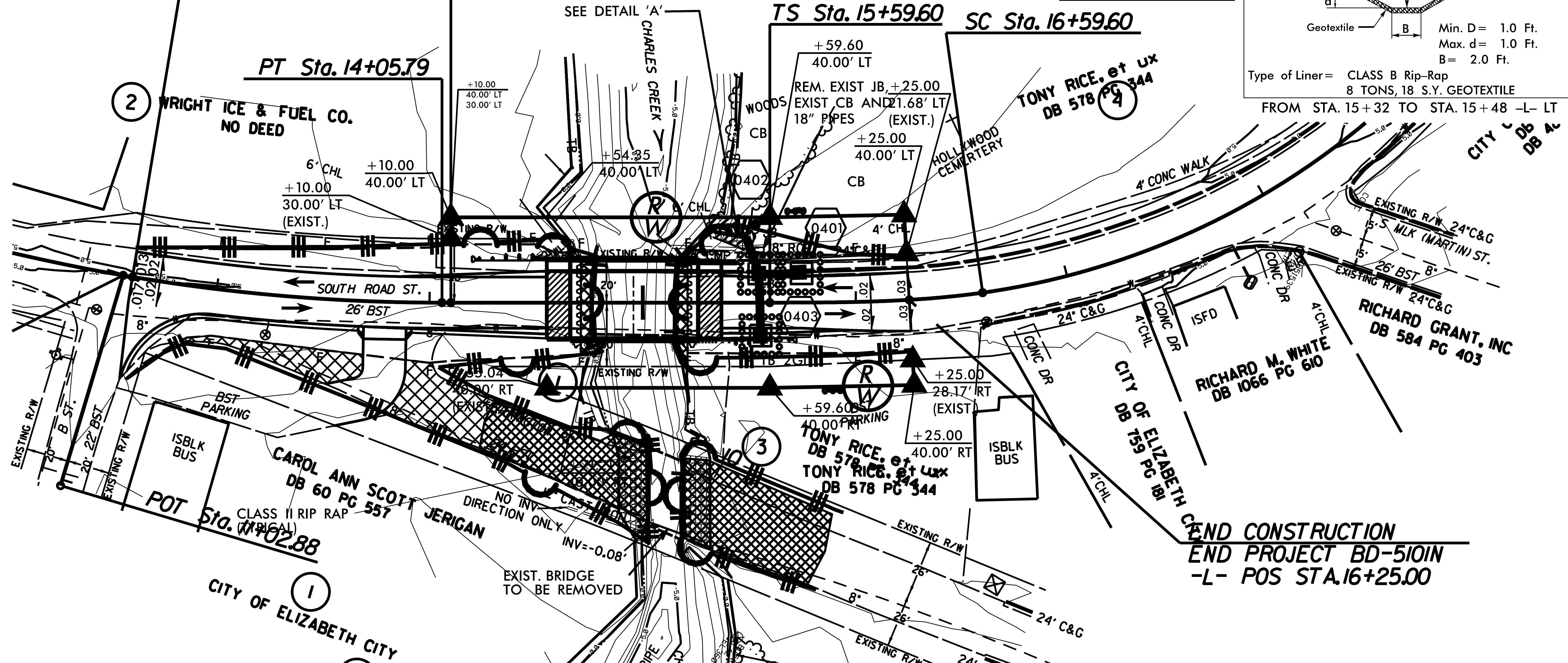
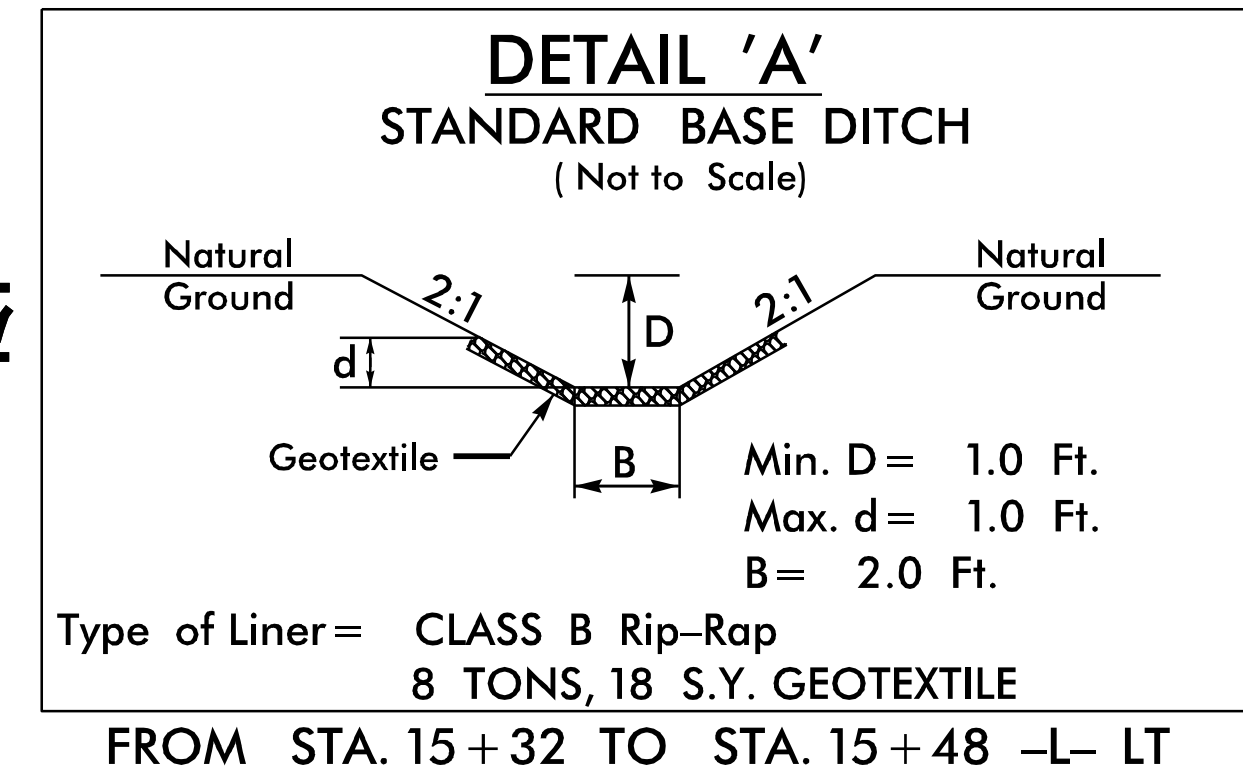
PROJECT REFERENCE NO.	SHEET NO.
BD-5101N	EC-1
Prepared by:	
MI ENGINEERING, PLLC	
Designed by: Melanie Nguyen, PE	
Level III: Designer of Erosion and Sediment Control Plans	
Certification Number: 3223 - Expires: 12/31/2015	
1011 SCHAUH DRIVE, SUITE 100	
RALEIGH, NC 27606	
919-851-6606 (PH)	
919-851-6645 (F)	
2012 STANDARD SPECIFICATIONS	



RUCTION
 CT BD-5101N
 A.12+60.00

-L- POT STA. 14+10.00

CS Sta. 18+60.9i



Sd. #	Description	Symbol	QUANTITY
1606.01	High Vis Temporary Silt Fence		1100 LF
1632.05	Rock Inlet Sediment Trap Type C		3 EA.
SP	Coir Fiber Wattle Barrier		130 LF

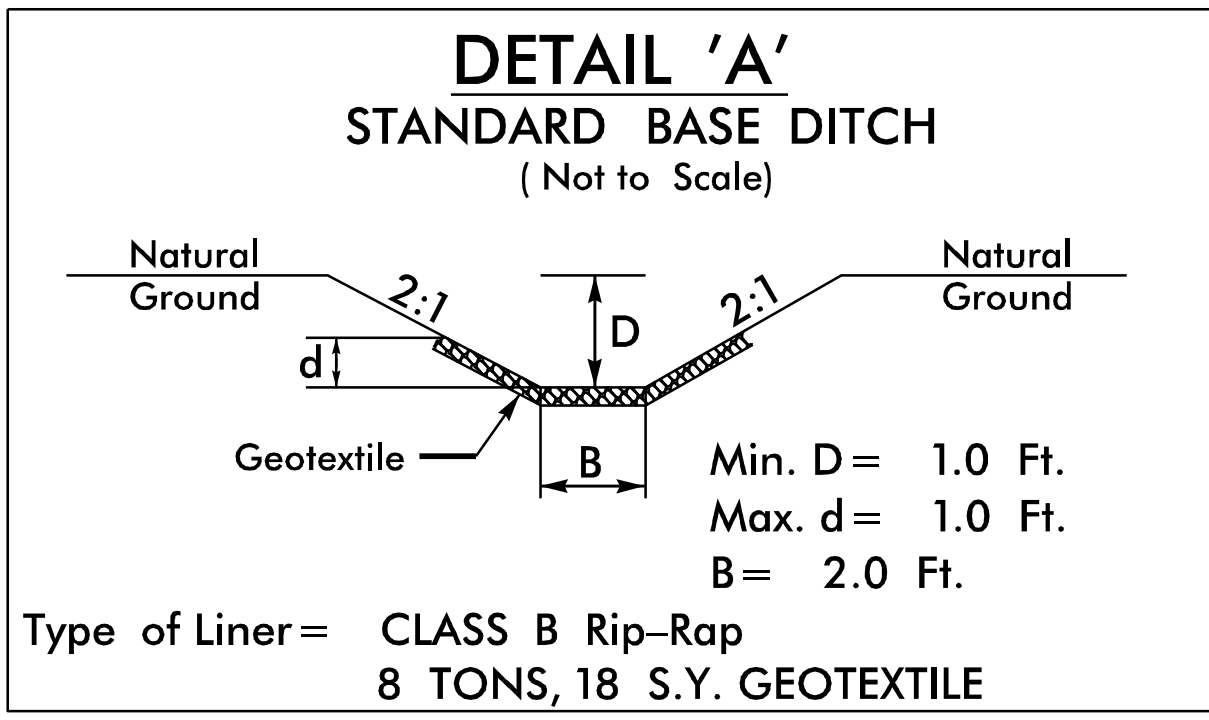
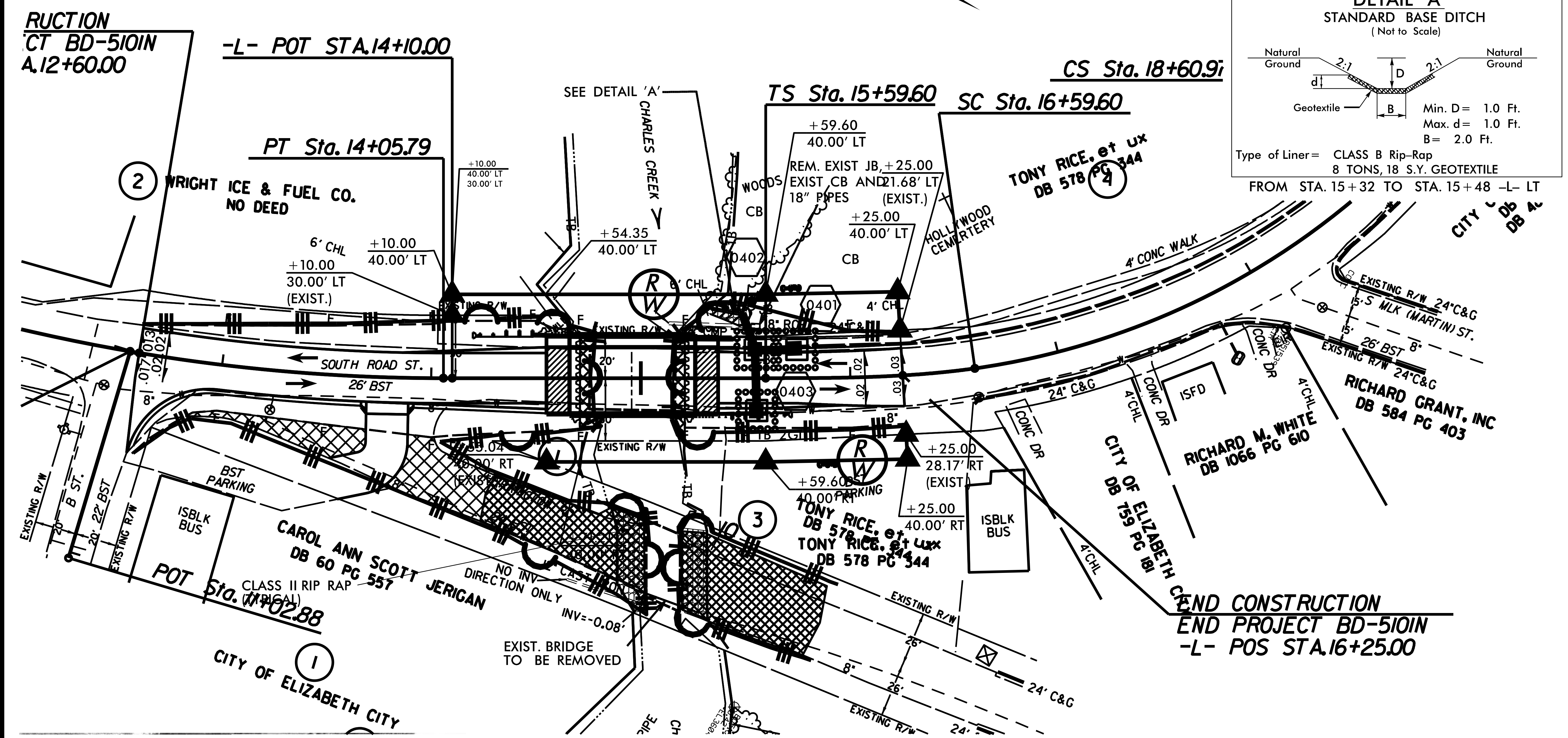
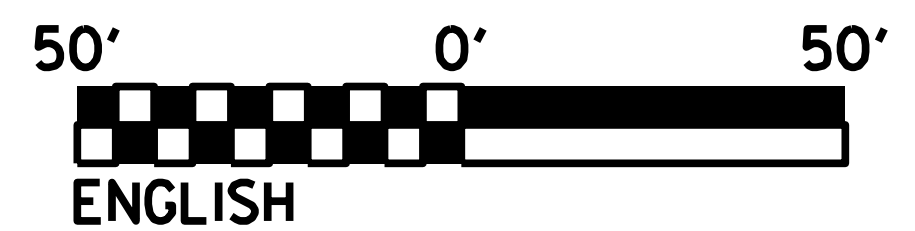
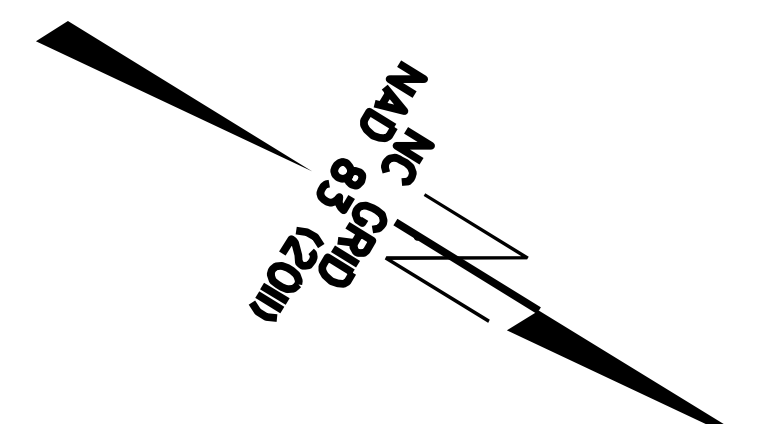
SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HOW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10' OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50' IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HOW ZONES.

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.
 ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.

8/17/19 8:17/199
 REVISIONS
 SYSTEM TIME
 10/12/2018 11:45:53 AM

EROSION CONTROL PLAN

PROJECT REFERENCE NO. **BD-5101N** SHEET NO. **EC-2**
 Prepared by:
MI ENGINEERING, PLLC
 Designed by: Melanie Nguyen, PE
 Level III, Designer of Erosion and Sediment Control Plans
 Certification Number: 3223 - Expires: 12/31/2015
 1011 SCHAUB DRIVE, SUITE 100
 RALEIGH, NC 27606
 919-851-6606 (PH)
 919-851-6645 (F)
 2012 STANDARD SPECIFICATIONS



FROM STA. 15+32 TO STA. 15+48 -L- LT

RUCTION
 CT BD-5101N
 A.12+60.00

-L- POT STA. 14+10.00

CS Sta. 18+60.9i

TS Sta. 15+59.60

SC Sta. 16+59.60

PT Sta. 14+05.79

2 WRIGHT ICE & FUEL CO.
 NO DEED

TONY RICE, et ux
 DB 578 PG 344

RICHARD M. WHITE
 DB 1066 PG 610

RICHARD GRANT, INC
 DB 584 PG 403

CAROL ANN SCOTT
 DB 60 PG 557

1 CITY OF ELIZABETH CITY

END CONSTRUCTION
 END PROJECT BD-5101N
 -L- POS STA. 16+25.00

Qty.	Description	Symbol	QUANTITY
1606.01	High Vis Temporary Silt Fence		1100 LF
1632.05	Rock Inlet Sediment Trap Type C		3 EA.
SP	Coir Fiber Wattle Barrier		130 LF

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HOW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10' OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50' IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HOW ZONES.

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.
 ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.

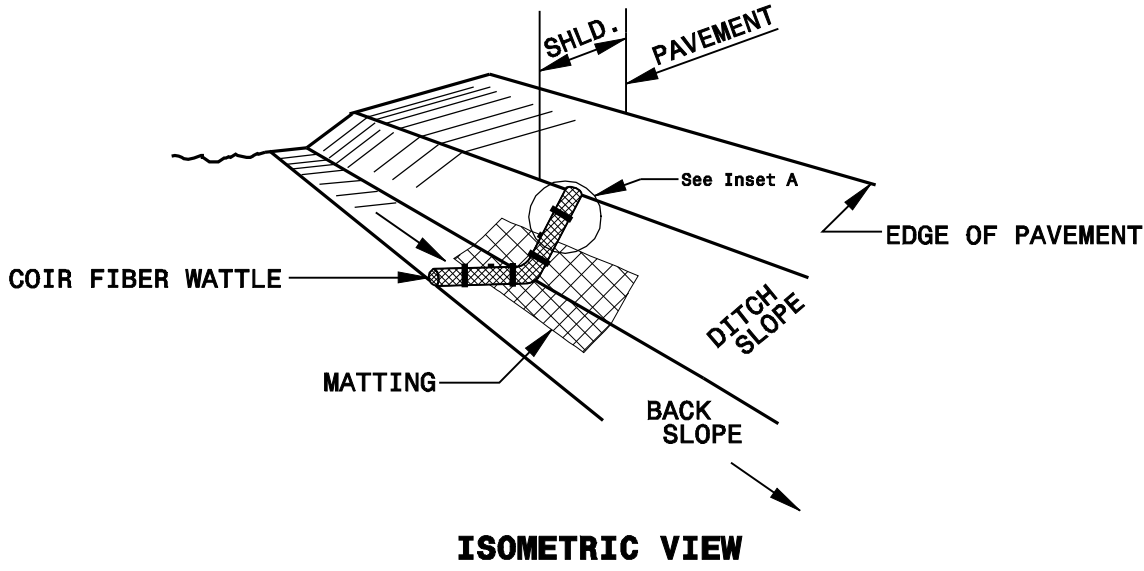
REVISIONS

8/17/19

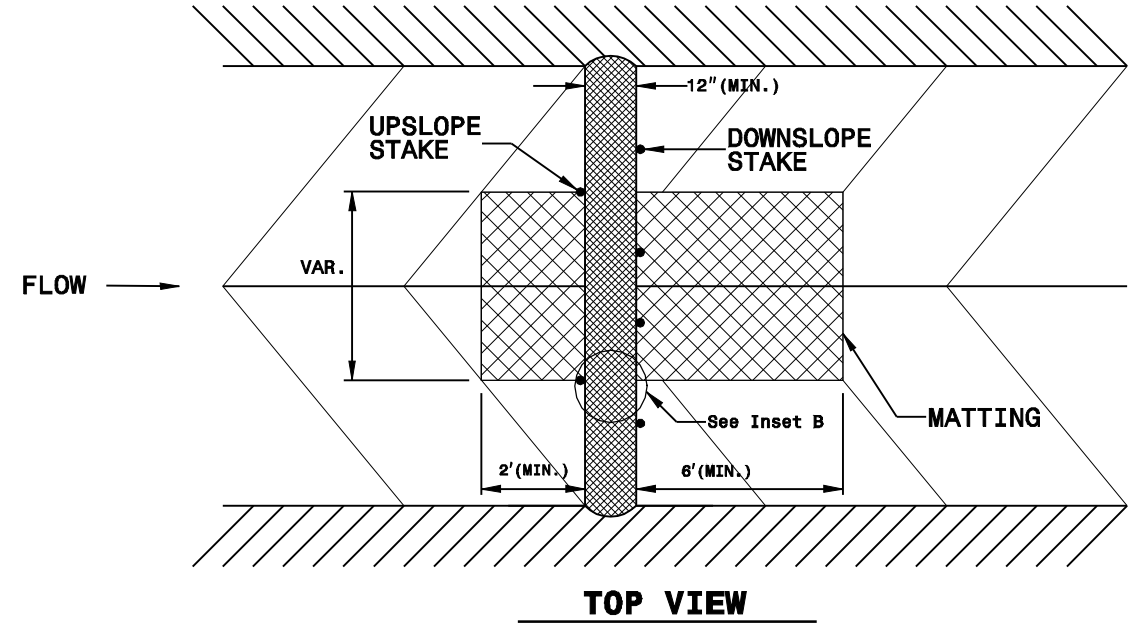
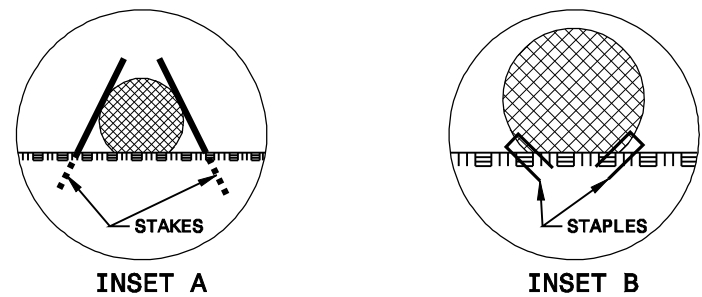
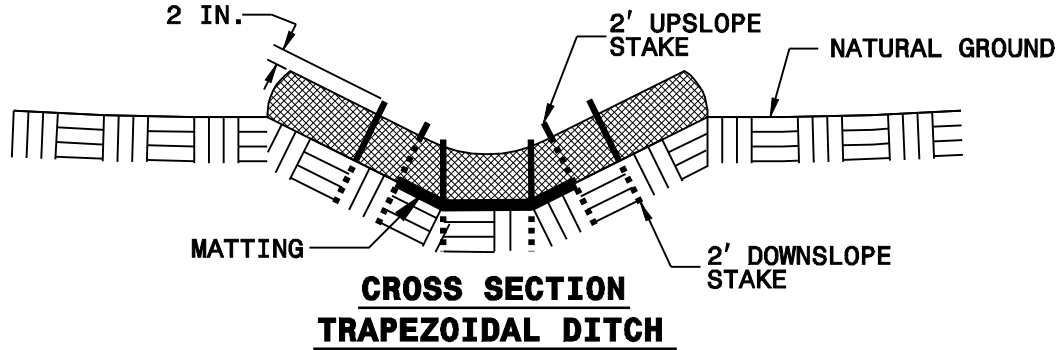
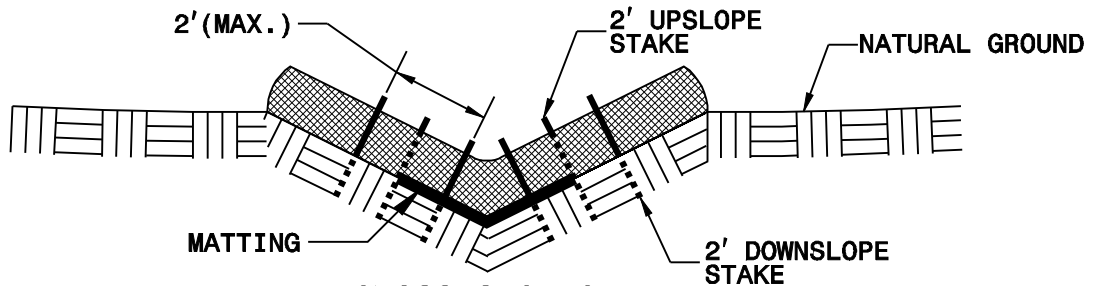
SYTIME

PROJECT REFERENCE NO. X-XXXX	SHEET NO. EC-26
RW SHEET NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

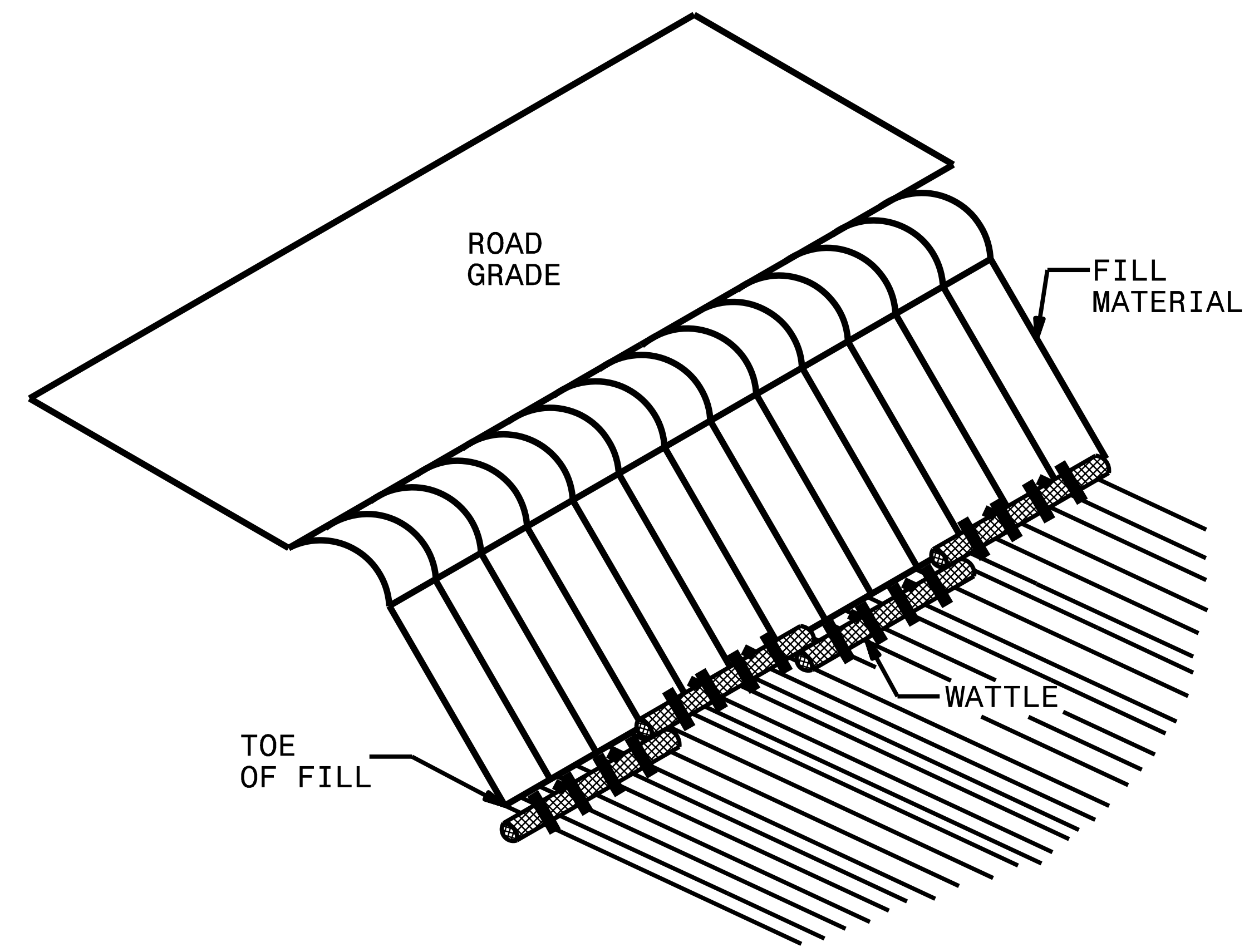
COIR FIBER WATTLE DETAIL



- NOTES:**
- USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.
 - USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.
 - ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.
 - INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.
 - PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.
 - INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.
 - INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.



COIR FIBER WATTLE BARRIER DETAIL



ISOMETRIC VIEW

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

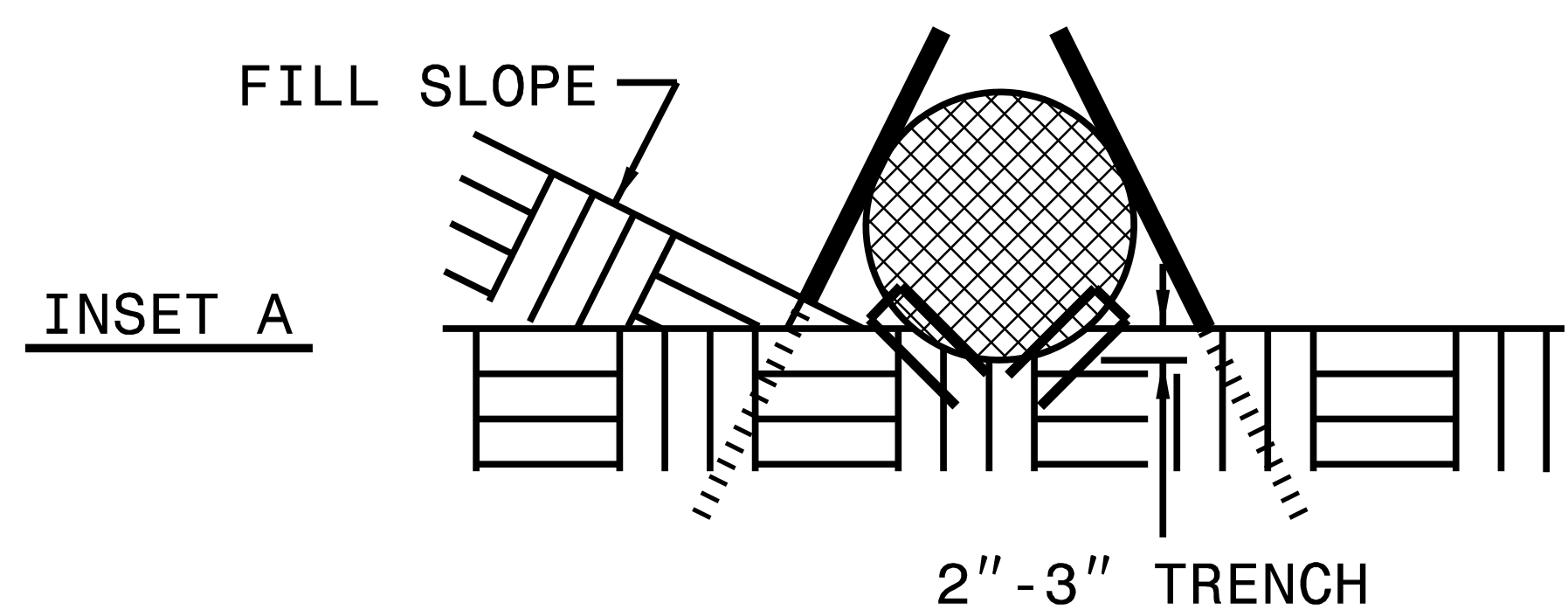
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

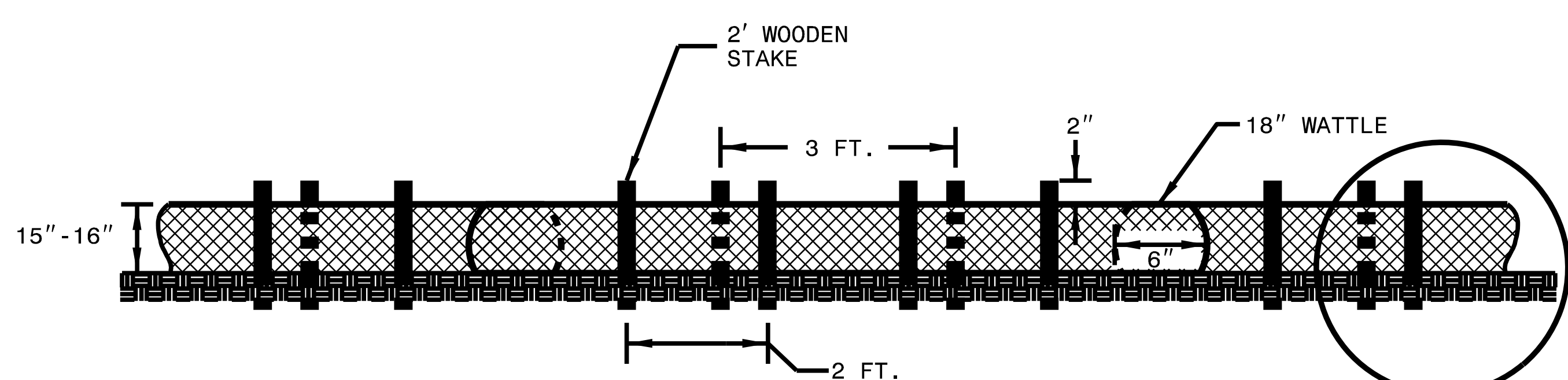
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

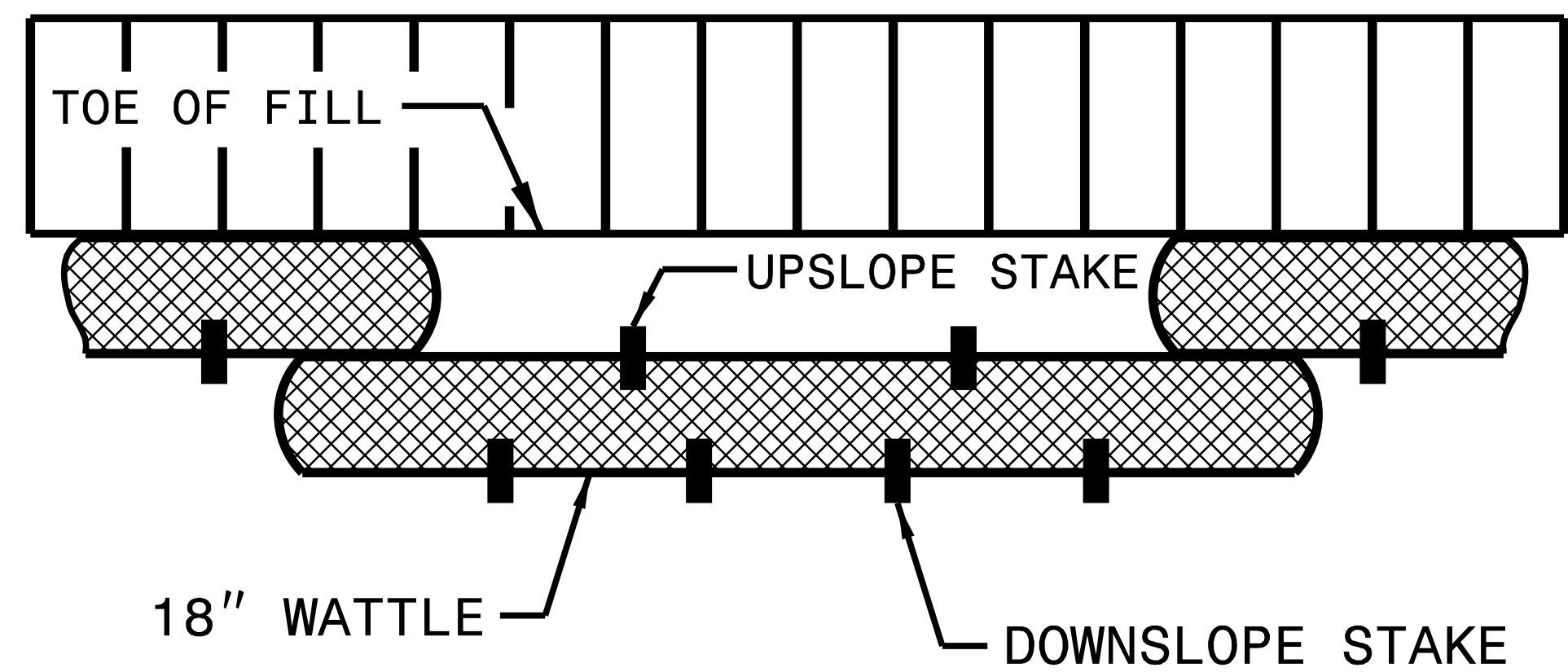
FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 20 FT.



INSET A



FRONT VIEW

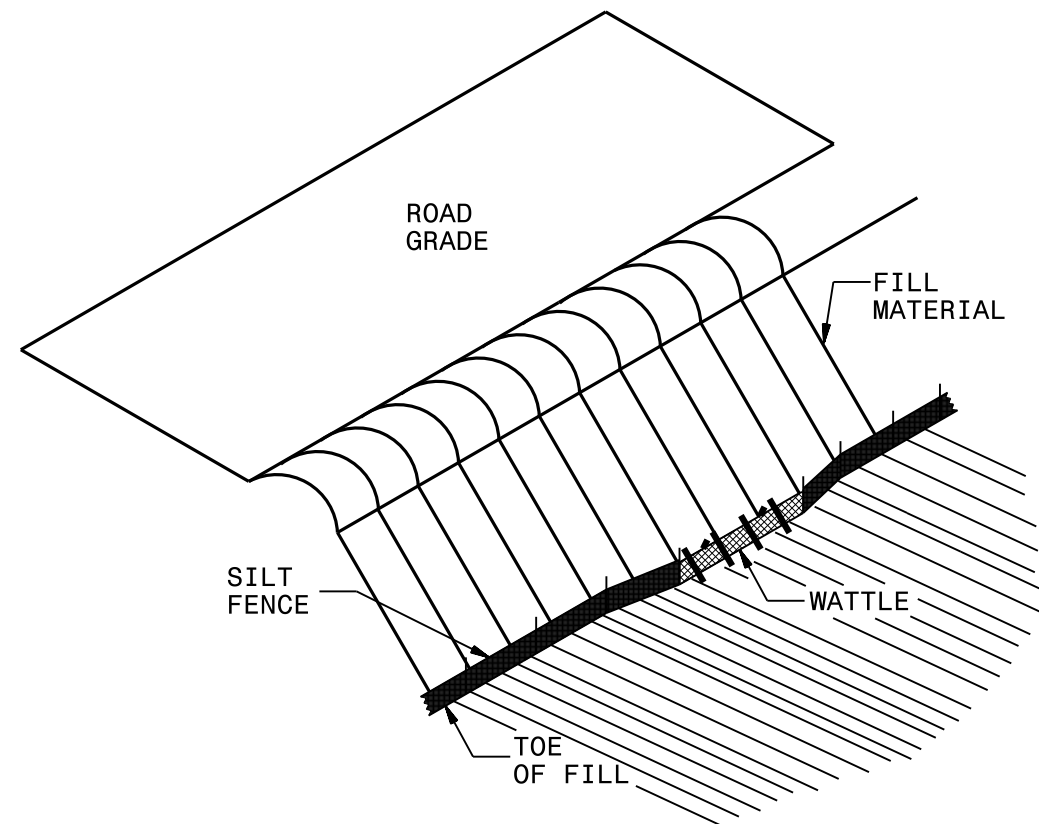


TOP VIEW

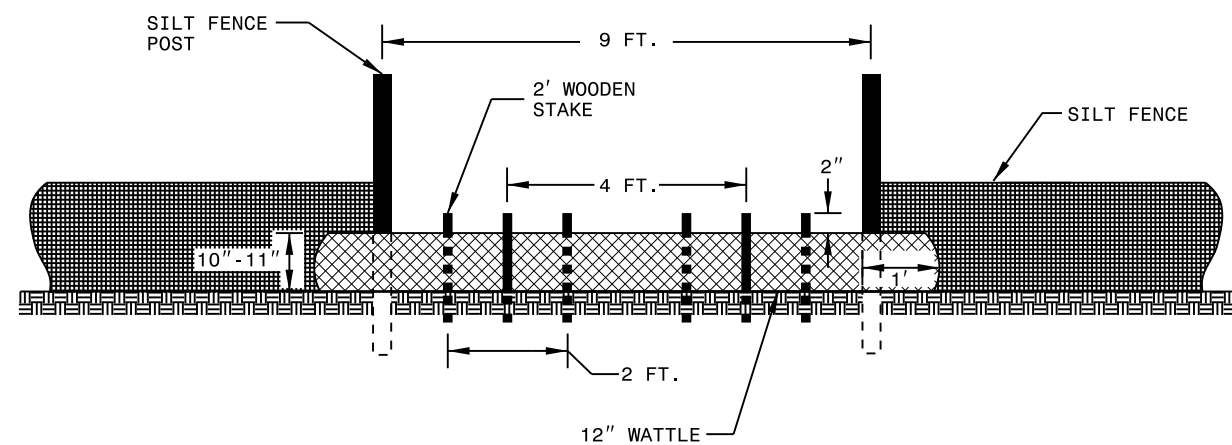
SEE INSET A

SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO. X-XXXX		SHEET NO. EC-2G	
RW SHEET NO.			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	



ISOMETRIC VIEW



VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

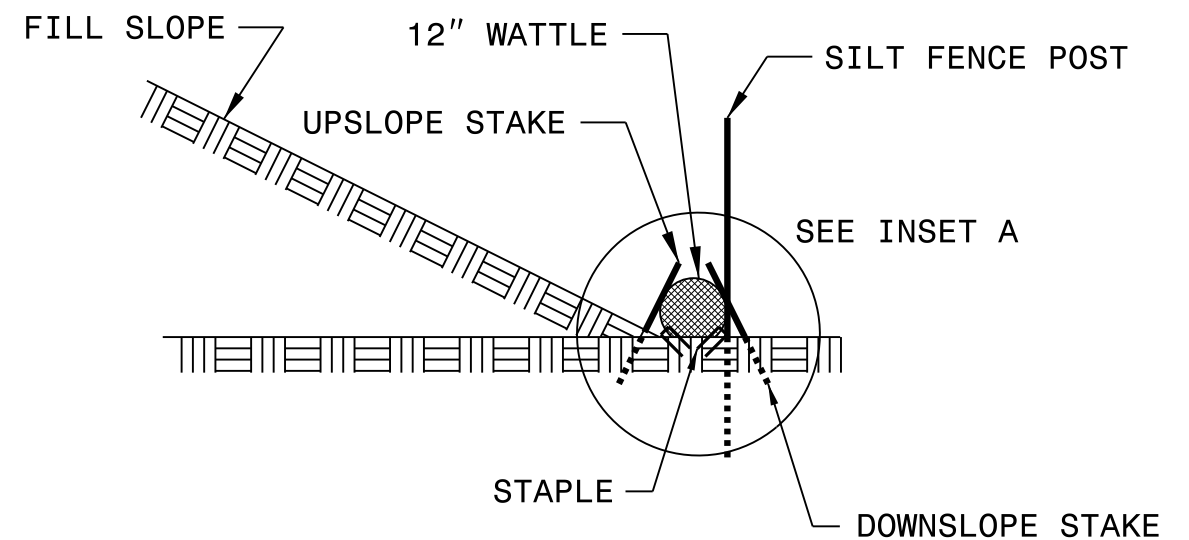
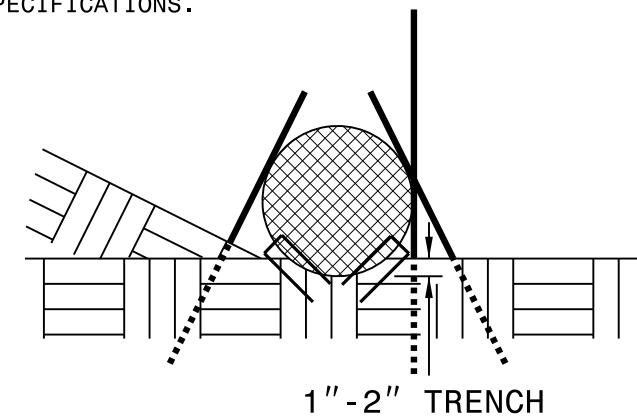
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

INSET A



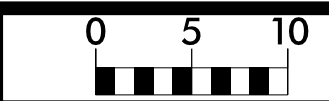
SIDE VIEW

STATE OF NORTH CAROLINA NCDOT RAIL DIVISION

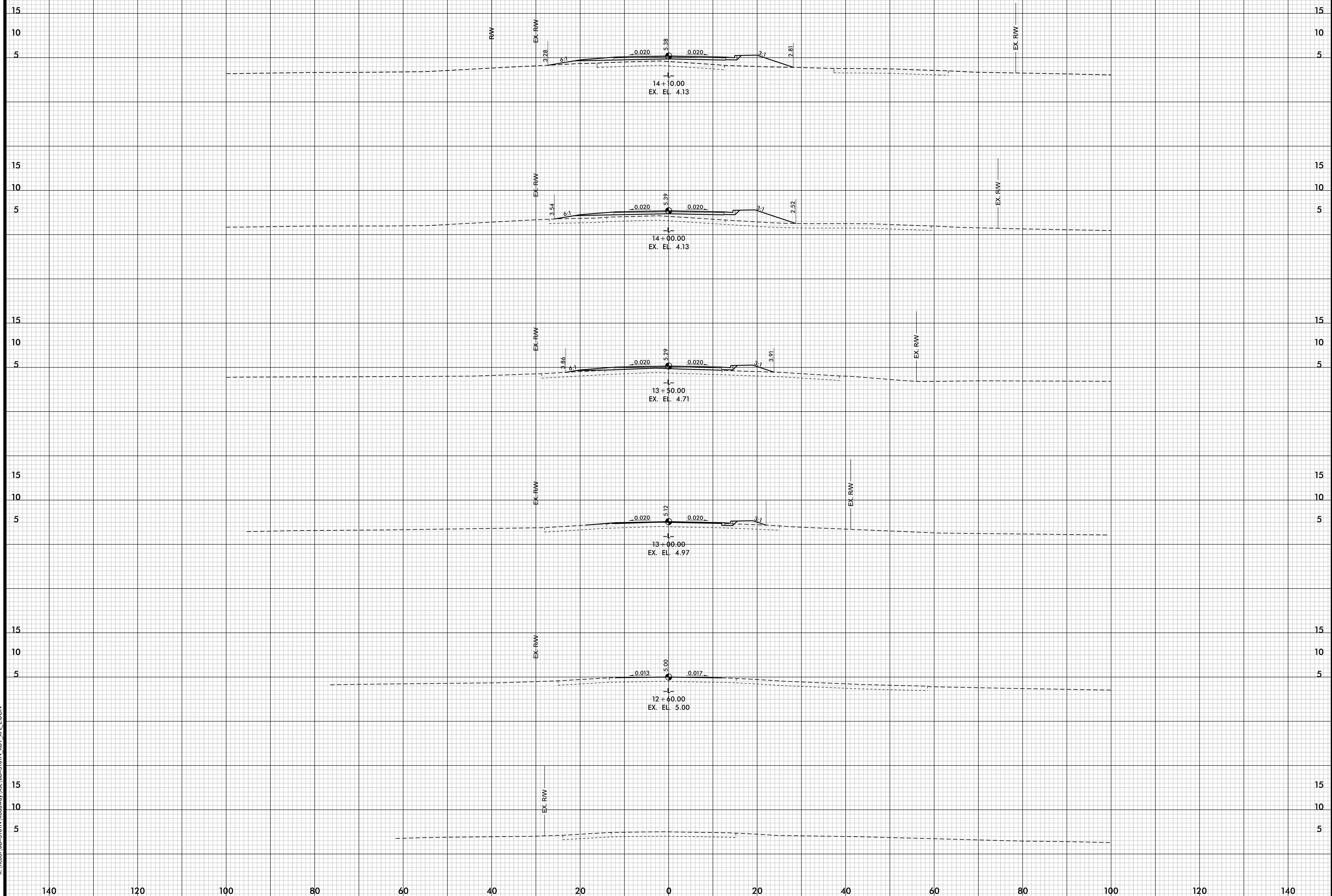
CROSS-SECTION SUMMARY

NOTE: EMBANKMENT COLUMN DOES NOT INCLUDE BACKFILL FOR UNDERCUT

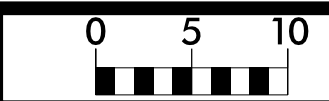
Station	Uncl. Exc.	Embt
L	(cu. yd.)	(cu. yd.)
12+60.00	0	0
13+00.00	0	4
13+50.00	1	14
14+00.00	0	56
14+10.00	0	18
14+50.00	3	58
14+55.00	1	4
15+26.00	0	0
15+50.00	1	6
16+00.00	1	8
16+25.00	1	3



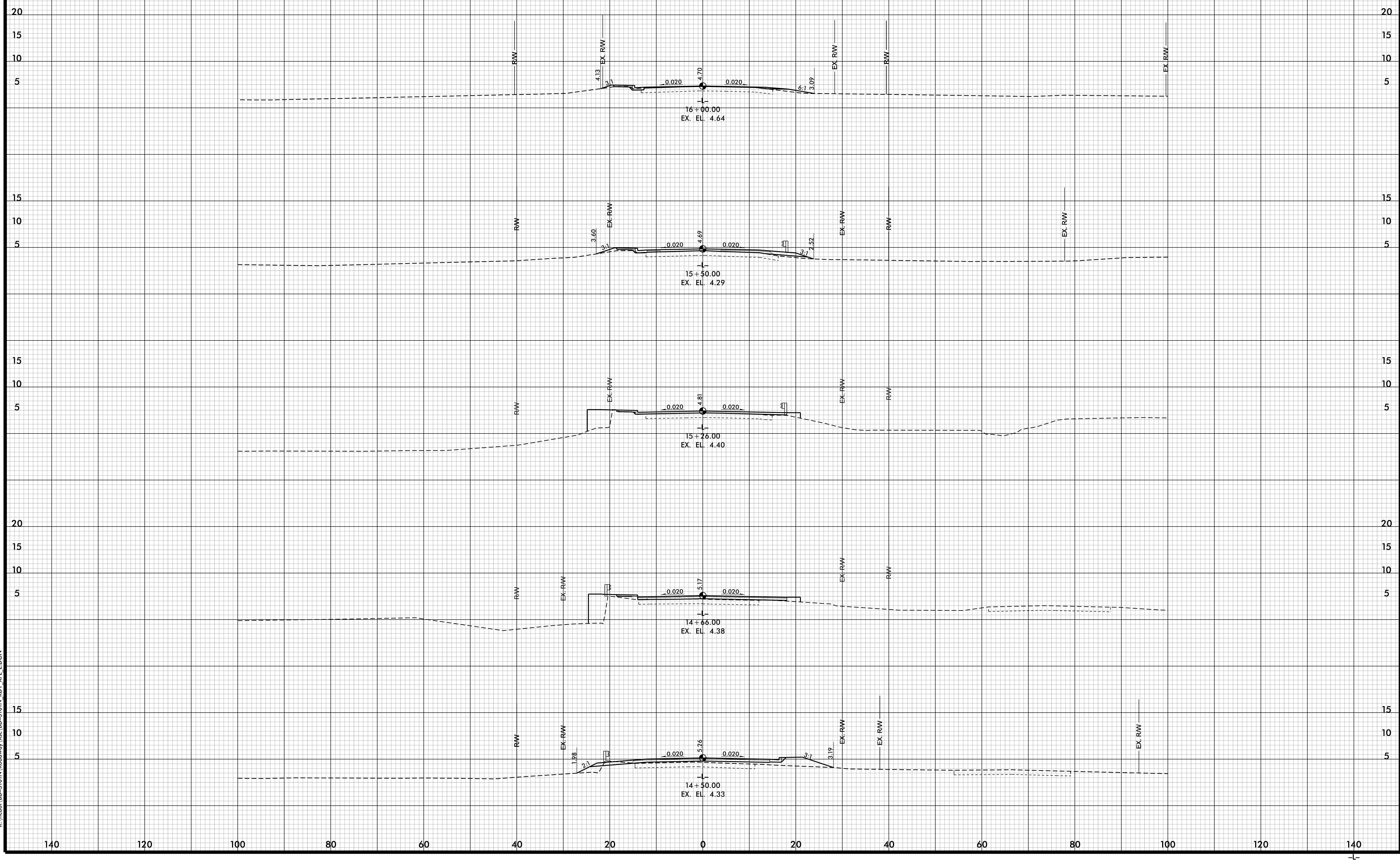
PROJ. REFERENCE NO.	SHEET NO.	TOTAL SHEETS
BD-5101N	X-2	



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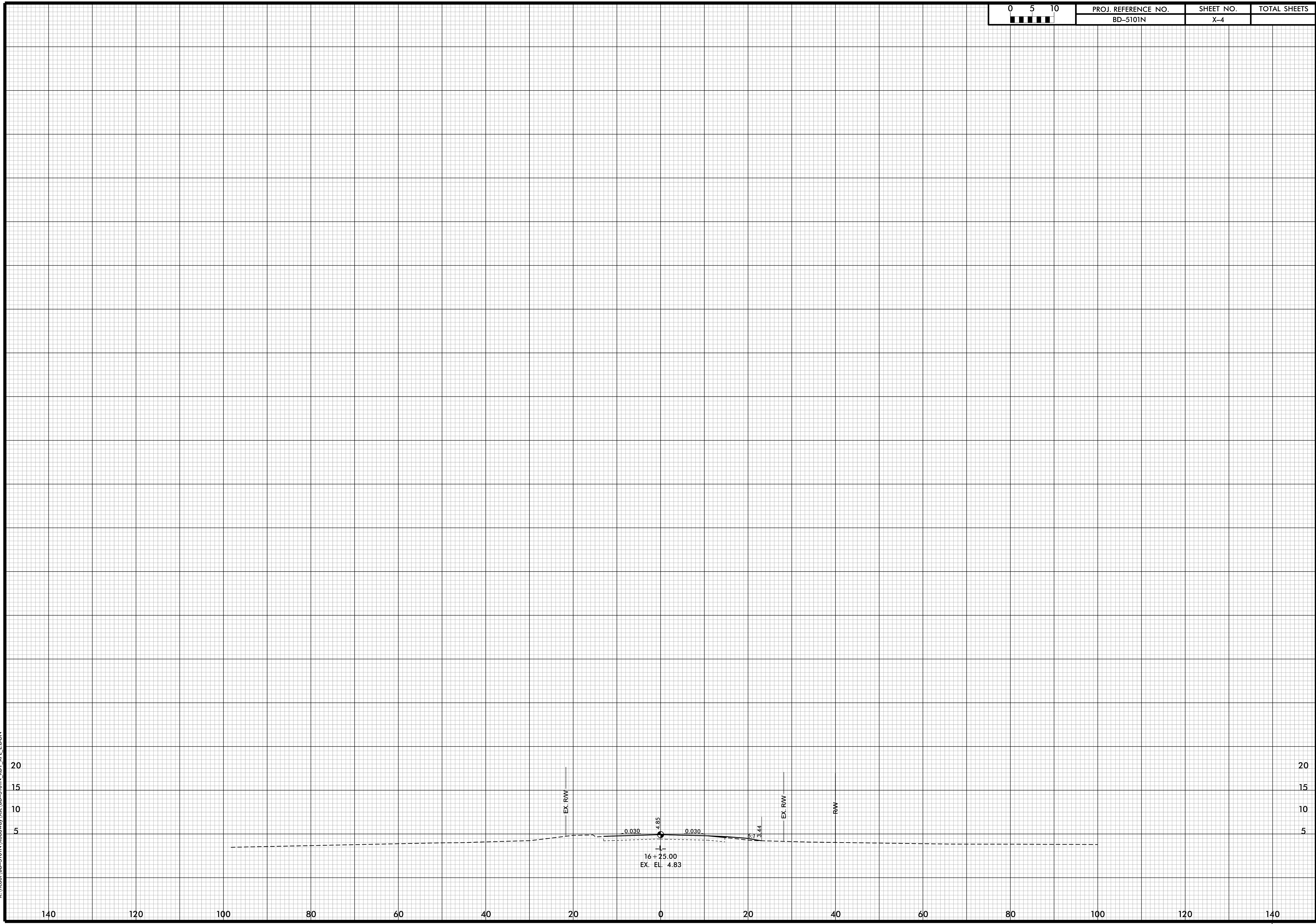
PROJ. REFERENCE NO.	SHEET NO.	TOTAL SHEETS
BD-5101N	X-3	

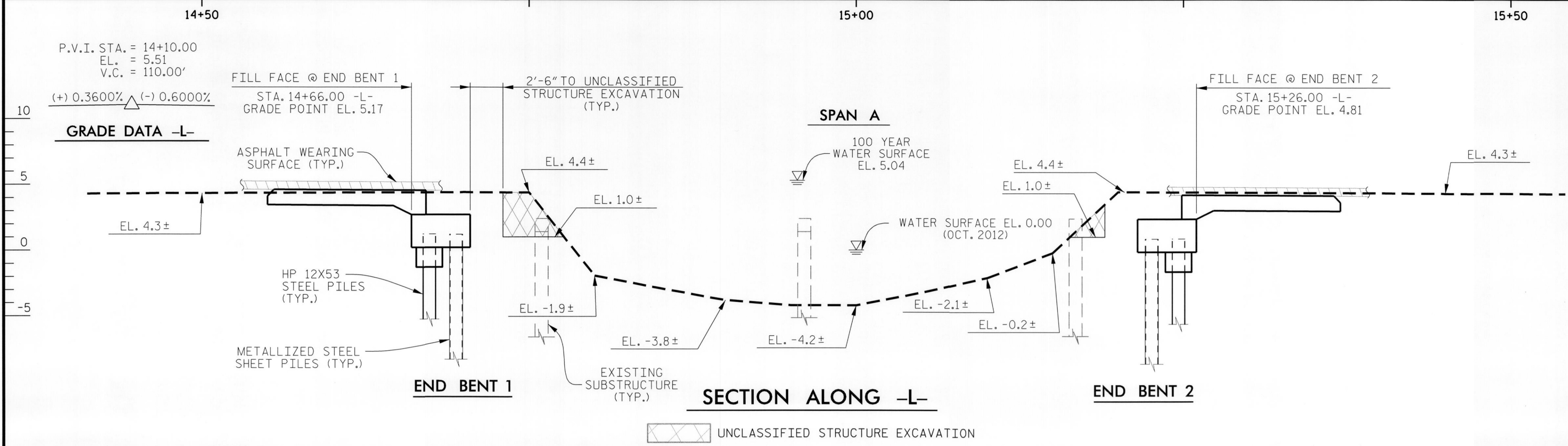


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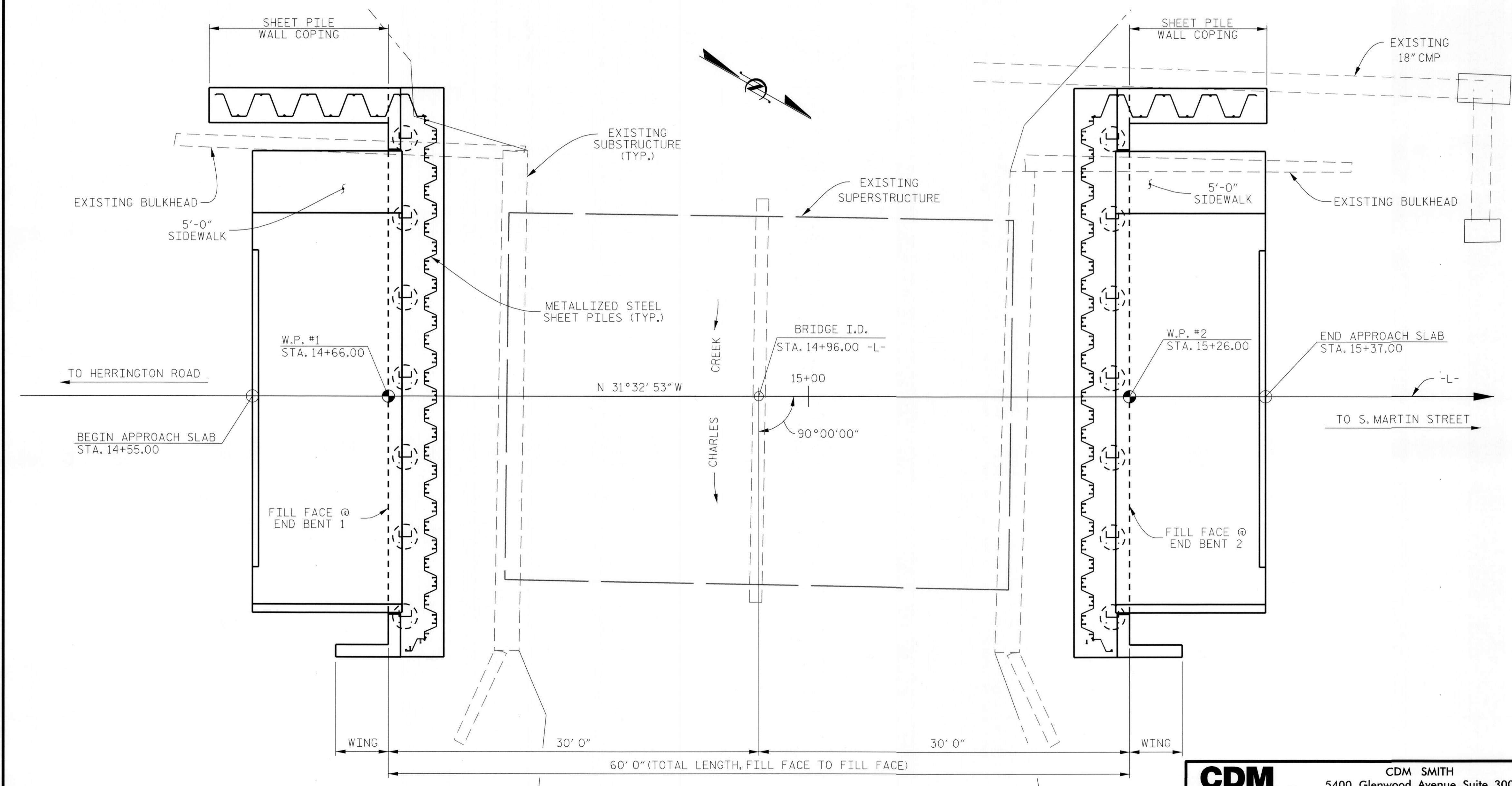
HYDRAULIC DATA

DESIGN DISCHARGE	= 710 CFS
FREQUENCY OF DESIGN FLOOD	= 10 YEARS
DESIGN HIGH WATER ELEVATION	= 3.5 FT.
DRAINAGE AREA	= 3.0 SQ. MI.
BASE DISCHARGE (Q100)	= 1,510 CFS
BASE HIGH WATER ELEVATION	= 5.04 FT.

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE	= 1,275 CFS
FREQUENCY OF OVERTOPPING FLOOD	= 50 YEARS
OVERTOPPING FLOOD ELEVATION	= 4.6 FT.

OVERTOPPING AT PROPOSED SAG AT STA. 15+74 -L-. PROPOSED DESIGN MAINTAINS EXISTING HYDRAULIC LEVEL OF SERVICE.



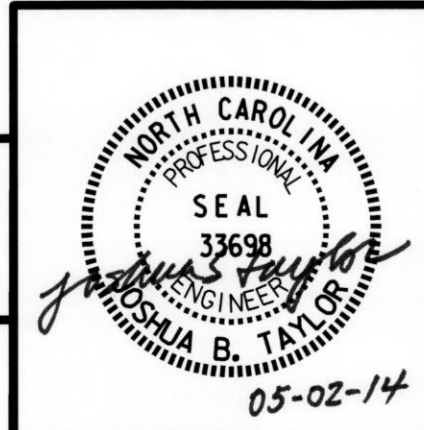
I HEREBY CERTIFY THESE PLANS ARE THE AS-BUILT PLANS.

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14 + 96.00 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING
BRIDGE OVER CHARLES CREEK ON
NC 34 (-L-) BETWEEN
HERRINGTON ROAD AND
S. MARTIN STREET



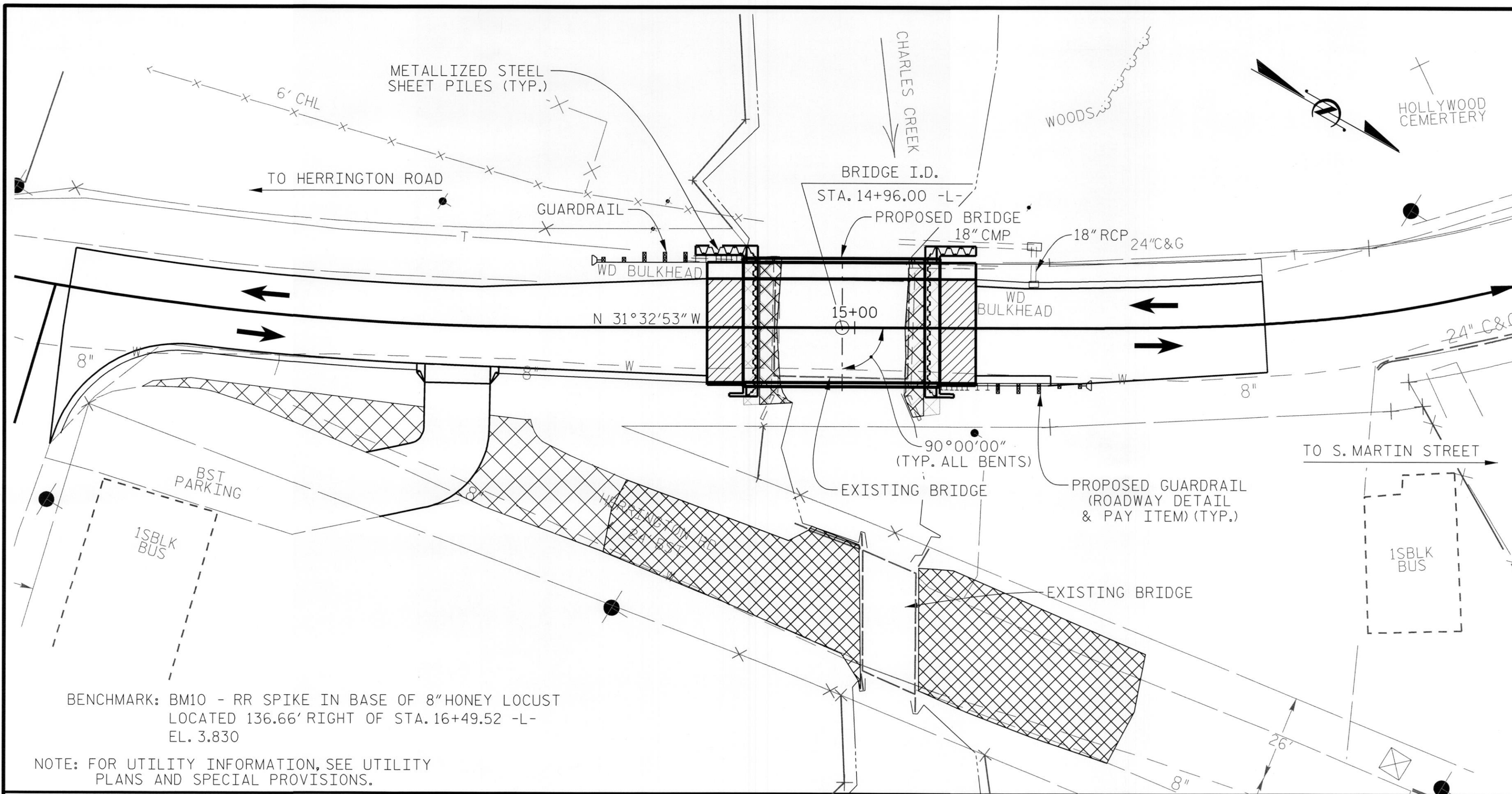
CDM Smith
5400 Glenwood Avenue, Suite 300
Raleigh, NC 27612-3228
NC COA No. F-1255

CDM SMITH
DRAWN BY: R. STATION / J. SLOAN DATE: 05-14
CHECKED BY: J. B. TAYLOR DATE: 05-14
DESIGN ENGINEER: J. B. TAYLOR DATE: 05-14

DWG. No. _____

REVISIONS				SHEET No.	
No.	BY:	DATE:	No.	BY:	DATE:
1			3		
2			4		

TOTAL SHEETS: S-1



LOCATION SKETCH

TOTAL BILL OF MATERIAL

	REMOVAL OF EXISTING STRUCTURE @ STA. 14+96.00 -L-	PDA TESTING EACH	UNCLASSIFIED STRUCTURE EXCAVATION @ STA. 14+96.00 -L-	CLASS AA CONCRETE CU. YDS.	BRIDGE APPROACH SLABS @ STA. 14+96.00 -L-	EPOXY COATED REINFORCING STEEL LBS.	HP 12 x 53 STEEL PILES		PILE REDRIVES EACH	METALLIZED STEEL SHEET PILES		1'-3" x 2'-6" CONCRETE PARAPET LIN. FT.	2 BAR METAL RAIL LIN. FT.	ELASTOMERIC BEARINGS LUMP SUM	3'-0" x 1'-9" PRESTRESSED CONCRETE CORED SLABS	
							No.	LIN. FT.		No.	SQ. FT.				No.	LIN. FT.
SUPERSTRUCTURE	-	-	-	8.8	LUMP SUM	376	-	-	-	-	-	115.5	100.5	LUMP SUM	13	750.75
END BENT 1	-	-	LUMP SUM	23.2	-	3047	7	665	-	40	3335	-	-	-	-	-
END BENT 2	-	-	LUMP SUM	22.9	-	3018	7	665	-	38	3050	-	-	-	-	-
TOTAL	LUMP SUM	1	LUMP SUM	54.9	LUMP SUM	6441	14	1330	1	78	6385	115.5	100.5	LUMP SUM	13	750.75

FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NOS.1 AND 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT END BENT NOS.1 AND 2 TO A REQUIRED DRIVING RESISTANCE OF 142 TONS PER PILE.

INSTALL PILES AT END BENT NOS.1 AND 2 TO A TIP ELEVATION NO HIGHER THAN -70 FT.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40,000 TO 45,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT NOS.1 AND 2.

THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

TESTING THE FIRST PRODUCTION PILE AT AN END BENT WITH THE PDA DURING DRIVING IS REQUIRED. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.

THE SCOUR CRITICAL ELEVATION FOR VERTICAL ABUTMENTS AT END BENT 1 AND END BENT 2 IS ELEVATION -9.0 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

STEEL SHEET PILES INSTALLED PARALLEL TO AND EMBEDDED IN THE END BENT CAP AT END BENT NO.1 AND END BENT NO.2 SHALL BE INSTALLED TO A SHEETING TIP ELEVATION OF -48.0 FEET.

STEEL SHEET PILES INSTALLED PARALLEL TO THE ROADWAY AT END BENT NO.1 AND END BENT NO.2 SHALL BE INSTALLED TO A SHEETING TIP OF -66.0 FEET.

STEEL SHEET PILING INSTALLED PARALLEL TO AND EMBEDDED IN THE END BENT CAP AT END BENT NO.1 AND END BENT NO.2 SHALL HAVE A MINIMUM SECTION MODULUS OF 30.2 IN³/FT.

STEEL SHEET PILING INSTALLED PARALLEL TO THE ROADWAY AT END BENT NO.1 AND END BENT NO.2 SHALL HAVE A MINIMUM SECTION MODULUS OF 80.2 IN³/FT.

THE CORED SLAB UNIT SUPERSTRUCTURE ACTS AS A BRACE BETWEEN THE SHEET PILE WALLS PARALLEL TO AND EMBEDDED IN THE END BENT CAP AT END BENT NO.1 AND END BENT NO.2. AT THE SCOUR CRITICAL ELEVATION, THE AASHTO LRFD STRENGTH LIMIT STATE BRACE LOAD IS 9.6 KIPS/FOOT OF WALL.

GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE EXISTING 2 SPAN STRUCTURE (2 @ 20'-0") WITH 6 LINES OF STEEL I-BEAMS AT 5'-0" SPACES SUPPORTING REINFORCED CONCRETE DECK WITH 30'-0" CLEAR ROADWAY WIDTH ON TIMBER CAP AND TIMBER PILES SHALL BE REMOVED. IN ADDITION, BULKHEADS AND BULKHEAD SUPPORT PILES SHALL BE REMOVED AND INCLUDED IN THE LUMP SUM PAY ITEM FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 14+96.00 -L-".

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 3.0 FT. BEYOND EACH SIDE OF THE PROPOSED STRUCTURE WIDTH AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 EVALUATING SCOUR AT BRIDGES".

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPlice OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 14+96.00 -L-".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR METALLIZED STEEL SHEET PILES, SEE SPECIAL PROVISIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

ALL METALLIZED SURFACES SHALL RECEIVE A SEAL COATING AS SPECIFIED IN THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE BENT CAPS AND SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR. FOR CALCIUM NITRITE CORROSION INHIBITOR, SEE SPECIAL PROVISIONS.

ALL BAR SUPPORTS USED IN THE PARAPET, SIDEWALK, AND BENT CAPS AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONCRETE IN THE END BENT CAPS OF END BENTS NO.1 & 2 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

PROJECT NO. **BD-5101N**
PASQUOTANK COUNTY
 STATION: **14 + 96.00 -L-**

SHEET 2 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 GENERAL DRAWING
**BRIDGE OVER CHARLES CREEK ON
 NC 34 (-L-) BETWEEN
 HERRINGTON ROAD AND
 S. MARTIN STREET**

CDM Smith
 CDM SMITH
 5400 Glenwood Avenue, Suite 300
 Raleigh, NC 27612-3228
 NC COA No. F-1255

DRAWN BY : R. STATION / J. SLOAN DATE : 05-14 DWG. No.
 CHECKED BY : J. B. TAYLOR DATE : 05-14
 DESIGN ENGINEER : J. B. TAYLOR DATE : 05-14



REVISIONS						SHEET No. S-2
No.	BY	DATE	No.	BY	DATE	
1			3			TOTAL SHEETS
2			4			

FILE: R:\indef\BD-5101N\Structures\PLANS\BD5101N_GD_02.dgn
 DATE: 5/28/14 12:27:50 PM

LOAD FACTORS:

DESIGN LOAD RATING FACTORS	LIMIT STATE	γ_{DC}	γ_{DW}
	STRENGTH I	1.25	1.50
	SERVICE III	1.00	1.00

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS																								
LEVEL	VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING (#)	MINIMUM RATING FACTORS (RF)	TONS = W x RF	STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE								COMMENT NUMBER
						MOMENT					SHEAR					MOMENT								
						LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)	LIVE-LOAD FACTORS (γ_{LL})	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (FT)		
DESIGN LOAD RATING	HL-93 (INVENTORY)	N/A	①	1.10	--	1.75	0.275	1.10	A	EL	28.375	0.517	1.14	A	EL	2.838	0.80	0.275	1.16	A	EL	28.375		
	HL-93 (OPERATING)	N/A	--	1.42	--	1.35	0.275	1.42	A	EL	28.375	0.517	1.48	A	EL	2.838	N/A	--	--	--	--	--		
	HS-20 (INVENTORY)	36.000	②	1.39	49.902	1.75	0.275	1.39	A	EL	28.375	0.517	1.39	A	EL	2.838	0.80	0.275	1.47	A	EL	28.375		
	HS-20 (OPERATING)	36.000	--	1.80	64.688	1.35	0.275	1.80	A	EL	28.375	0.517	1.80	A	EL	2.838	N/A	--	--	--	--	--		
LEGAL LOAD RATING	SINGLE VEHICLE (SV)	SNSH	13.500	--	3.13	42.188	1.40	0.275	3.69	A	EL	28.375	0.517	4.02	A	EL	2.838	0.80	0.275	3.13	A	EL	28.375	
		SNGARBS2	20.000	--	2.41	48.129	1.40	0.275	2.84	A	EL	28.375	0.517	2.90	A	EL	2.838	0.80	0.275	2.41	A	EL	28.375	
		SNAGRIS2	22.000	--	2.31	50.887	1.40	0.275	2.73	A	EL	28.375	0.517	2.70	A	EL	2.838	0.80	0.275	2.31	A	EL	28.375	
		SNCOTTS3	27.250	--	1.56	42.436	1.40	0.275	1.84	A	EL	28.375	0.517	2.01	A	EL	2.838	0.80	0.275	1.56	A	EL	28.375	
		SNAGGRS4	34.925	--	1.33	46.480	1.40	0.275	1.57	A	EL	28.375	0.517	1.69	A	EL	2.838	0.80	0.275	1.33	A	EL	28.375	
		SNS5A	35.550	--	1.30	46.194	1.40	0.275	1.53	A	EL	28.375	0.517	1.73	A	EL	2.838	0.80	0.275	1.30	A	EL	28.375	
		SNS6A	39.950	--	1.20	48.136	1.40	0.275	1.42	A	EL	28.375	0.517	1.59	A	EL	2.838	0.80	0.275	1.20	A	EL	28.375	
		SNS7B	42.000	--	1.15	48.213	1.40	0.275	1.36	A	EL	28.375	0.517	1.57	A	EL	2.838	0.80	0.275	1.15	A	EL	28.375	
	TRUCK TRACTOR SEMI-TRAILER (TTST)	TNAGRIT3	33.000	--	1.47	48.613	1.40	0.275	1.74	A	EL	28.375	0.517	1.88	A	EL	2.838	0.80	0.275	1.47	A	EL	28.375	
		TNT4A	33.075	--	1.48	49.055	1.40	0.275	1.75	A	EL	28.375	0.517	1.82	A	EL	2.838	0.80	0.275	1.48	A	EL	28.375	
		TNT6A	41.600	--	1.23	50.967	1.40	0.275	1.45	A	EL	28.375	0.517	1.70	A	EL	2.838	0.80	0.275	1.23	A	EL	28.375	
		TNT7A	42.000	--	1.24	51.999	1.40	0.275	1.46	A	EL	28.375	0.517	1.63	A	EL	2.838	0.80	0.275	1.24	A	EL	28.375	
		TNT7B	42.000	--	1.29	54.296	1.40	0.275	1.53	A	EL	28.375	0.517	1.53	A	EL	2.838	0.80	0.275	1.29	A	EL	28.375	
		TNAGRIT4	43.000	--	1.22	52.545	1.40	0.275	1.44	A	EL	28.375	0.517	1.47	A	EL	2.838	0.80	0.275	1.22	A	EL	28.375	
TNAGT5A	45.000	--	1.15	51.584	1.40	0.275	1.35	A	EL	28.375	0.517	1.48	A	EL	2.838	0.80	0.275	1.15	A	EL	28.375			
TNAGT5B	45.000	③	1.13	50.729	1.40	0.275	1.33	A	EL	28.375	0.517	1.40	A	EL	2.838	0.80	0.275	1.13	A	EL	28.375			

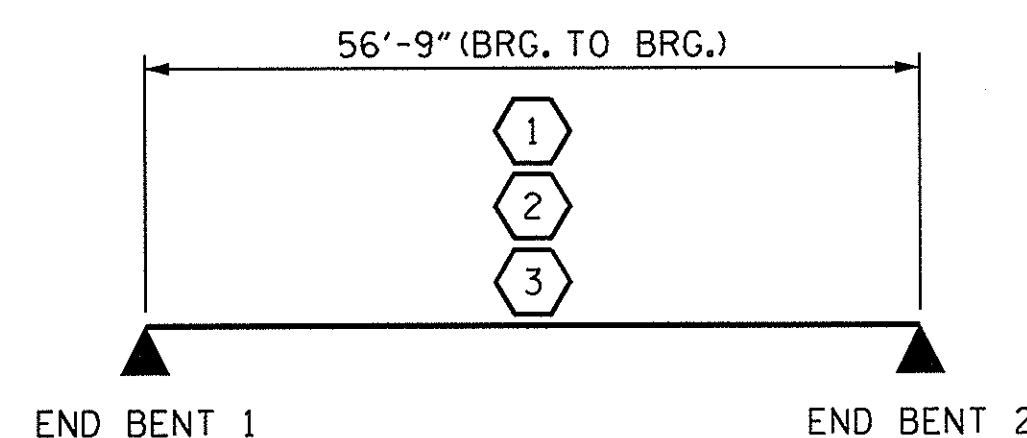
NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.
ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

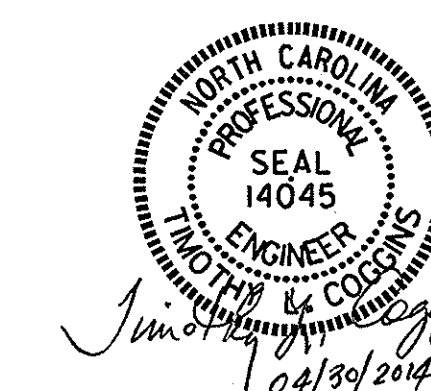
- 1.
- 2.
- 3.
- 4.

#	CONTROLLING LOAD RATING
①	DESIGN LOAD RATING (HL-93)
②	DESIGN LOAD RATING (HS-20)
③	LEGAL LOAD RATING **
** SEE CHART FOR VEHICLE TYPE	
GIRDER LOCATION	
I - INTERIOR GIRDER	
EL - EXTERIOR LEFT GIRDER	
ER - EXTERIOR RIGHT GIRDER	



LRFR SUMMARY

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

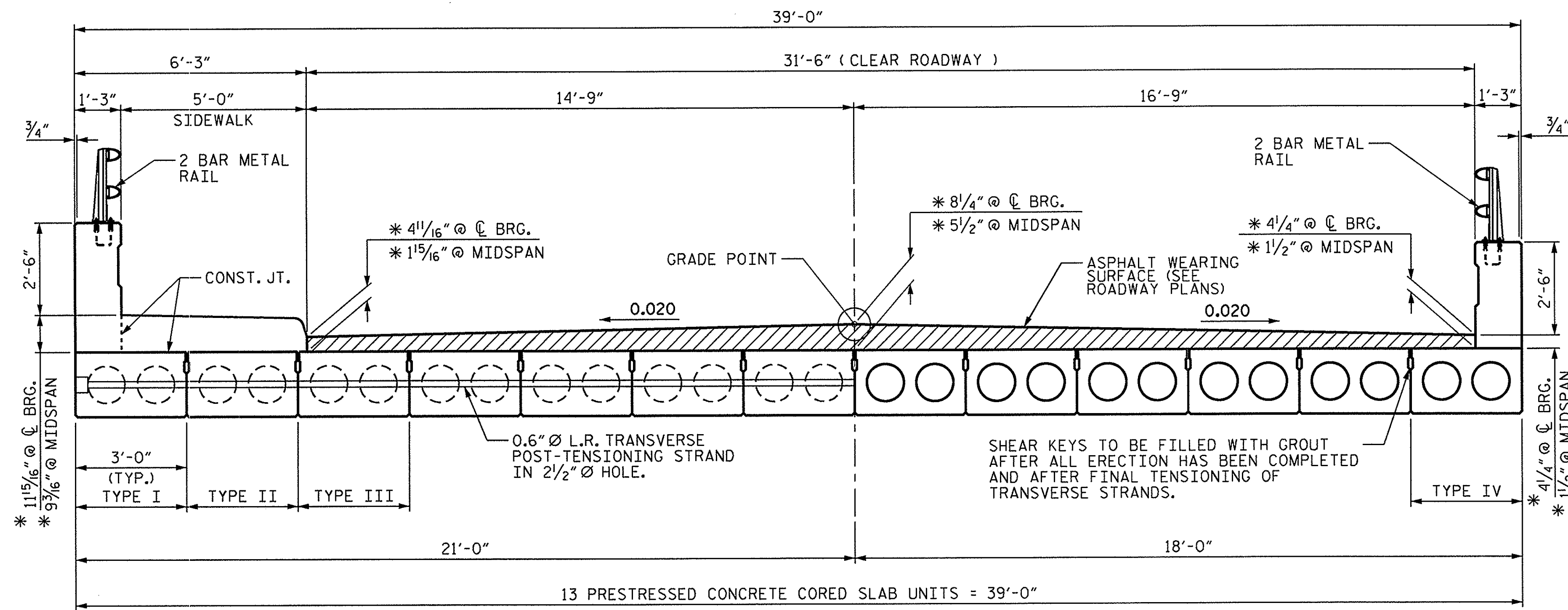


STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD
 LRFR SUMMARY FOR
 PRESTRESSED CORED
 SLAB UNIT 90° SKEW
 (NON-INTERSTATE TRAFFIC)

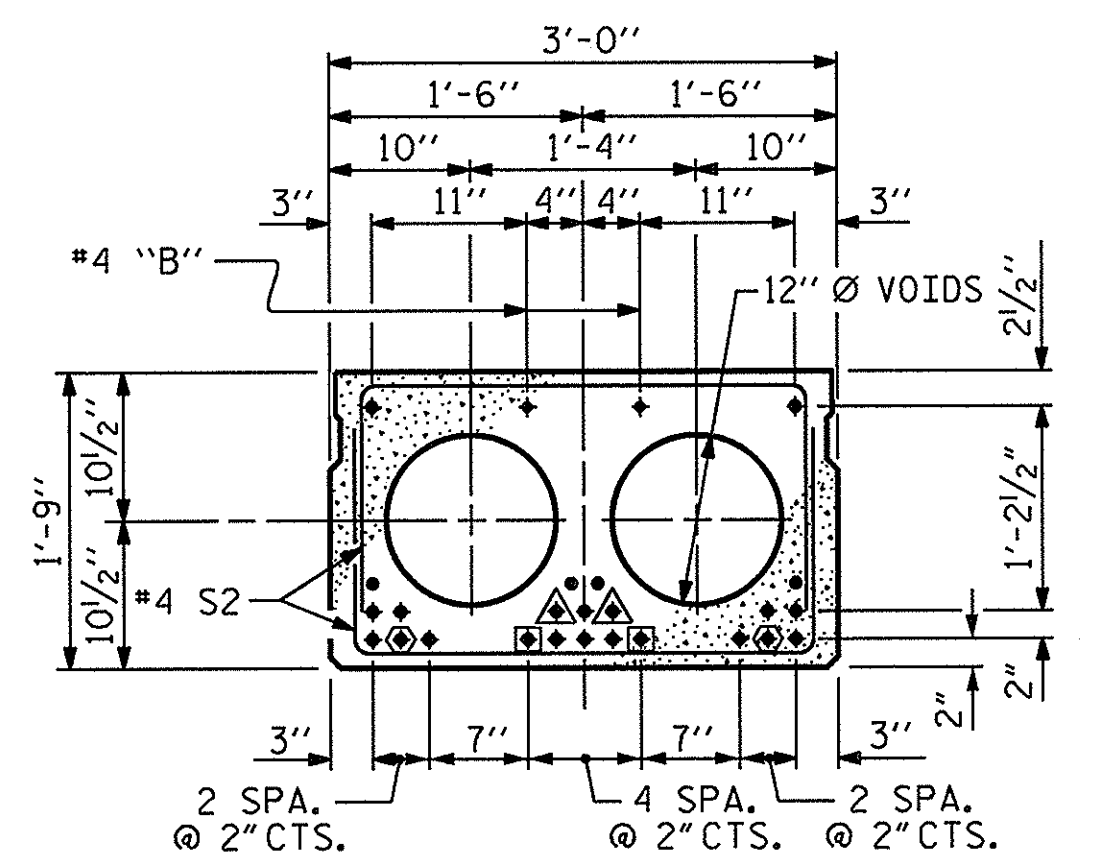
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-3
1			3			TOTAL SHEETS
2			4			

ASSEMBLED BY: M. Ruffin DATE: 4/10/14
 CHECKED BY: T. L. COGGINS DATE: 4/21/14
 DRAWN BY: MAA 1/08 REV. 11/12/08RR MAA/GM
 CHECKED BY: GM/DI 2/08 REV. 10/1/11 MAA/GM



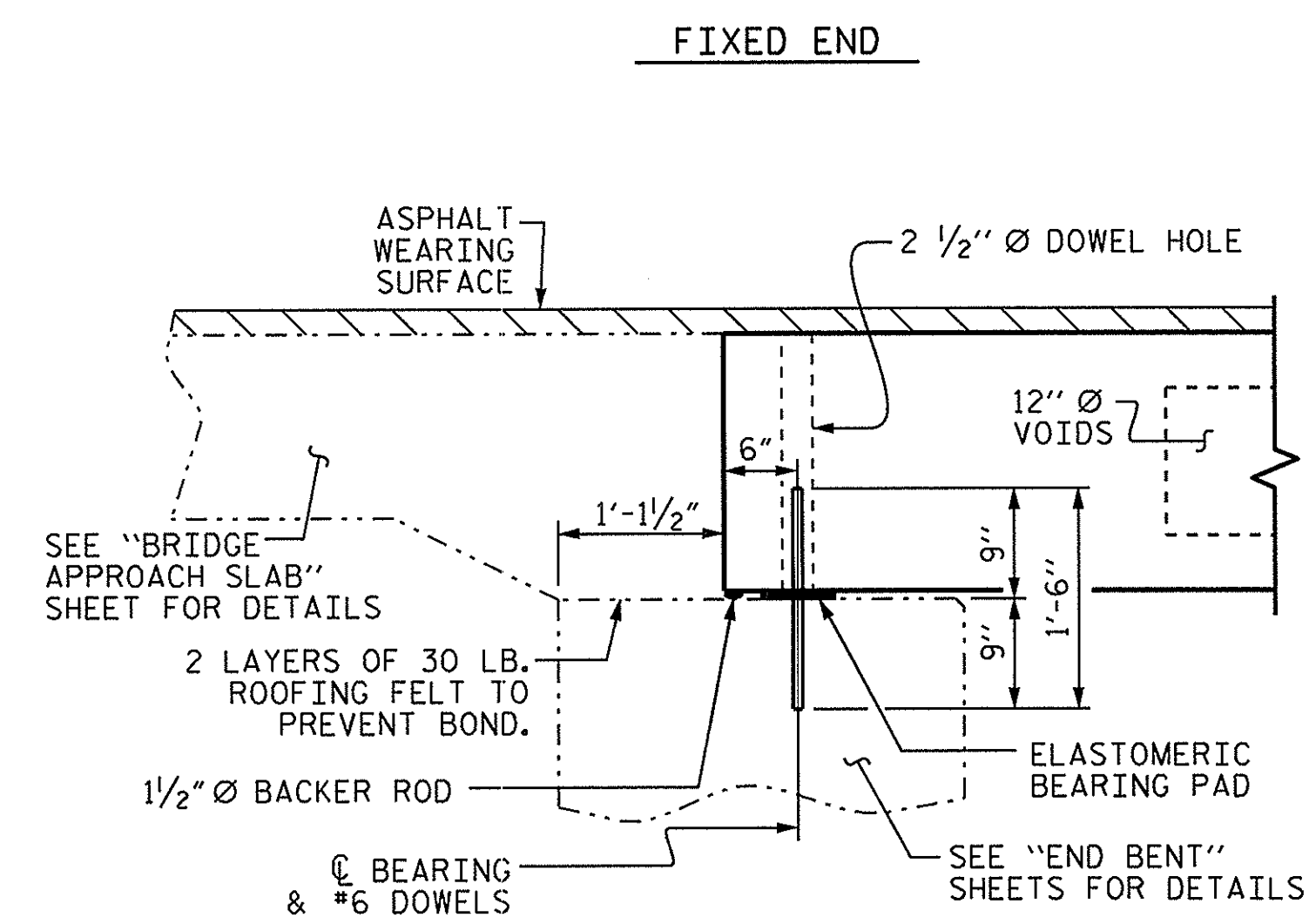
HALF SECTION AT INTERMEDIATE DIAPHRAGMS
 HALF SECTION THROUGH VOIDS
TYPICAL SECTION

* - THE MINIMUM AND MAXIMUM CONCRETE PARAPET HEIGHTS, SIDEWALK HEIGHT AND ASPHALT THICKNESSES ARE SHOWN. THE HEIGHT OF THE SIDEWALK, CONCRETE PARAPET AND ASPHALT WEARING SURFACE THICKNESSES VARY WHILE THE TOP OF THE CONCRETE PARAPET FOLLOWS THE PROFILE OF THE GUTTERLINE.

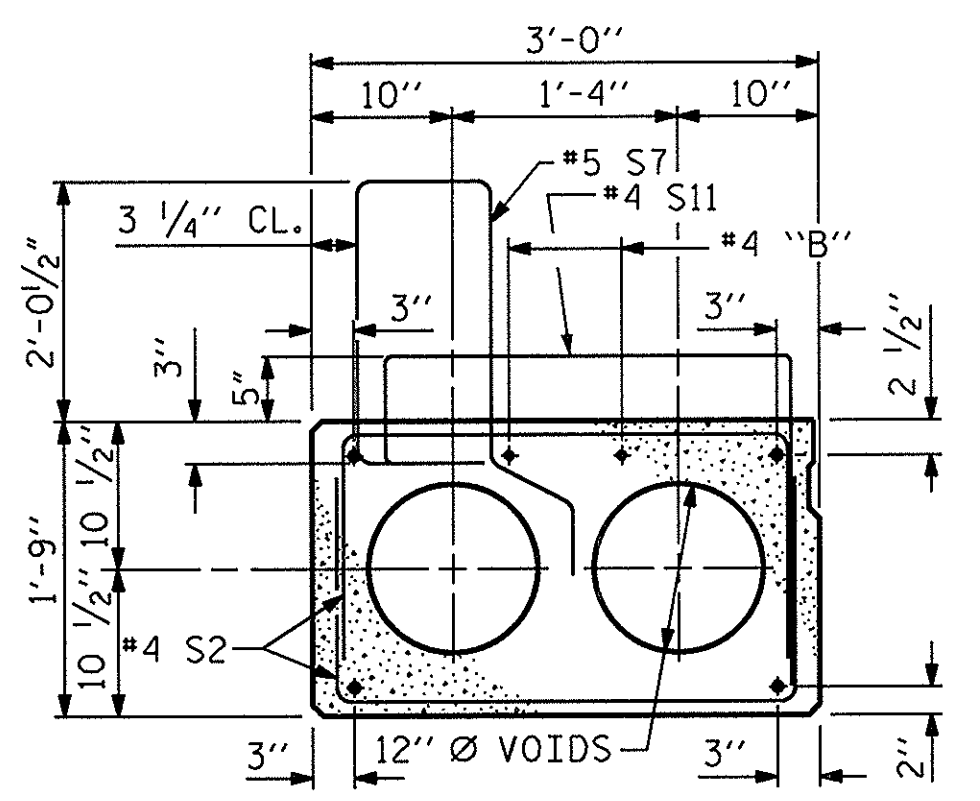


TYPE III - INTERIOR SLAB SECTION
 SPAN A (57'-9" UNIT)
 (24 STRANDS REQUIRED)
0.6" Ø LOW RELAXATION STRAND LAYOUT

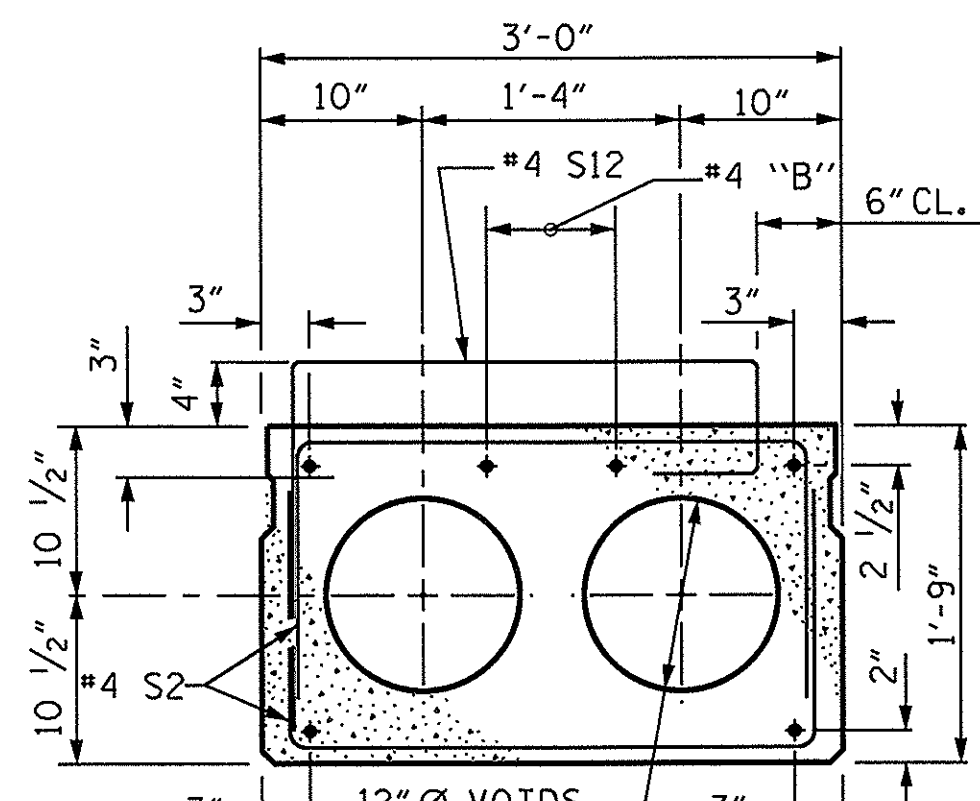
- DEBONDING LEGEND**
- ▲ BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
 - BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 8'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
 - BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 14'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.



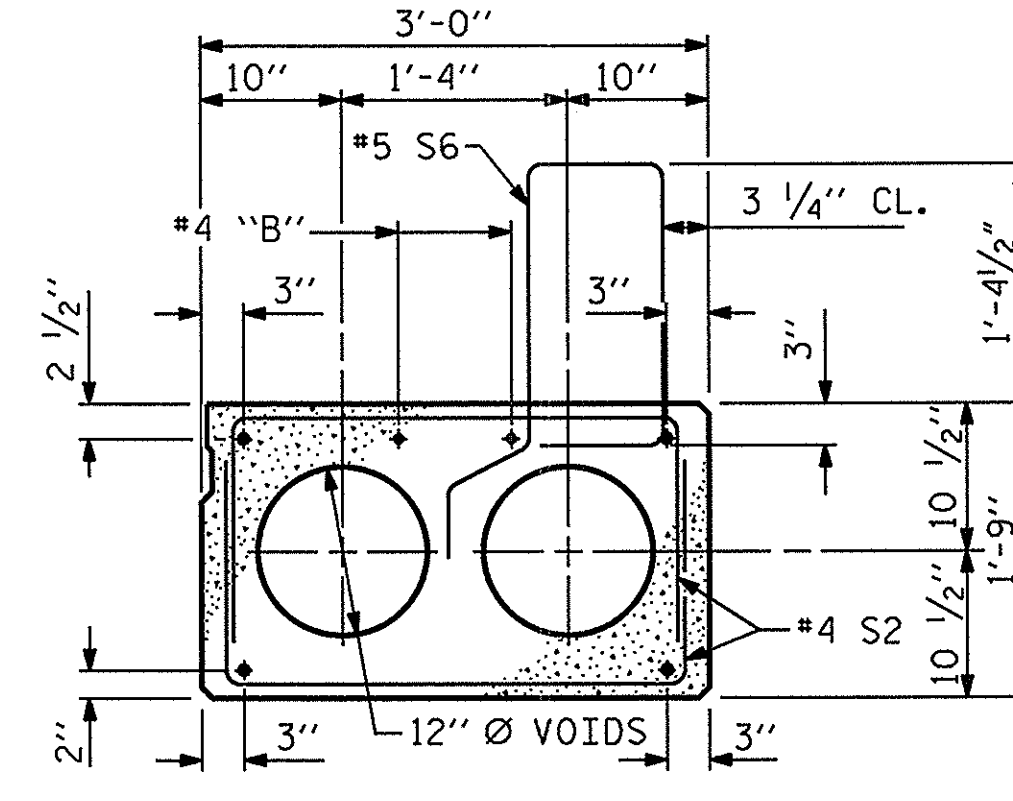
SECTION AT END BENT



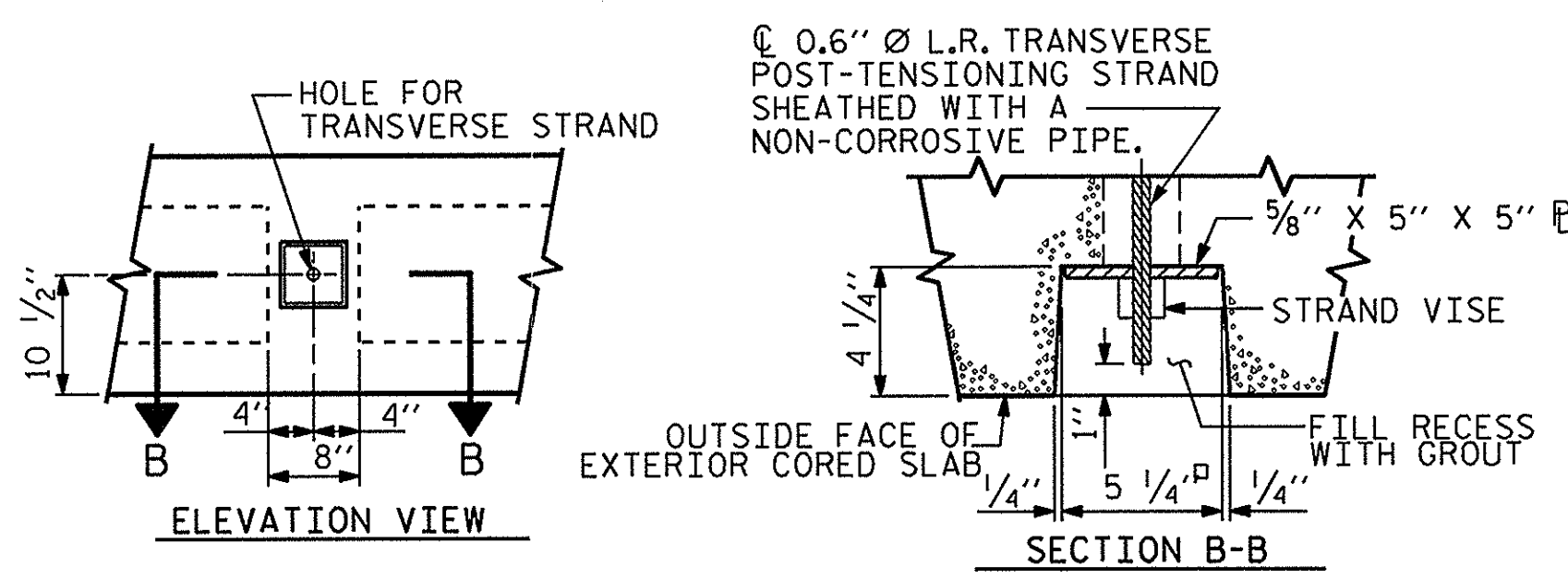
TYPE I - EXTERIOR SLAB SIDEWALK SECTION
 (FOR PRESTRESSED STRAND LAYOUT, SEE TYPE III - INTERIOR SLAB SECTION)



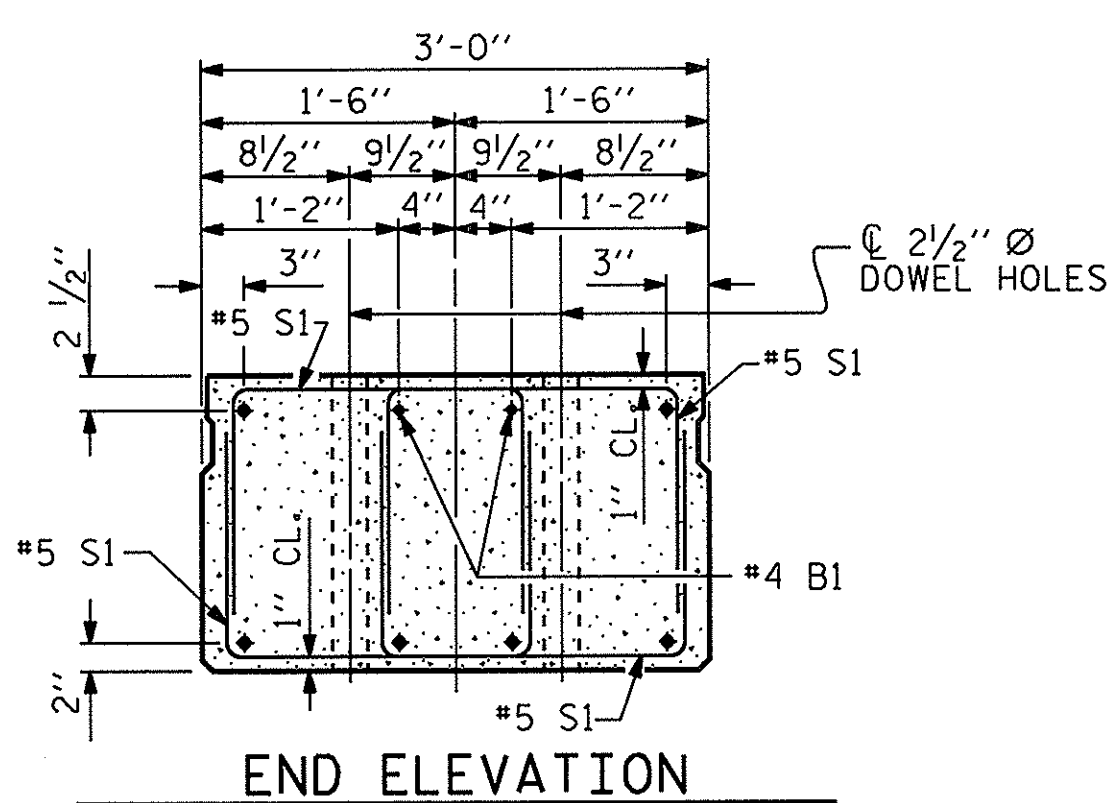
TYPE II - INTERIOR SLAB SIDEWALK SECTION
 (FOR PRESTRESSED STRAND LAYOUT, SEE TYPE III - INTERIOR SLAB SECTION)



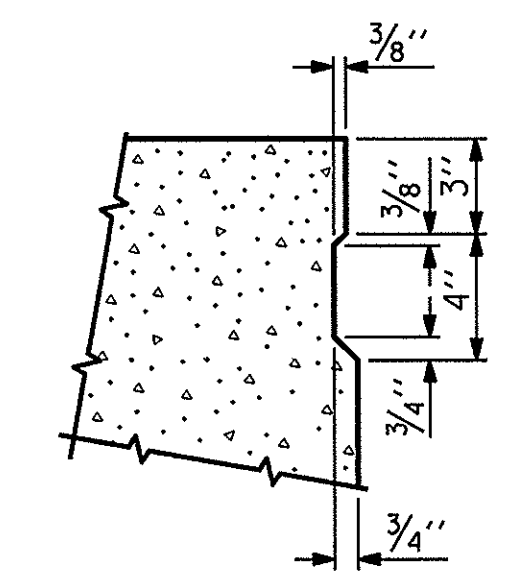
TYPE IV - EXTERIOR SLAB SECTION
 (FOR PRESTRESSED STRAND LAYOUT, SEE TYPE III - INTERIOR SLAB SECTION)



GROUTED RECESS AT END OF POST-TENSIONED STRAND-CORED SLABS



END ELEVATION
 SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN). INTERIOR SLAB SECTION SHOWN-EXTERIOR SLAB SECTION SIMILAR EXCEPT SHEAR KEY LOCATION.



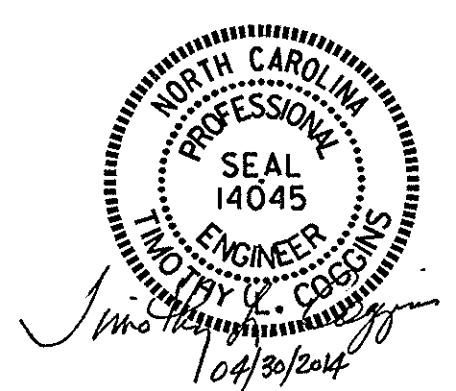
SHEAR KEY DETAIL
 NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

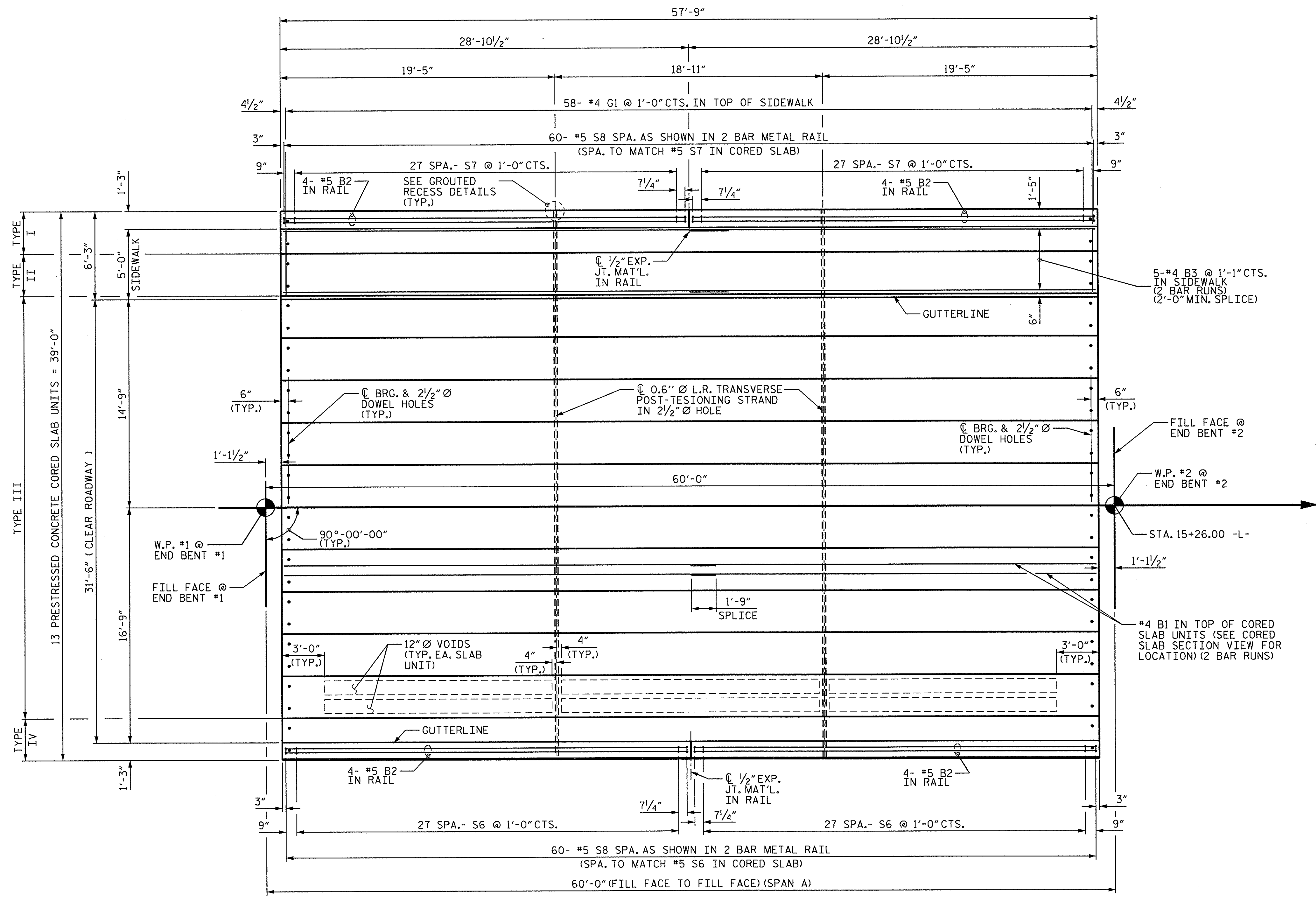
DRAWN BY: N. Ruffin DATE: 4/10/14
 CHECKED BY: T.L. COGGINS DATE: 4/21/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

30-APR-2014 14:42 S:\DPROJ\Tim\Division 1 LIBR\BD-5101N\ruffin\BD5101N_sm.Design.01.dgn nruffin

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						SHEET NO. S-4
SUPERSTRUCTURE 3'-0" x 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT						
REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			
2			4			





PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 1 OF 3

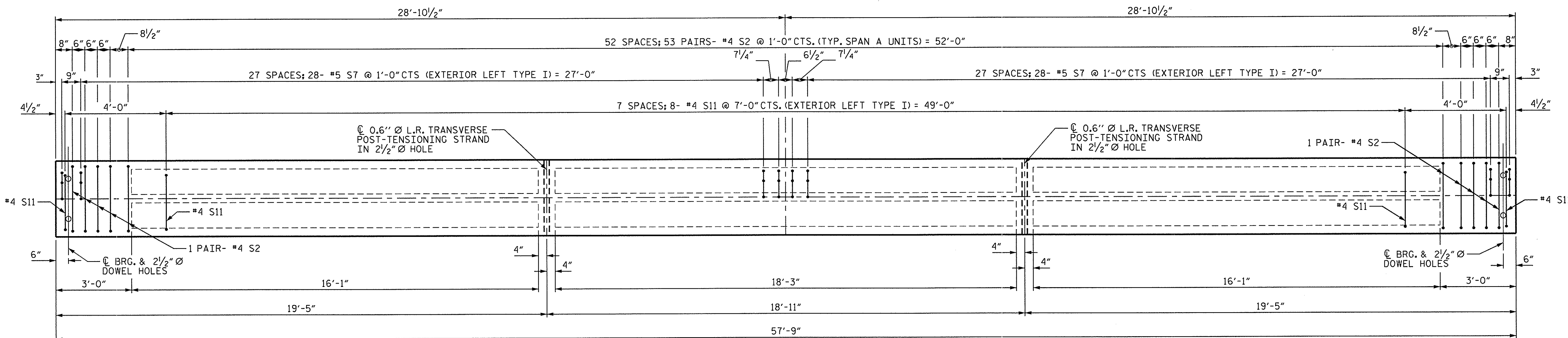
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN
 SPAN A



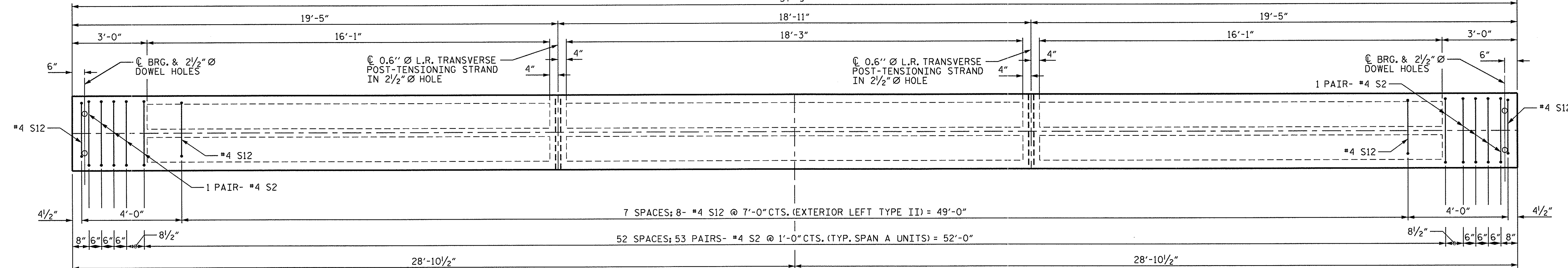
DRAWN BY : N. Ruffin DATE : 4/10/14
 CHECKED BY : T. L. COGGINS DATE : 4/22/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE : 4/30/14

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-5	
1			3			TOTAL SHEETS	
2			4				



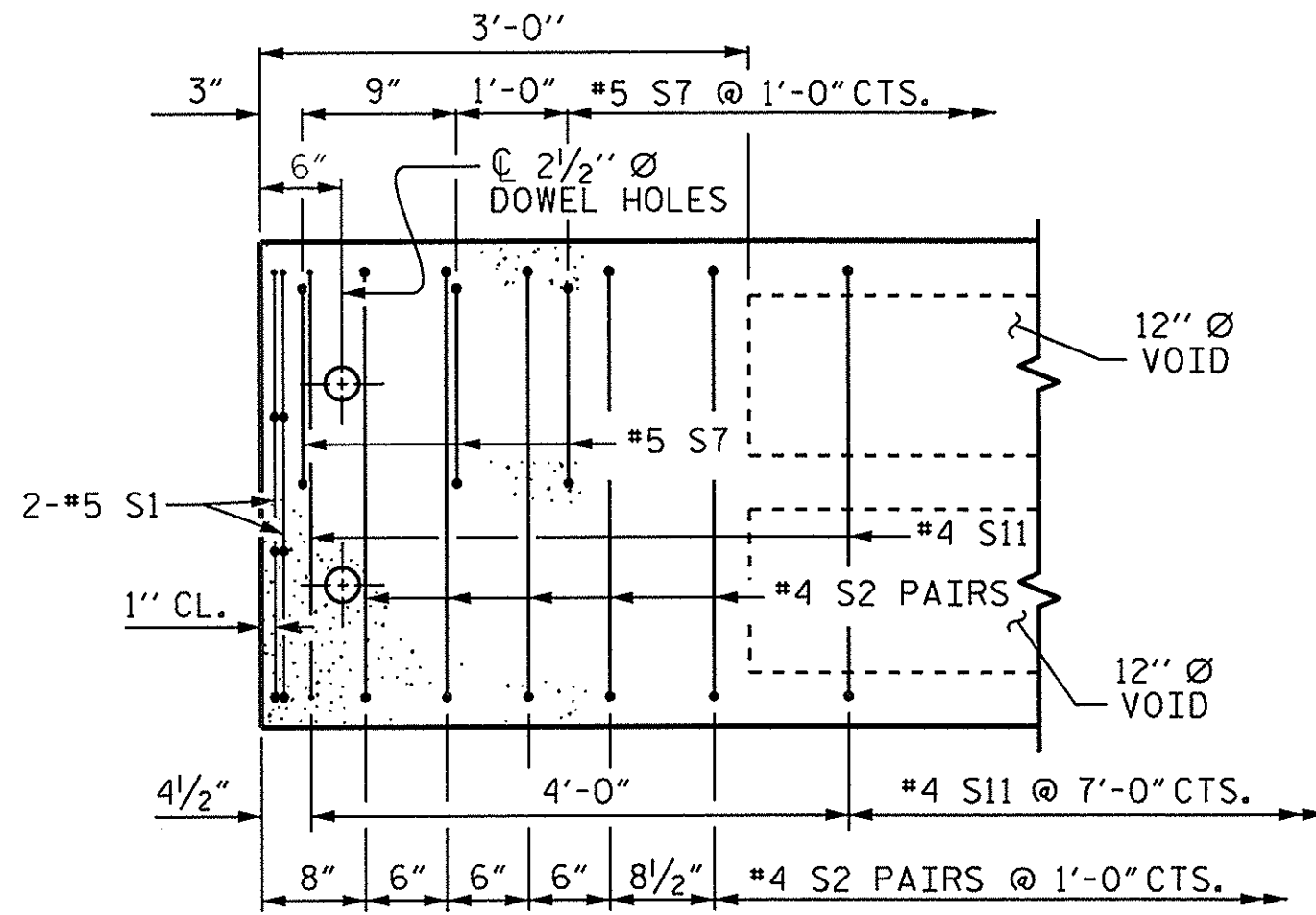
PLAN OF TYPE I CORED SLAB UNIT

FOR LOCATION OF S1 BARS AT ENDS OF SLAB, SEE "PART PLAN - EXTERIOR SECTIONS"



PLAN OF TYPE II CORED SLAB UNIT

FOR LOCATION OF S1 BARS AT ENDS OF SLAB, SEE "PART PLAN - EXTERIOR SECTIONS"



TYPE I EXTERIOR SECTION

TYPE II SIMILAR EXCEPT OMIT S7 BARS AND SUBSTITUTE #4 S11 BARS WITH #4 S12 BARS

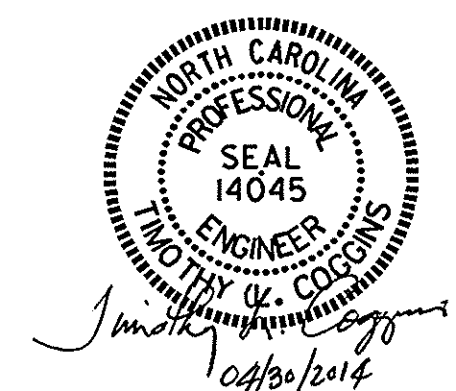
PART PLAN - EXTERIOR SECTIONS

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 2 OF 3

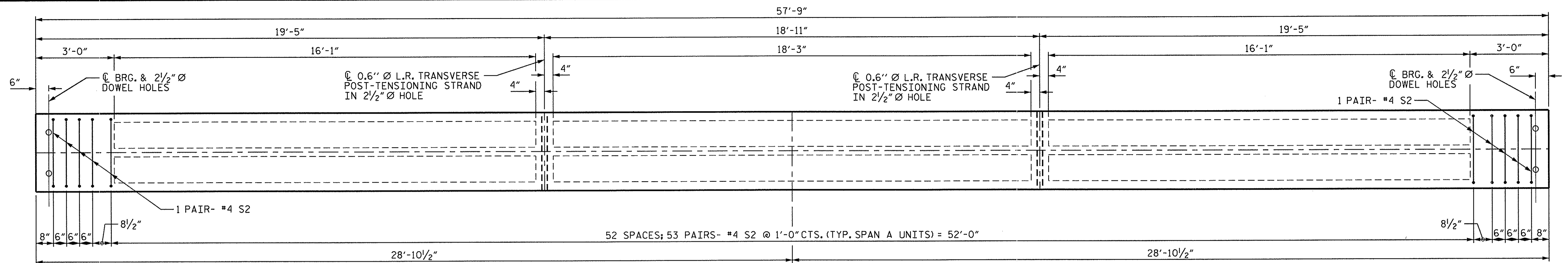
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN
 SPAN A
 DETAILS

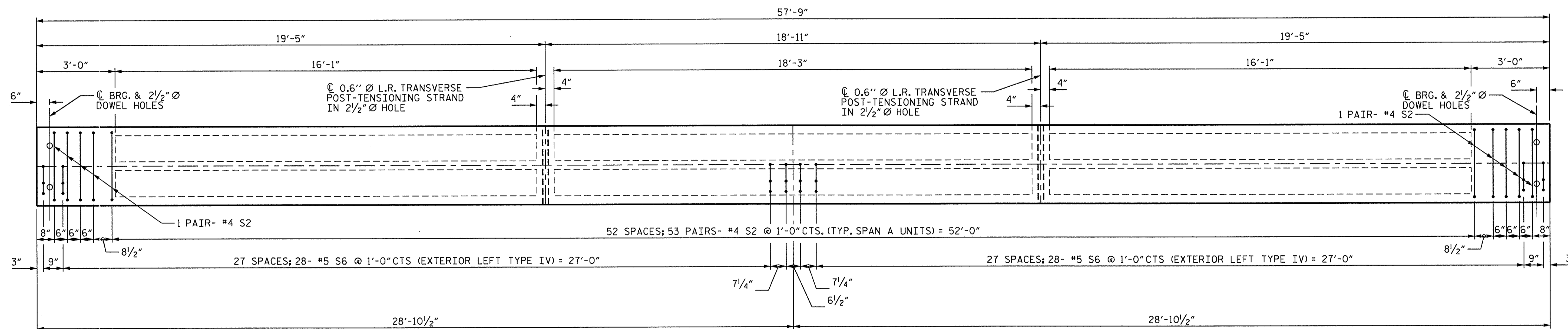


DRAWN BY: N. Ruffin DATE: 4/11/14
 CHECKED BY: T.L. COGGINS DATE: 4/22/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

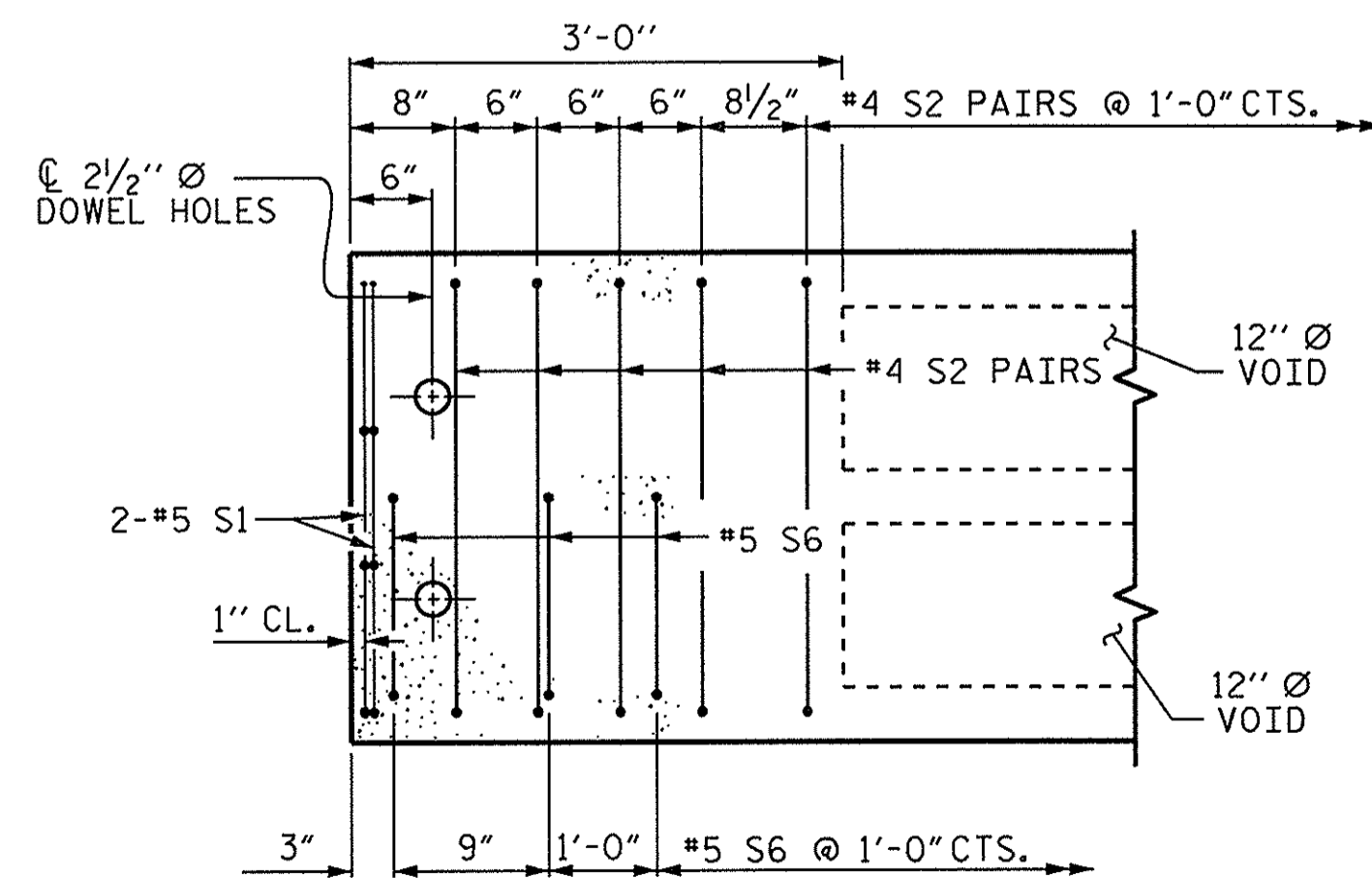
REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			3
2			4			4



PLAN OF TYPE III CORED SLAB UNIT
 FOR LOCATION OF S1 BARS AT ENDS OF SLAB, SEE "PART PLAN - EXTERIOR SECTIONS"



PLAN OF TYPE IV CORED SLAB UNIT
 FOR LOCATION OF S1 BARS AT ENDS OF SLAB, SEE "PART PLAN - EXTERIOR SECTIONS"



TYPE IV EXTERIOR SECTION
 TYPE III SIMILAR EXCEPT OMIT S6 BARS

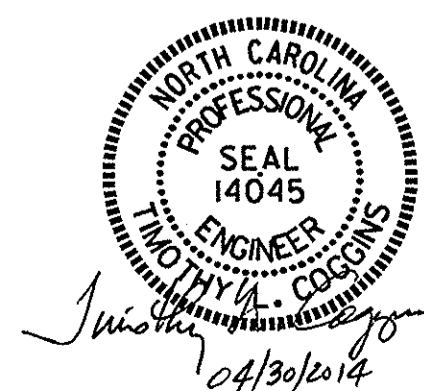
PART PLAN - EXTERIOR SECTIONS

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 3 OF 3

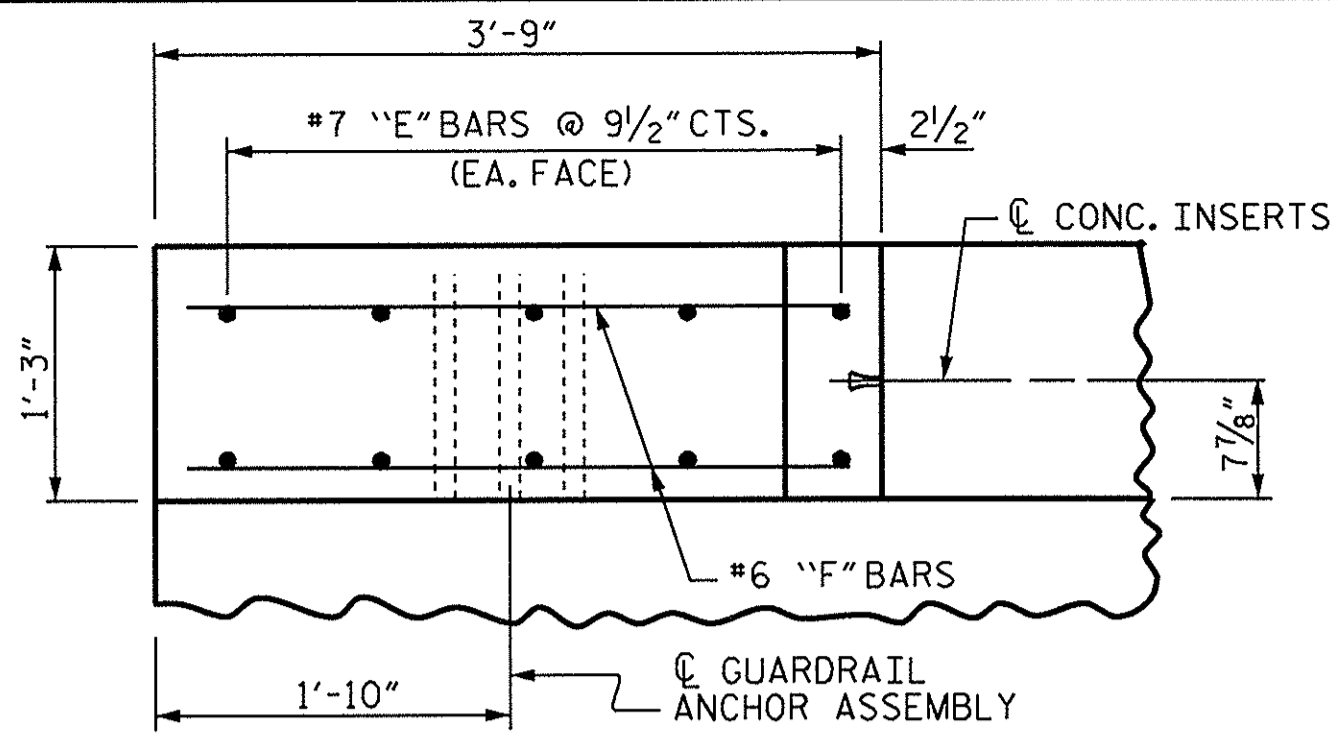
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUPERSTRUCTURE
 PLAN OF SPAN
 SPAN A
 DETAILS

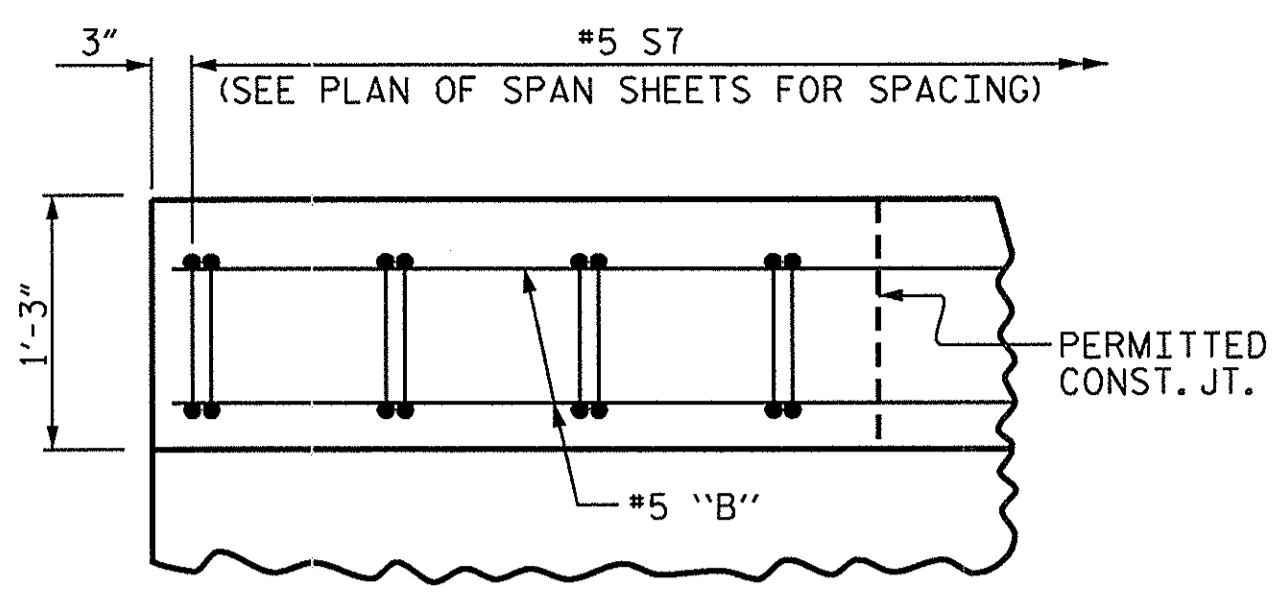


DRAWN BY: N. Ruffin DATE: 4/11/14
 CHECKED BY: T. L. COGGINS DATE: 4/22/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

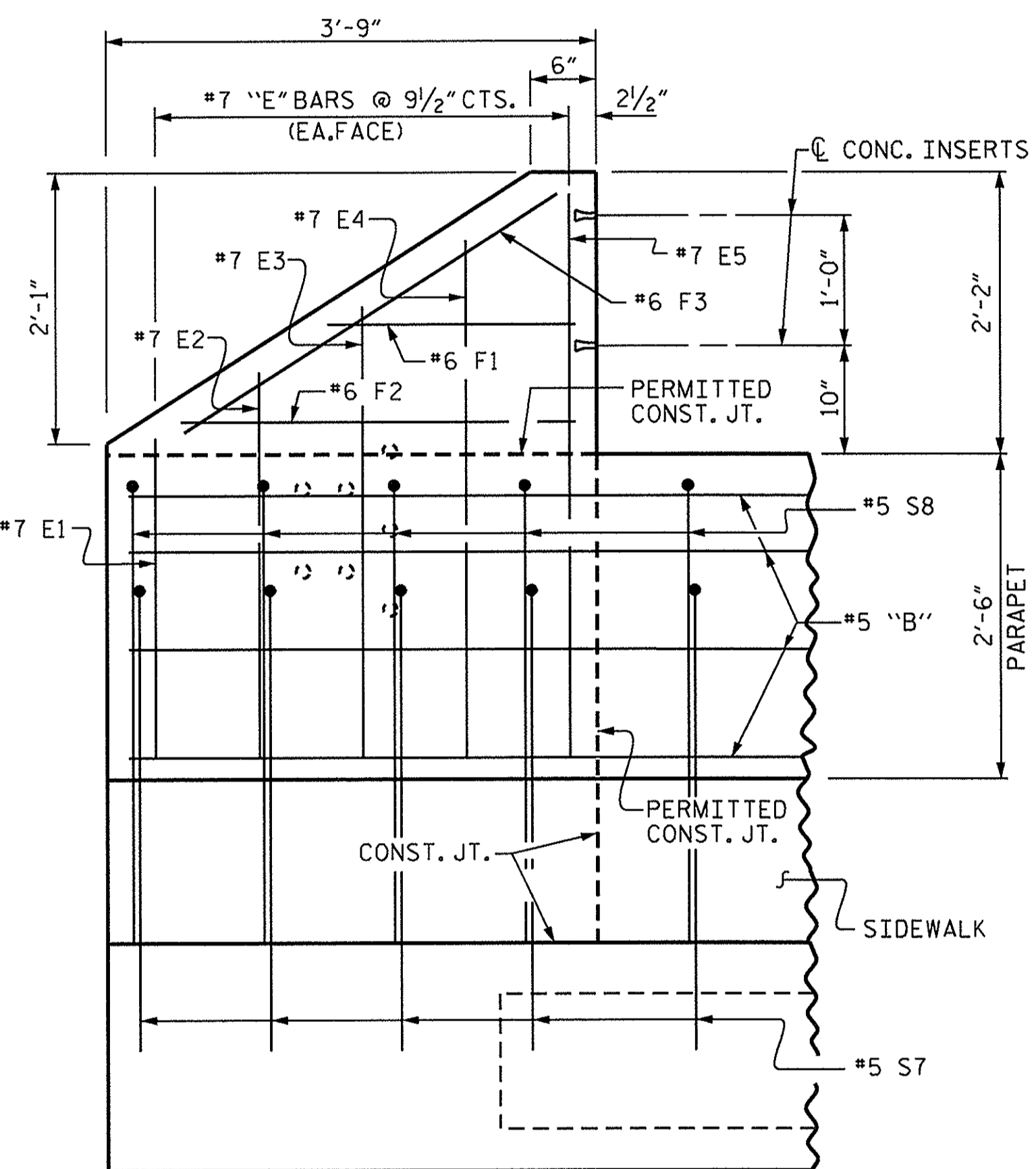
REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-7	
1			3			TOTAL SHEETS	
2			4				



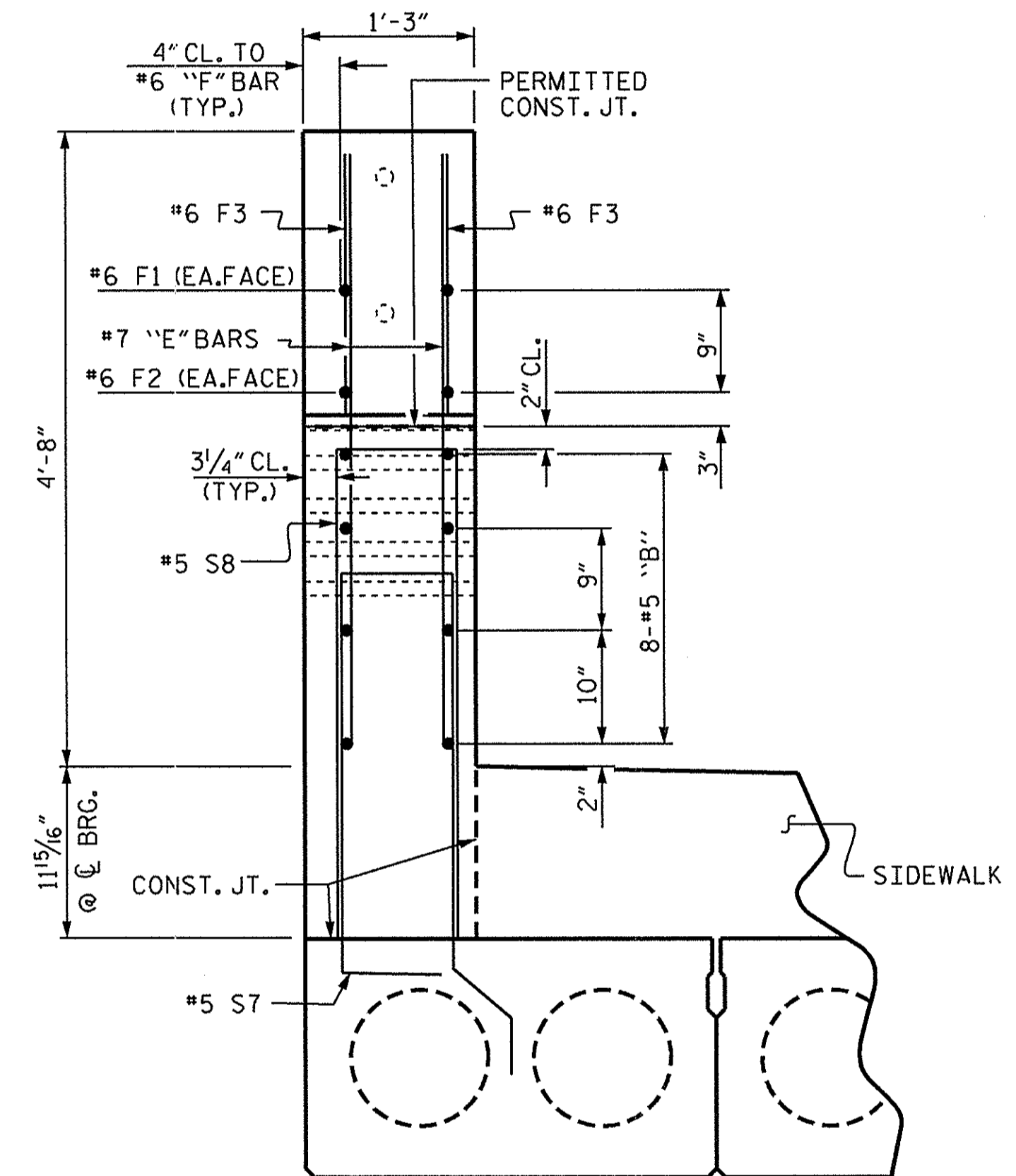
PLAN OF END POST



PLAN OF PARAPET



ELEVATION

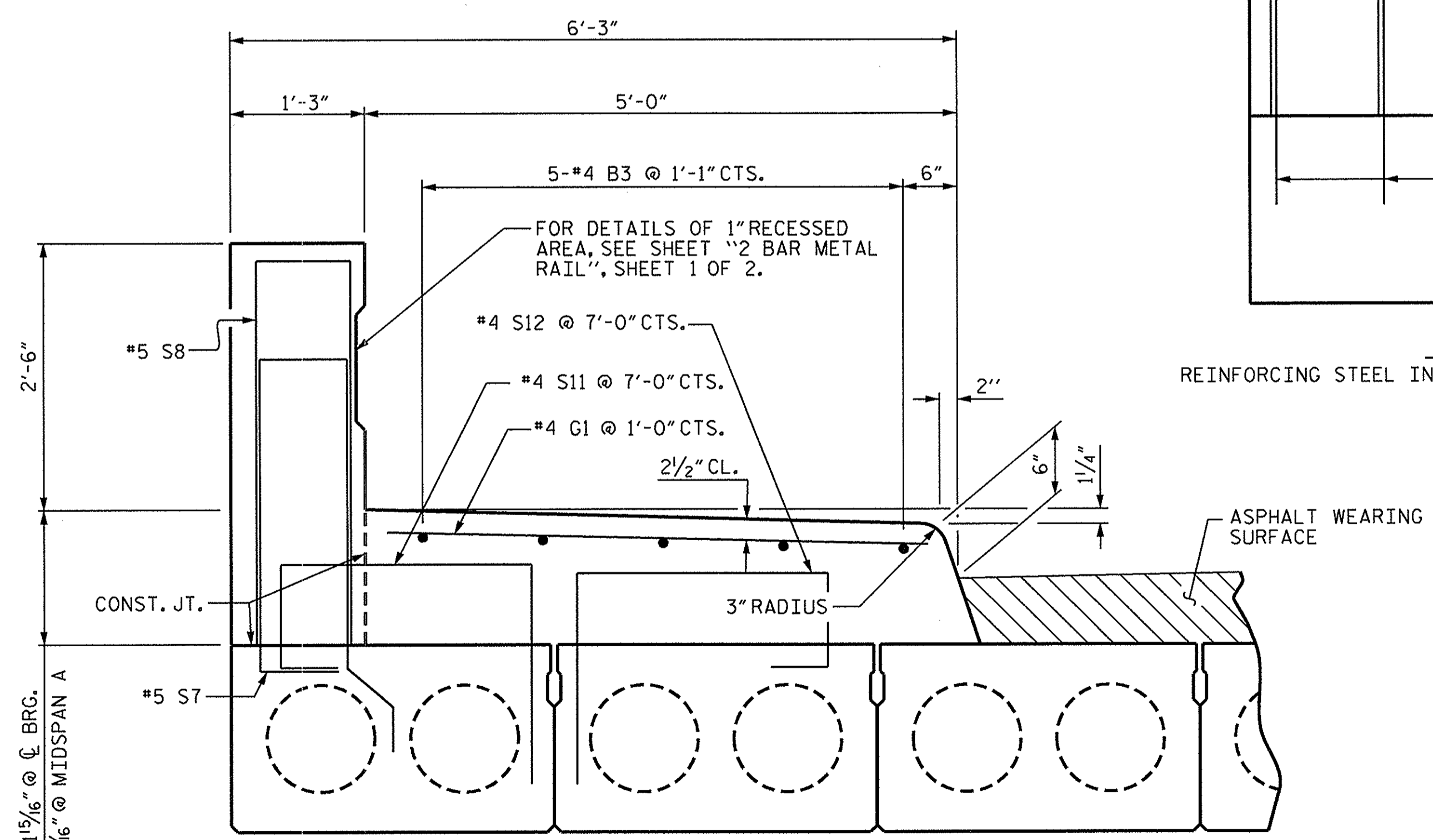


END VIEW

REINFORCING STEEL IN SIDEWALK NOT SHOWN FOR CLARITY

PARAPET AND END POST FOR TWO BAR RAIL

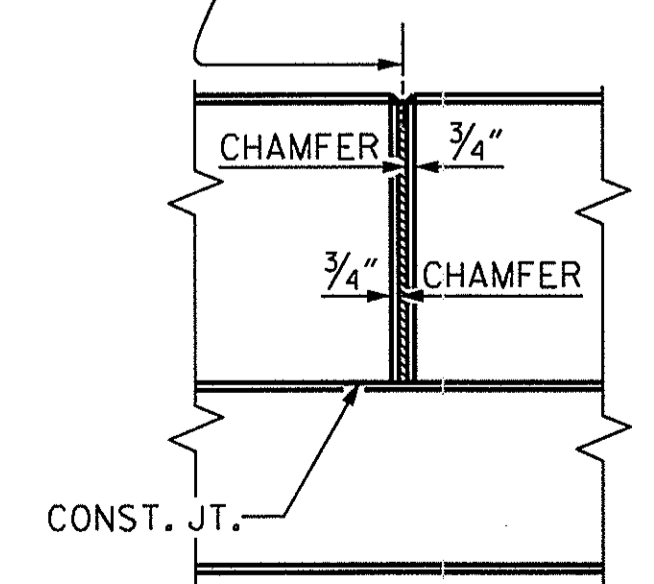
THE #5S8 MAY BE SHIFTED AS NECESSARY TO PROVIDE CLEARANCE TO THE 1/4" Ø HOLES.



SECTION THROUGH SIDEWALK

#5 'B' BARS IN PARAPET NOT SHOWN FOR CLARITY
SEE PLAN OF SPAN SHEETS FOR LOCATION OF STEEL IN SIDEWALK

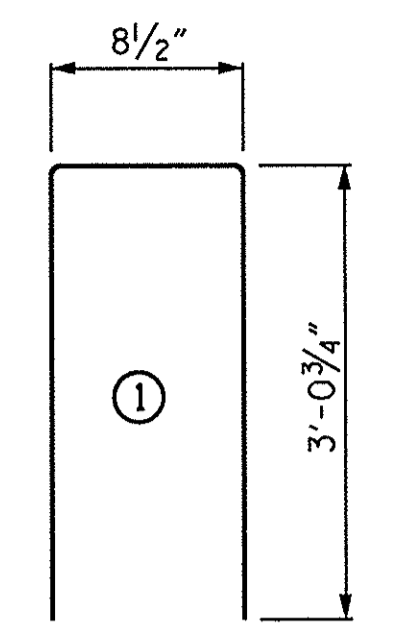
1/2" EXP. JT. MAT'L. IN PARAPET & SIDEWALK HELD IN PLACE WITH GALVANIZED NAILS.



ELEVATION AT EXPANSION JOINTS

PARAPET DETAILS

BAR TYPE



ALL BAR DIMENSIONS ARE OUT TO OUT

BILL OF MATERIAL

PARAPET AND END POSTS

BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT
*B2	16	#5	STR.	28'-6"	476
*E1	4	#7	STR.	2'-5"	20
*E2	4	#7	STR.	2'-11"	24
*E3	4	#7	STR.	3'-5"	28
*E4	4	#7	STR.	3'-11"	32
*E5	4	#7	STR.	4'-4"	35
*F1	4	#6	STR.	1'-10"	11
*F2	4	#6	STR.	3'-0"	18
*F3	4	#6	STR.	3'-4"	20
*S8	60	#5	1	6'-10"	428

* EPOXY COATED REINFORCING STEEL 1,092 LBS.
CLASS AA CONCRETE (PARAPET & END POSTS) 9.5 CU. YDS.
1'-3" X 2'-6" CONCRETE PARAPET 57.75 LIN. FT

SIDEWALK

BAR NO.	NO.	SIZE	TYPE	LENGTH	WEIGHT
*B2	10	#4	STR.	29'-8"	198
*G1	58	#4	STR.	4'-7"	178

* EPOXY COATED REINFORCING STEEL (SIDEWALK) 376 LBS.
CLASS AA CONCRETE (SIDEWALK) 8.8 CU. YDS.

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

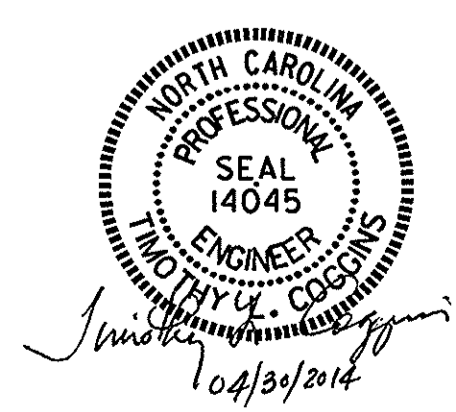
SHEET 1 OF 2

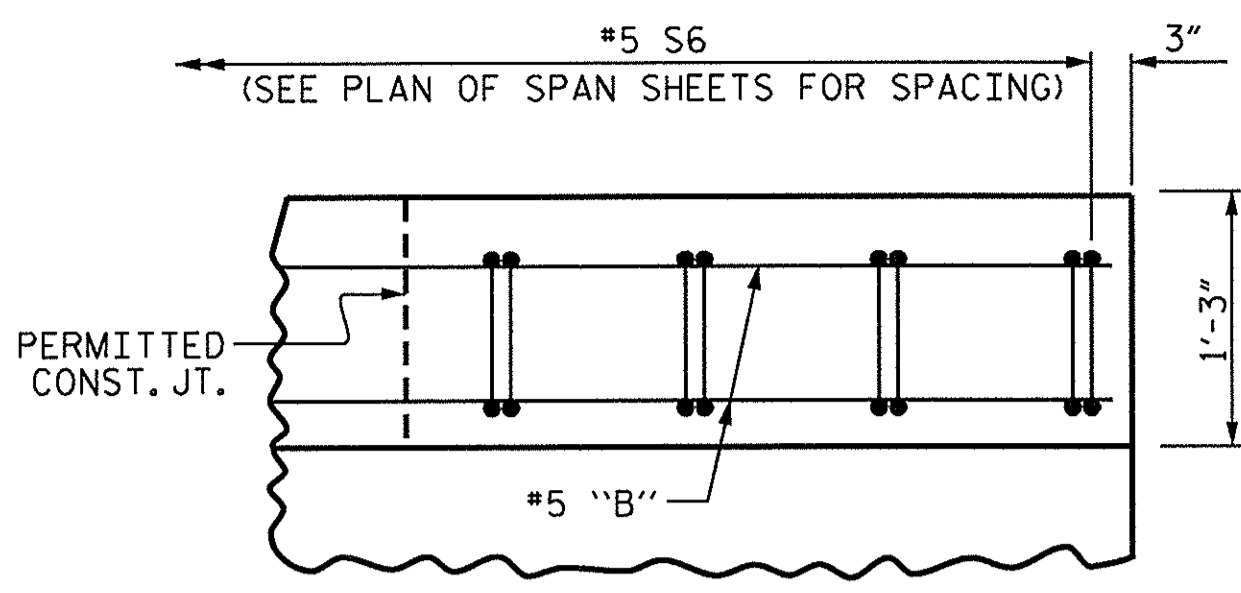
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUPERSTRUCTURE
CONCRETE PARAPET,
END POST, &
SIDEWALK DETAILS
LEFT SIDE

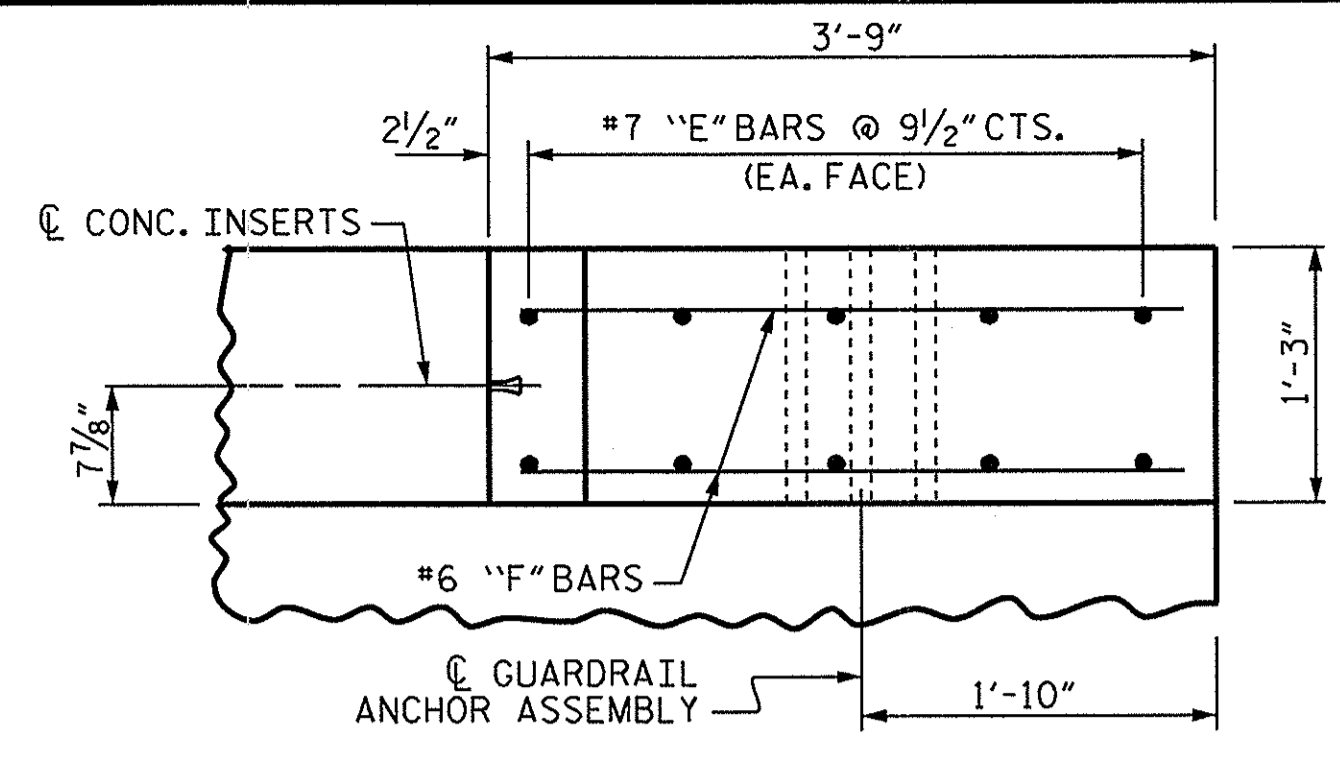
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-8
1			3			TOTAL SHEETS
2			4			

DRAWN BY: N. Ruffin DATE: 4/11/14
CHECKED BY: T.L. COGGINS DATE: 4/23/14
DESIGN ENGINEER OF RECORD: N.M. RUFFIN DATE: 4/30/14



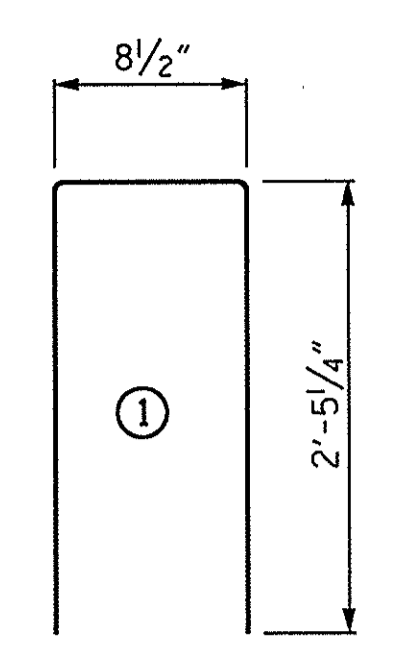


PLAN OF PARAPET



PLAN OF END POST

BAR TYPE



ALL BAR DIMENSIONS ARE OUT TO OUT

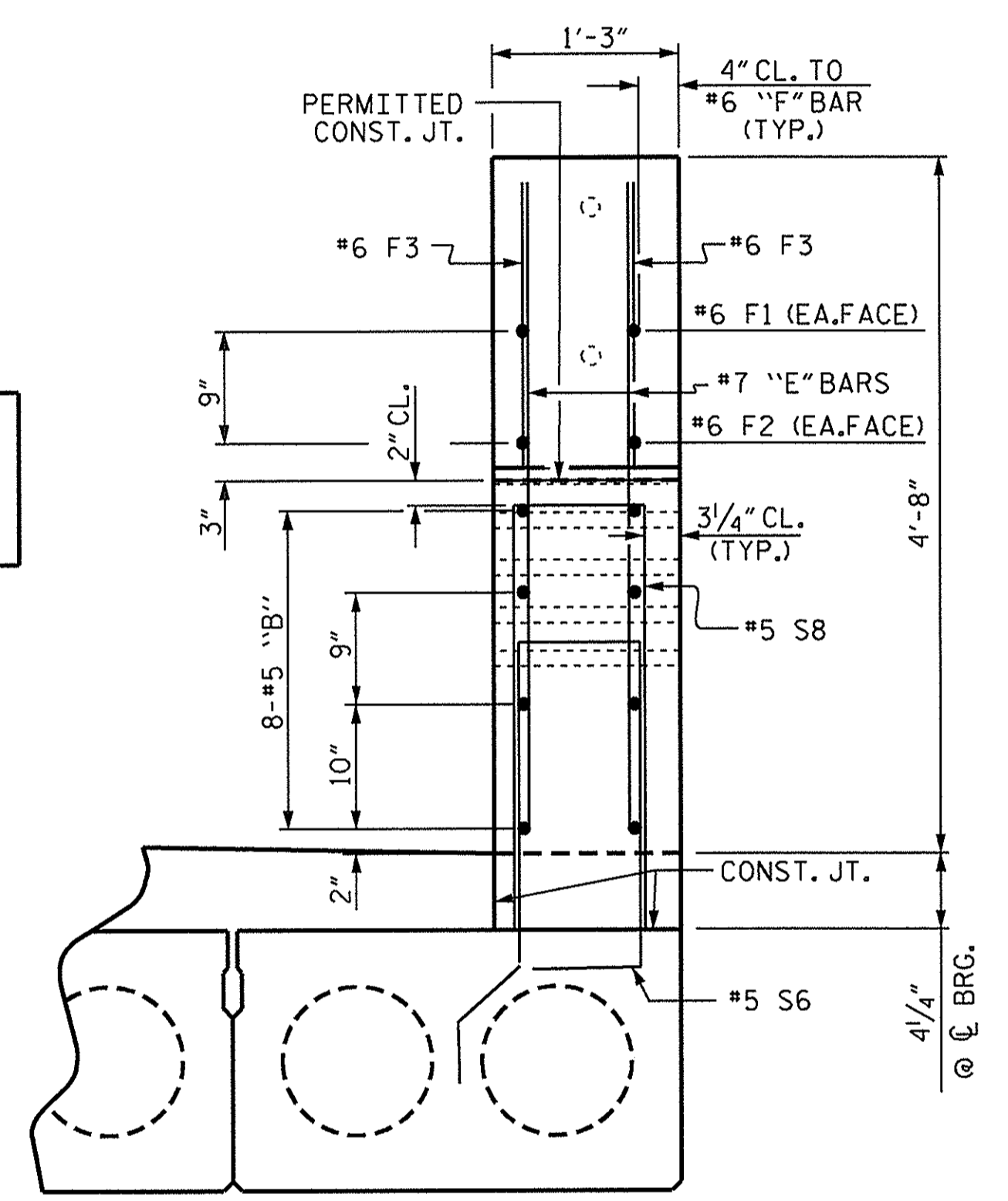
BILL OF MATERIAL

PARAPET AND END POSTS

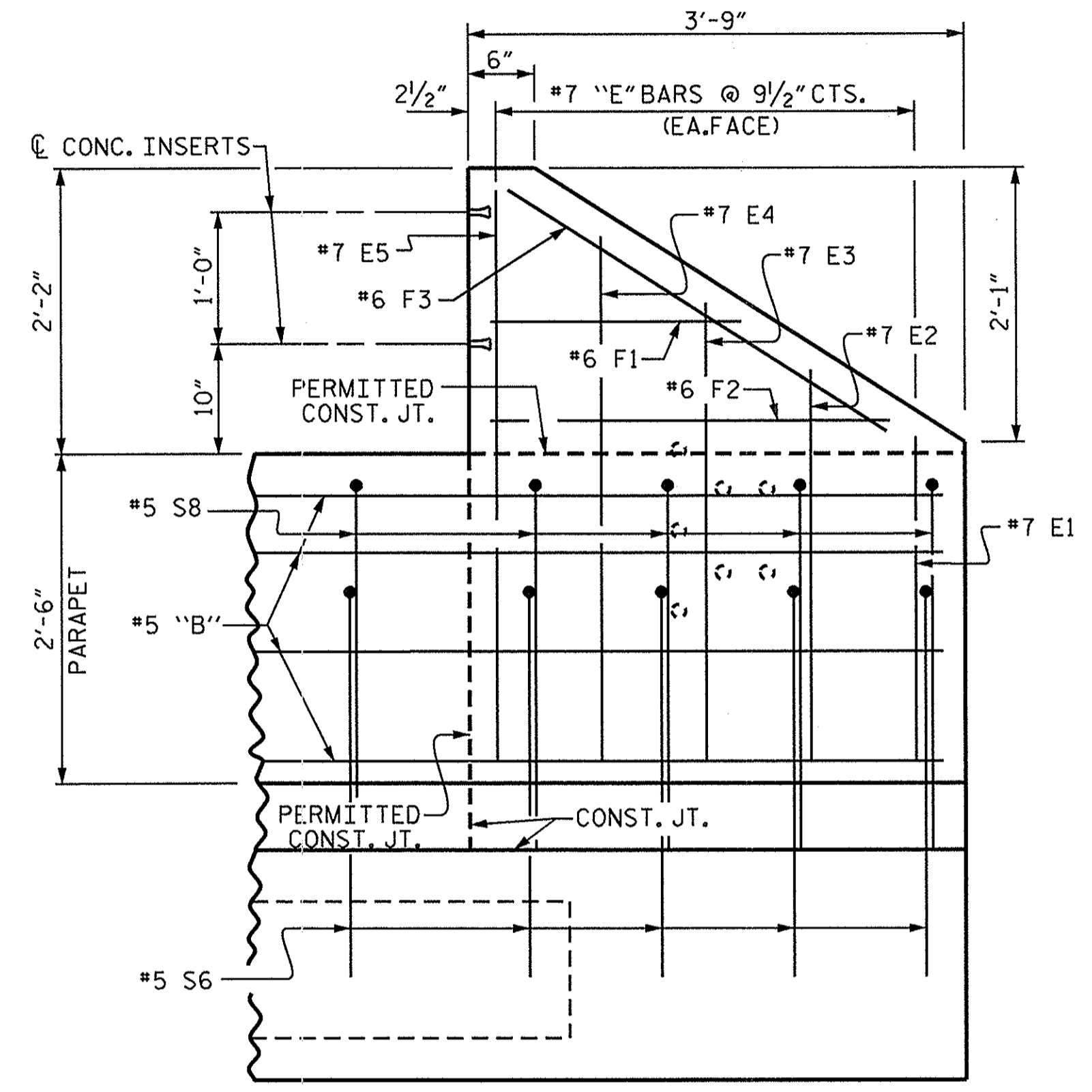
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
*B2	16	#5	STR.	28'-6"	476
*E1	4	#7	STR.	2'-5"	20
*E2	4	#7	STR.	2'-11"	24
*E3	4	#7	STR.	3'-5"	28
*E4	4	#7	STR.	3'-11"	32
*E5	4	#7	STR.	4'-4"	35
*F1	4	#6	STR.	1'-10"	11
*F2	4	#6	STR.	3'-0"	18
*F3	4	#6	STR.	3'-4"	20
*S8	60	#5	1	5'-7"	349

* EPOXY COATED REINFORCING STEEL 1,013 LBS.
 CLASS AA CONCRETE (PARAPET & END POSTS) 7.8 CU. YDS.
 1'-3" X 2'-6" CONCRETE PARAPET 57.75 LIN. FT

THE #5S8 MAY BE SHIFTED AS NECESSARY TO PROVIDE CLEARANCE TO THE 1/4" Ø HOLES.

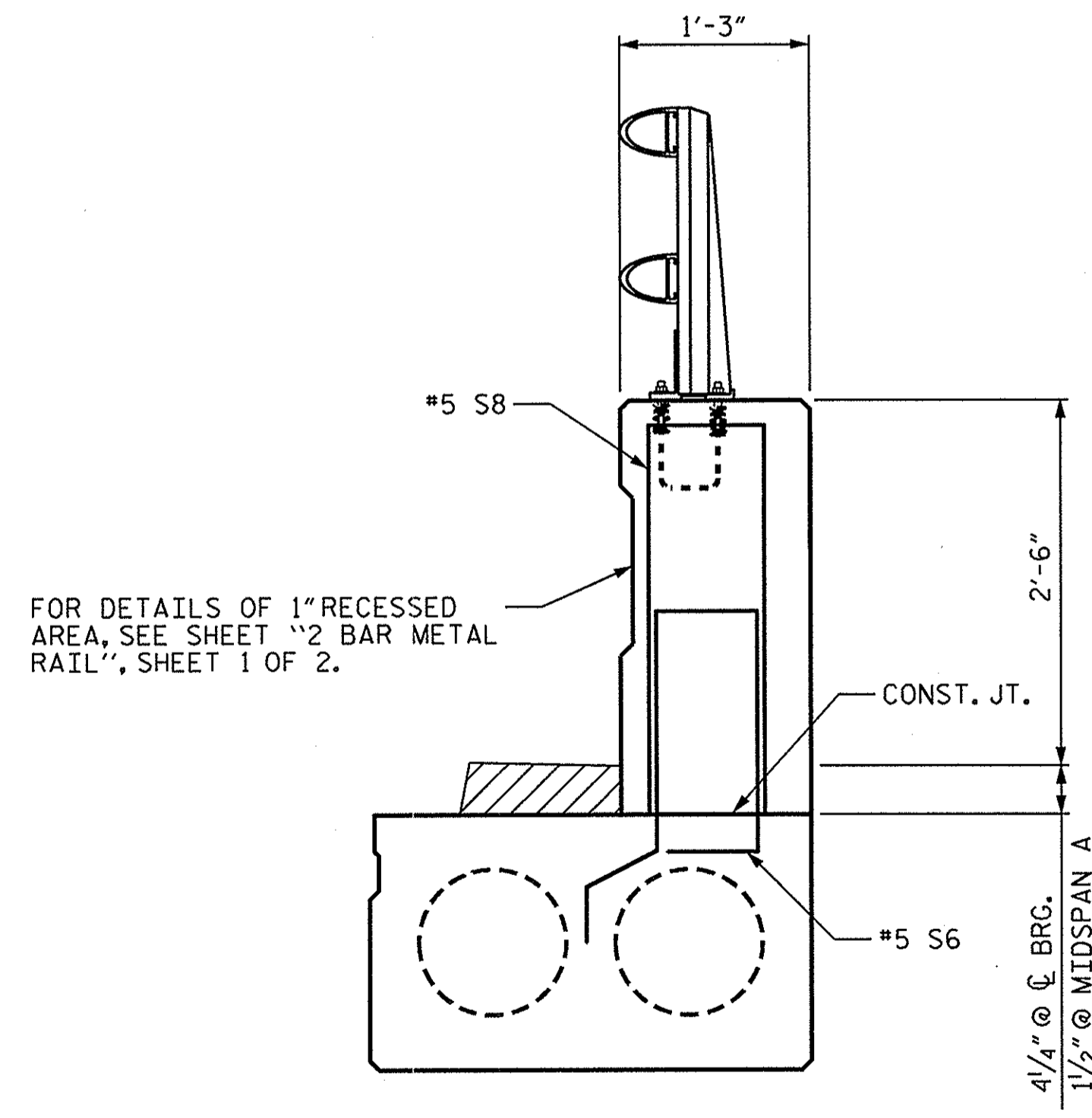


END VIEW



ELEVATION

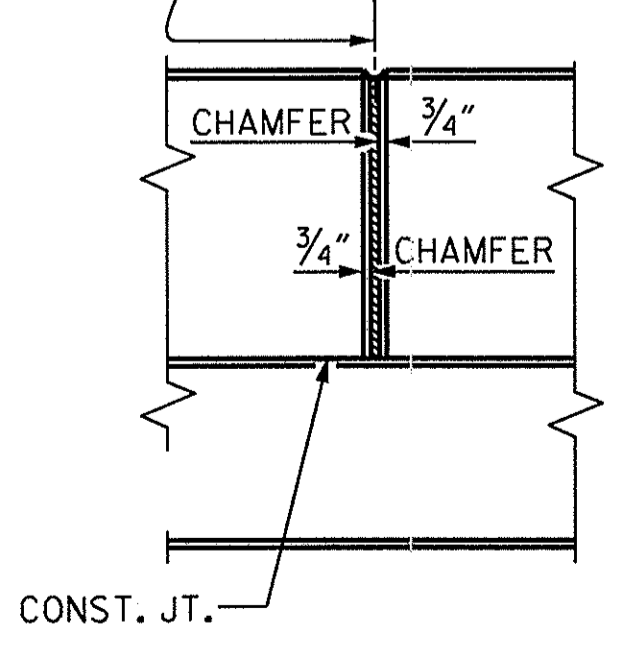
PARAPET AND END POST FOR TWO BAR RAIL



SECTION THROUGH PARAPET

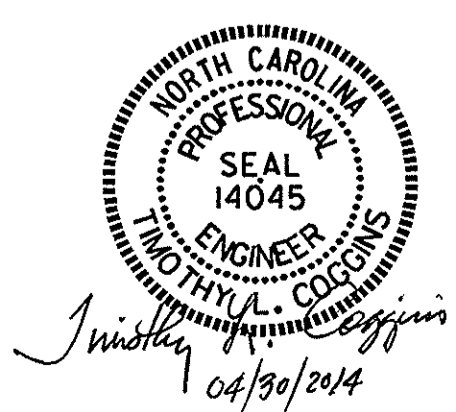
*5 'B' BARS IN PARAPET NOT SHOWN FOR CLARITY SEE "PARAPET AND END POST FOR TWO BAR RAIL".

1/2" EXP. JT. MAT'L. IN PARAPET HELD IN PLACE WITH GALVANIZED NAILS.



ELEVATION AT EXPANSION JOINTS

PARAPET DETAILS



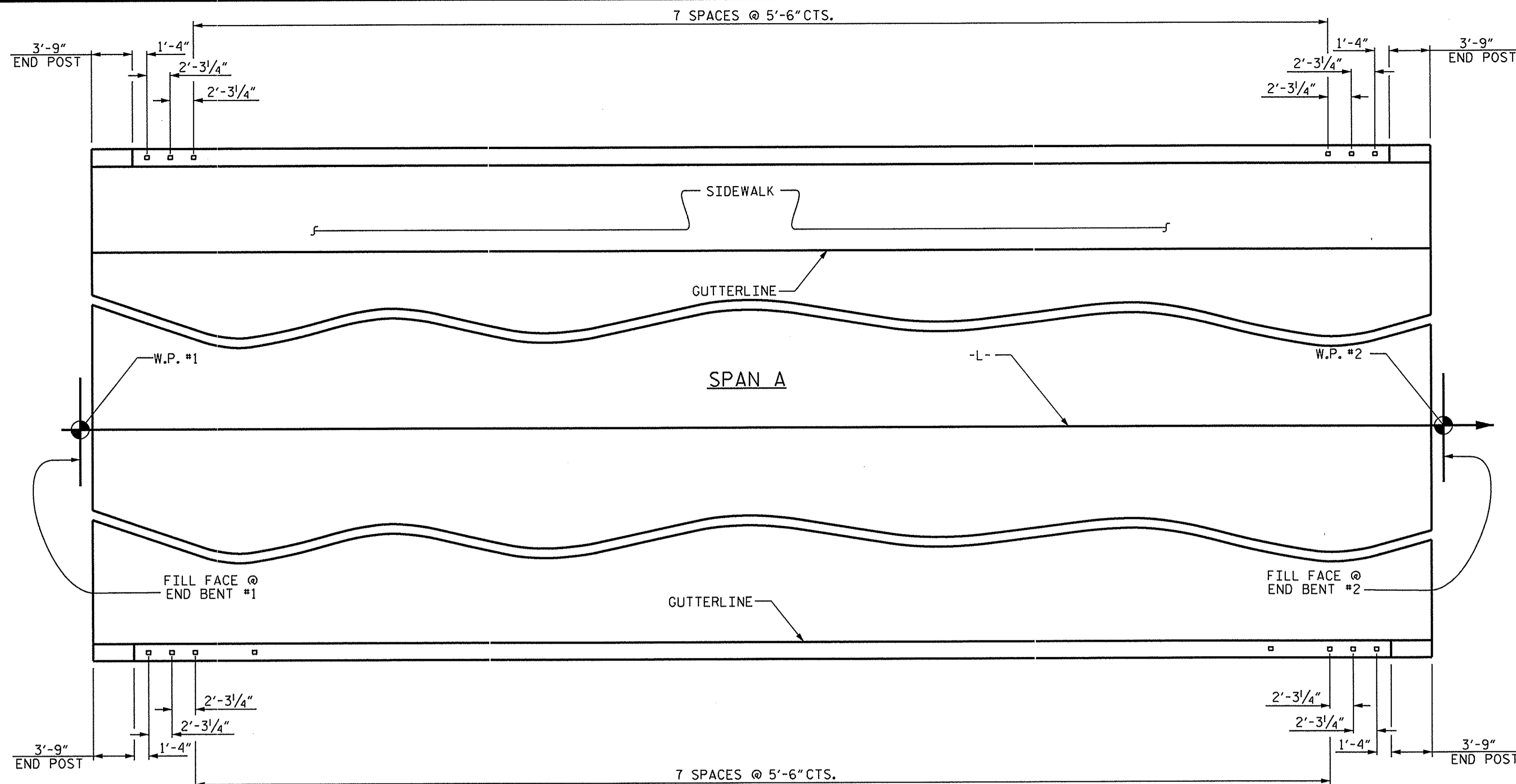
PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 2 OF 2

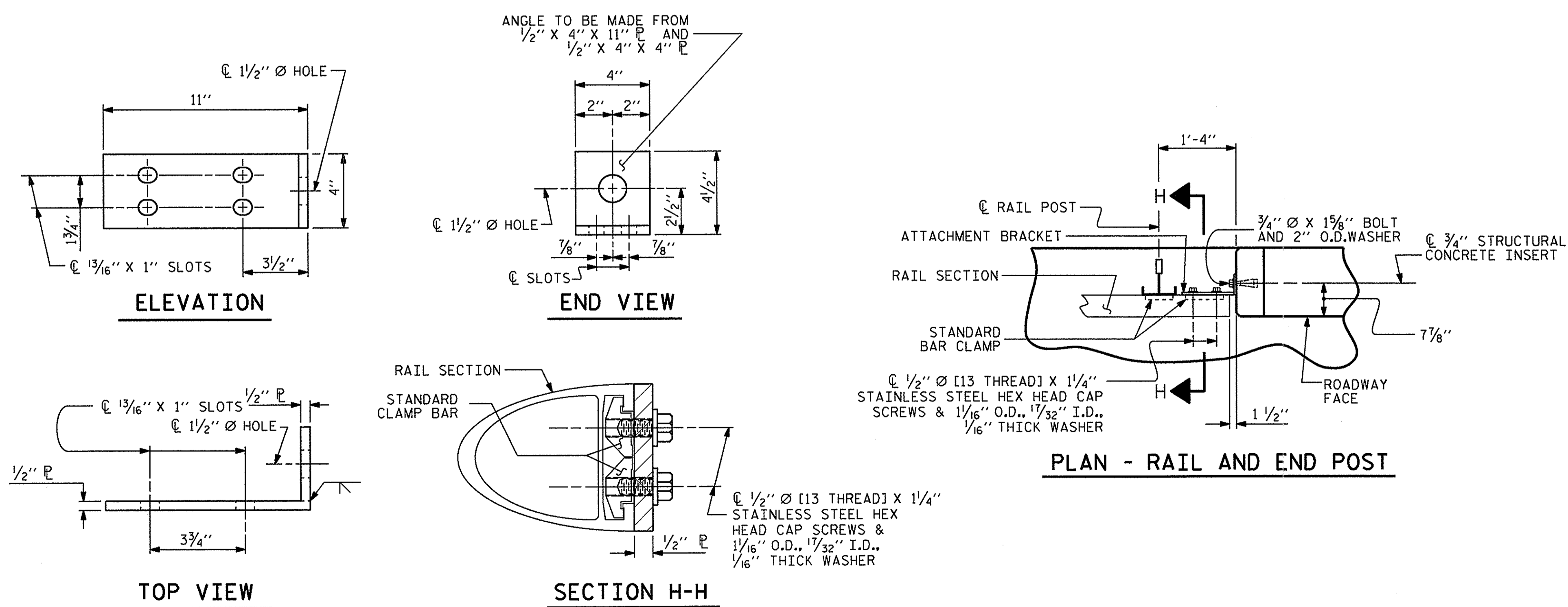
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUPERSTRUCTURE
 CONCRETE PARAPET
 & END POST DETAILS
 RIGHT SIDE

REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	
1			3			9
2			4			9

DRAWN BY: N. Ruffin DATE: 4/11/14
 CHECKED BY: T.L. COGGINS DATE: 4/24/14
 DESIGN ENGINEER OF RECORD: N.M. RUFFIN DATE: 4/30/14



PLAN OF RAIL POST SPACINGS



NOTES
STRUCTURAL CONCRETE INSERT

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 1 1/2".

B. 1 - 3/4" Ø X 1 5/8" BOLT WITH WASHER, BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 1 5/8" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 1/16" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES
METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

A. 1/2" PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.

B. 3/4" STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A 3/4" Ø X 1 5/8" BOLT WITH 2" O.D. WASHER IN PLACE. THE 3/4" Ø X 1 5/8" BOLT SHALL HAVE N.C. THREADS.

C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°.

D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).

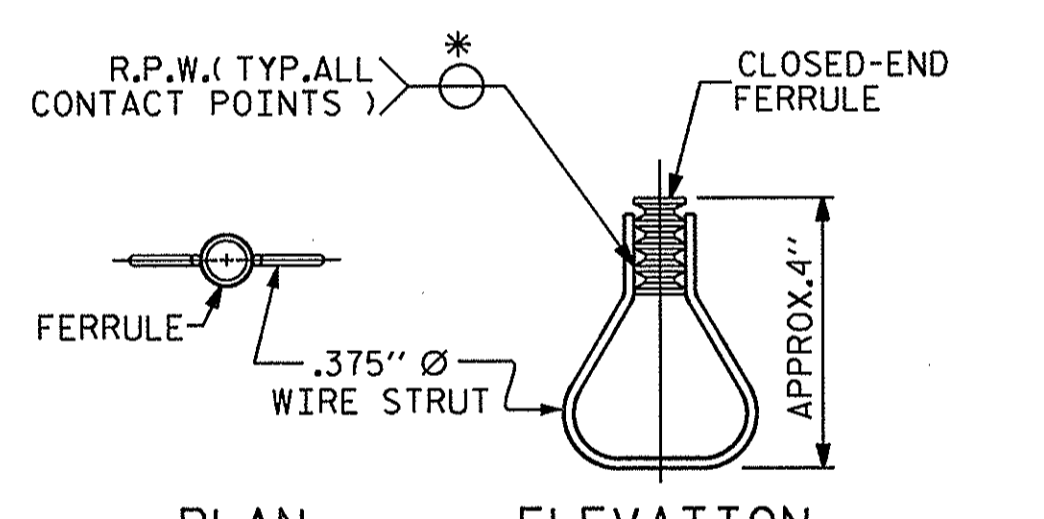
E. 1/2" Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 2 BAR METAL RAILS.

THE 3/4" STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE 3/4" STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE 1/2" PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE 3/4" Ø X 1 5/8" BOLT WITH WASHER SHALL BE REPLACED WITH A 3/4" Ø X 6 1/2" BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE 3/4" Ø X 1 5/8" BOLT SHALL APPLY TO THE 3/4" Ø X 6 1/2" BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



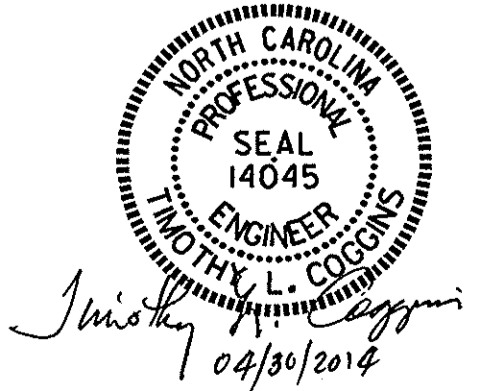
PLAN ELEVATION
STRUCTURAL CONCRETE INSERT

* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

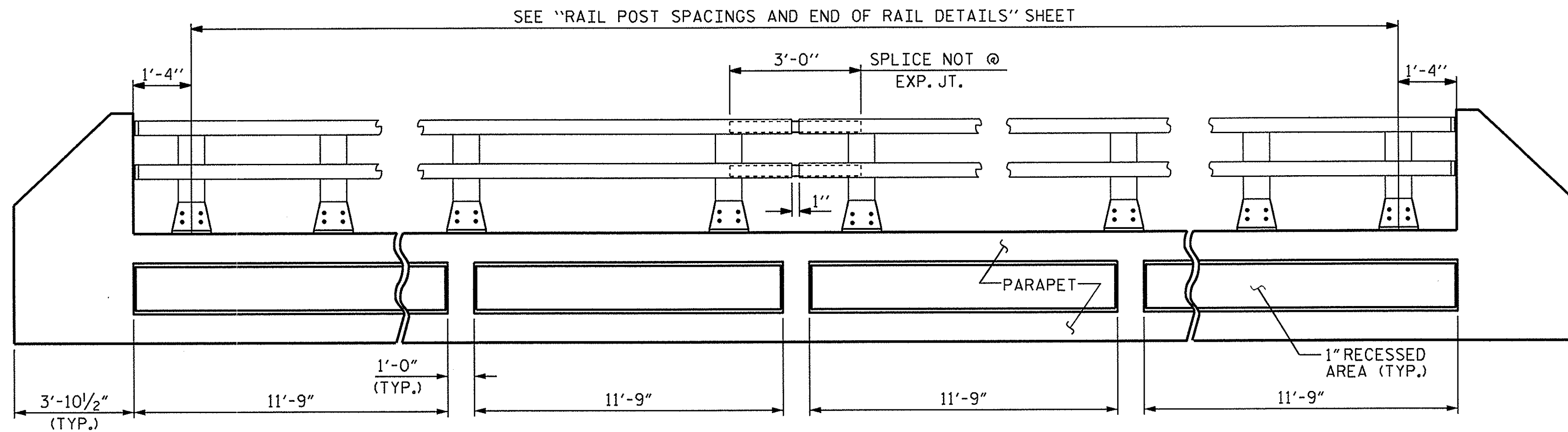
PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
RAIL POST SPACINGS
AND
END OF RAIL DETAILS
FOR TWO BAR METAL RAILS

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-10	
1			3			TOTAL SHEETS	
2			4				

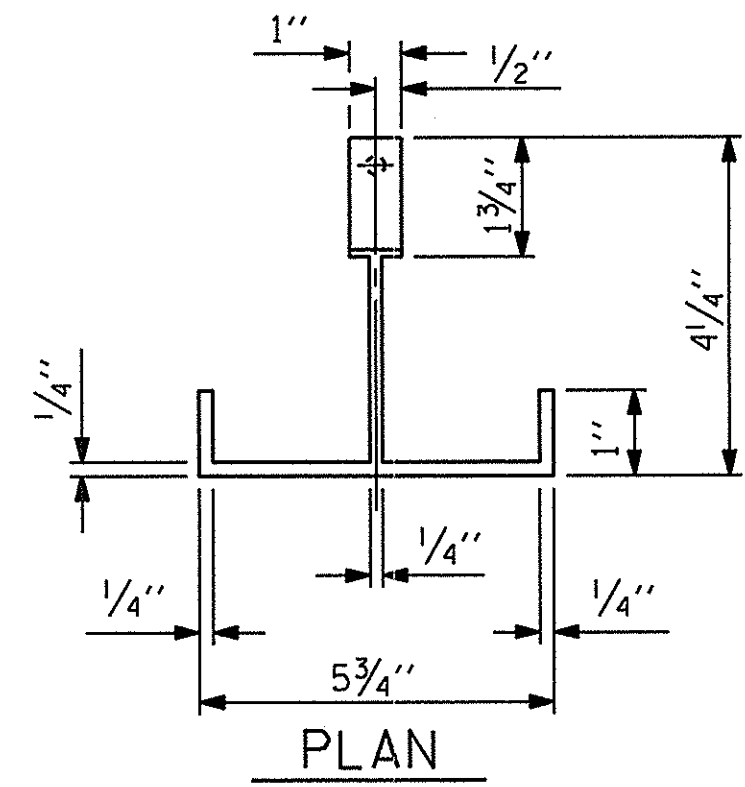


ASSEMBLED BY: M. Ruffin DATE: 4/11/14
CHECKED BY: T.L. COGGINS DATE: 4/24/14
DRAWN BY: FCJ 1/88 REV. 5/7/03 RWW/JTE
CHECKED BY: CRK 3/89 REV. 5/1/06 TLA/GM
REV. 10/1/11 MAA/GM

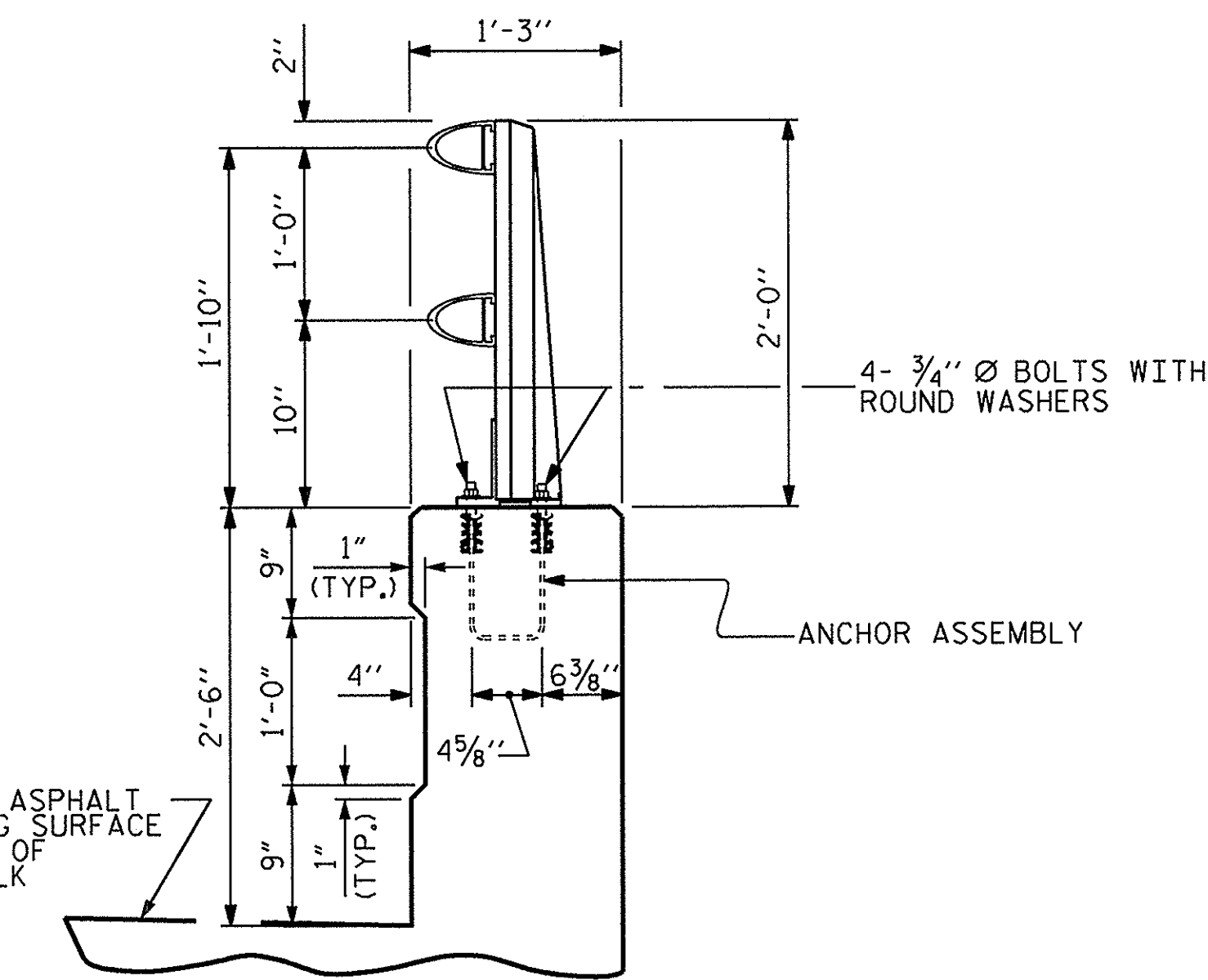


ELEVATION

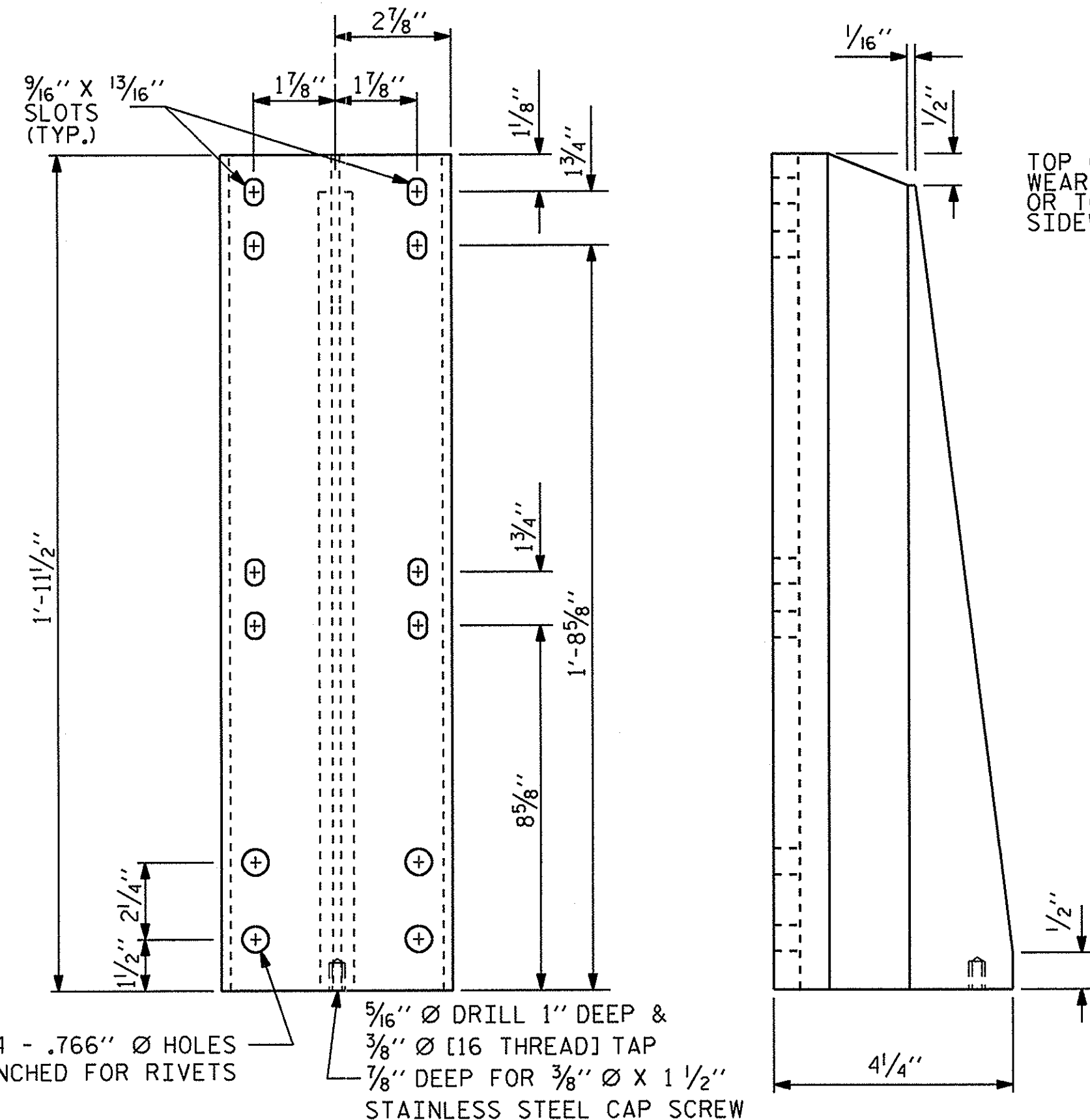
NOTE: FOR ATTACHMENT OF METAL RAIL TO END POST, SEE STANDARD NO. BMR2.



PLAN



SECTION THRU PARAPET AND RAIL

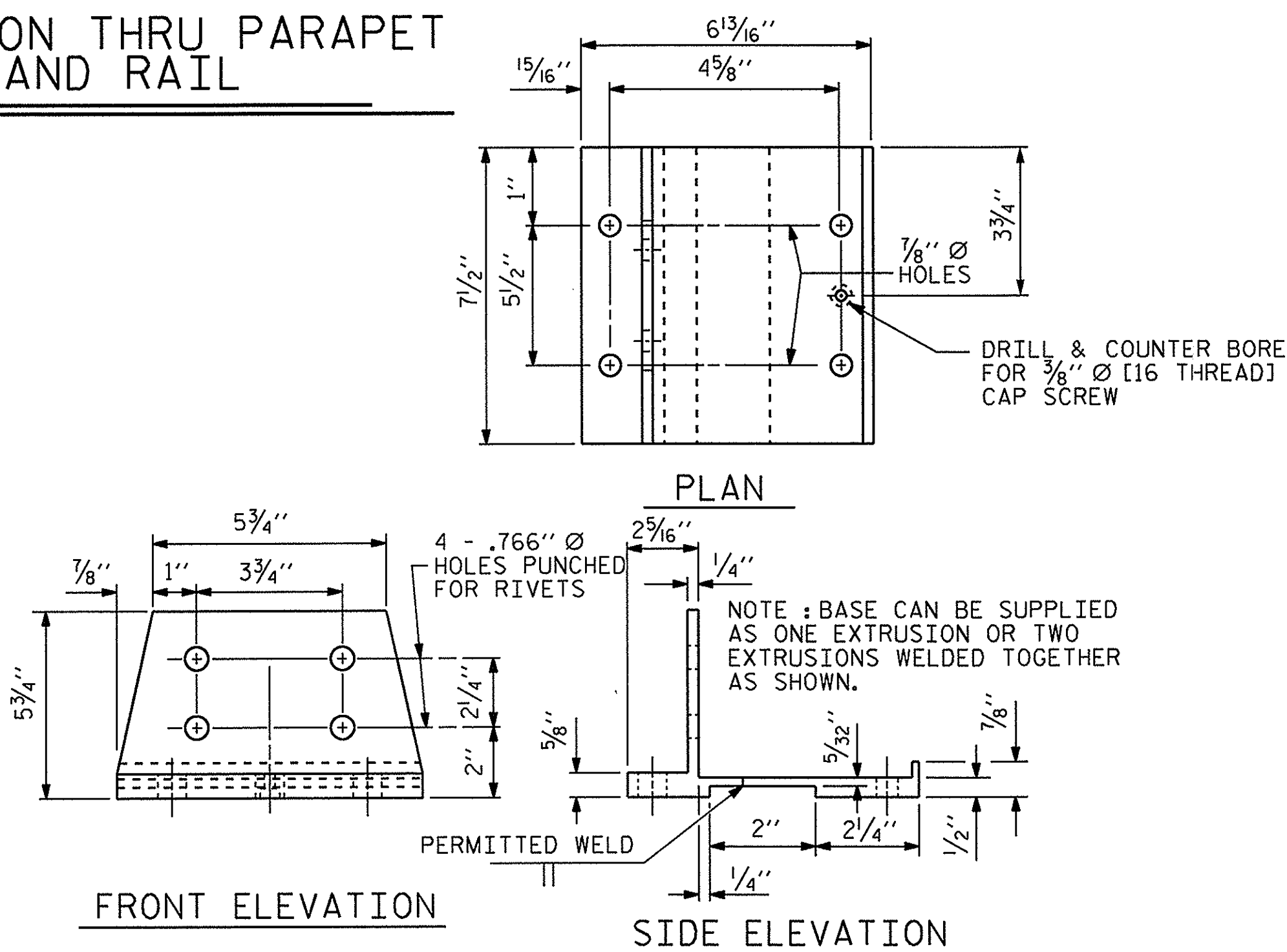


FRONT ELEVATION

SIDE ELEVATION

DETAILS OF POST

ASSEMBLED BY: *N. Ruffin* DATE: 4/11/14
 CHECKED BY: T.L. COGGINS DATE: 4/24/14
 DRAWN BY: EEM 6/94 REV. 5/1/06 TLA/GM
 CHECKED BY: RGW 6/94 REV. 10/1/11 MAA/GM
 REV. 6/13 MAA/GM

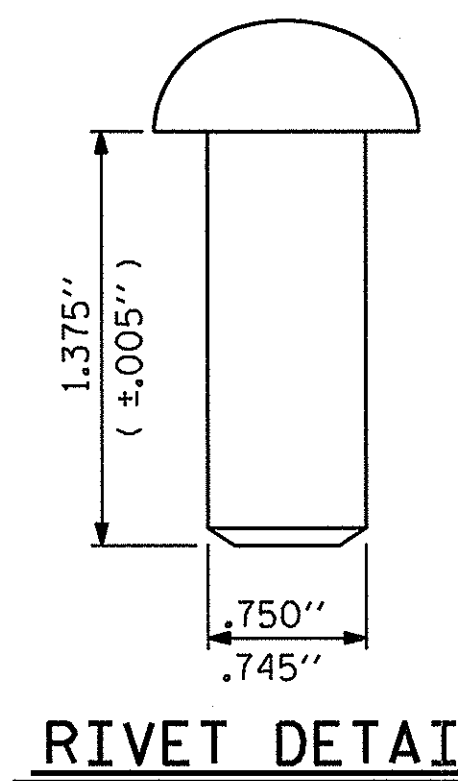


FRONT ELEVATION

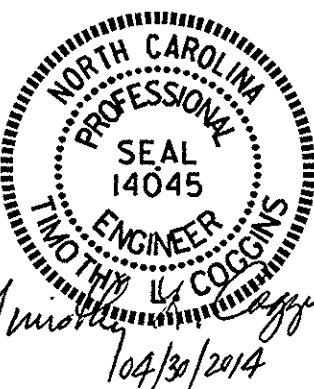
SIDE ELEVATION

POST BASE DETAILS

PAY LENGTH = 100.50 LIN. FT.



RIVET DETAIL



NOTES

METAL RAIL SHALL BE ALUMINUM IN ACCORDANCE WITH THE REQUIREMENTS OF THE GENERAL NOTES AND THE FOLLOWING SPECIFICATIONS FOR THE MATERIALS.

UNLESS OTHERWISE REQUIRED IN THE CONTRACT DOCUMENTS, THE CONTRACTOR HAS THE OPTION TO USE AN ALTERNATE TO THE 2 BAR METAL RAIL. THE ALTERNATE RAIL SHALL MEET THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS AND MUST BE LISTED ON THE DEPARTMENT'S APPROVED PRODUCTS LIST (APL) UNDER "2 BAR METAL RAIL ALTERNATE". ADJUSTMENTS TO THE CONCRETE PARAPET WILL NOT BE ALLOWED.

ALUMINUM RAILS

MATERIAL FOR POSTS, BASES AND RAILS, EXPANSION BARS AND CLAMP BARS SHALL BE ASTM B-221 ALLOY 6061-T6. MATERIAL FOR RIVETS SHALL BE ASTM B316 ALLOY 6061-T6. RIVETS SHALL BE STANDARD BUTTON HEAD AND CONE POINT COLD DRIVEN AS PER DRAWING.

THE BASE OF RAIL POSTS, OR ANY OTHER ALUMINUM SURFACE IN CONTACT WITH CONCRETE SHALL BE THOROUGHLY COATED WITH AN ALUMINUM IMPREGNATED CAULKING COMPOUND OF APPROVED QUALITY.

MATERIAL FOR SHIMS TO BE ASTM B209 ALLOY 6061-T6.

GENERAL NOTES

RAILING SHALL BE CONTINUOUS FROM END POST TO END POST OF BRIDGE. EACH JOINT IN RAIL LENGTH SHALL BE SPLICED AS DETAILED. PANEL LENGTHS OF RAIL SHALL BE ATTACHED TO A MINIMUM OF THREE POSTS.

FOR END OF RAIL TO CLEAR FACE OF CONCRETE END POST DIMENSION, SEE STANDARD NO. BMR2.

CAP SCREWS SHALL BE ASTM F593 ALLOY 305 STAINLESS STEEL. WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.

CERTIFIED MILL REPORTS ARE REQUIRED FOR RAILS AND POSTS. SHOP INSPECTION IS NOT REQUIRED.

METAL RAIL POSTS SHALL BE SET NORMAL TO CURB GRADE.

METHOD OF MEASUREMENT FOR METAL RAILS: FOR LENGTH OF METAL RAILS TO BE PAID FOR, SEE THE STANDARD SPECIFICATIONS.

CURVED RAIL USAGE: WHERE RAILS ARE TO BE USED ON BRIDGES ON HORIZONTAL AND/OR VERTICAL CURVATURE THE CONTRACTOR MAY, AT HIS OPTION, HAVE THE REQUIRED CURVATURE IN THE RAIL FORMED IN THE SHOP OR IN THE FIELD. IN EITHER EVENT, THE RAIL SHALL CONFORM WITHOUT BUCKLING OR KINKING TO THE REQUIRED CURVATURE IN A UNIFORM MANNER ACCEPTABLE TO THE ENGINEER.

TO INSURE FUTURE IDENTIFICATION OF THE FABRICATOR, A PERMANENT IDENTIFYING MARK SHALL BE PLACED ON EACH POST. THE METHOD OF MARKING AND LOCATION SHALL BE SUCH THAT IT DOES NOT DETRACT FROM THE APPEARANCE OF THE POST, BUT REMAINS VISIBLE AFTER RAIL PLACEMENT.

SHIMS SHALL BE USED AS NECESSARY FOR POST ALIGNMENT.

ALLOY 6351-T5 MAY BE SUBSTITUTED FOR ALLOY 6061-T6 WHERE APPLICABLE.

MINOR VARIATIONS IN DETAILS OF METAL RAIL WILL BE CONSIDERED. DETAILS OF SUCH VARIATIONS, IF DESIRED, SHALL BE SUBMITTED FOR APPROVAL.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD
 2 BAR METAL RAIL

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-11	
1			3			TOTAL SHEETS	
2			4				

STD. NO. BMR3

NOTES

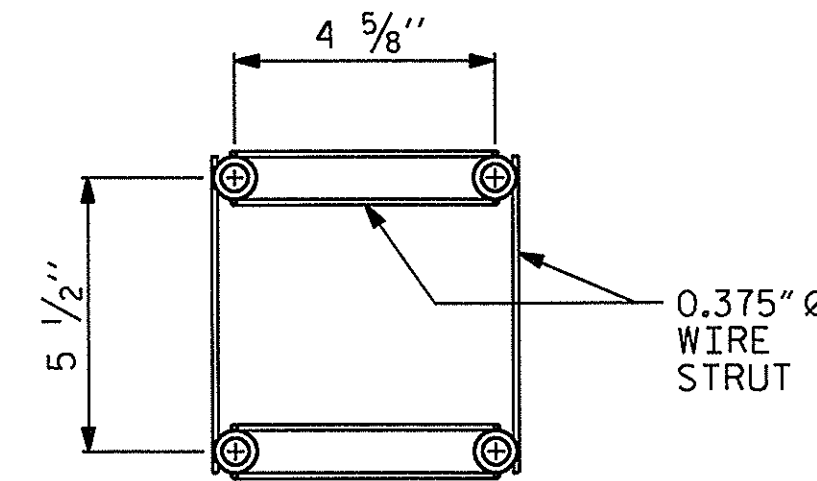
STRUCTURAL CONCRETE ANCHOR ASSEMBLY

THE STRUCTURAL CONCRETE ANCHOR ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS :

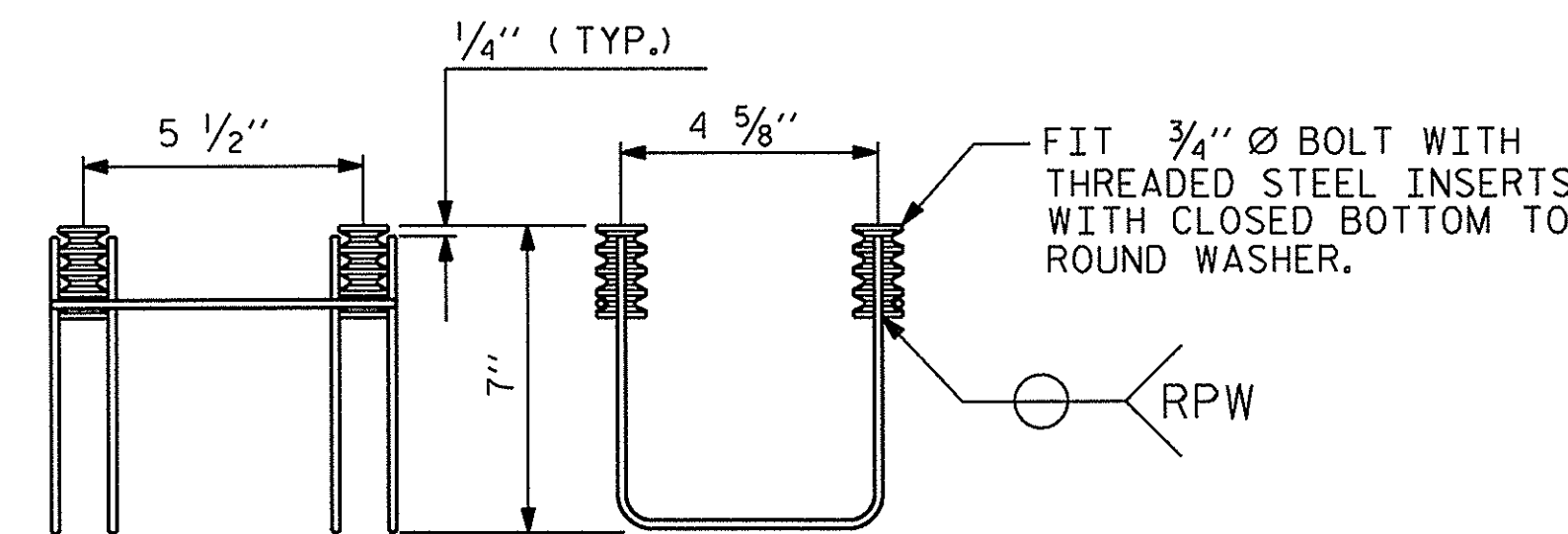
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 2" FOR 3/4" FERRULES.
- B. 4 - 3/4" Ø X 2 1/2" BOLTS WITH WASHERS. BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 3/4" Ø X 2 1/2" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.
- C. WIRE STRUT SHOWN IN THE CONCRETE ANCHOR ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 1/6" Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.
- D. THE METAL RAIL ANCHOR ASSEMBLIES TO BE HOT DIPPED GALVANIZED TO CONFORM TO REQUIREMENTS OF AASHTO M111.
- E. THE COST OF THE METAL RAIL ANCHOR ASSEMBLY WITH BOLTS AND WASHERS COMPLETE IN PLACE SHALL BE INCLUDED IN THE PRICE BID FOR LINEAR FEET OF METAL RAIL.
- F. BOLTS TO BE TIGHTENED ONE-HALF TURN WITH A WRENCH FROM A FINGER-TIGHT POSITION.

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF THE METAL RAIL ANCHOR ASSEMBLY. LEVEL ONE FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 3/4" Ø BOLT IS 10 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE THE STANDARD SPECIFICATIONS.

WHEN ADHESIVELY ANCHORED ANCHOR BOLTS ARE USED, BOLTS SHALL MEET THE REQUIREMENTS OF ASTM F593 ALLOY 304 STAINLESS STEEL WITH MINIMUM 75,000 PSI ULTIMATE STRENGTH. NUTS SHALL MEET THE REQUIREMENTS OF ASTM F594 ALLOY 304 STAINLESS STEEL AND WASHERS SHALL MEET THE REQUIREMENTS OF ASTM F844 EXCEPT THEY SHALL BE MADE FROM ALLOY 304 STAINLESS STEEL.



PLAN

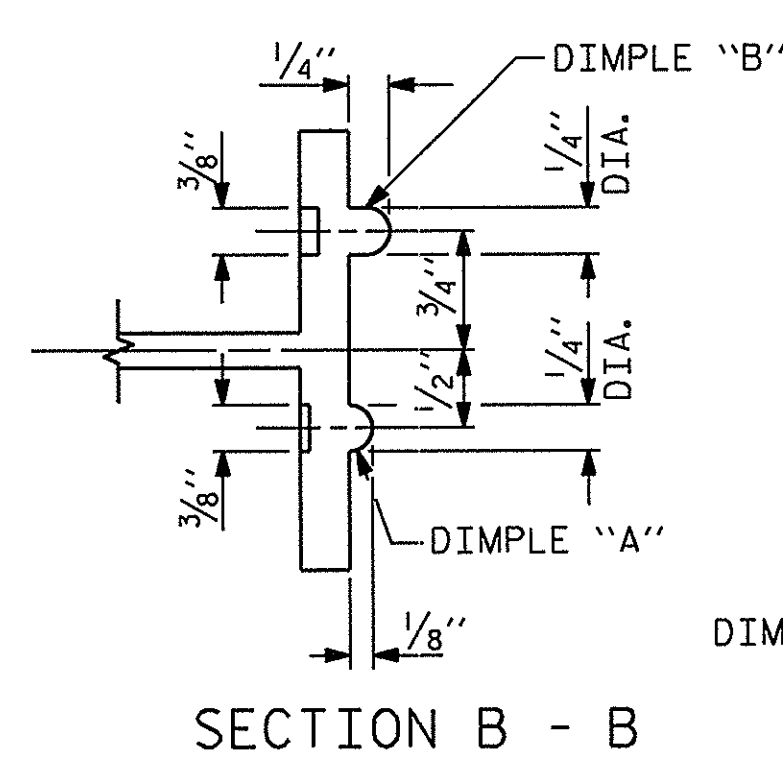


SIDE VIEW

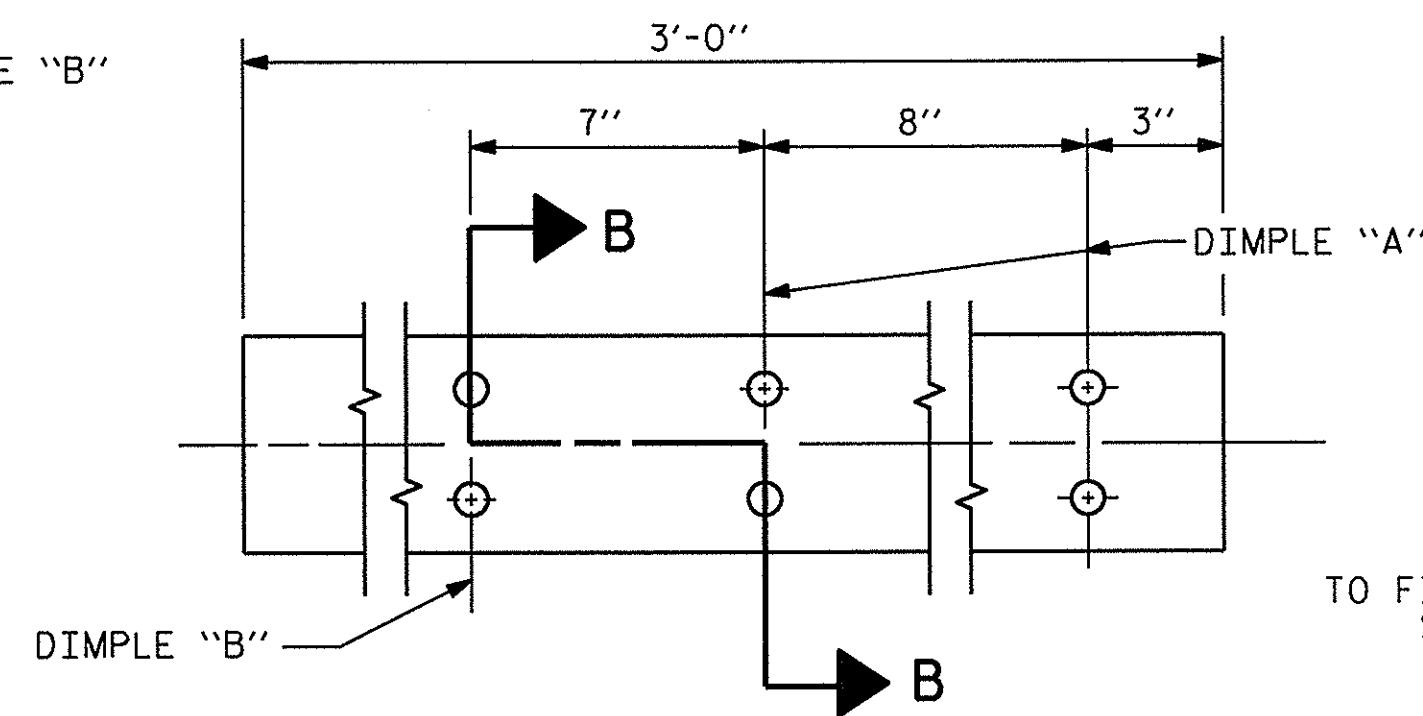
ELEVATION

4-BOLT METAL RAIL ANCHOR ASSEMBLY

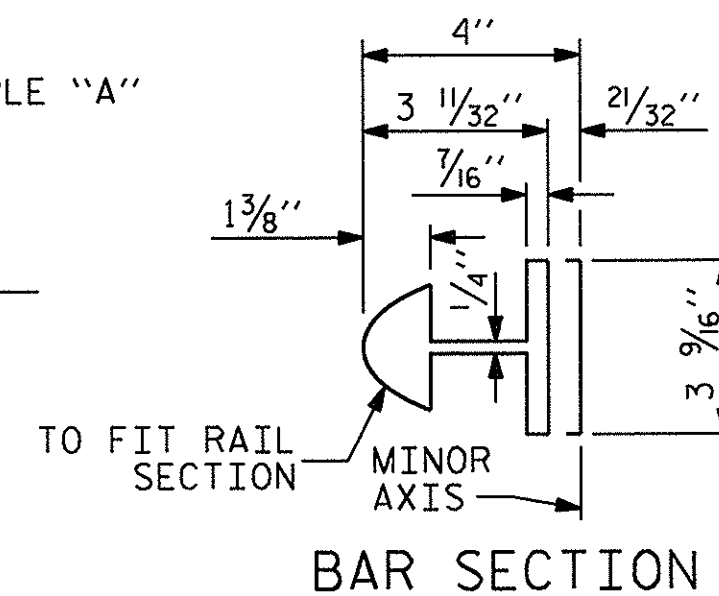
(24 ASSEMBLIES REQUIRED)



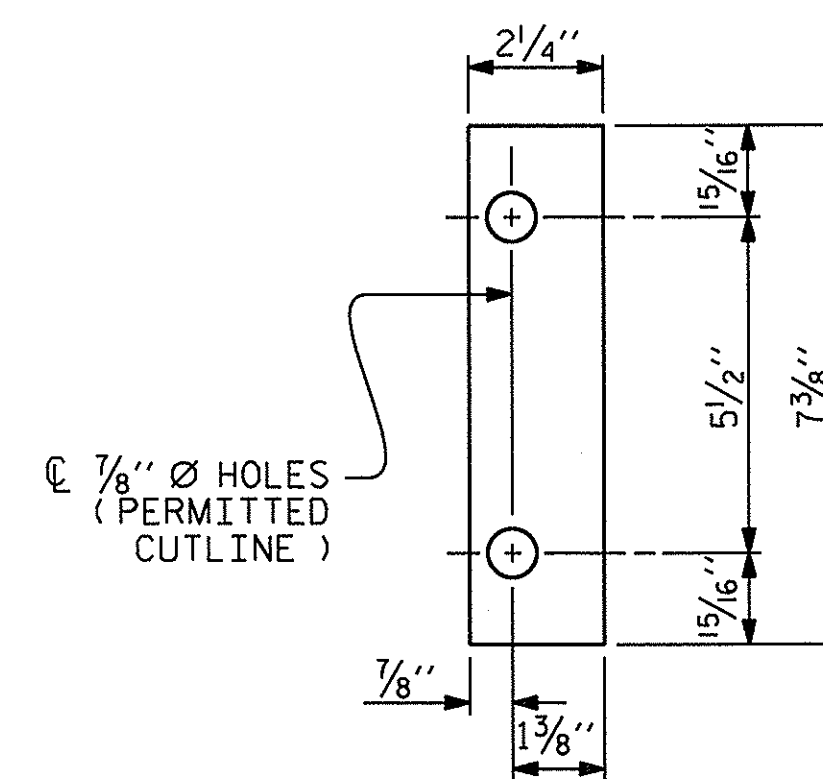
SECTION B - B



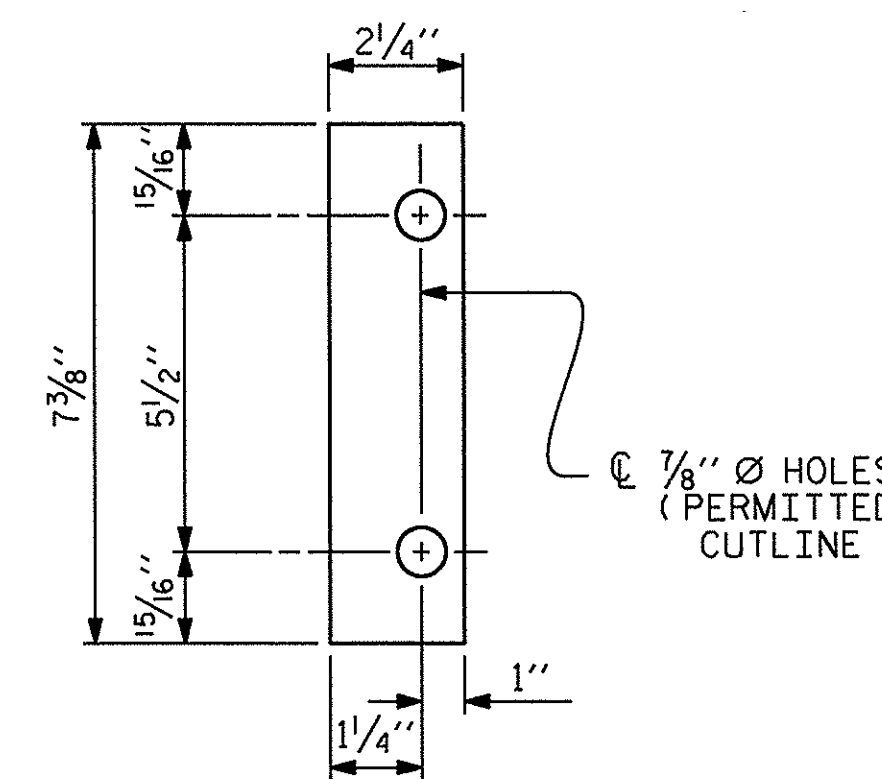
EXPANSION BAR DETAILS



BAR SECTION



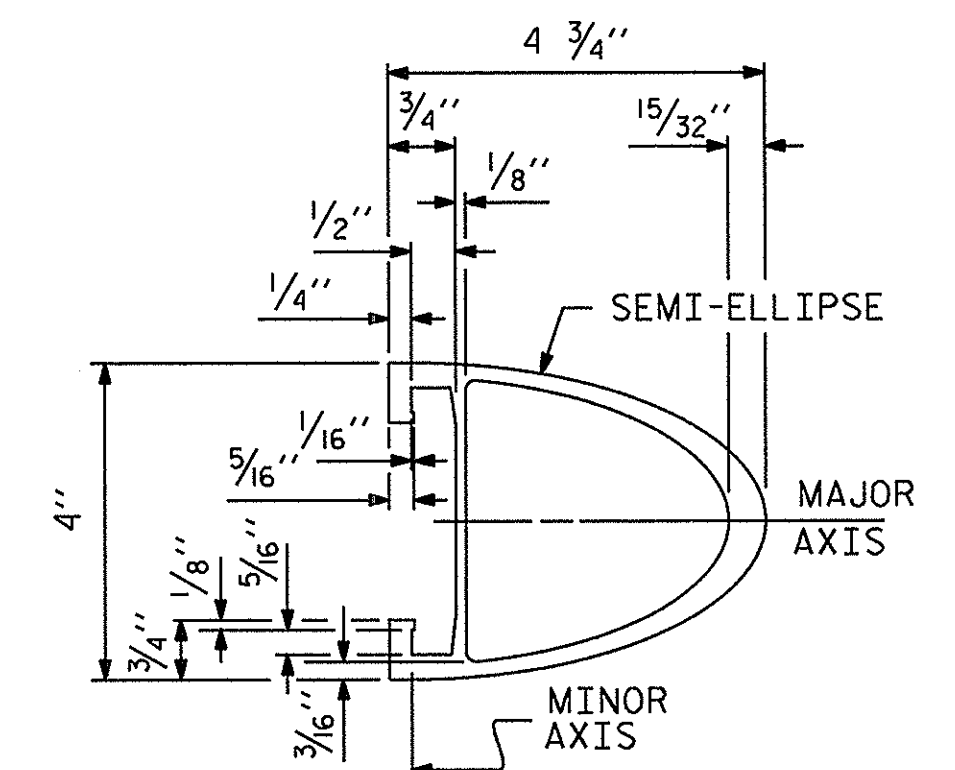
FRONT PLATE



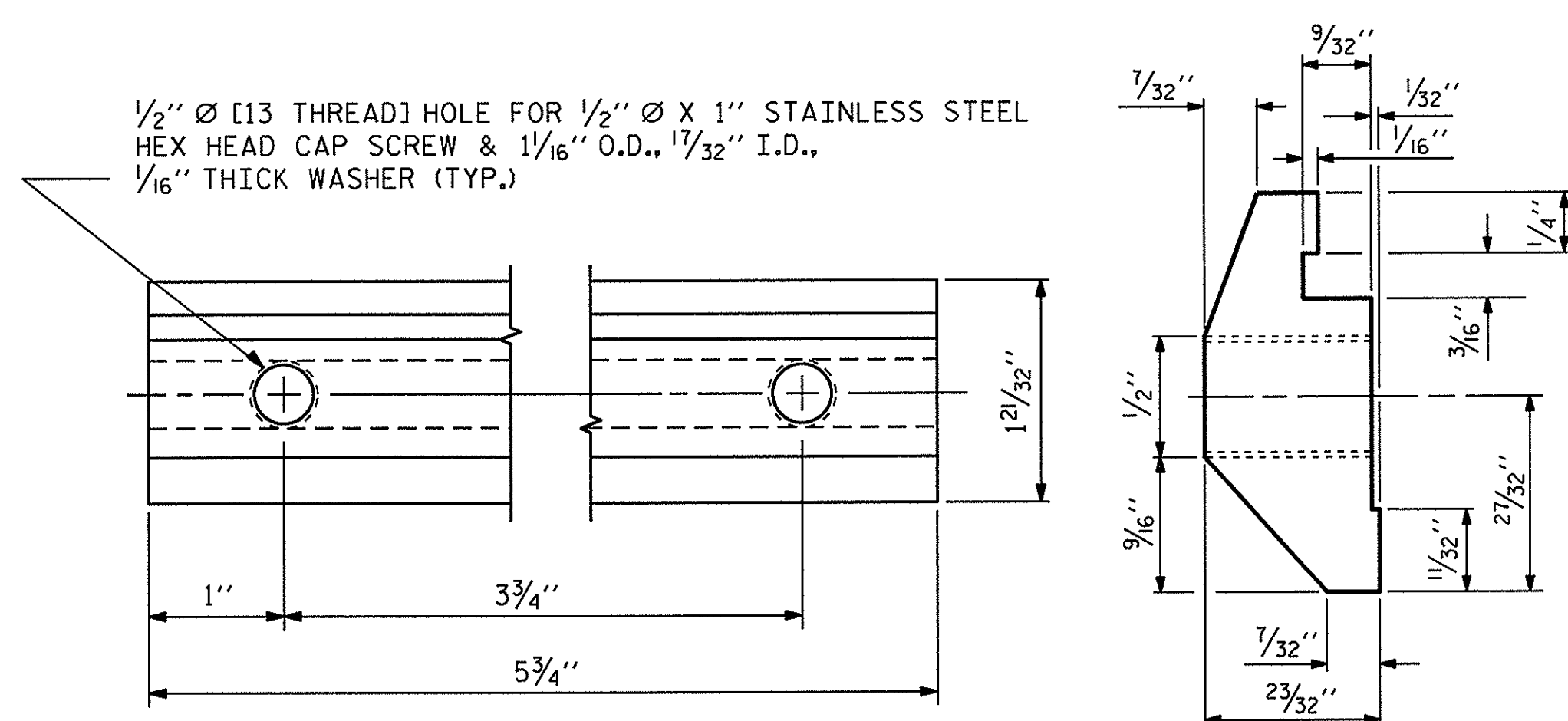
REAR PLATE

SHIM DETAILS

NOTE : SHIMS MAY BE CUT ALONG PERMITTED CUTLINE OR SLOTTED TO EDGE OF PLATE TO FACILITATE PLACEMENT.

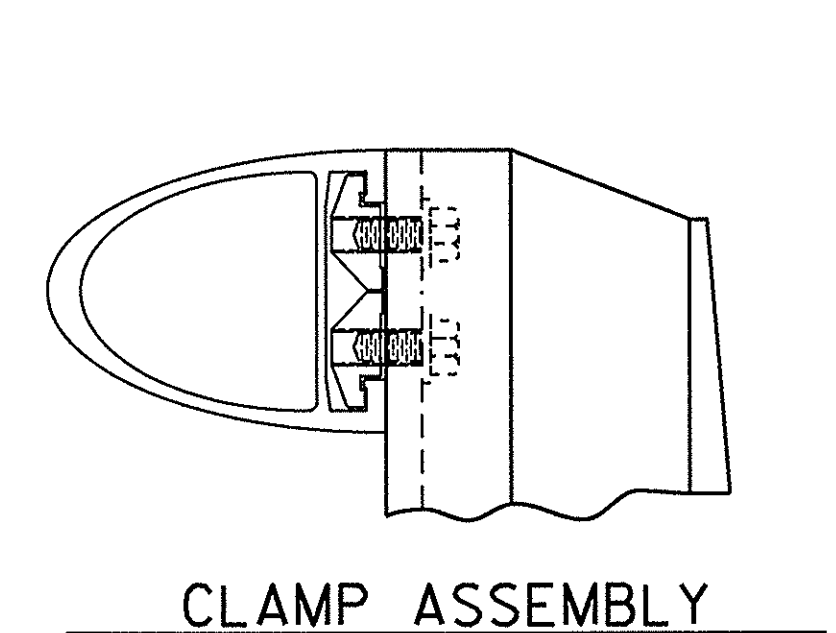


RAIL SECTION

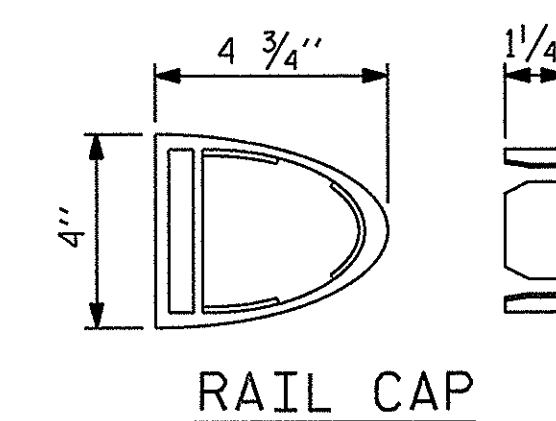


CLAMP BAR DETAIL

(4 REQUIRED PER POST)



CLAMP ASSEMBLY



RAIL CAP

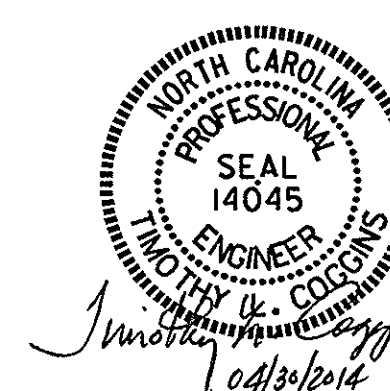
PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

STANDARD

2 BAR METAL RAIL



ASSEMBLED BY : <i>M. Ruffin</i>	DATE : 4/11/14
CHECKED BY : T.L. COGGINS	DATE : 4/24/14
DRAWN BY : EEM 6/94	REV. 8/16/99 MAB/LES
CHECKED BY : RGW 6/94	REV. 5/1/06R KMM/GM
	REV. 10/1/11 MAA/GM

REVISIONS						TOTAL SHEETS
NO.	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			1
2			4			1

STD. NO. BMR4

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 - 7/8" Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

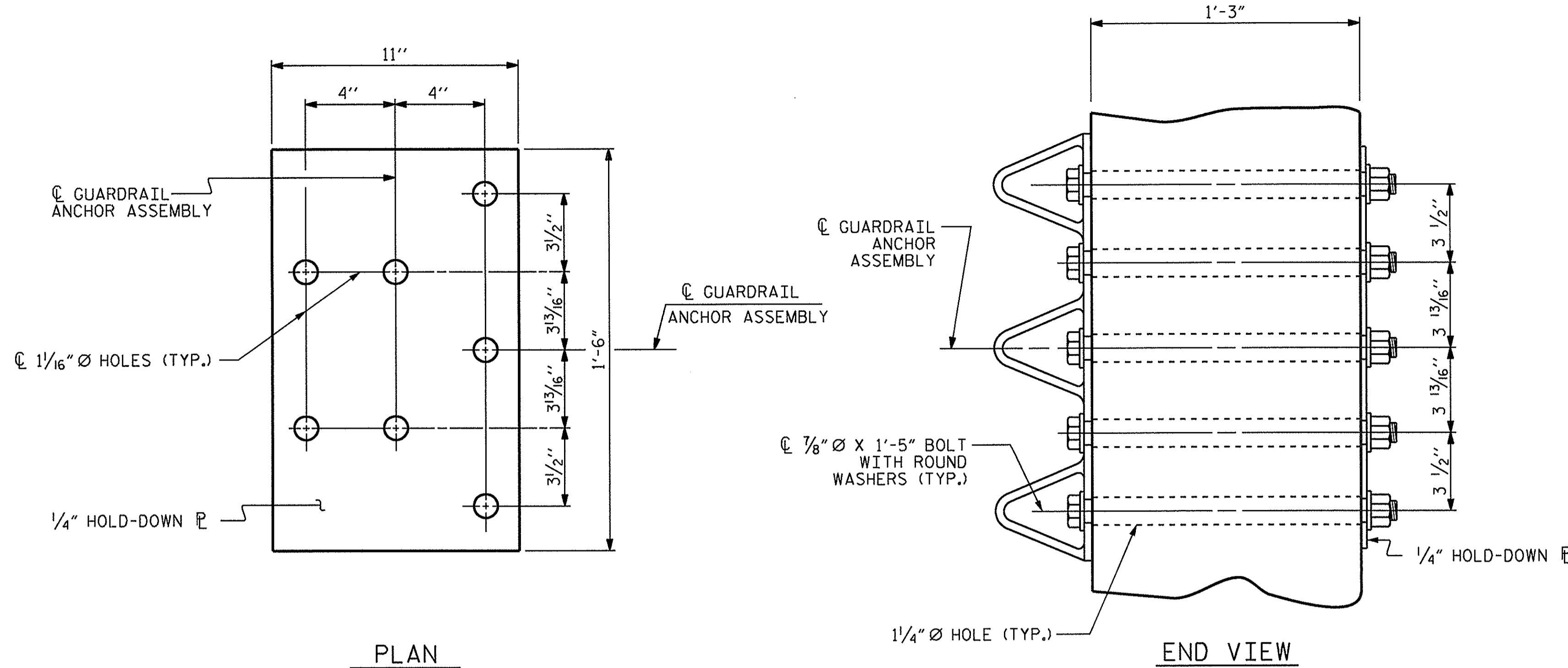
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

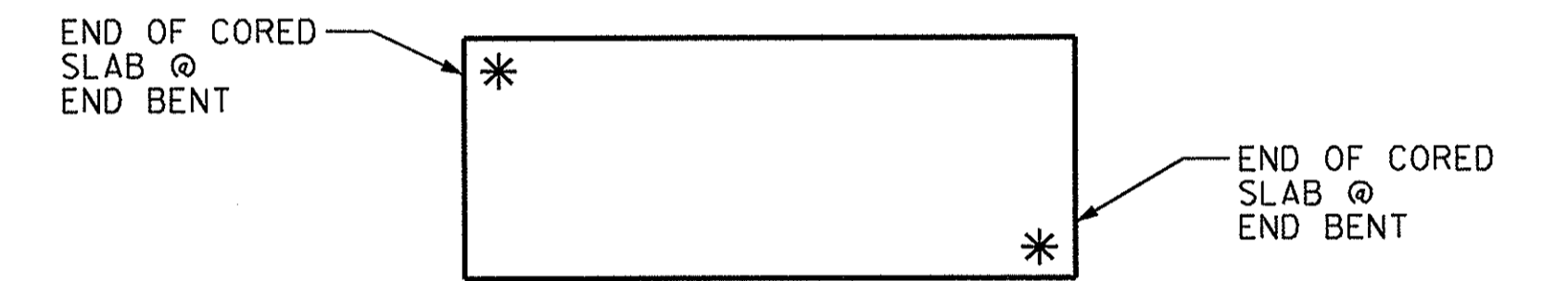
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

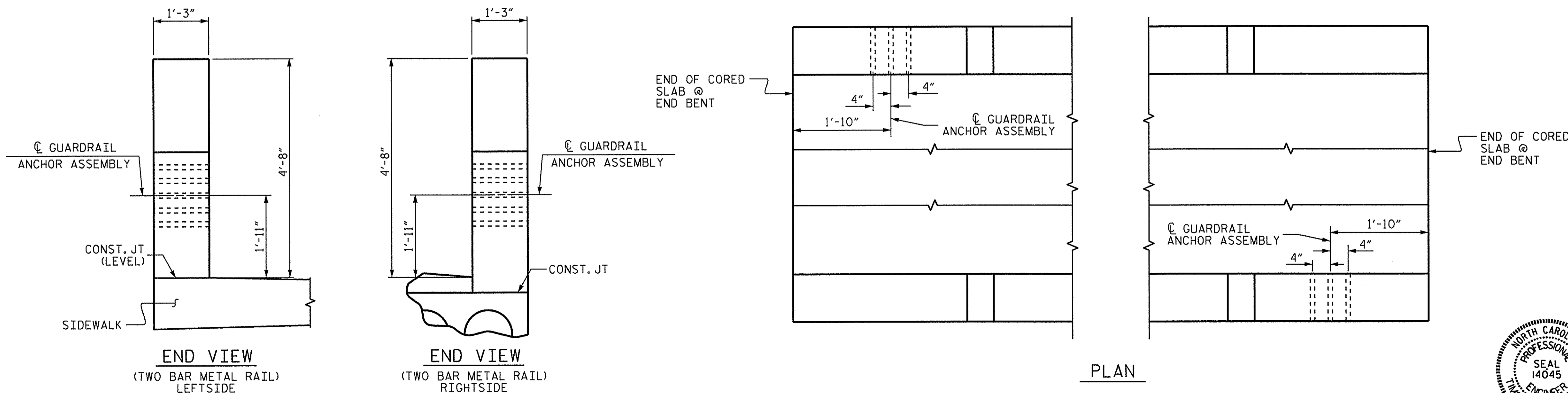
THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



PLAN
END VIEW
GUARDRAIL ANCHOR ASSEMBLY DETAILS



SKETCH SHOWING POINTS OF ATTACHMENT
* LOCATION OF GUARDRAIL ATTACHMENT



LOCATION OF GUARDRAIL ANCHOR AT END POST

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD
GUARDRAIL ANCHORAGE
DETAILS
FOR METAL RAILS

ASSEMBLED BY : <i>N. Ruffin</i>	DATE : 4/14/14
CHECKED BY : T.L. COGGINS	DATE : 4/23/14
DRAWN BY : MAA 5/10	REV. 10/1/11 MAA/GM
CHECKED BY : GM 5/10	REV. 12/5/11 MAA/GM
	REV. 6/13 MAA/GM

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-13
1			3			TOTAL SHEETS
2			4			

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 2 1/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

PLACEMENT OF THE ASPHALT WEARING SURFACE SHALL OCCUR AFTER CASTING THE CONCRETE PARAPET AND SIDEWALK.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 5300 PSI.

ALL REINFORCING STEEL IN THE CONCRETE PARAPET AND SIDEWALK SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE CONCRETE PARAPET AND SIDEWALK IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE CENTERED BETWEEN EACH RECESSED AREA. EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

QUANTITIES FOR SIDEWALK ITEMS SHALL BE INCLUDED IN CLASS AA CONCRETE AND EPOXY COATED REINFORCING STEEL TOTALS.

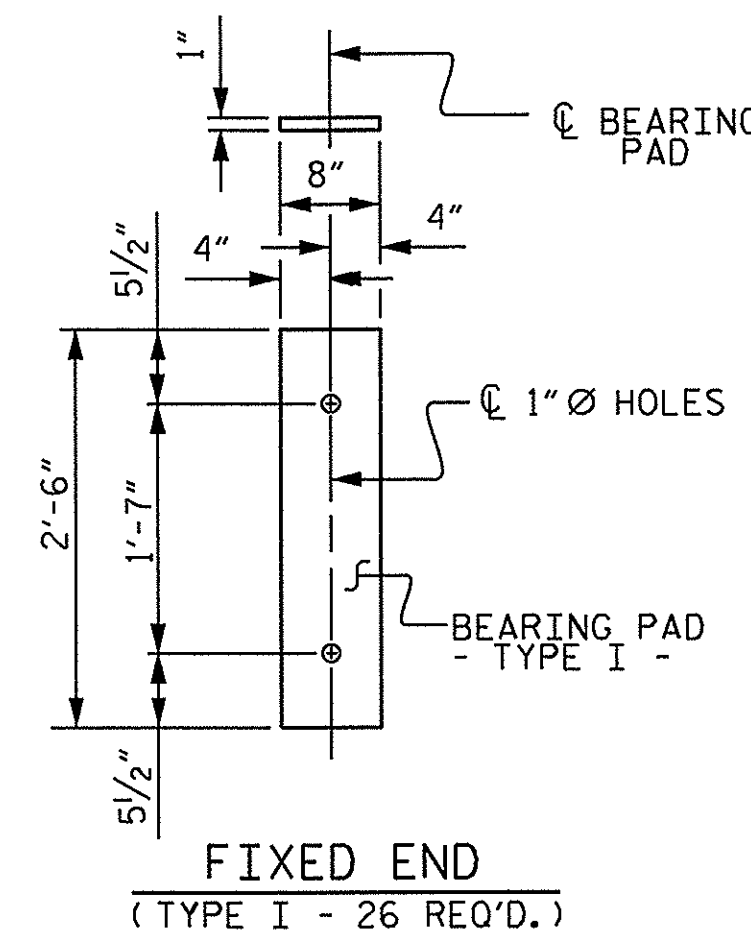
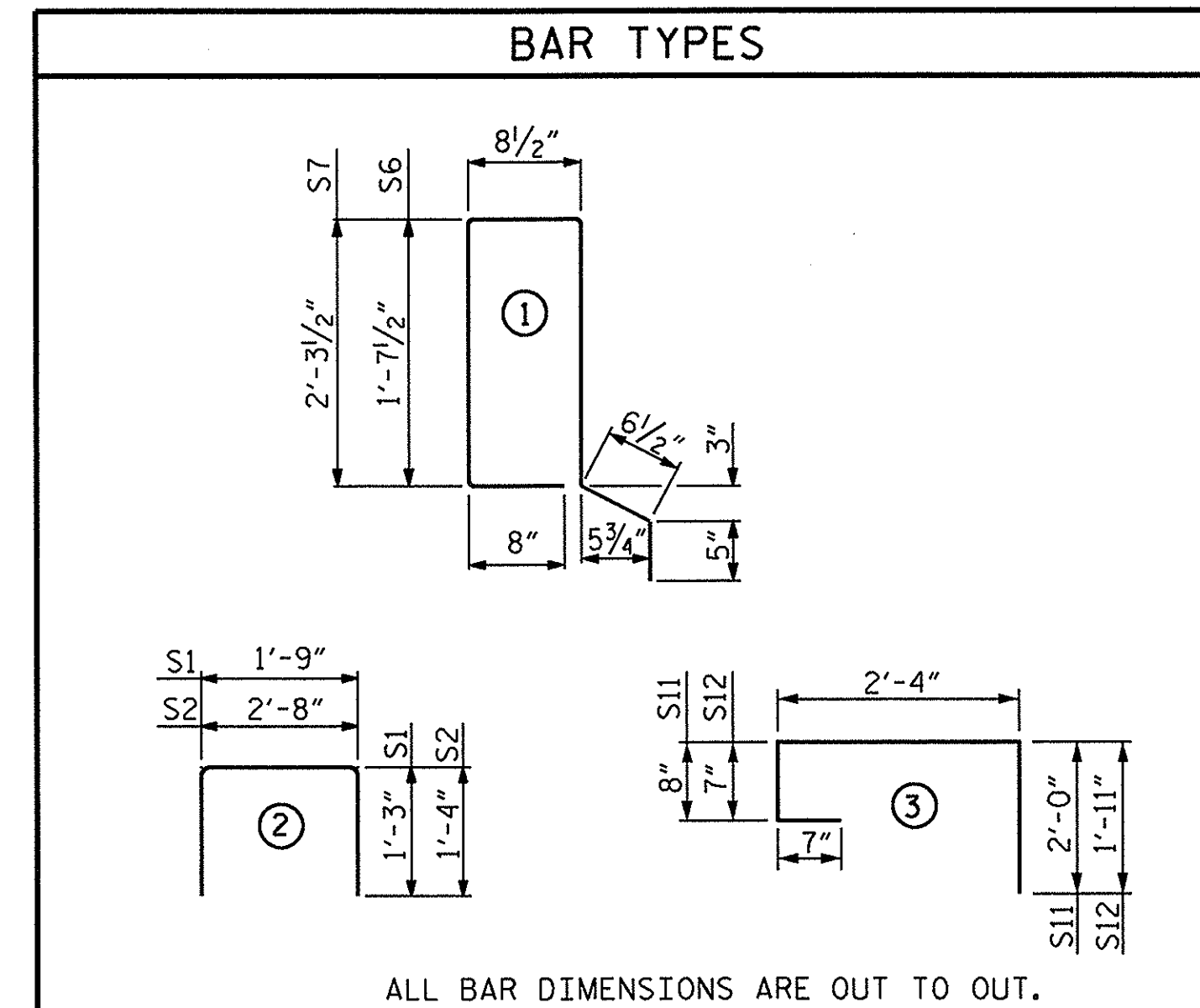
THE #4 S2 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR 0 PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

BILL OF MATERIAL FOR ONE CORED SLAB SECTION											
SPAN A											
		TYPE I		TYPE II		TYPE III		TYPE IV			
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	LENGTH	WEIGHT	LENGTH	WEIGHT
B1	4	#4	STR	29'-7"	79	29'-7"	79	29'-7"	79	29'-7"	79
S1	8	#5	2	4'-3"	35	4'-3"	35	4'-3"	35	4'-3"	35
S2	122	#4	2	5'-4"	435	5'-4"	435	5'-4"	435	5'-4"	435
* S6	60	#5	1							5'-7"	349
* S7	60	#5	1	6'-11"	433						
* S11	10	#4	3	5'-7"	37						
* S12	10	#4	3			5'-5"	36				
REINFORCING STEEL				LBS. 549		LBS. 549		LBS. 549		LBS. 549	
* EPOXY COATED REINFORCING STEEL				LBS. 470		LBS. 36				349	
7500 P.S.I. CONCRETE				8.2 CU. YDS.		8.2 CU. YDS.		8.2 CU. YDS.		8.2 CU. YDS.	
0.6" Ø L.R. STRANDS No.				24		24		24		24	

GRADE 270 STRANDS	
AREA (SQUARE INCHES)	0.6" Ø L.R.
ULTIMATE STRENGTH (LBS. PER STRAND)	58,600
APPLIED PRESTRESS (LBS. PER STRAND)	43,950



ELASTOMERIC BEARING DETAIL

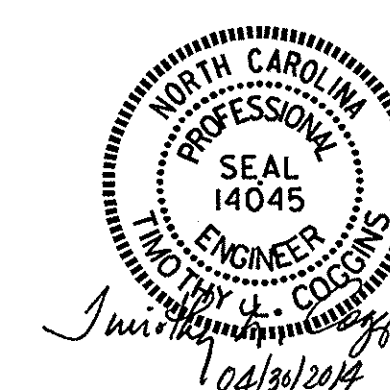
ELASTOMER IN BEARINGS SHALL BE 50 DUROMETER HARDNESS.

DEAD LOAD DEFLECTION AND CAMBER		
3'-0" x 1'-9" 0.6" Ø L.R. STRAND	SPAN A	
CAMBER (SLAB ALONE IN PLACE)	↑	3 3/8"
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	↓	5/8"
FINAL CAMBER	↑	2 3/4"

** INCLUDES FUTURE WEARING SURFACE

CORED SLABS REQUIRED				
SPAN A	TYPE I	1	57'-9"	57'-9"
	TYPE II	1	57'-9"	57'-9"
	TYPE III	10	57'-9"	577'-6"
	TYPE IV	1	57'-9"	57'-9"
TOTAL		13		750'-9"

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

3'-0 X 1'-9"
 PRESTRESSED CONCRETE
 CORED SLAB UNIT

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-14
1			3			TOTAL SHEETS
2			4			

DRAWN BY: N. Ruffin DATE: 4/8/14
 CHECKED BY: T. L. COGGINS DATE: 4/25/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

--- NOTES ---

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

FOR PILE SPLICE DETAILS, SEE END BENT #1 SHEET 4 OF 4.

INSTALL THE 4" DIA. DRAIN PIPE THROUGH THE WING WALL AND SHEET PILING AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

FOR ANCHOR STUD DETAILS AND LOCATION, SEE END BENT #1 SHEET 2 OF 4.

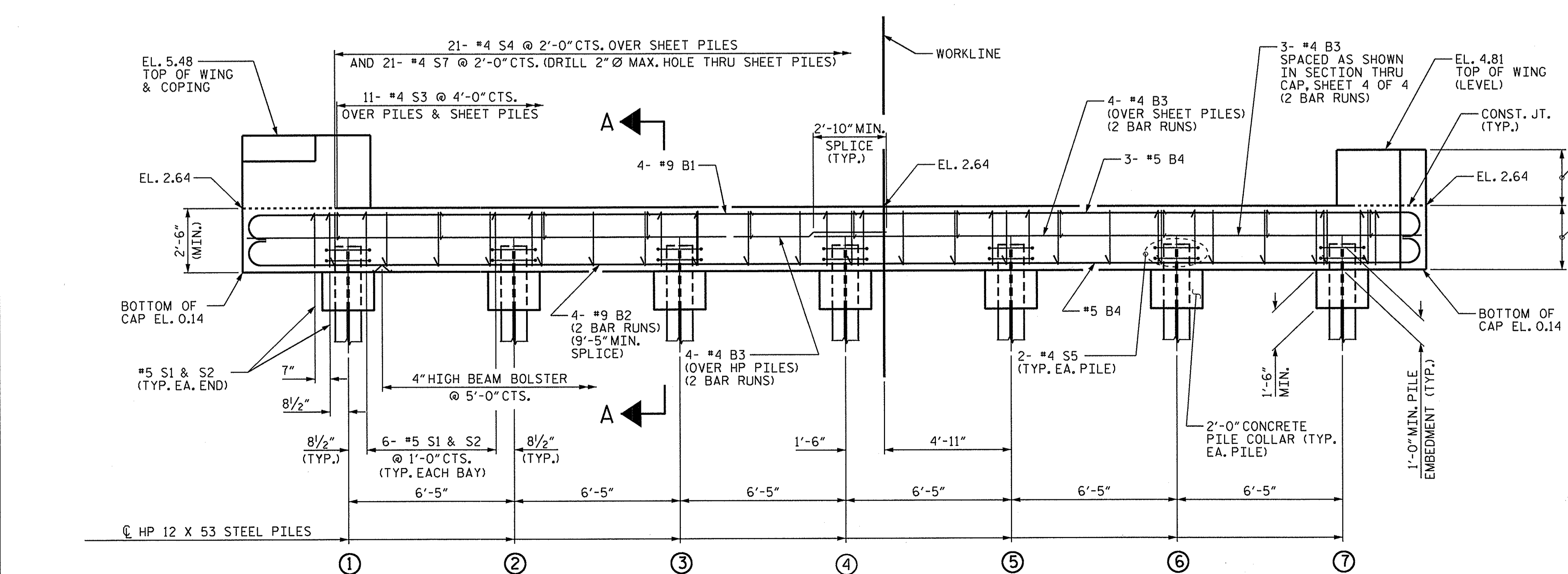
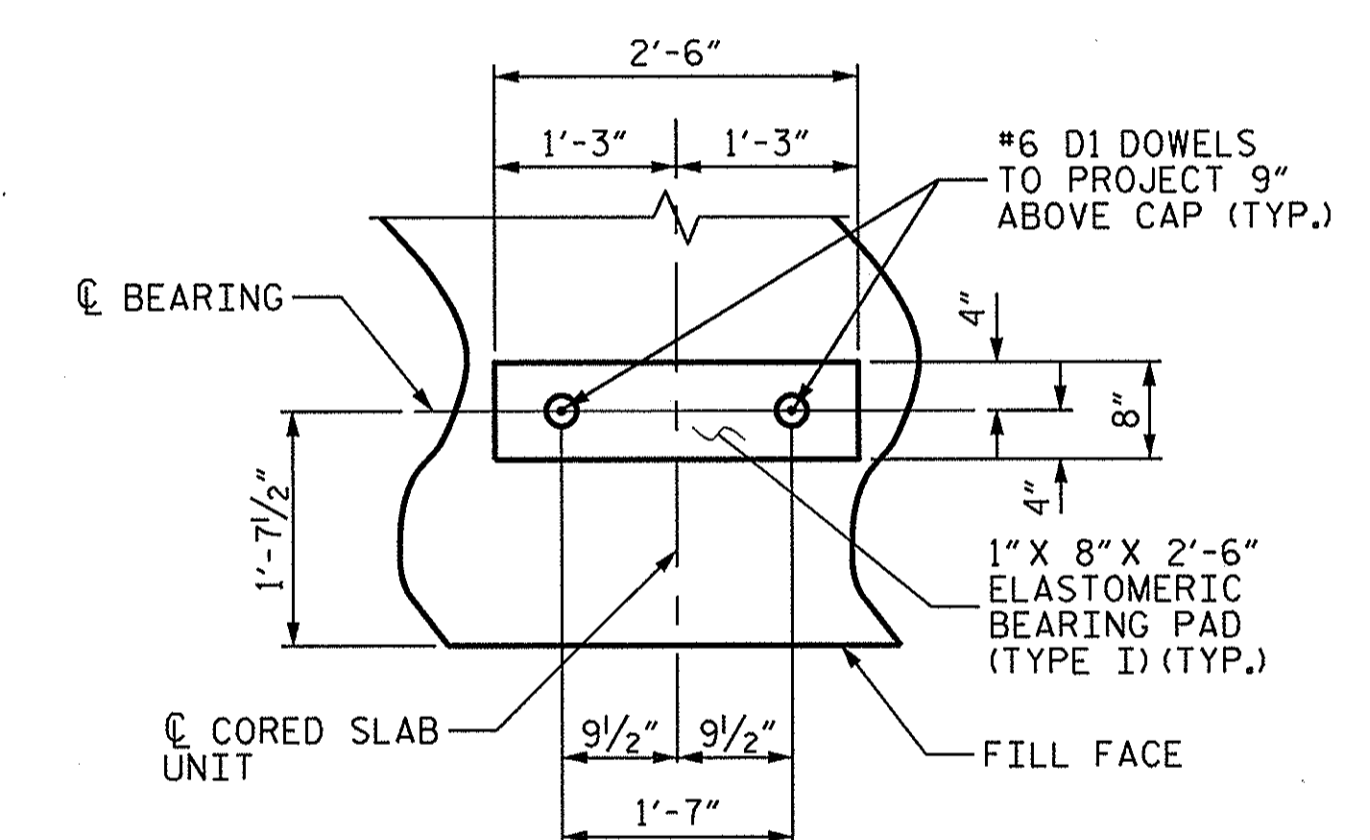
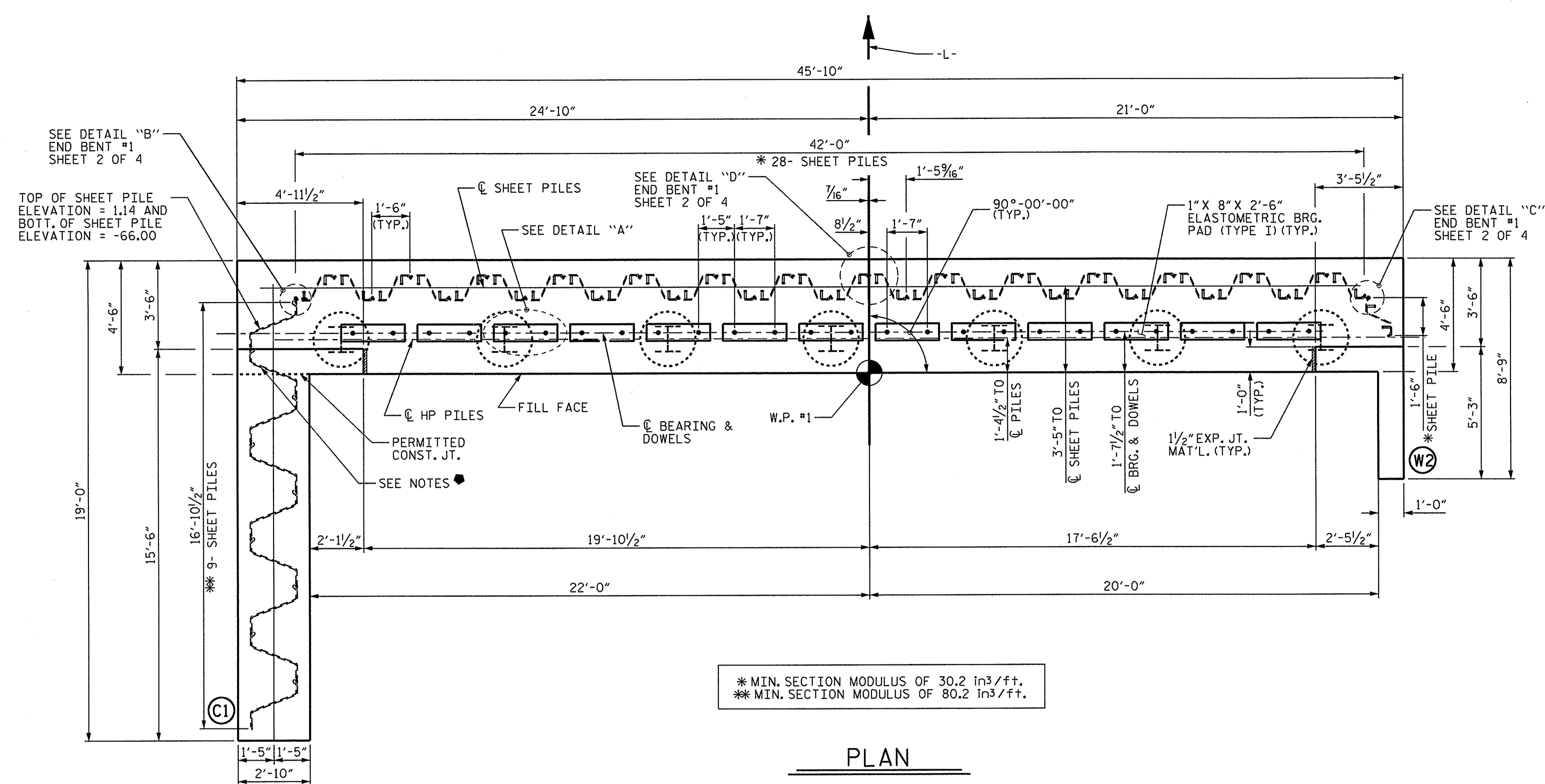
FOR STEEL SHEET PILES, SEE SPECIAL PROVISIONS.

FOR ELEVATION VIEW OF SHEET PILING, SEE END BENT #1 SHEET 2 OF 4.

THE CONCRETE IN THE END BENT CAP OF END BENT NO. 1 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

STEEL SHEET PILES SHALL BE METALLIZED. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

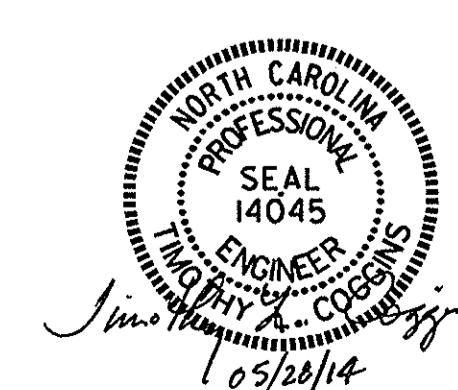
FOR THE SECOND SHEET PILE TOP OF SHEET PILE ELEVATION = 4.98 BOTTOM OF SHEET PILE ELEVATION = -66.00 TRIM BACK TO 6" FROM THE FILL FACE AND DOWN TO THE PREVIOUS SHEET PILE ELEVATION OF 1.14



PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

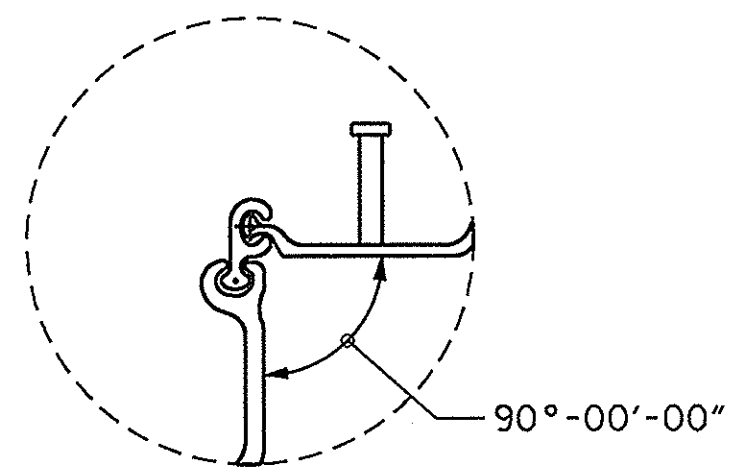
SHEET 1 OF 4

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-15
1			3			TOTAL SHEETS
2			4			

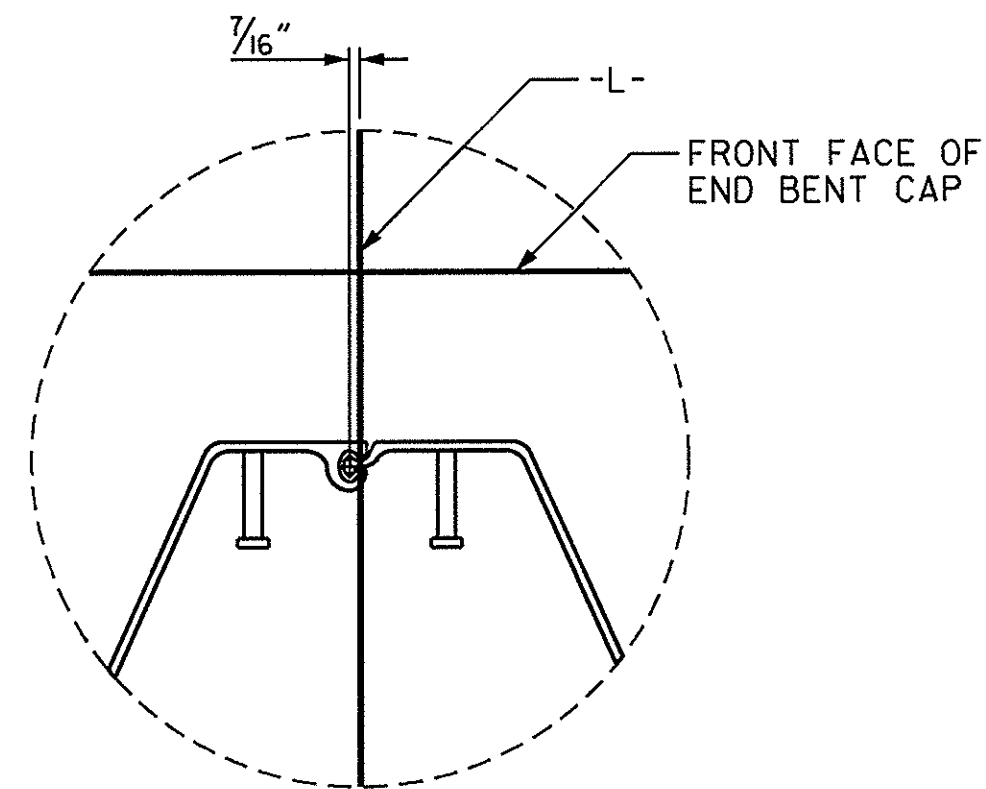


DRAWN BY: N. Ruffin DATE: 2/12/14
CHECKED BY: T.L. COGGINS DATE: 4/02/14
DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

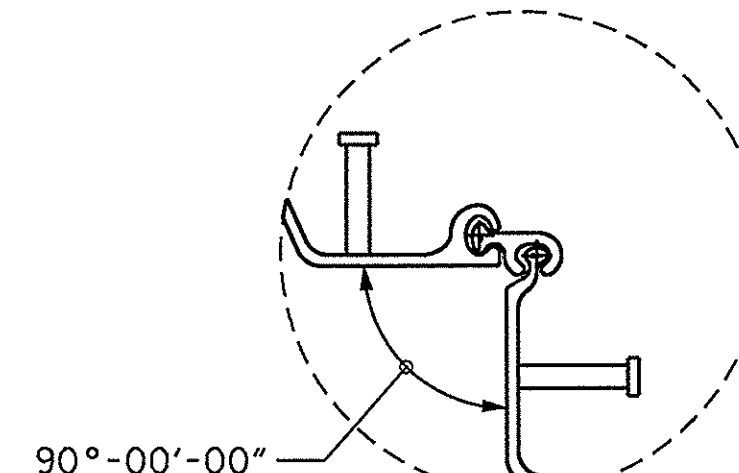
SHEET PILING NOT SHOWN FOR CLARITY.



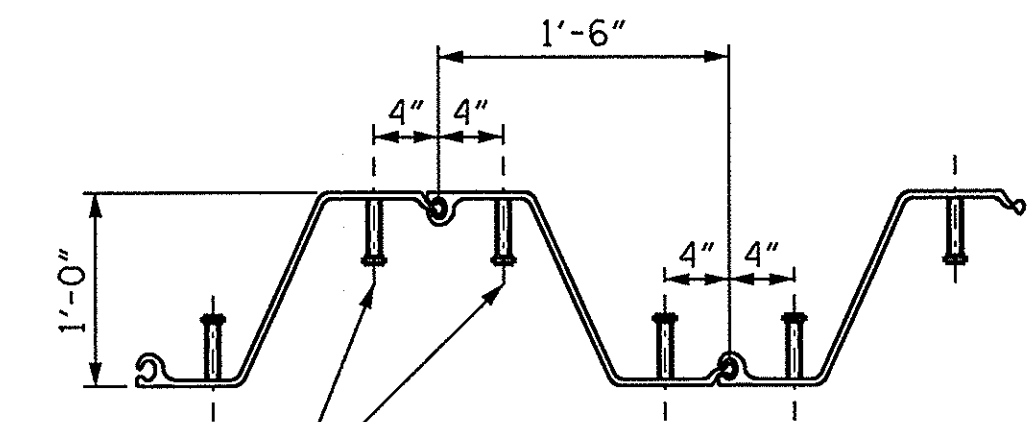
DETAIL "B"



DETAIL "D"

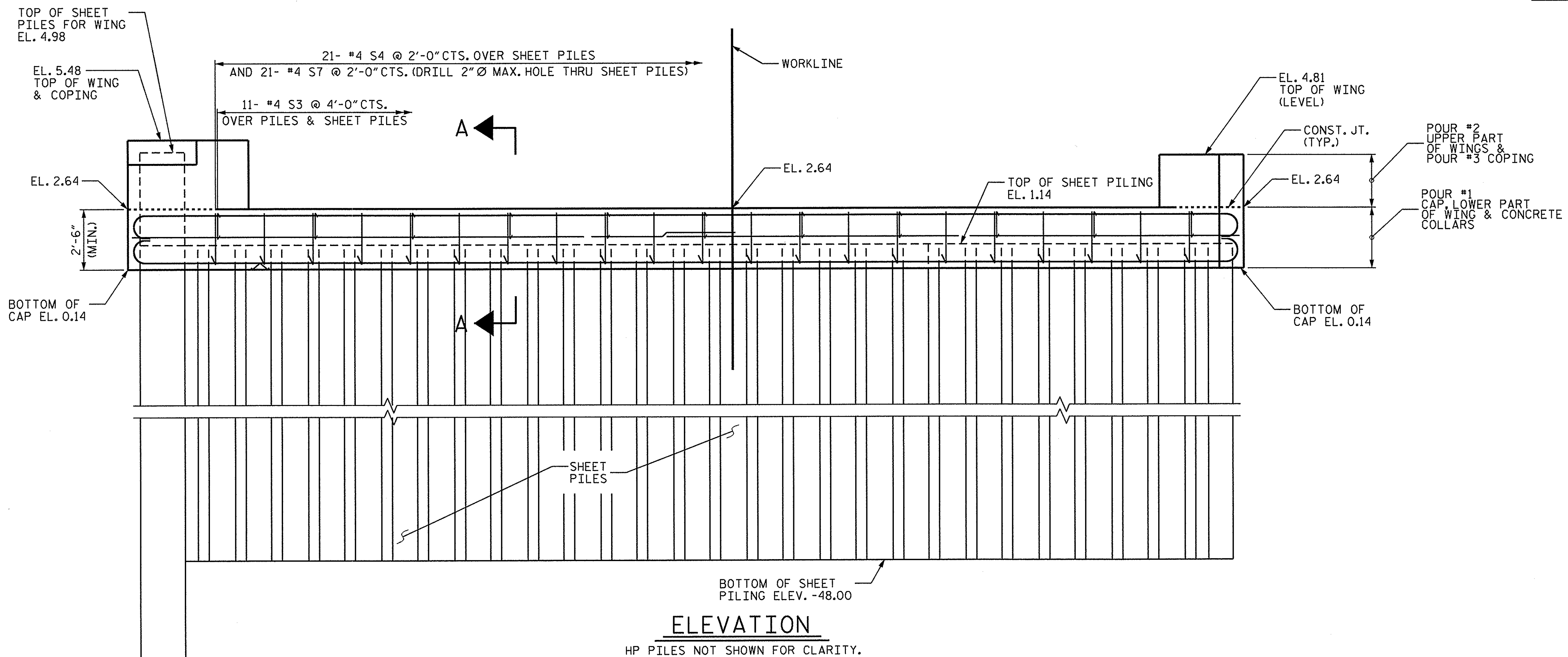


DETAIL "C"



3/4" Ø x 5" ANCHOR STUDS
WELDED TO SHEET PILES
(2 STUDS PER SHEET PILE
ONLY IN THE CAP)

ANCHOR STUD DETAILS



ELEVATION

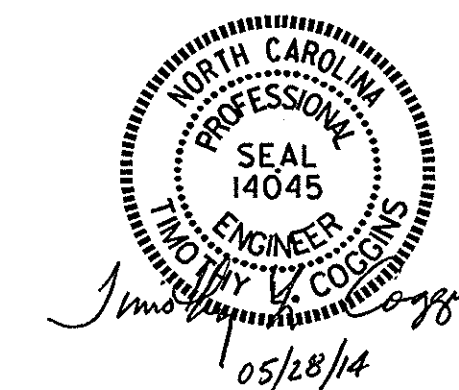
HP PILES NOT SHOWN FOR CLARITY.

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

SHEET 2 OF 4

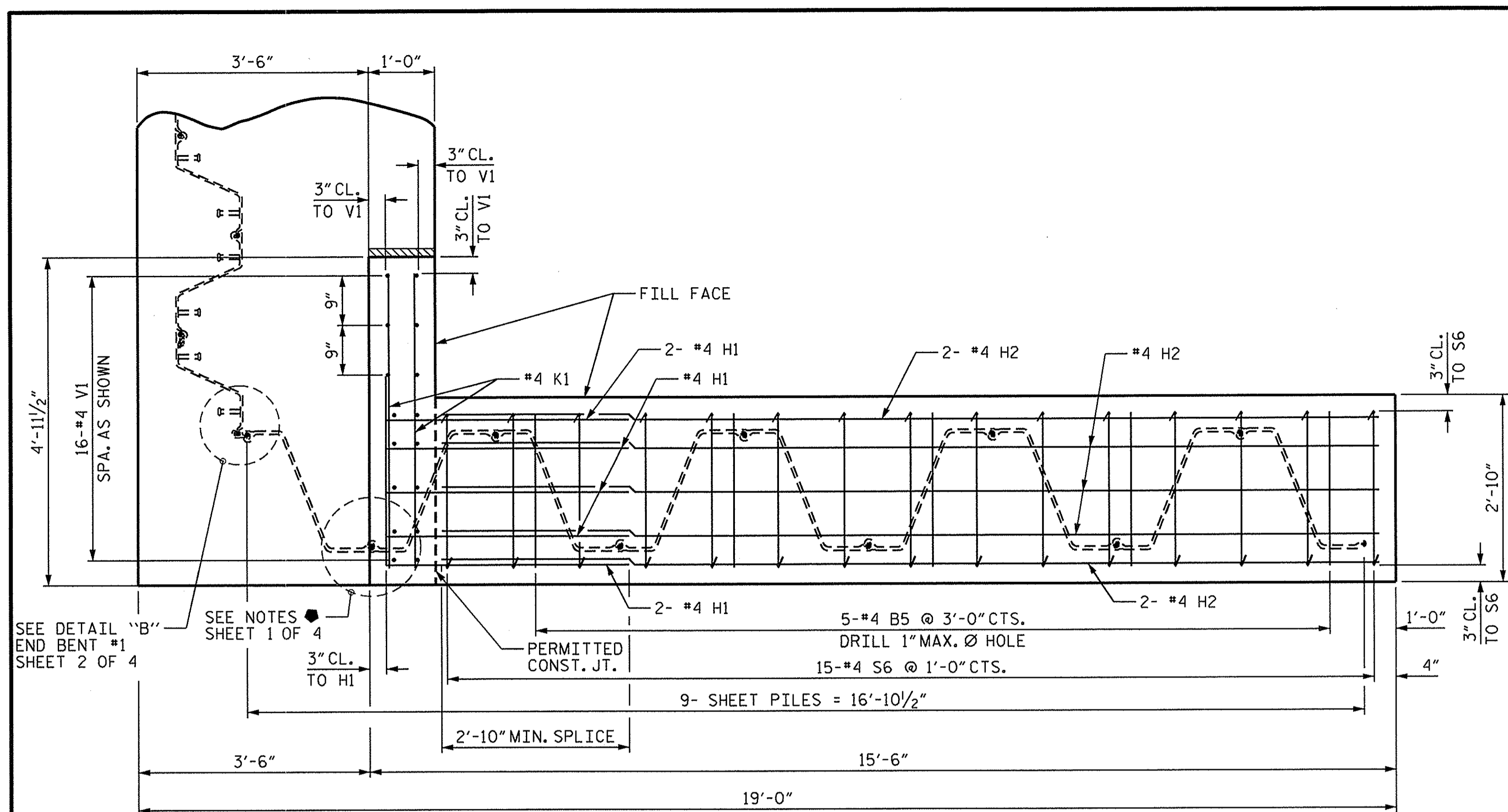
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE
END BENT #1

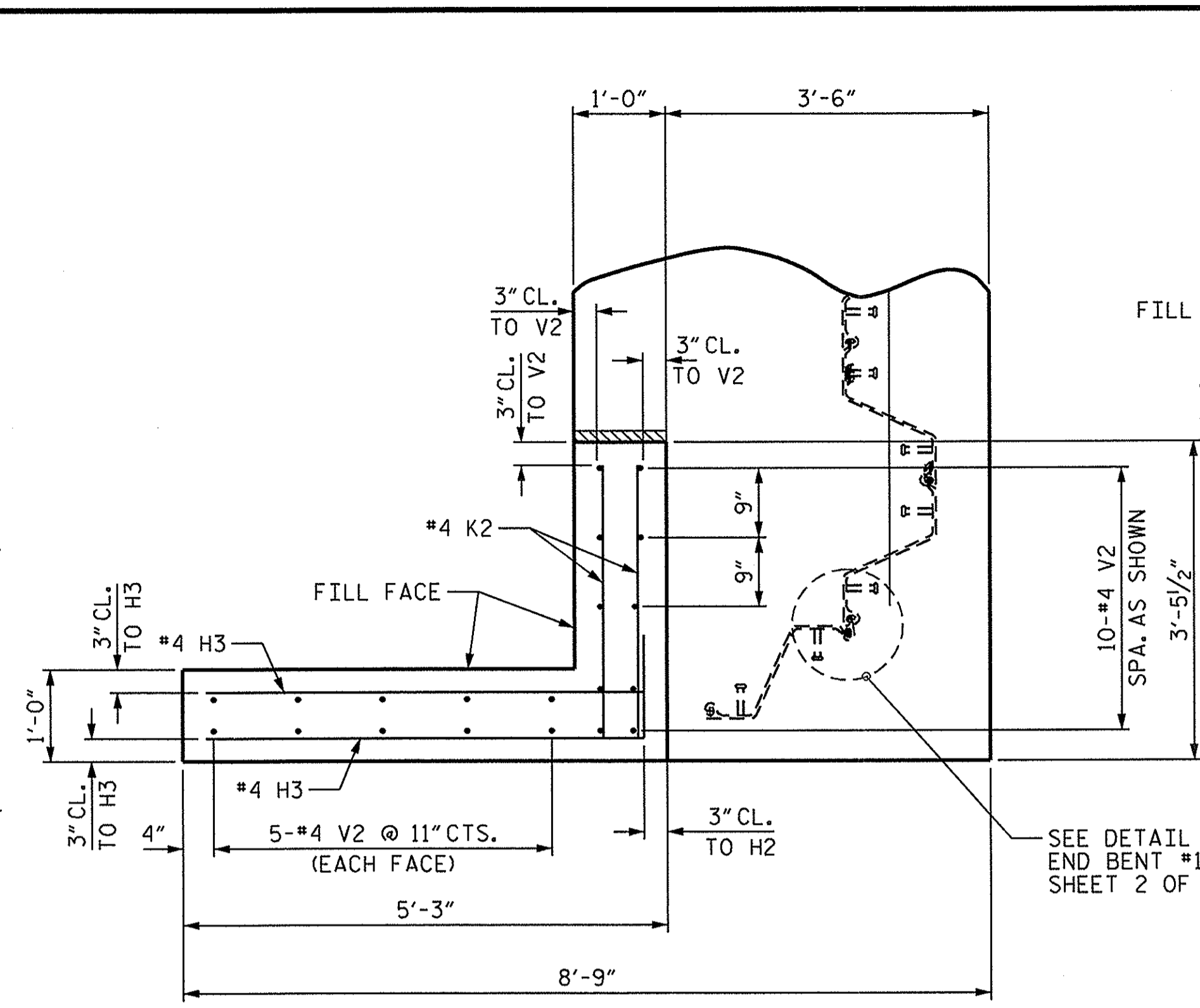


DRAWN BY: N. Ruffin DATE: 2/12/14
CHECKED BY: T.L. COGGINS DATE: 4/2/14
DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

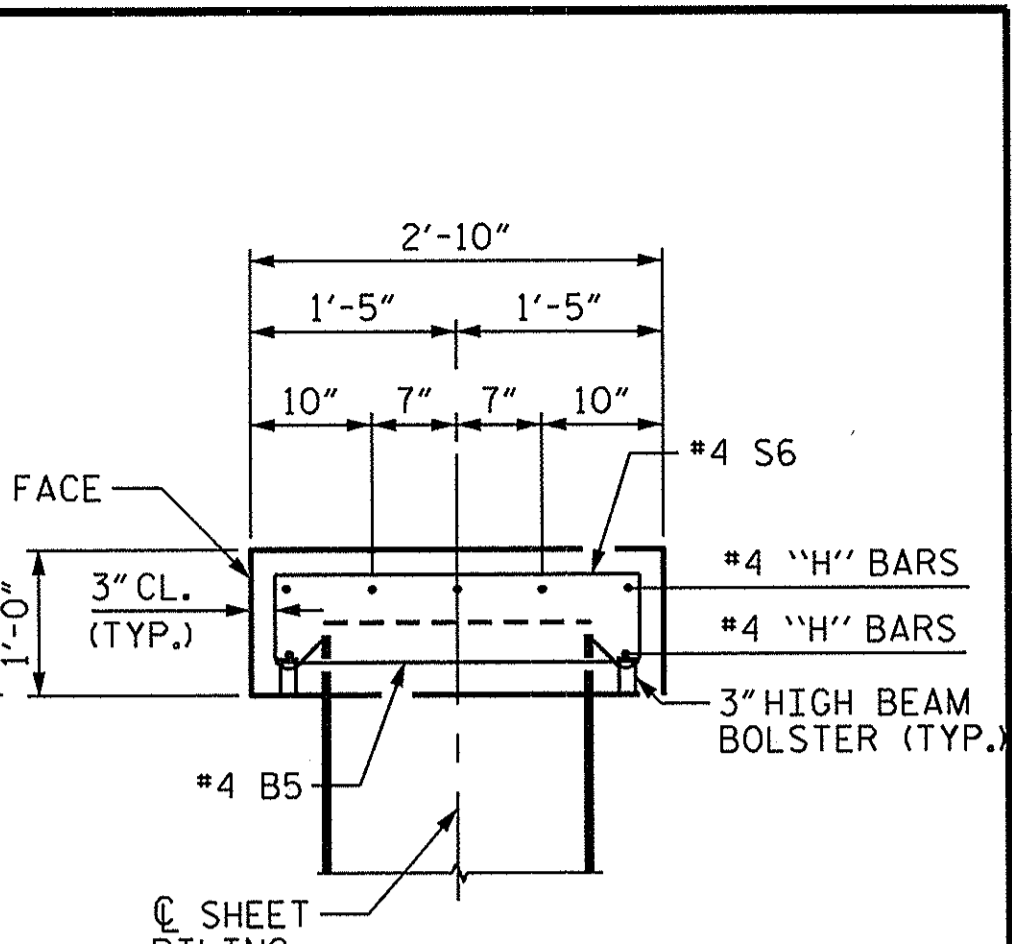
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16
1			3			TOTAL SHEETS
2			4			



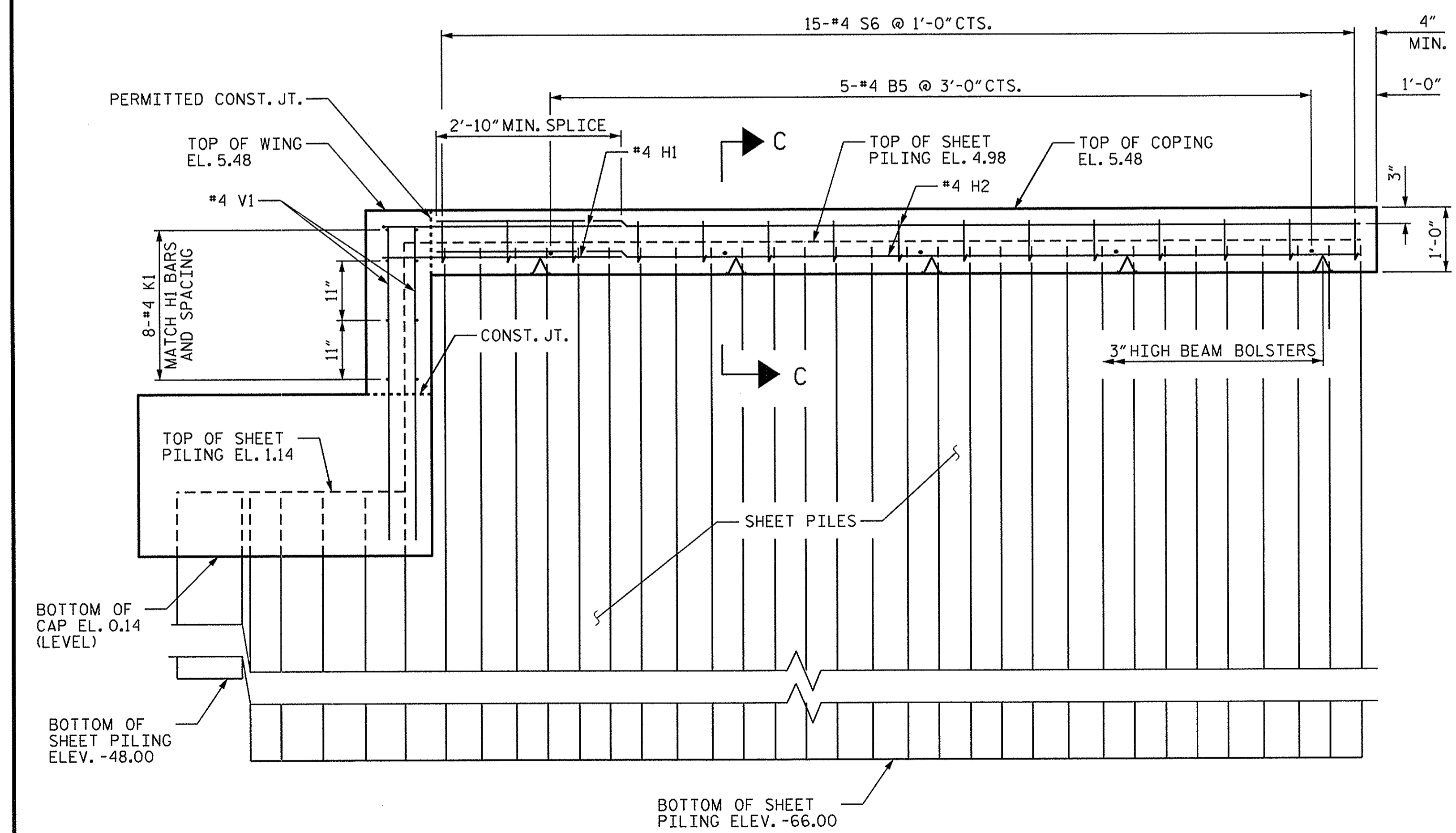
PLAN OF COPING C1



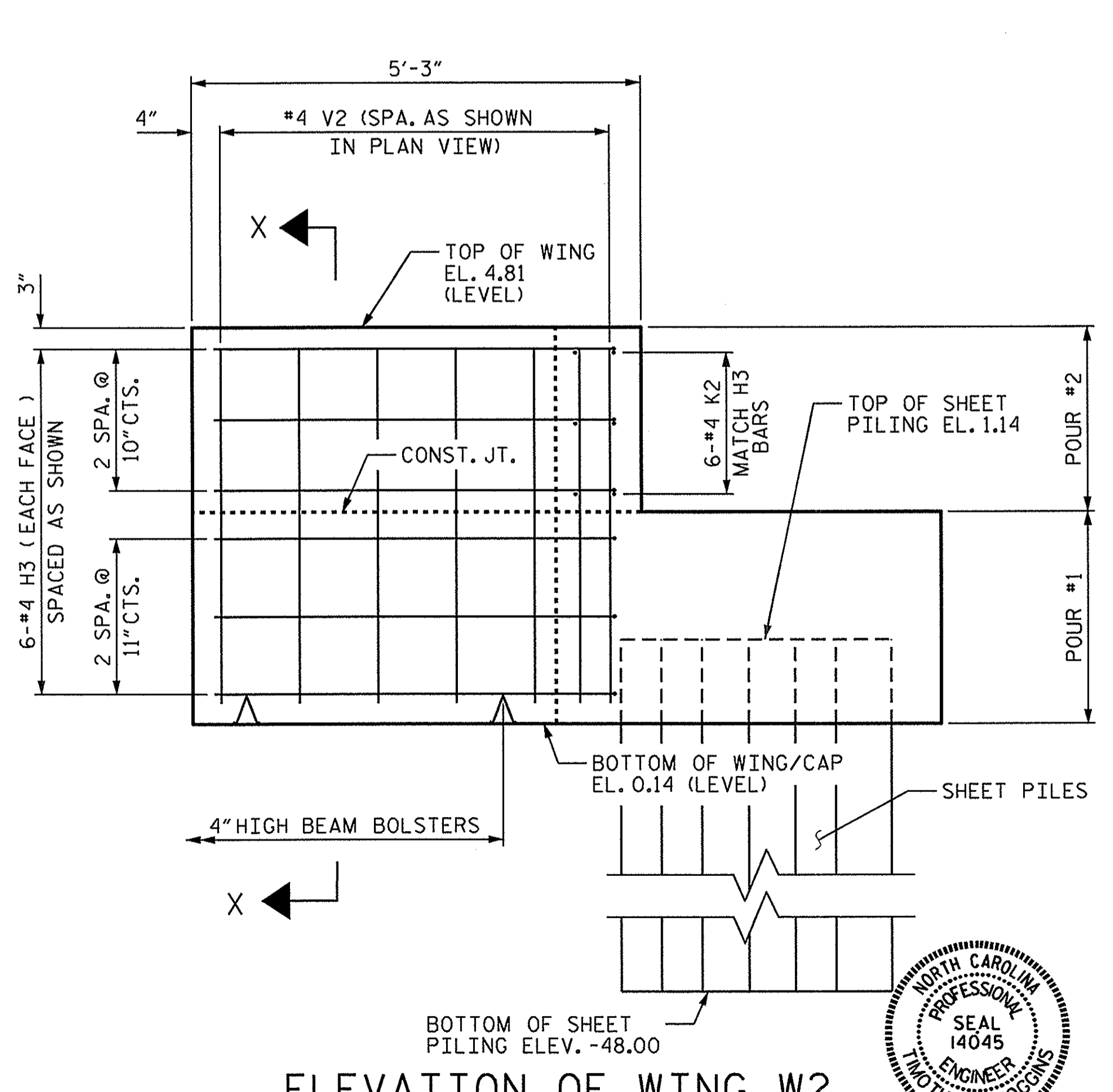
PLAN OF WING W2



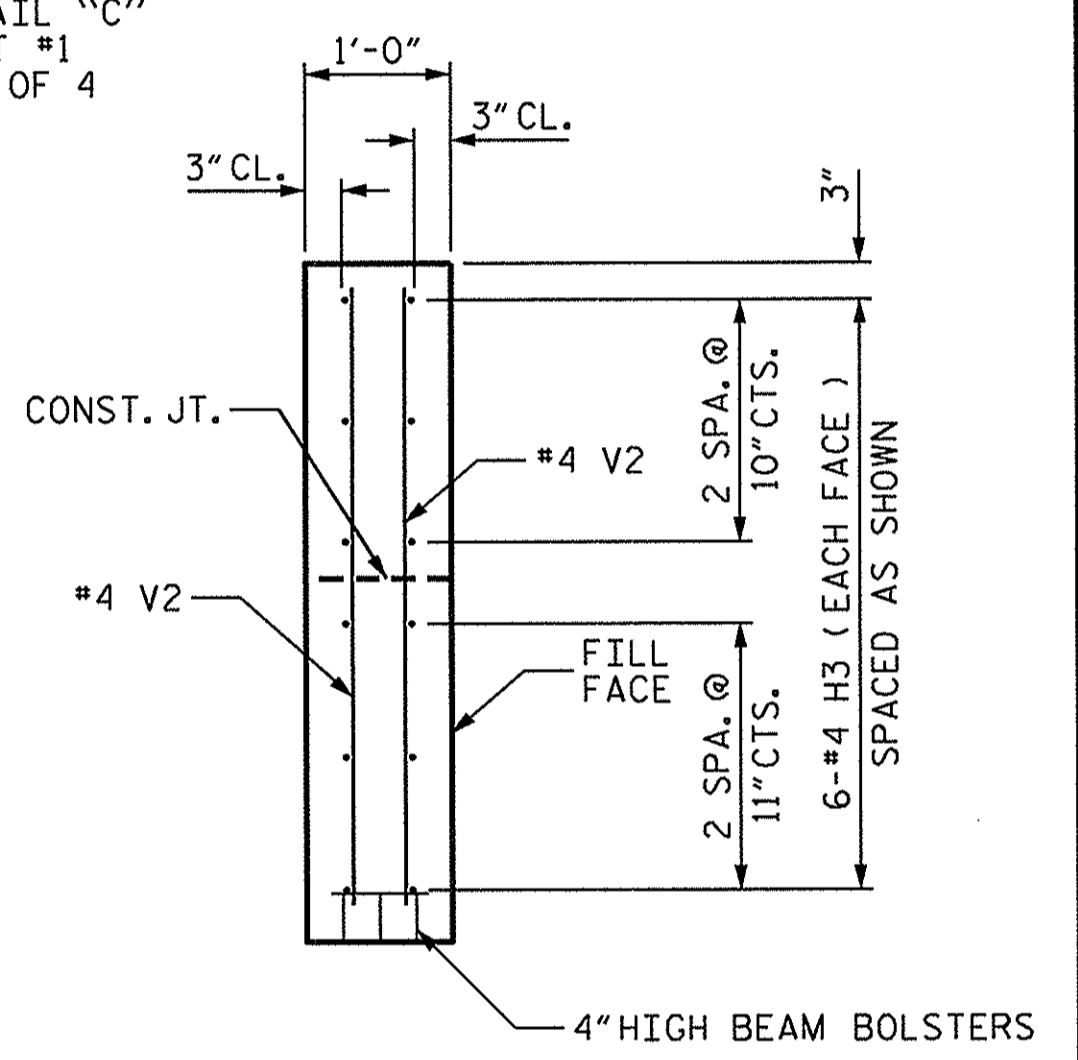
SECTION C-C



ELEVATION OF COPING C1



ELEVATION OF WING W2

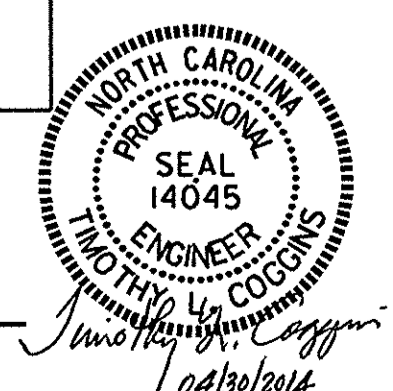


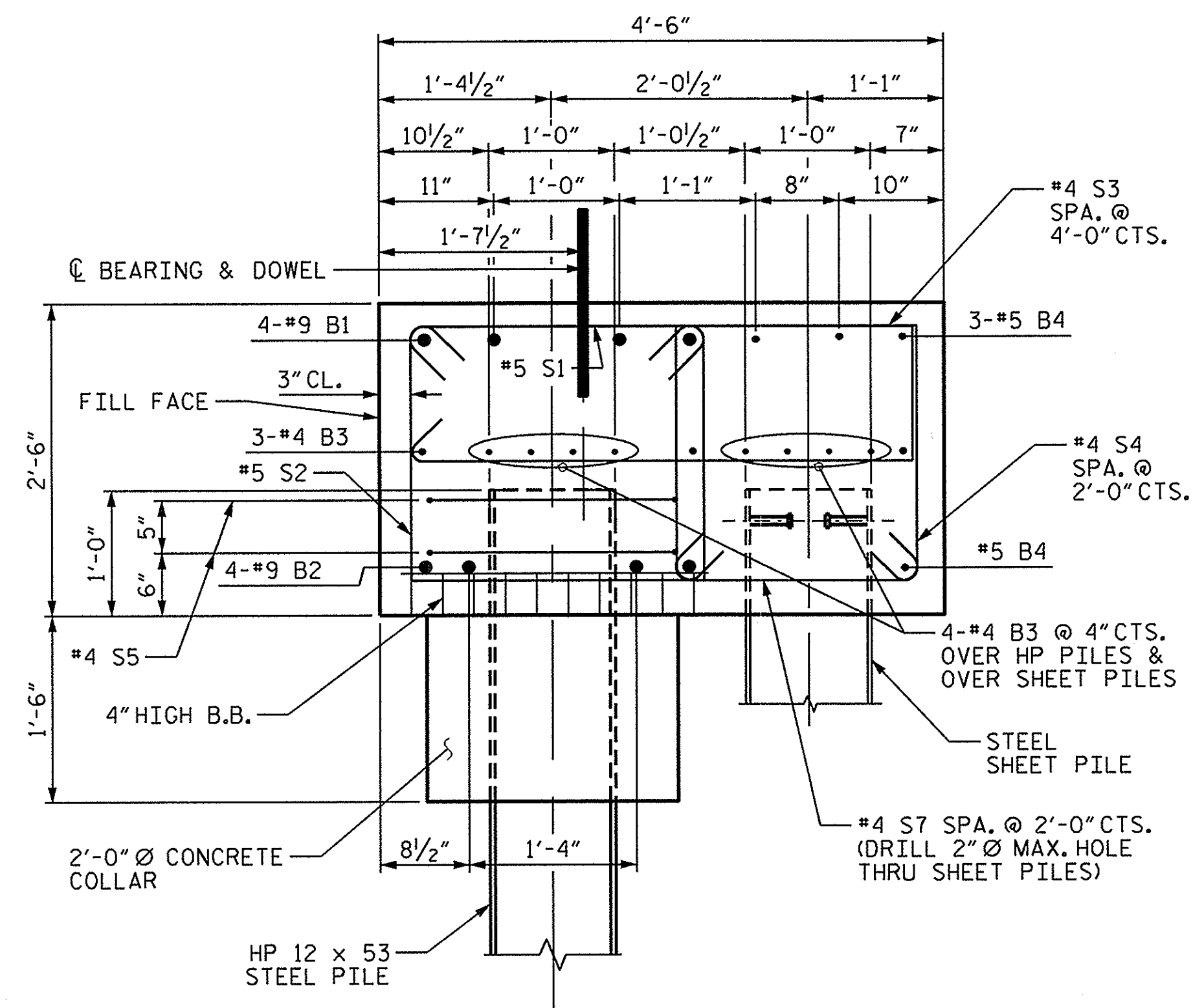
SECTION X-X

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-
 SHEET 3 OF 4

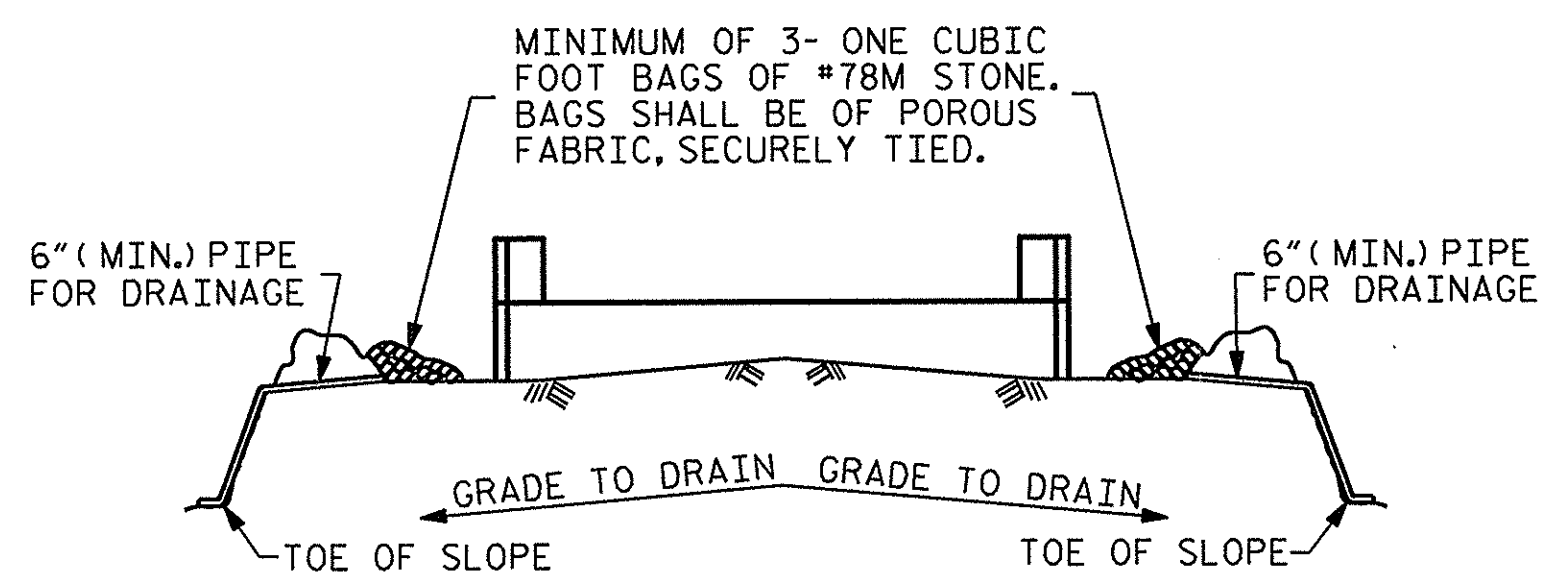
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE END BENT #1					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S-17 TOTAL SHEETS

DRAWN BY: N. Ruffin DATE: 2/18/14
 CHECKED BY: T.L. COGGINS DATE: 4/2/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14





SECTION A-A THRU CAP

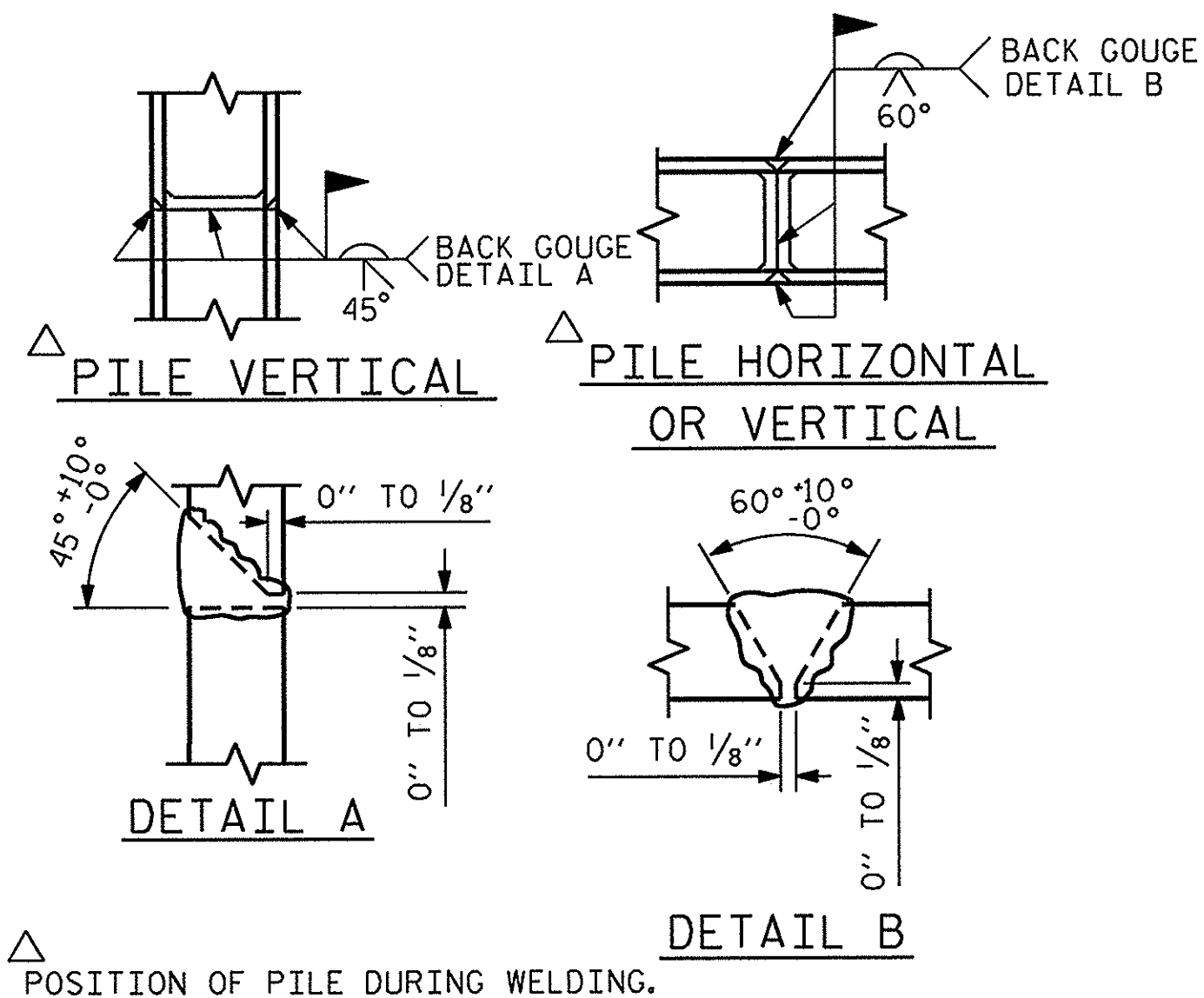


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

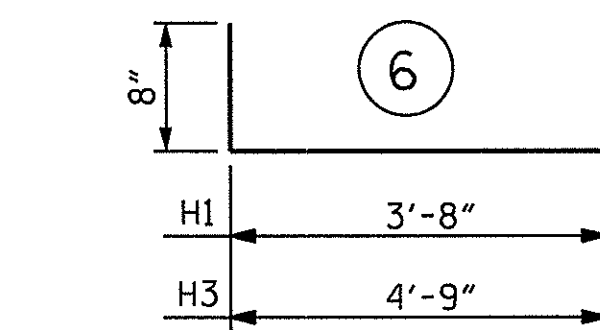
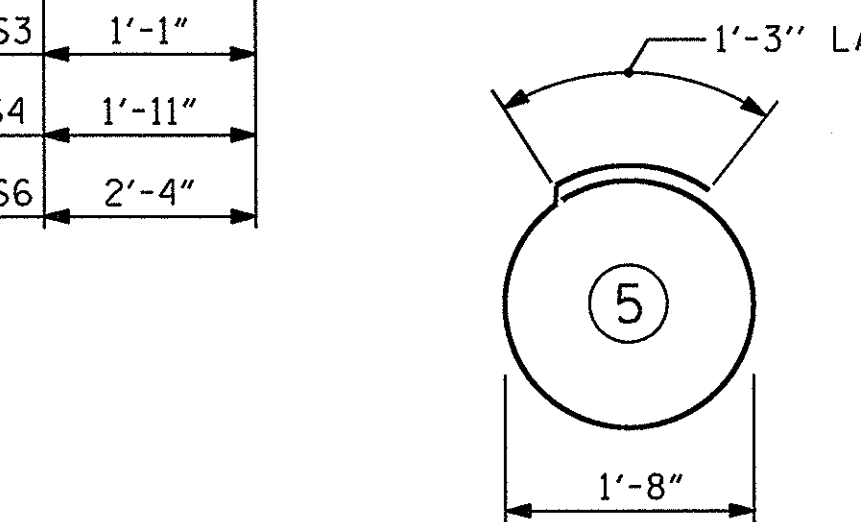
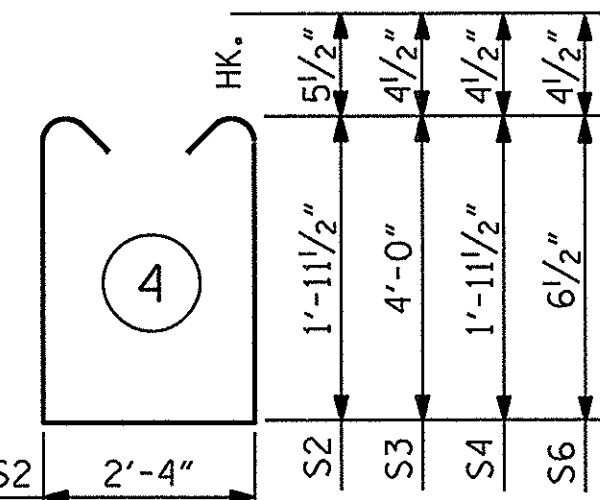
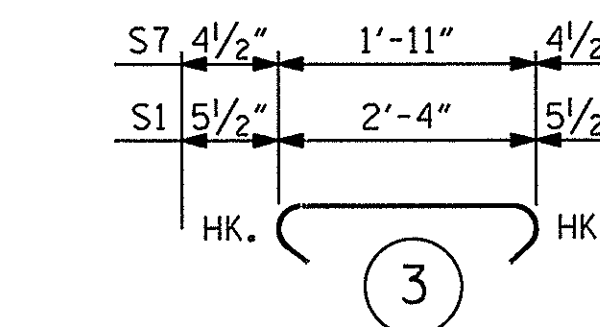
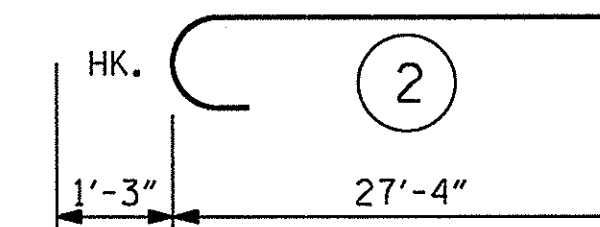
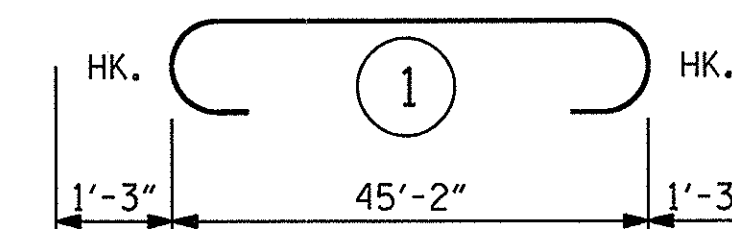


PILE SPLICE DETAILS

DRAWN BY: N. Ruffin DATE: 2/18/14
 CHECKED BY: T. L. COGGINS DATE: 4/2/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

28-MAY-2014 08:31
 S:\DPG\Tim\Division 1 LIBR\BD-5101N\ruffin\BD5101N.sm.Design.01.dgn
 nruffin

BAR TYPES



ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL
 END BENT #1

BAR	NO	SIZE	TYPE	LENGTH	WEIGHT
*B1	4	9	1	47'-8"	648
*B2	8	9	2	28'-7"	777
*B3	22	4	STR	24'-1"	354
*B4	4	5	STR	45'-4"	189
*B5	5	4	STR	2'-4"	8
*D1	26	6	STR	1'-6"	59
*H1	7	4	6	4'-4"	20
*H2	7	4	STR	14'-2"	66
*H3	12	4	6	5'-5"	43
*K1	8	4	STR	4'-6"	24
*K2	6	4	STR	2'-11"	12
*S1	40	5	3	3'-3"	136
*S2	40	5	4	7'-2"	299
*S3	11	4	4	9'-10"	72
*S4	21	4	4	6'-7"	92
*S5	14	4	5	6'-6"	61
*S6	15	4	4	4'-2"	42
*S7	21	4	3	2'-8"	37
*V1	16	4	STR	4'-10"	52
*V2	20	4	STR	4'-2"	56

*EXOPY COATED REINFORCING STEEL LBS. 3,047

CLASS "AA" CONCRETE BREAKDOWN
 POUR #1 CAP & LOWER PART OF WINGS & COLLARS 20.7 C.Y.
 POUR #2 UPPER PART OF WINGS 1 C.Y.
 POUR #3 COPING 1.5 C.Y.
 TOTAL 23.2 C.Y.

HP 12 x 53 STEEL PILES No. = 7 LIN. FEET. 665

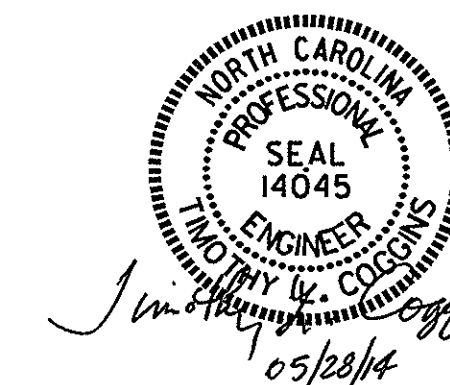
PILE REDRIVES EA. 1

STEEL SHEET PILES
 NO. 90 = 2
 NO. PILES = 38
 TOTAL NO. = 40 SQ. FT. 3335

PROJECT NO. BD-5101N
 PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 SUBSTRUCTURE
 END BENT #1



REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-18
1			3			TOTAL SHEETS
2			4			-

NC006

--- NOTES ---

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

FOR PILE SPLICE DETAILS, SEE END BENT #2 SHEET 4 OF 4.

INSTALL THE 4" DIA. DRAIN PIPE THROUGH THE WING WALL AND SHEET PILING AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

FOR ANCHOR STUD DETAILS AND LOCATION, SEE END BENT #2 SHEET 2 OF 4.

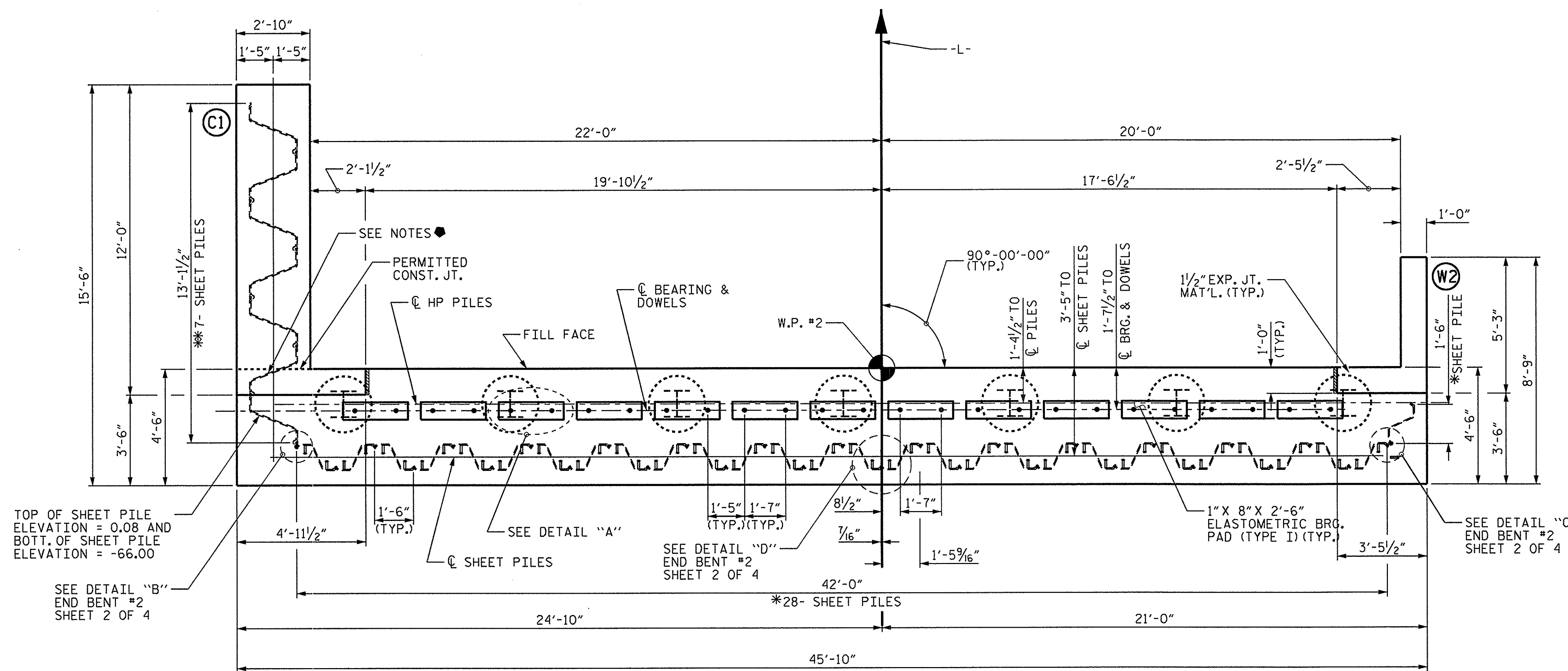
FOR STEEL SHEET PILES, SEE SPECIAL PROVISIONS.

FOR ELEVATION VIEW OF SHEET PILING, SEE END BENT #2 SHEET 2 OF 4.

THE CONCRETE IN THE END BENT CAP OF END BENT NO. 2 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

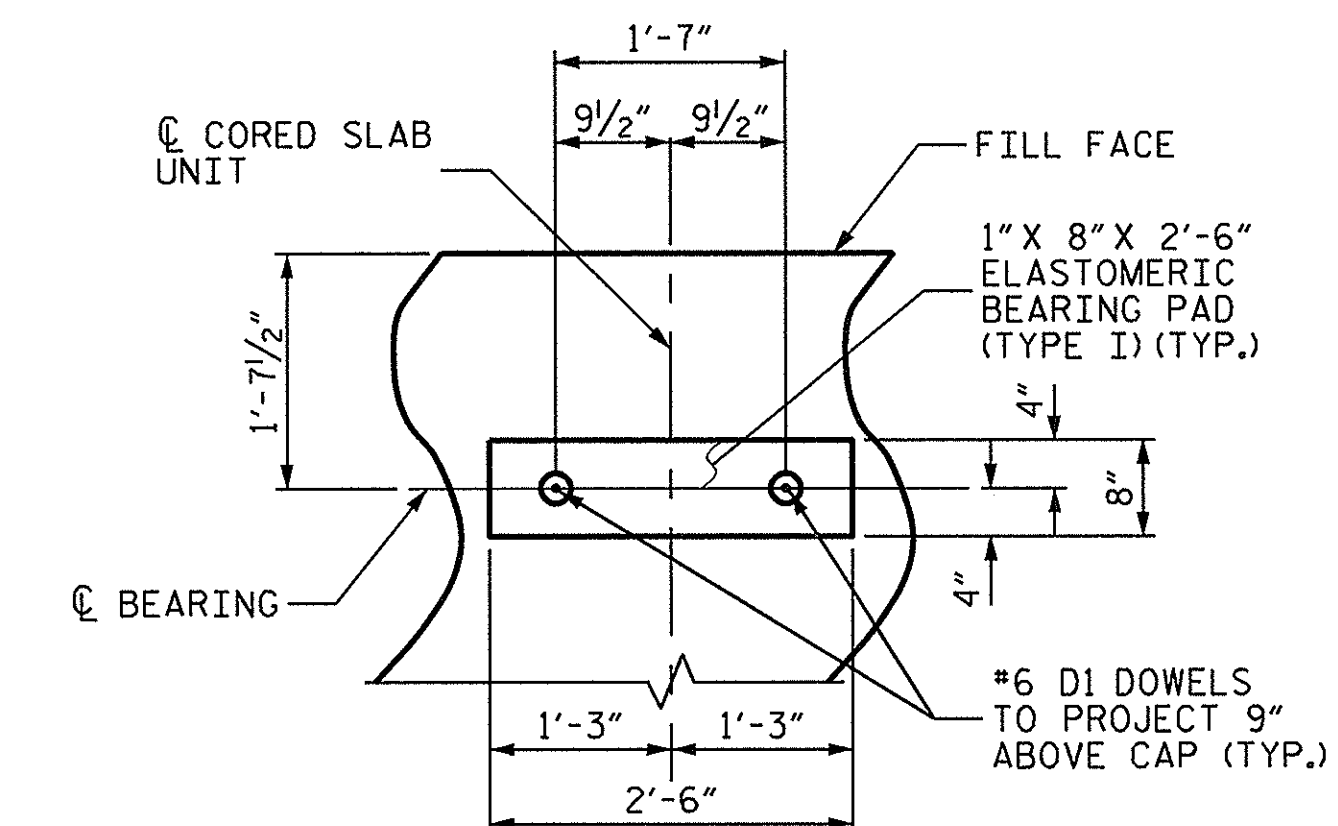
STEEL SHEET PILES SHALL BE METALLIZED. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR THE SECOND SHEET PILE TOP OF SHEET PILE ELEVATION = 4.64 BOTTOM OF SHEET PILE ELEVATION = -66.00 TRIM BACK TO 6" FROM THE FILL FACE AND DOWN TO THE PREVIOUS SHEET PILE ELEVATION OF 0.08

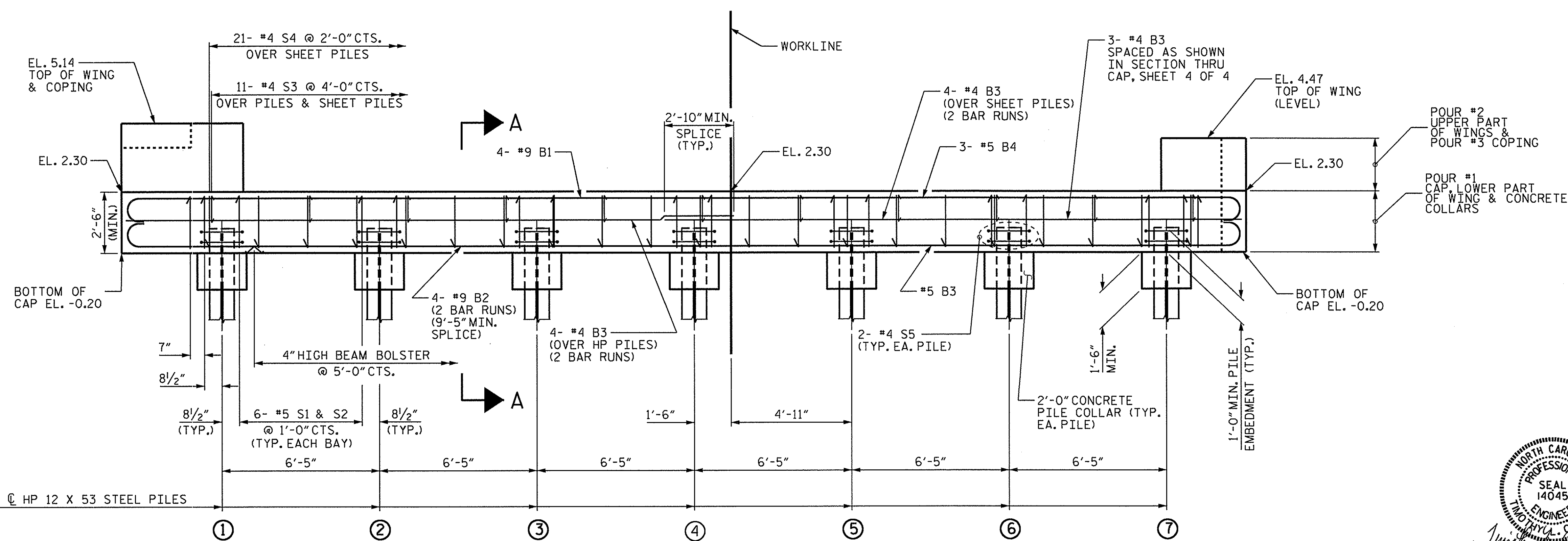


* MIN. SECTION MODULUS OF 30.2 in³/ft.
 ** MIN. SECTION MODULUS OF 80.2 in³/ft.

PLAN



DETAIL "A"



ELEVATION

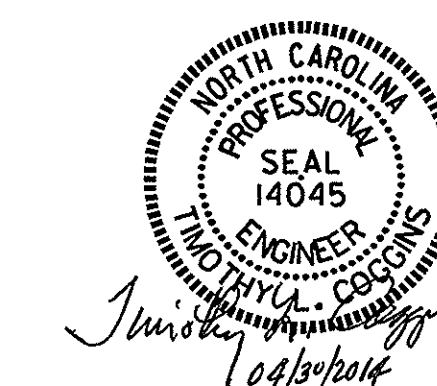
SHEET PILING NOT SHOWN FOR CLARITY.

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 1 OF 4

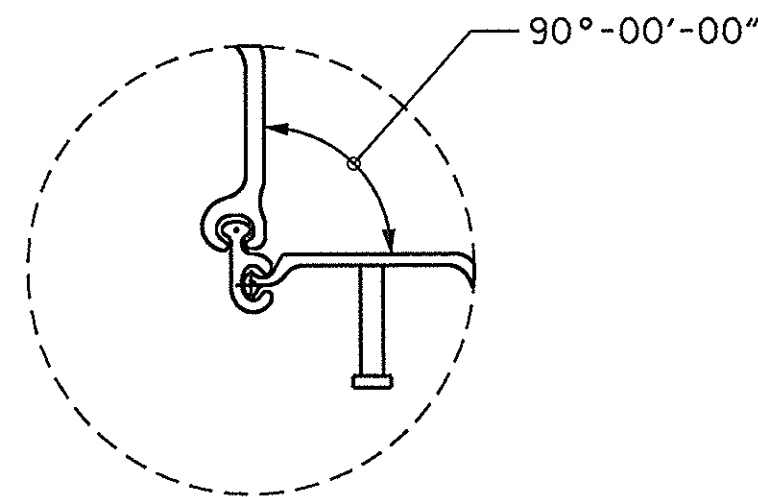
STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH

SUBSTRUCTURE
 END BENT #2

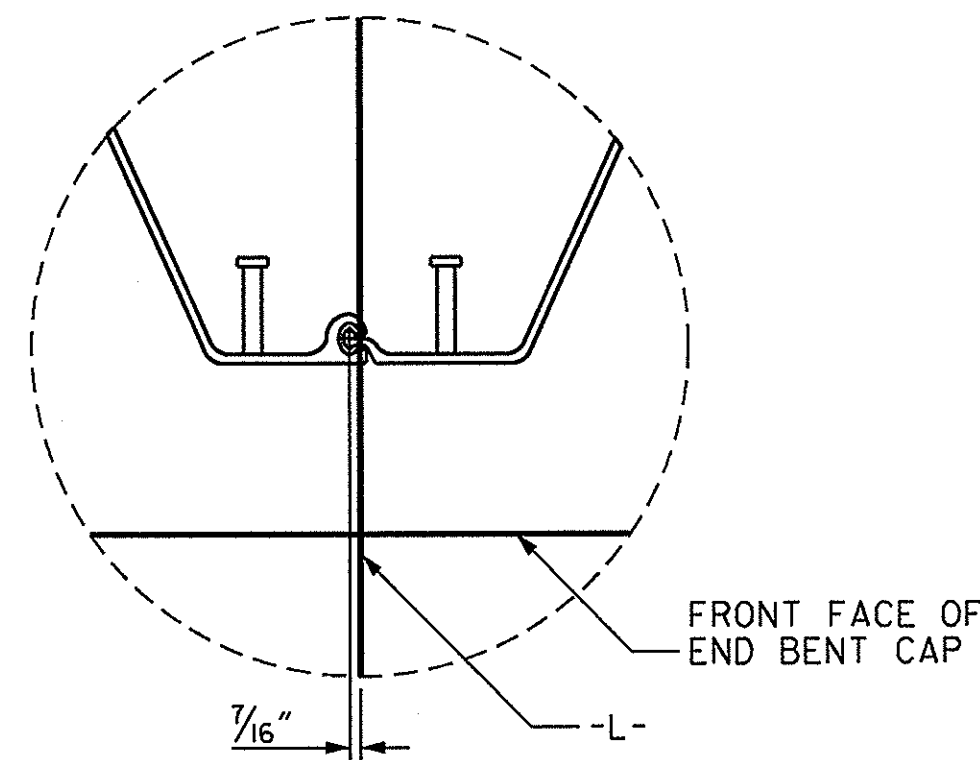


DRAWN BY: N. Ruffin DATE: 2/12/14
 CHECKED BY: T.L. COGGINS DATE: 4/2/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

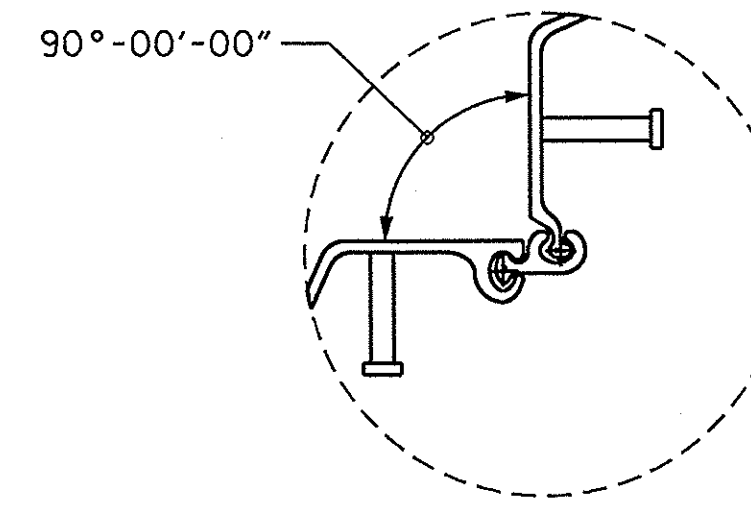
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-19
1			3			TOTAL SHEETS
2			4			



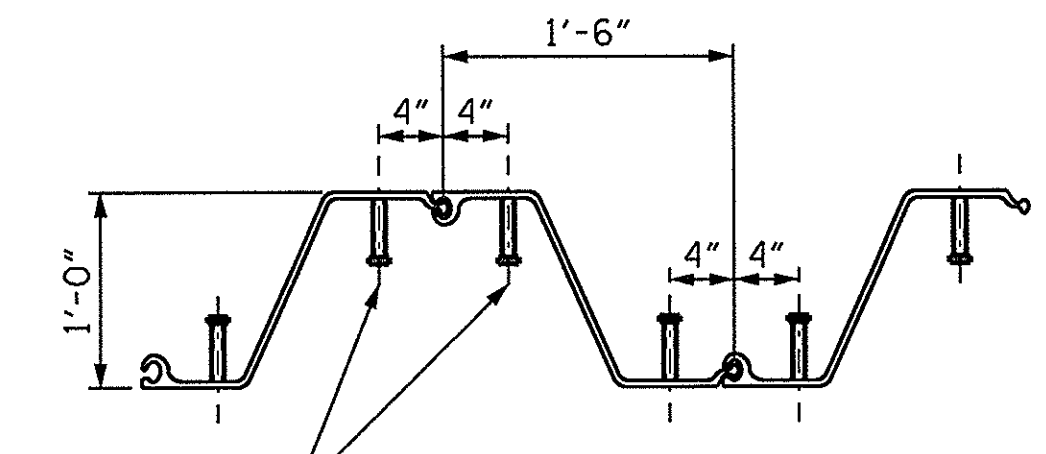
DETAIL "B"



DETAIL "D"

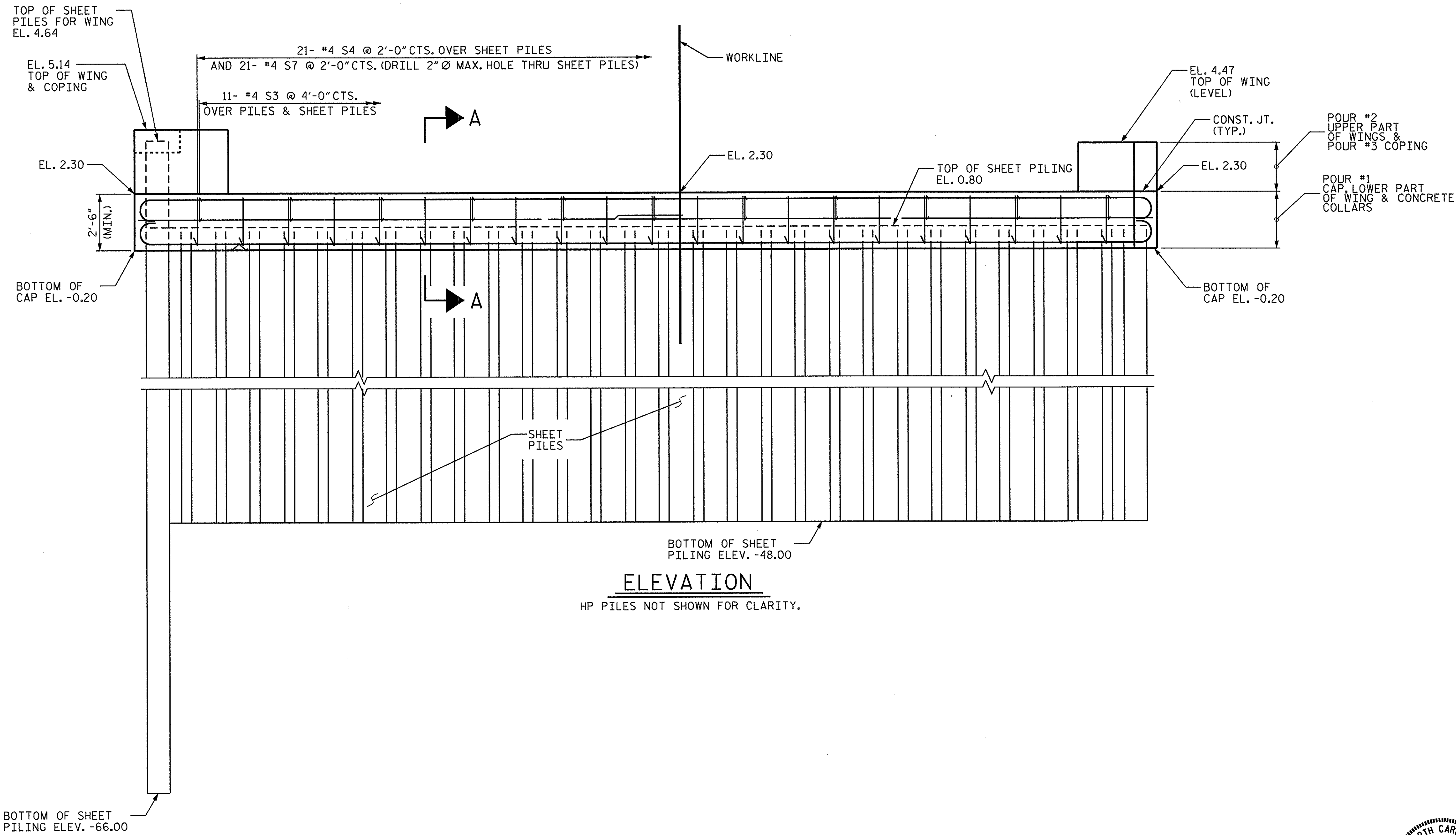


DETAIL "C"



3/4" Ø x 5" ANCHOR STUDS
WELDED TO SHEET PILES
(2 STUDS PER SHEET PILE
ONLY IN THE CAP)

ANCHOR STUD DETAILS



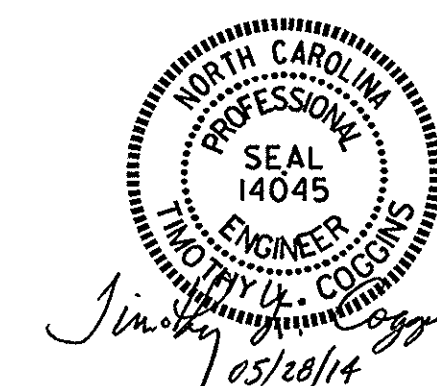
ELEVATION
HP PILES NOT SHOWN FOR CLARITY.

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

SHEET 2 OF 4

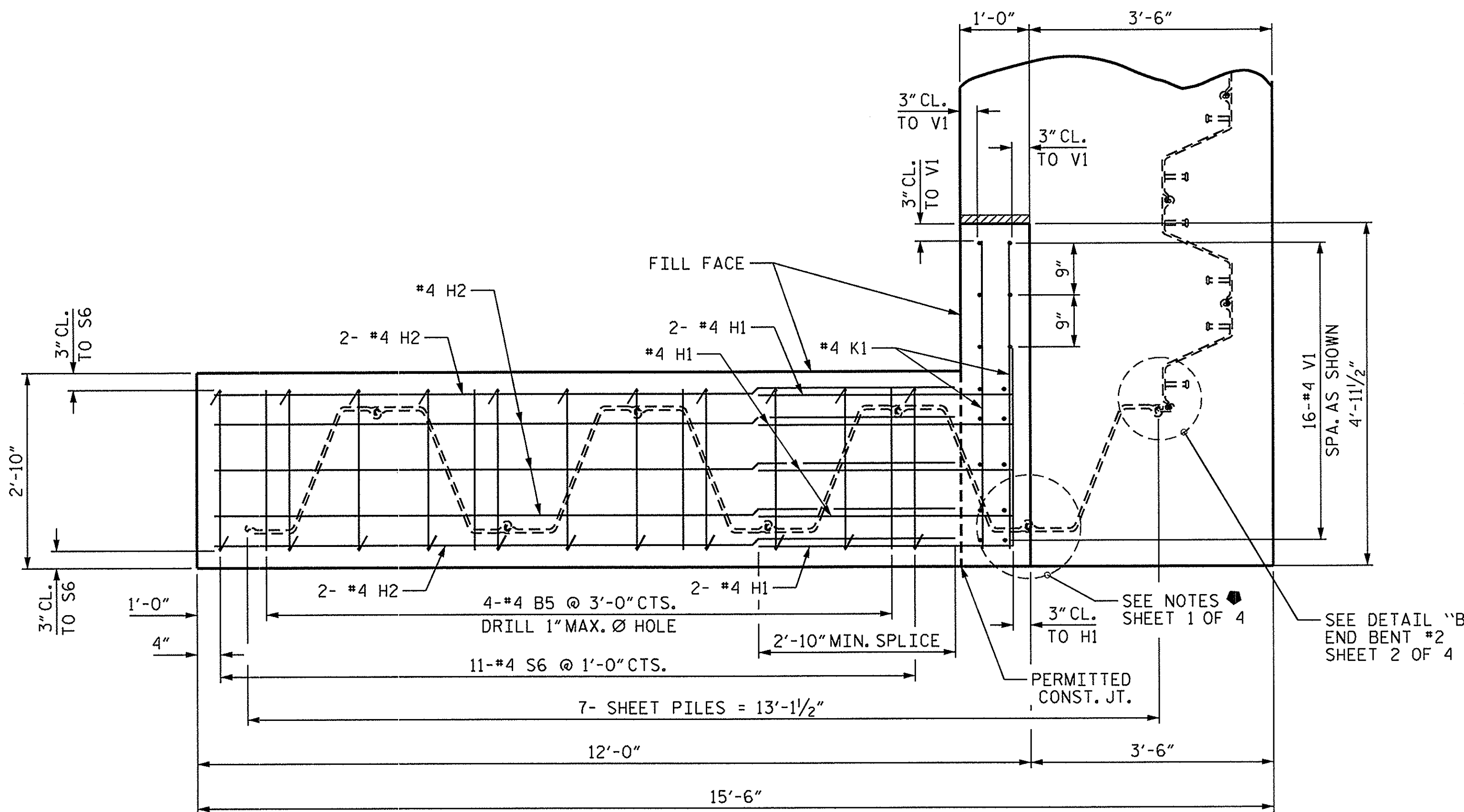
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE
END BENT #2

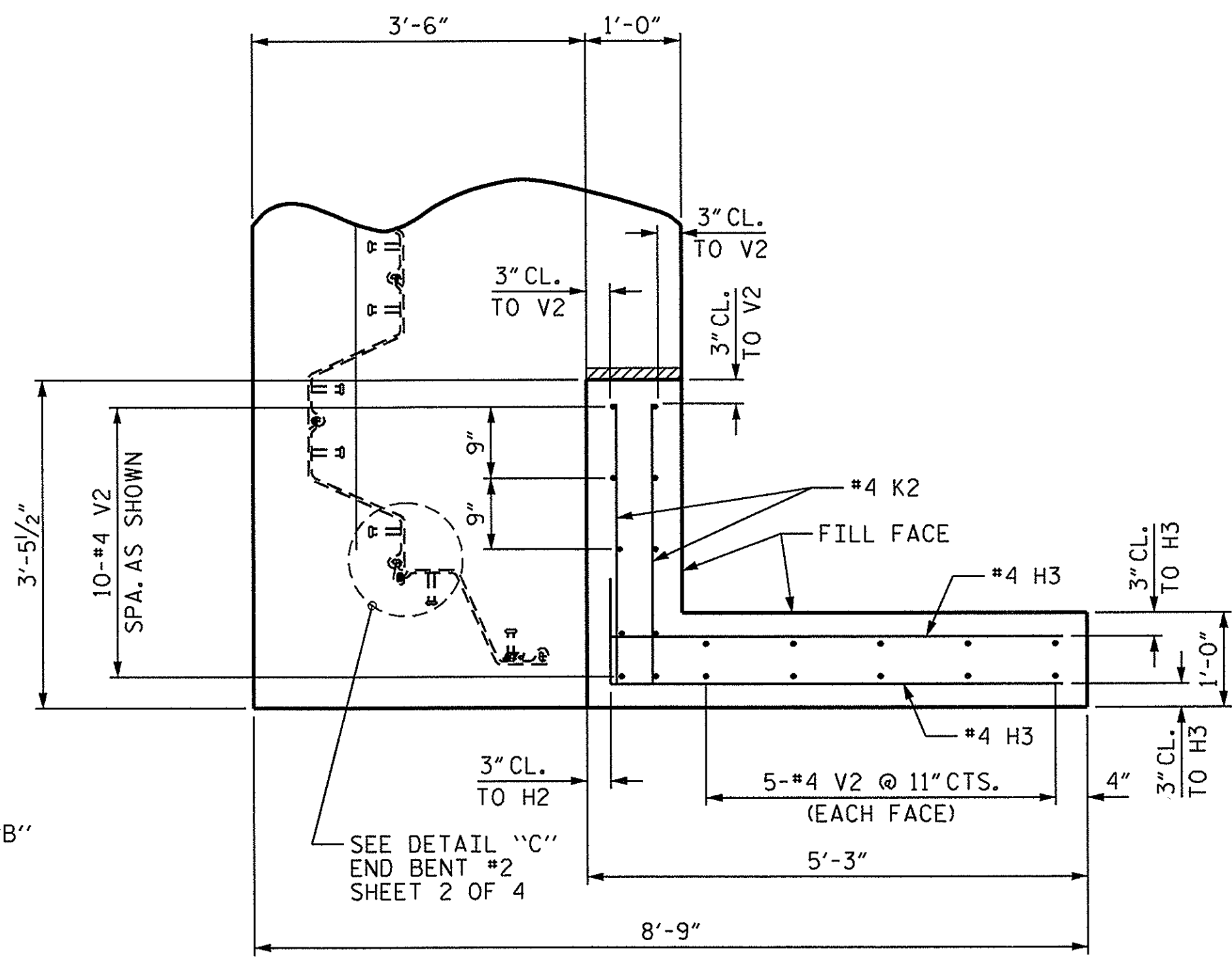


DRAWN BY: N. Ruffin DATE: 2/12/14
CHECKED BY: T.L. COGGINS DATE: 4/2/14
DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

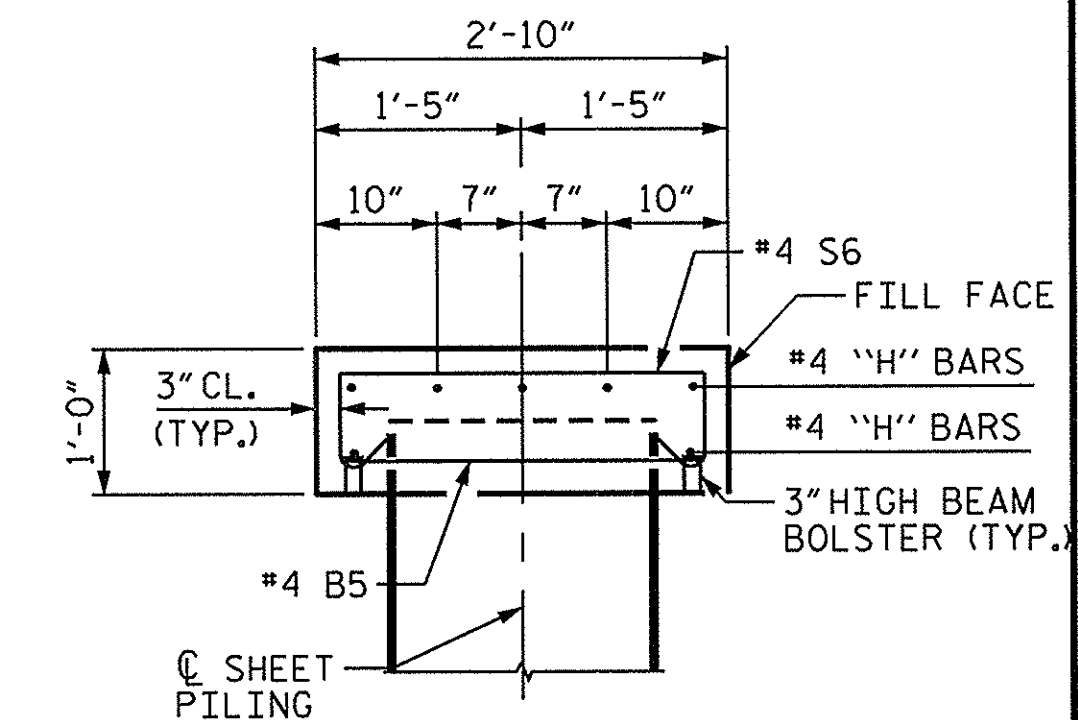
REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-20
1			3			TOTAL SHEETS
2			4			



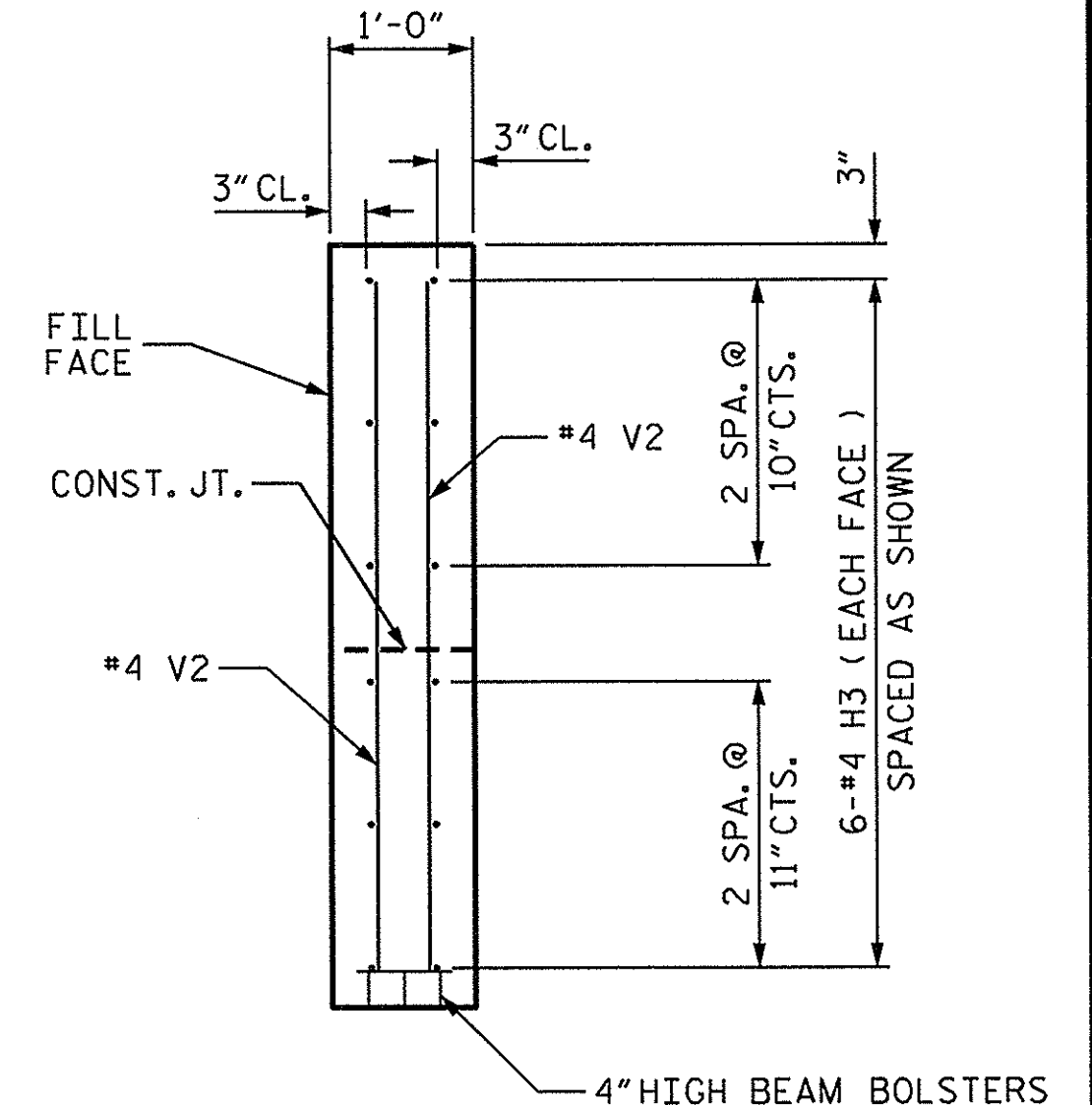
PLAN OF COPING C1



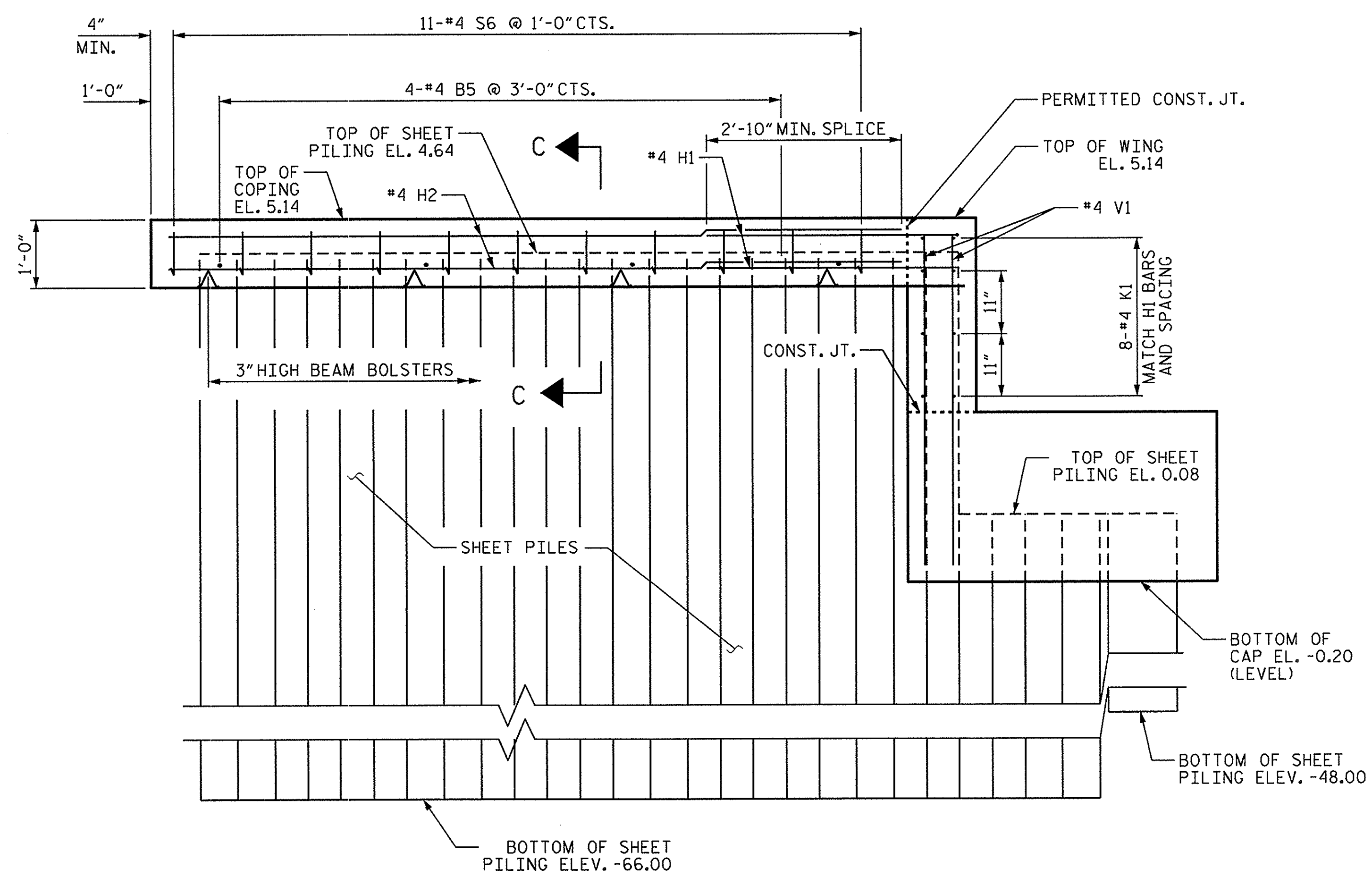
PLAN OF WING W2



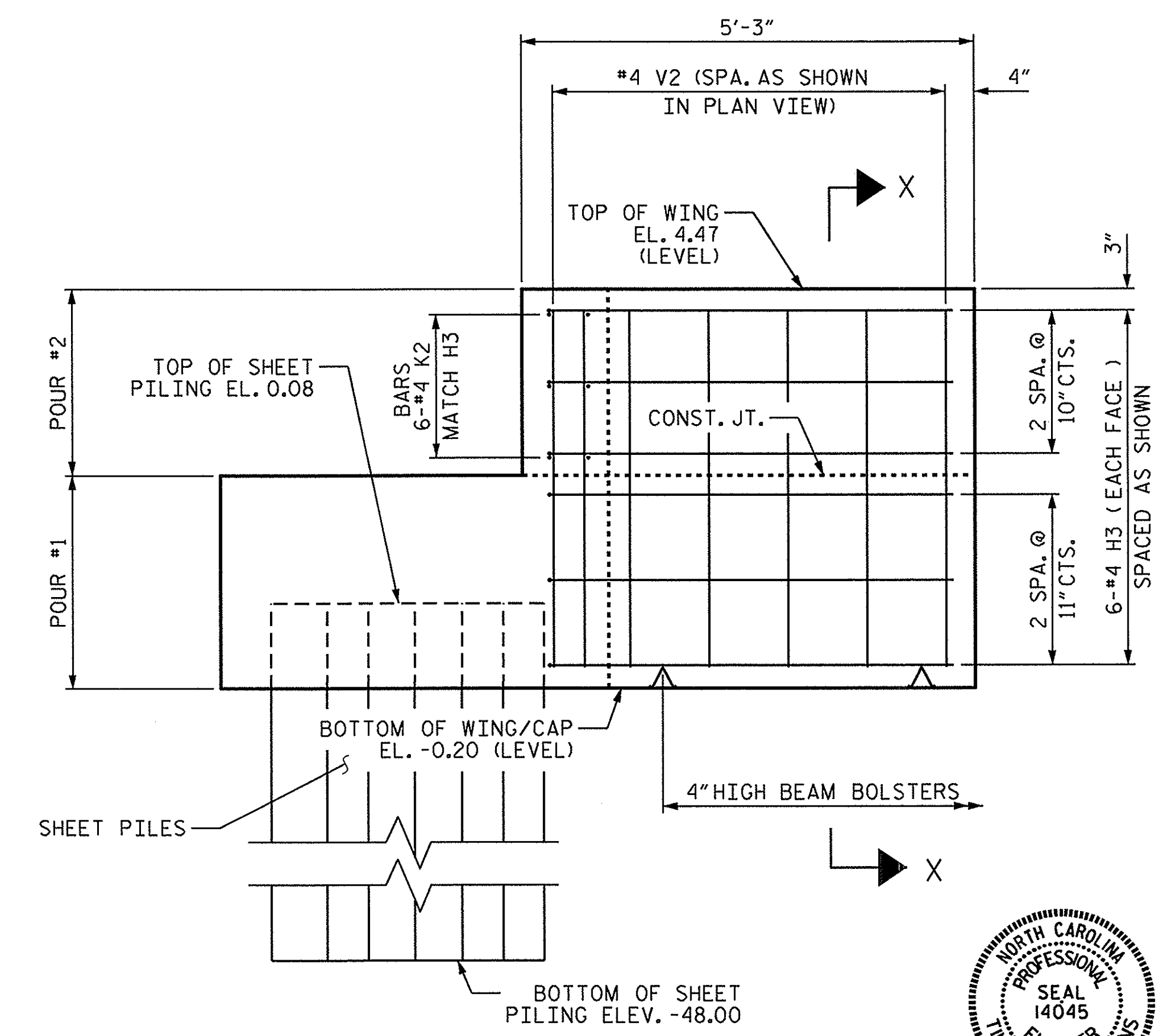
SECTION C-C



SECTION X-X



ELEVATION OF COPING C1



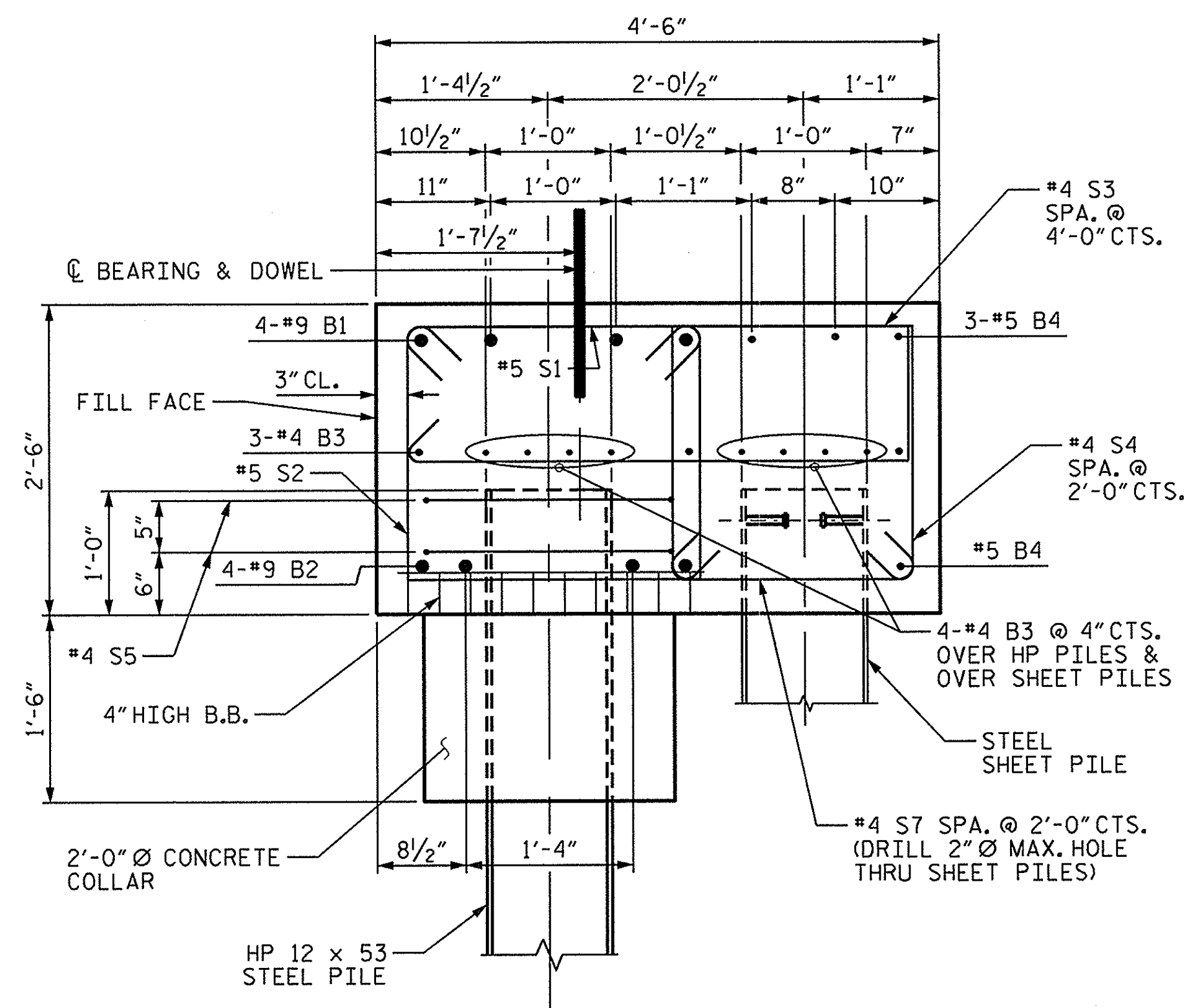
ELEVATION OF WING W2

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-
 SHEET 3 OF 4

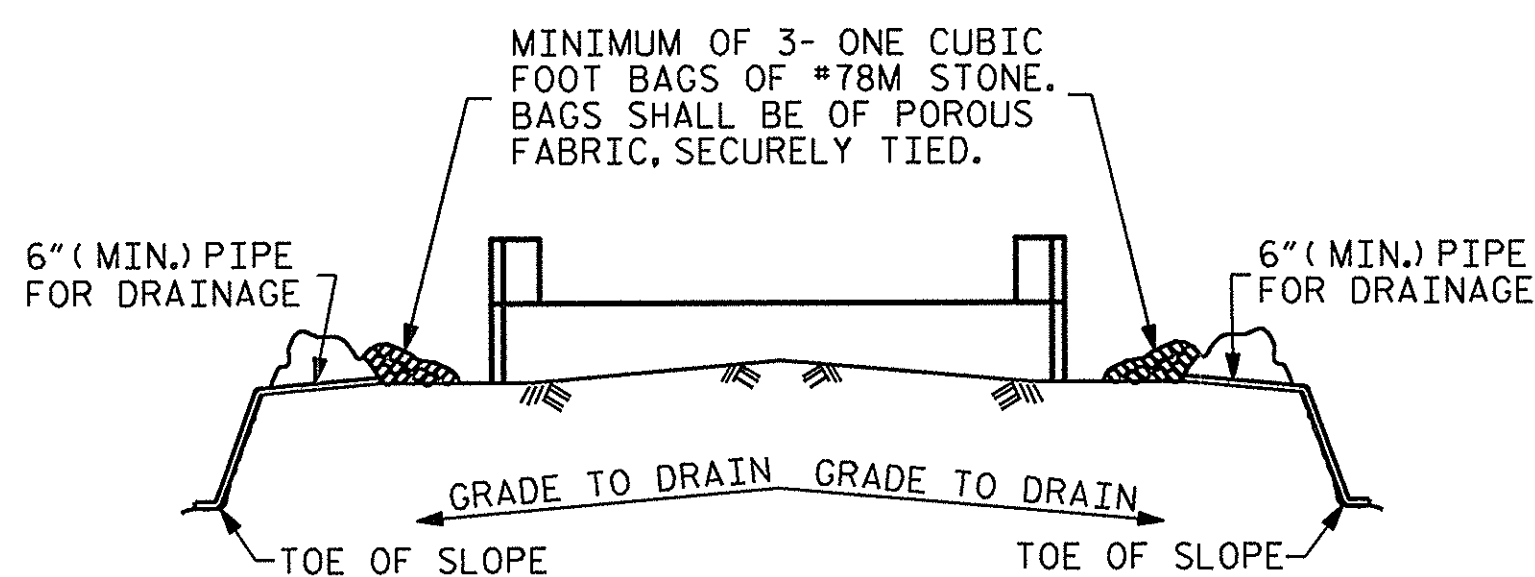
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
SUBSTRUCTURE END BENT #2					
REVISIONS					
NO.	BY:	DATE:	NO.	BY:	DATE:
1			3		
2			4		
					SHEET NO. S-21
					TOTAL SHEETS

DRAWN BY: N. Ruffin DATE: 2/18/14
 CHECKED BY: T. L. COGGINS DATE: 4/2/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14





SECTION A-A THRU CAP

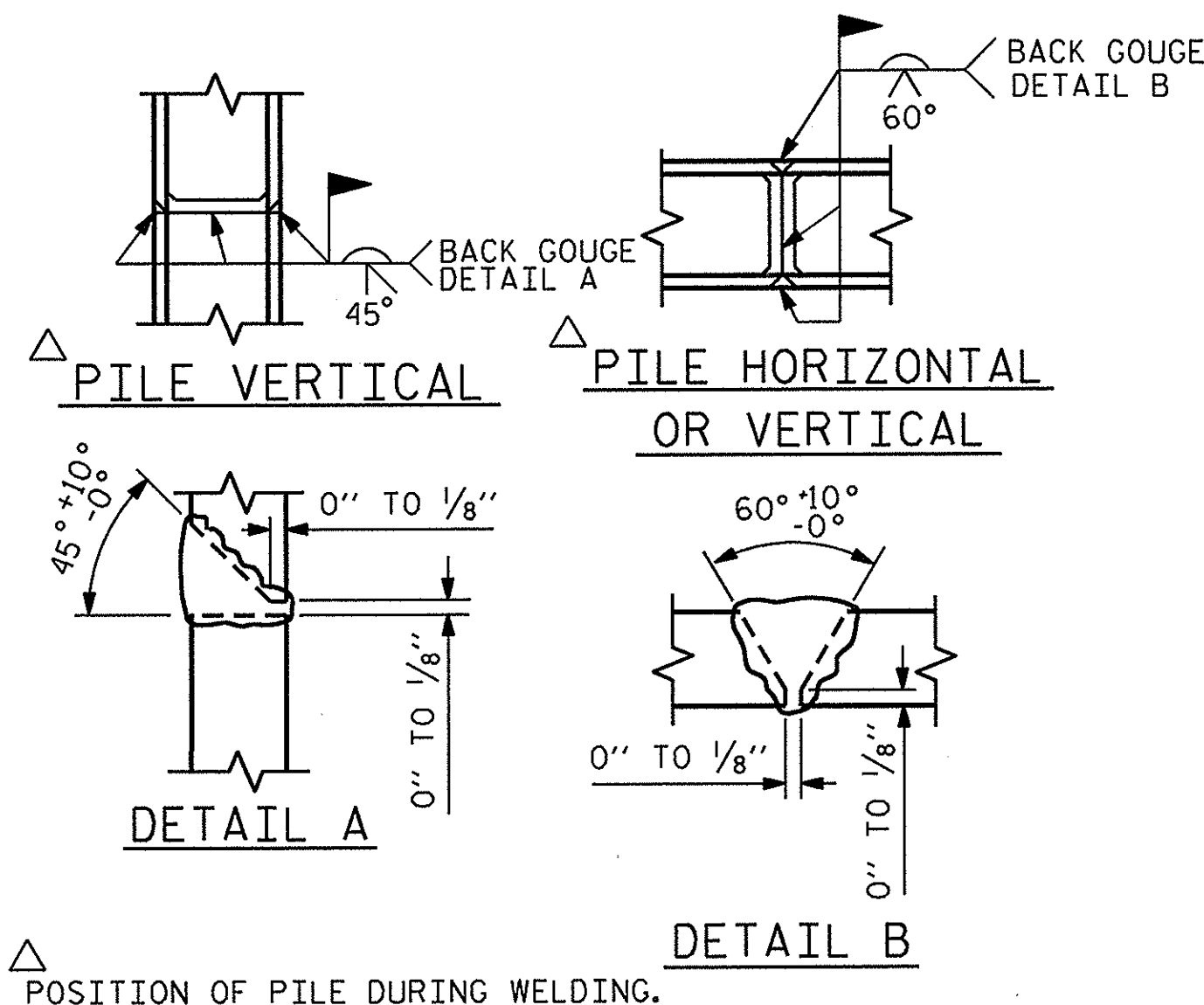


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETERMINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

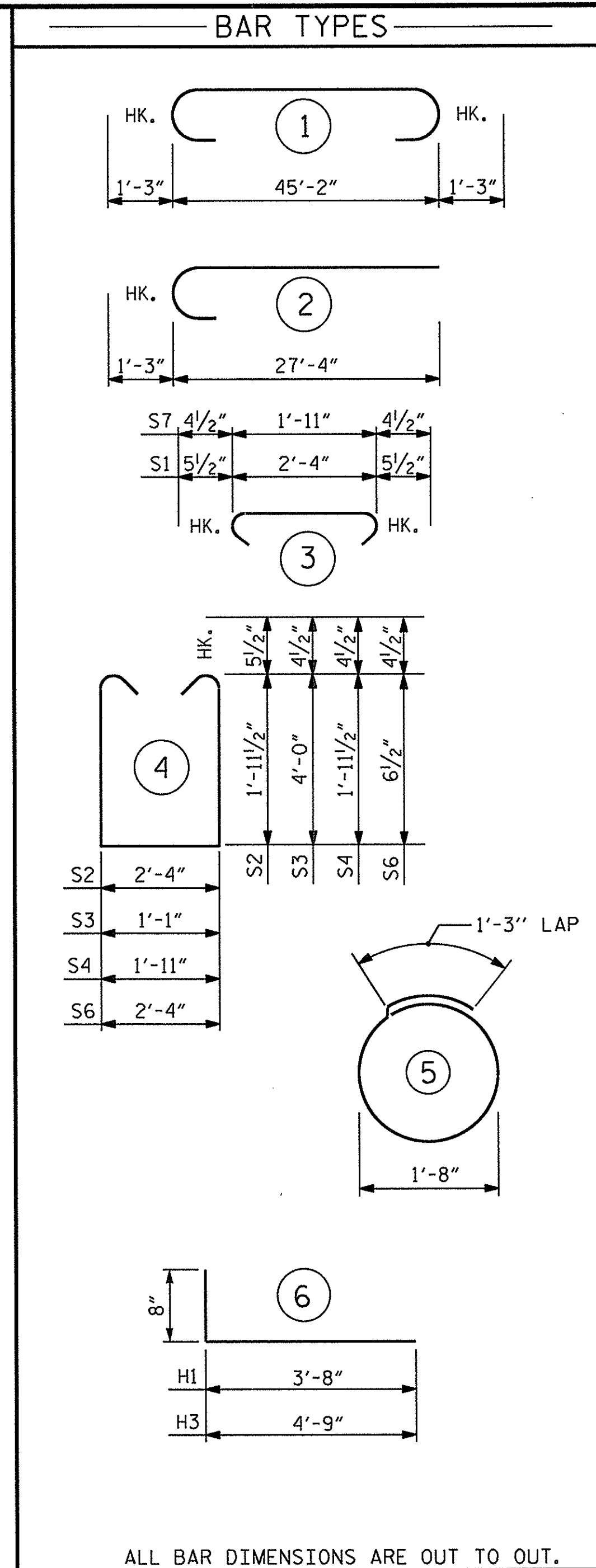
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS



ALL BAR DIMENSIONS ARE OUT TO OUT.

**BILL OF MATERIAL
END BENT #2**

BAR	NO	SIZE	TYPE	LENGTH	WEIGHT
*B1	4	9	1	47'-8"	648
*B2	8	9	2	28'-7"	777
*B3	22	4	STR	24'-1"	354
*B4	4	5	STR	45'-4"	189
*B5	4	4	STR	2'-4"	6
*D1	26	6	STR	1'-6"	59
*H1	7	4	6	4'-4"	20
*H2	7	4	STR	10'-8"	50
*H3	12	4	6	5'-5"	43
*K1	8	4	STR	4'-6"	24
*K2	6	4	STR	2'-11"	12
*S1	40	5	3	3'-3"	136
*S2	40	5	4	7'-2"	299
*S3	11	4	4	9'-10"	72
*S4	21	4	4	6'-7"	92
*S5	14	4	5	6'-6"	61
*S6	11	4	4	4'-2"	31
*S7	21	4	3	2'-8"	37
*V1	16	4	STR	4'-10"	52
*V2	20	4	STR	4'-2"	56

*EXOPY COATED REINFORCING STEEL LBS. 3,018

CLASS "AA" CONCRETE BREAKDOWN
POUR #1 CAP & LOWER PART OF WINGS & COLLARS 20.7 C.Y.
POUR #2 UPPER PART OF WINGS 1 C.Y.
POUR #3 COPING 1.2 C.Y.
TOTAL 22.9 C.Y.

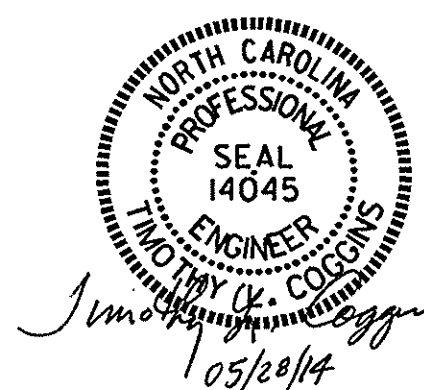
HP 12 x 53 STEEL PILES
No. = 7 LIN. FEET. 665

STEEL SHEET PILES
NO. 90 = 2
NO. PILES = 36
TOTAL NO. = 38 SO. FT. 3050

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

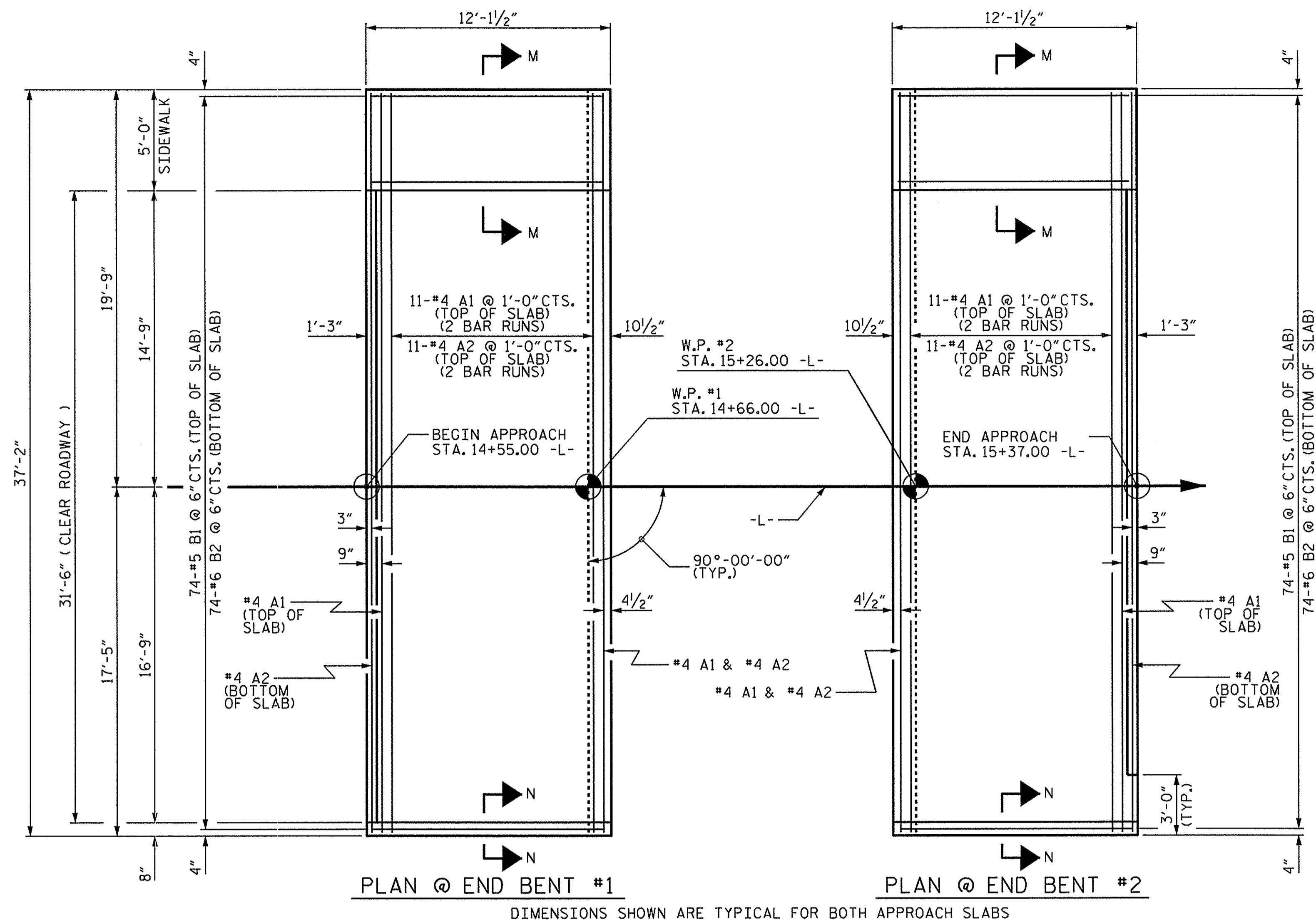
SHEET 4 OF 4

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
SUBSTRUCTURE
END BENT #2

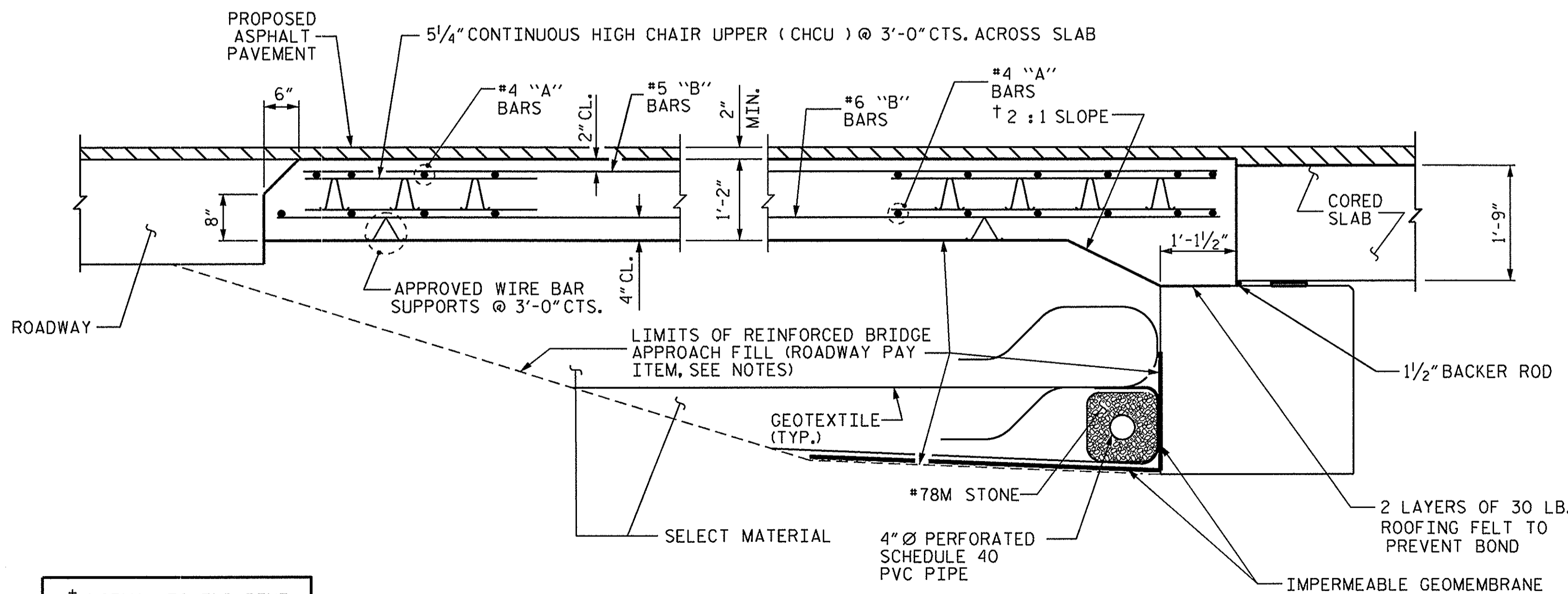


DRAWN BY: N. Ruffin DATE: 2/18/14
CHECKED BY: T.L. COGGINS DATE: 4/2/14
DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-22
1			3			TOTAL SHEETS
2			4			



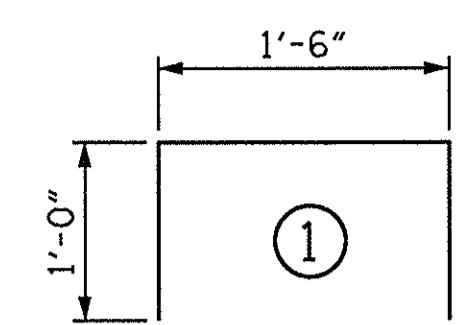
PLAN @ END BENT #1 PLAN @ END BENT #2
DIMENSIONS SHOWN ARE TYPICAL FOR BOTH APPROACH SLABS



SECTION THRU SLAB

† NORMAL TO END BENT

BAR TYPES



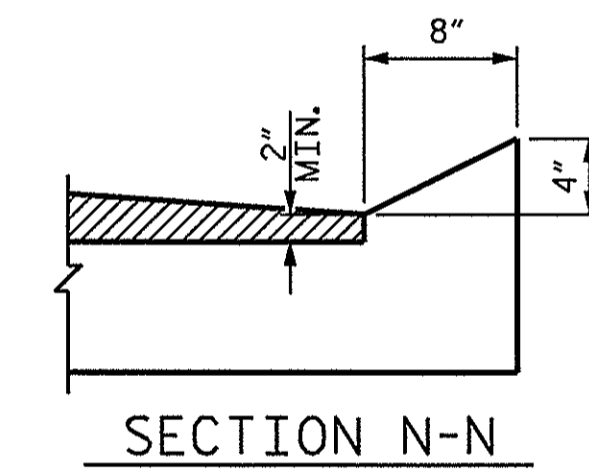
BILL OF MATERIAL

APPROACH SLAB AT EB #1					
BAR NO	SIZE	TYPE	LENGTH	WEIGHT	
*A1	26	4	STR	19'-5"	337
*A2	26	4	STR	19'-5"	337
*B1	74	5	STR	11'-3"	868
*B2	74	6	STR	11'-8"	1297
*B3	5	4	STR	11'-8"	39
*G1	13	4	STR	4'-7"	40
*U1	10	4	1	3'-6"	23
*EPOXY COATED REINFORCING STEEL LBS.					2,941
CLASS AA CONCRETE BREAKDOWN					
APPROACH SLAB					21.8 C.Y.
SIDEWALK					1.6 C.Y.
TOTAL CLASS AA CONCRETE					23.4 C.Y.

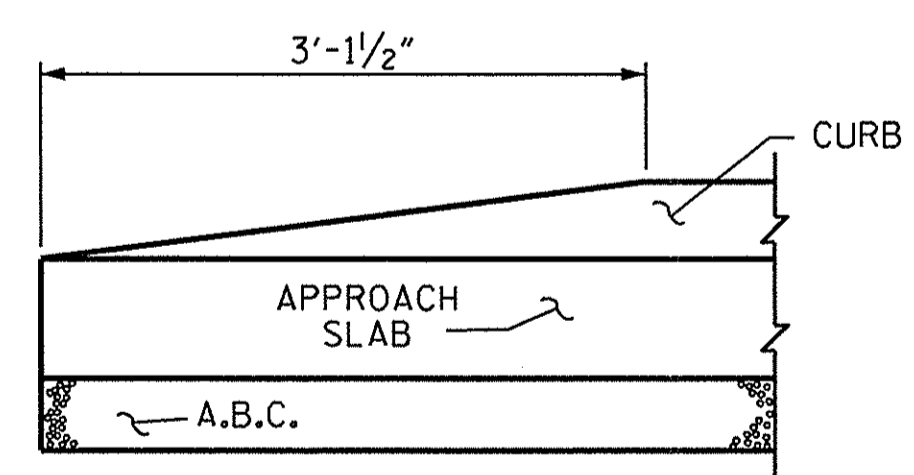
APPROACH SLAB AT EB #2 RT					
BAR NO	SIZE	TYPE	LENGTH	WEIGHT	
*A1	26	4	STR	19'-5"	337
*A2	26	4	STR	19'-5"	337
*B1	74	5	STR	11'-3"	868
*B2	74	6	STR	11'-8"	1297
*B3	5	4	STR	11'-8"	39
*G1	13	4	STR	4'-7"	40
*U1	10	4	1	3'-6"	23
*EPOXY COATED REINFORCING STEEL LBS.					2,941
CLASS AA CONCRETE BREAKDOWN					
APPROACH SLAB					21.8 C.Y.
SIDEWALK					1.6 C.Y.
TOTAL CLASS AA CONCRETE					23.4 C.Y.

NOTES

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE ROADWAY PLANS.
AREA BETWEEN THE WINGWALL OR SHEET PILING WITH COPING AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.
APPROACH SLAB GROOVING IS NOT REQUIRED.



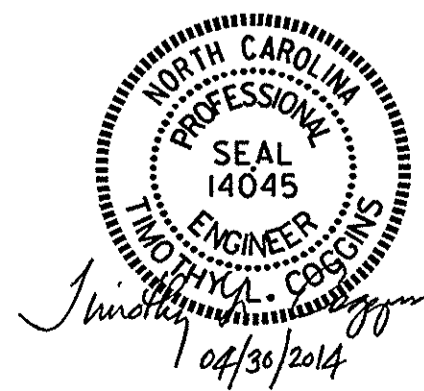
SECTION N-N



END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS

SPLICE LENGTHS		
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3'-10"	2'-7"

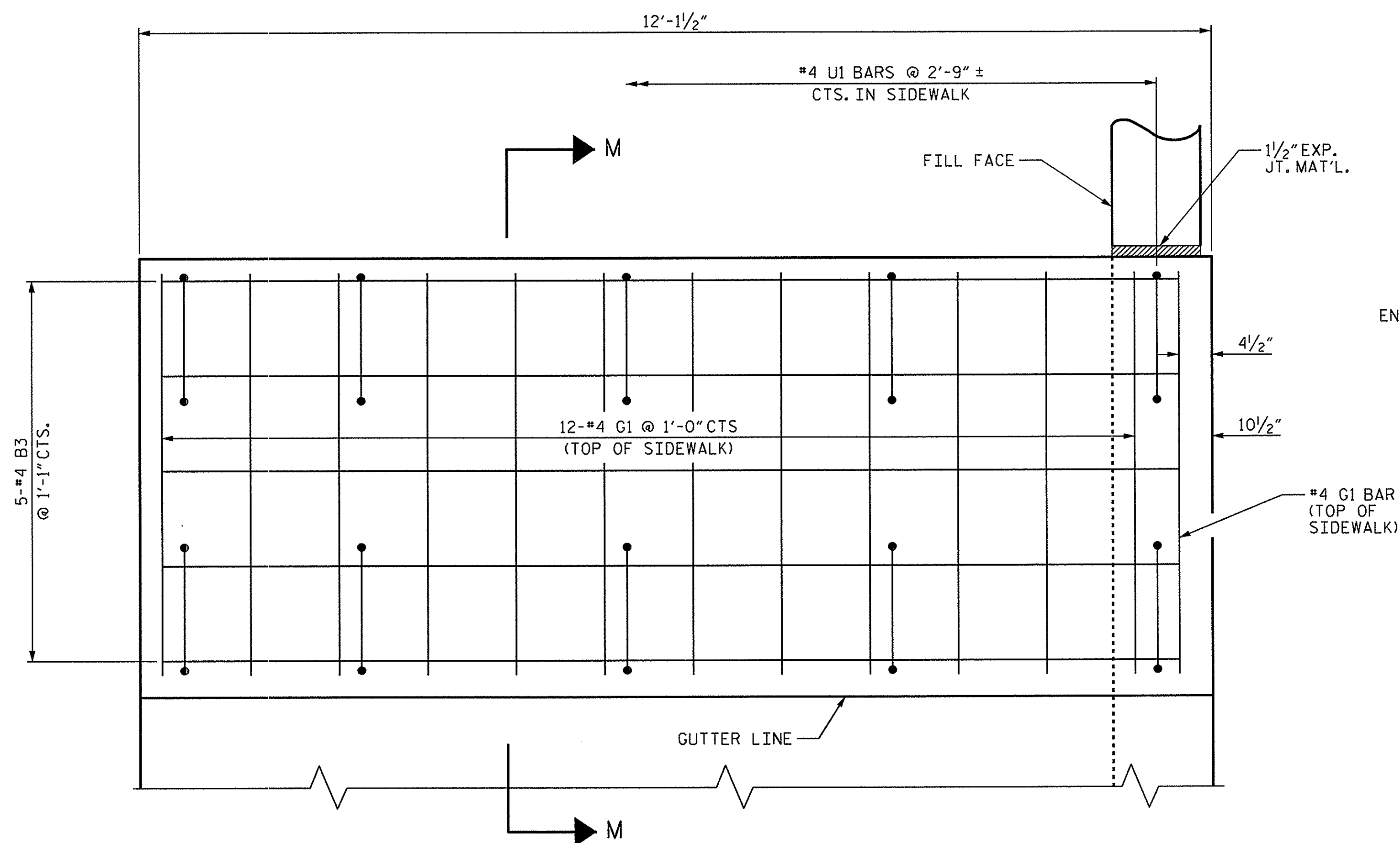


PROJECT NO. BD-5101N
PASQUOTANK COUNTY
STATION: 14+96.00 -L-

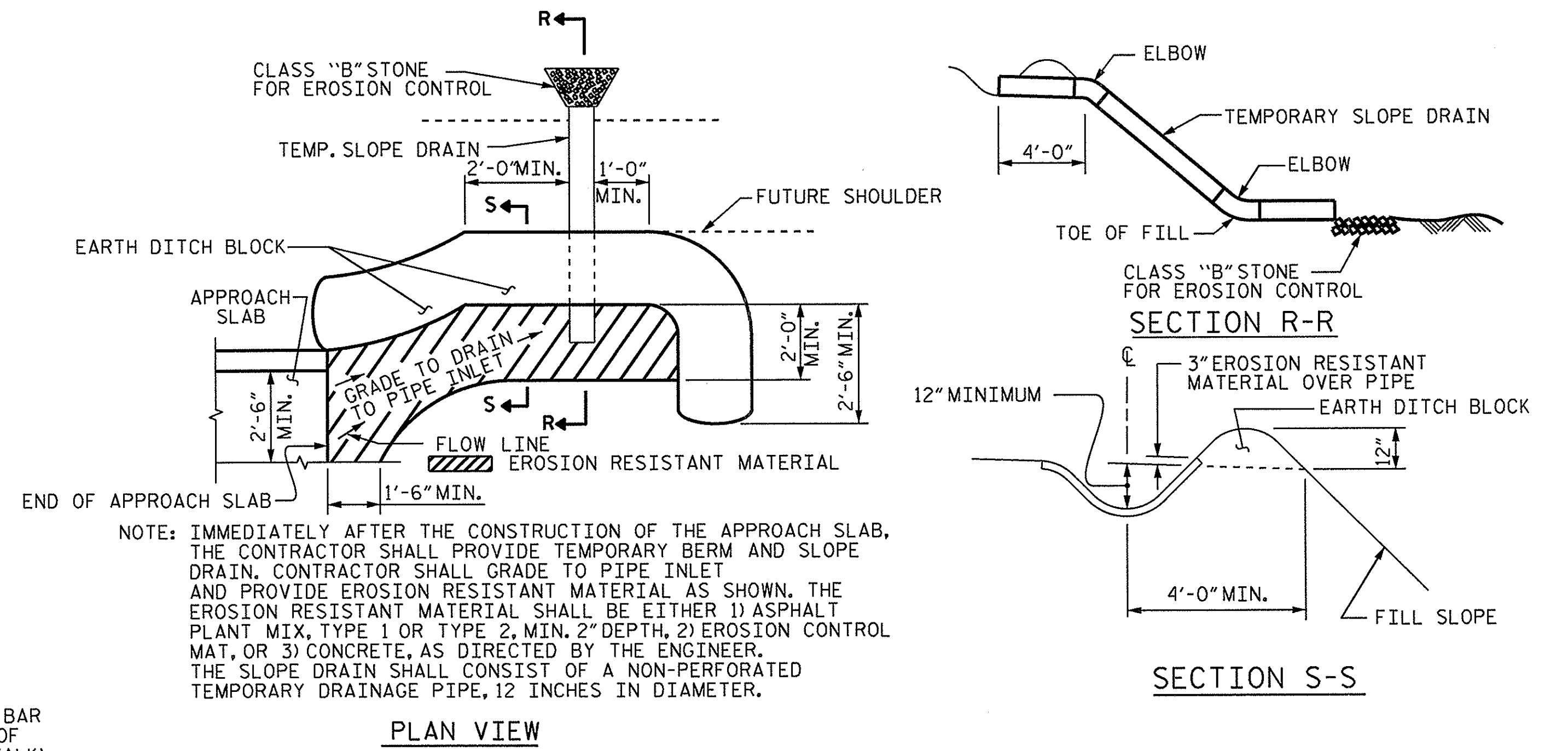
SHEET 1 OF 2
STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
RALEIGH
BRIDGE APPROACH SLAB
FOR PRESTRESSED
CONCRETE CORED SLAB
(SUB-REGIONAL TIER)

REVISIONS						SHEET NO.	
NO.	BY:	DATE:	NO.	BY:	DATE:	S-23	
1			3			TOTAL SHEETS	
2			4				

DRAWN BY: N. Ruffin DATE: 4/15/14
CHECKED BY: T.L. COGGINS DATE: 4/25/14
DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

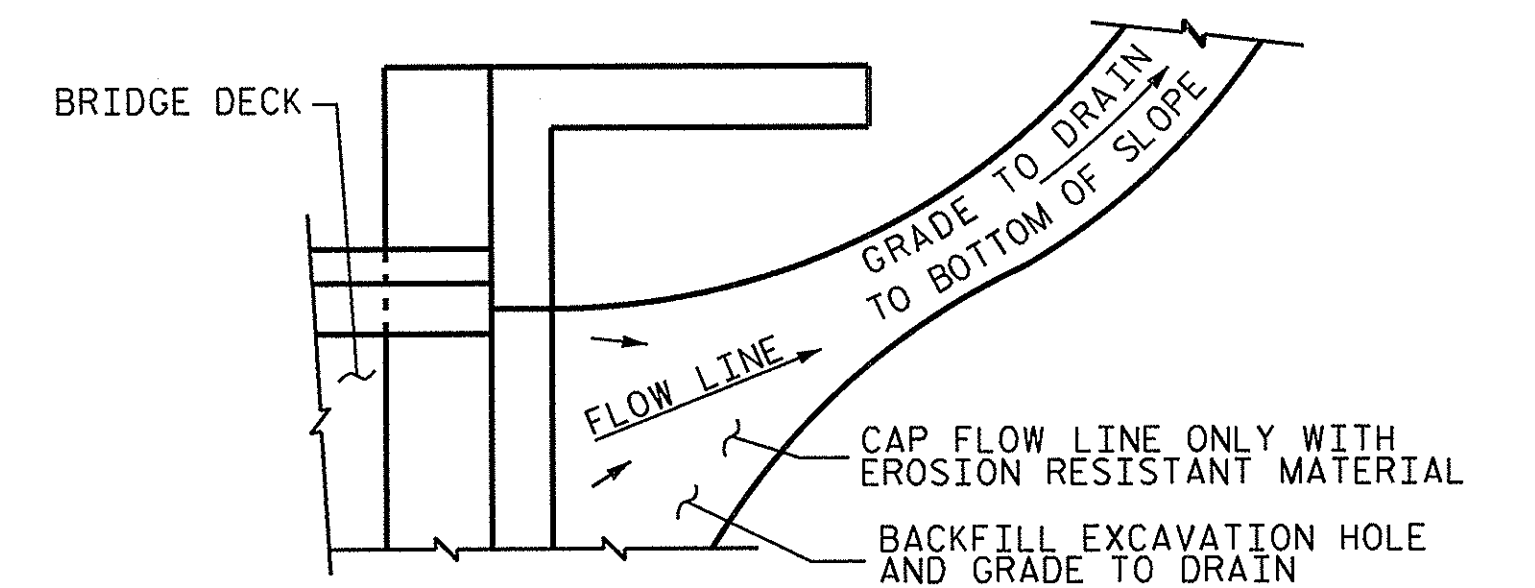


DETAILS OF SIDEWALK ON APPROACH SLAB



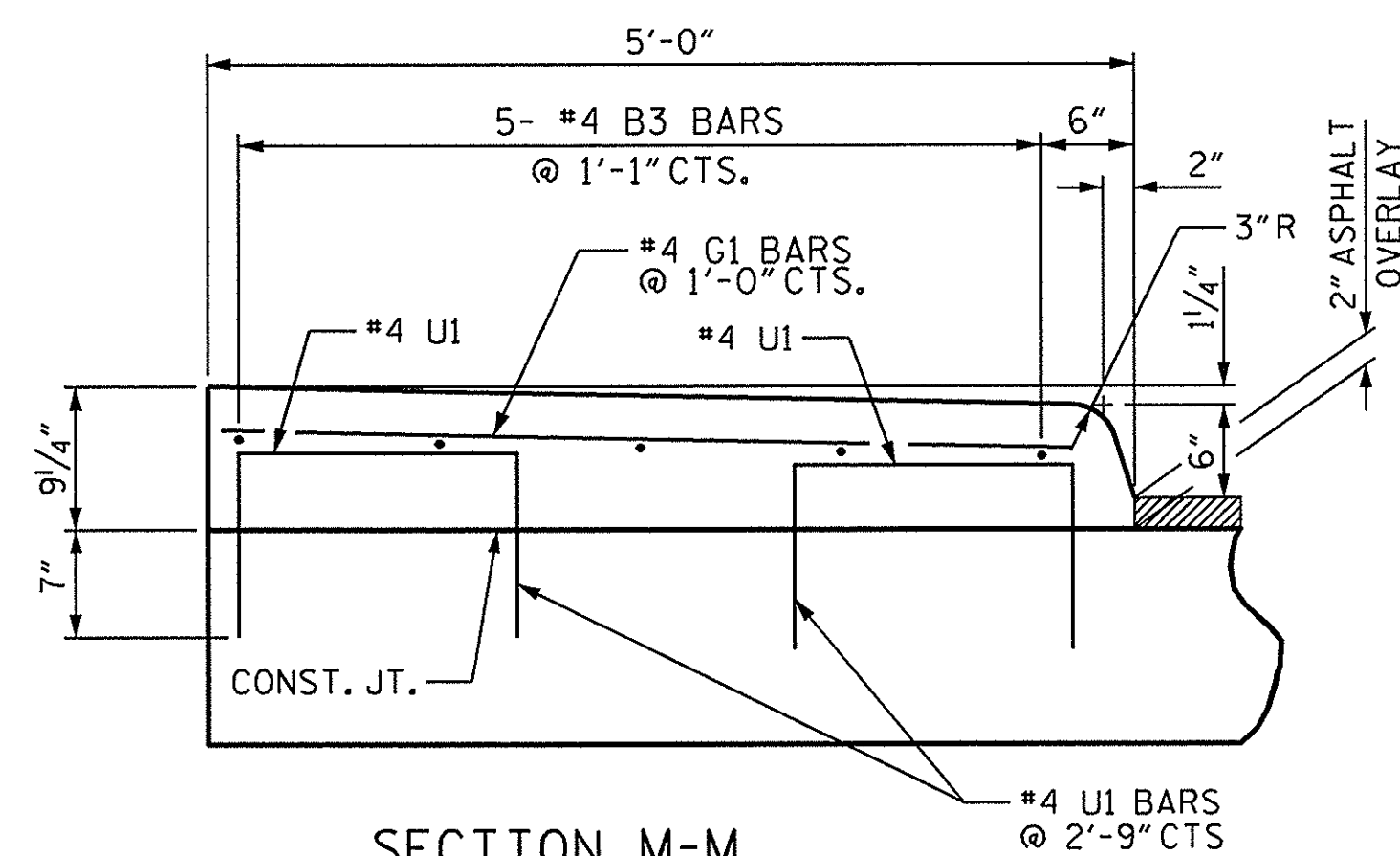
TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



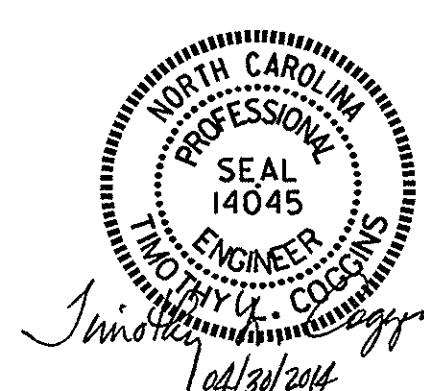
SIDEWALK DETAILS

DRAWN BY: N. Ruffin DATE: 4/15/14
 CHECKED BY: T. L. COGGINS DATE: 4/25/14
 DESIGN ENGINEER OF RECORD: N. RUFFIN DATE: 4/30/14

30-APR-2014 14:41
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 nruffin

PROJECT NO. BD-5101N
PASQUOTANK COUNTY
 STATION: 14+96.00 -L-

SHEET 2 OF 2



STATE OF NORTH CAROLINA
 DEPARTMENT OF TRANSPORTATION
 RALEIGH
 BRIDGE APPROACH SLAB
 FOR PRESTRESSED
 CONCRETE CORED SLAB
 (SUB-REGIONAL TIER)

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	S-24
1			3			TOTAL SHEETS
2			4			