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M. SELLS

CHECKED BY : _____A. TEAGUE ____ DATE : ____05/14

DRAWN BY : _

__ DATE : ____05/14___

					– TO	TAL BILI	L OF MA	TE	RIAL							
	REMOVAL OF EXISTING STRUCTURE AT STA.12+29.00 -L-	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	UNCLASSIFIED STRUCTURE EXCAVATION AT STA.12+29.00 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS STA.12+29.00 -L-	REINFORCING STEEL	HP STE	12 x 53 El PILES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O"THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0" X PRESTRE CONCRE BOX BE	2'-9" ESSED ETE EAMS	ASBESTOS ASSESSMENT
	LUMP SUM	LIN.FT.	LIN.FT.	LUMP SUM	CU.YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO. LI	N.FT.	LUMP SUM
SUPERSTRUCTURE										160.00				10 80	00.00	
END BENT NO.1		27	25		23.8		3342	5	75		126	140				
END BENT NO.2		22	25		23.8		3342	5	75		122	135				
TOTAL	LUMP SUM	49	50	LUMP SUM	47.6	LUMP SUM	6684	10	150	160.00	248	275	LUMP SUM	10 80	00.00	LUMP SUM

HYDRAULIC DATA

DESIGN DISCHARGE	= 1200 CFS
FREQUENCY OF DESIGN FLOOD	= 25 YR
DESIGN HIGH WATER ELEVATION	= 251.8
DRAINAGE AREA	= 3.7 SQ.MI.
BASE DISCHARGE (Q100)	= 1700 CFS
BASE HIGH WATER FLEVATION	= 252.51
DAGE HIGH WATER ELEVATION	202101

OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE = 5150 CFS FREQUENCY OF OVERTOPPING FLOOD = > 500 YR OVERTOPPING FLOOD ELEVATION = 256.9

NOTES:

AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING. PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE. PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT END BENT NO.2. EXCAVATE HOLES AT PILE LOCATIONS TO 240.6 FT (LT) AND 240.8 FT (RT). THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1. CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT END BENT NO.2. ALL PILES ARE TO BE INSTALLED PLUMB WITH THE STRONG AXIS ORIENTED PARALLEL TO THE BRIDGE DECK ALIGNMENT. A MINIMUM OF 5 FEET ROCK SOCKET IS REQUIRED FOR ALL PILES. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS. ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ACTIVITIES, SEE SPECIAL PROVISIONS. ROADWAY PLANS. REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR "EVALUATING SCOUR AT BRIDGES", MAY, 2001. SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS. INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR SEE SECTION 412 OF THE STANDARD SPECIFICATIONS. "REMOVAL OF EXISTING STRUCTURE AT STATION 12+29.00 -L-". FOR UTILITY INFORMATION, SEE ROADWAY PLANS. THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE. THE EXISTING STRUCTURE CONSISTING OF 1 SPAN (1 @ 40'-8") WITH A STEEL PLANK DECK ON I-BEAMS AND A CLEAR ROADWAY OF 24'-1" ON TIMBER CAPS, POSTS, AND CONCRETE SILLS AND TIMBER BULKHEADS, AND LOCATED AT THE PROPOSED SITE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. PILE EXCAVATION IS REQUIRED TO INSTALL PILES AT END BENT NO.1. EXCAVATE HOLES AT PILE LOCATIONS TO 239.3 FT (LT) AND 240.9 FT (RT). FOR PILE EXCAVATION, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. CONCRETE IS REQUIRED TO FILL HOLES FOR PILE EXCAVATION AT END BENT NO.1. **PROJECT NO.** <u>17BP.10.R.64</u> ANSON _ COUNTY

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION FOR A DISTANCE OF 20 FT. EACH SIDE OF CENTERLINE ROADWAY AS CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.

12+29.00 -L-STATION:__

SHEET 2 OF 2



		· · · · · · · · · · · · · · · · · · ·									STRE	ENGTH	ILIN	IT ST	ATE				SE	RVICE	III	LIMI	r sta	TE	
		· · ·									MOMENT	z				SHEAR	Z					MOMENT	Z		
LEVEL			VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	L I VELOAD F ACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATIO	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATIC	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATIC	DISTANCE FROM LEFT END OF SPAN (f†)	
		·	HL-93(Inv)	N/A		1.155		1.75	0.273	1.72	Α	EL	39.25	0.502	1.51	А	EL	7.85	0.80	0.273	1.15	А	EL	39.25	
DESIGN			HL-93(0pr)	N/A		1.958		1.35	0.273	2.23	Α	EL	39.25	0.502	1.96	Α	EL	7.85	N/A			 		70.05	
LOAD RATING		:	HS-20(Inv)	36.000	2	1.533	55.181	1.75	0.273	2.28	A	EL.	39.25	0.502	1.91	A	EL	(.85	0.80	0.273	1.53	A 		39.25	
			HS-20(0pr)	36.000		2.473	89.021	1.35	0.273	2.96	A 		39.25	0.502	5.73	A 	EL E1	7.85	0.80	0 273	3 51		FI	39.25	
				13.500		2.209	41.316 51.00	1.4 1 <i>/</i>	0.213	0.00 4 82	Α 		39.20	0.502	4.06	Δ	сс Fl	7.85	0.80	0.273	2,59	Δ	FI	39,25	
			SNACRTS2	22.000		2.004	57.85	1.4	0.273	4.55	Δ	F1	39.25	0.502	3.76	A	EL	7.85	0.80	0.273	2.45	A	EL	39.25	
		х 	SNCOTTS3	27.250		1.746	47.571	1.4	0.273	3.25	Α	EL	39.25	0.502	2.86	Α	EL	7.85	0.80	0.273	1.75	A	EL	39.25	
	۶۷	· ·	SNAGGRS4	34.925		1.451	50.667	1.4	0.273	2.7	Δ	EL	39.25	0.502	2.36	Α	EL	7.85	0.80	0.273	1.45	Α	EL	39.25	
			SNS5A	35.550		1.419	50.453	1.4	0.273	2.64	Α	EL	39.25	0.502	2.38	A	EL	7.85	0.80	0.273	1.42	А	EL	39.25	
			SNS6A	39.950		1.299	51.885	1.4	0.273	2.42	А	EL	39.25	0.502	2.17	Α	EL	7.85	0.80	0.273	1.30	Α	EL	39.25	·
FCAL			SNS7B	42.000		1.237	51.941	1.4	0.273	2.3	А	EL	39.25	0.502	2.13	А	EL	7.85	0.80	0.273	1.24	Α	EL	39.25	
			TNAGRIT3	33.000		1.583	52.231	1.4	0.273	2.94	А	EL	39.25	0.502	2 . 59	Α	EL	7.85	0.80	0.273	1.58	А	EL	39.25	
RATING			TNT4A	33.075		1.589	52.55	1.4	0.273	2.96	۵	EL	39.25	0.502	2.53	А	EL	7.85	0.80	0.273	1.59	Α	EL	39.25	
		2 -	TNT6A	41.600		1.296	53.907	1.4	0.273	2.41	А	EL	39.25	0.502	2.25	Α	EL	7.85	0.80	0.273	1.30	A	EL	39.25	
	TTST		TNT7A	42.000		1.301	54.625	1.4	0.273	2.42	А	EL	39.25	0.502	2.21	Α.	EL	7.85	0.80	0.273	1.30	Α	EL	39.25	
			TNT7B	42.000		1.341	56.333	1.4	0.273	2.49	Α	EL	39.25	0.502	2.08	A	EL	7.85	0.80	0.273	1.34	Α	EL	39.25	
			TNAGRIT4	43.000		1.279	55.001	1.4	0.273	2.38	A	EL	39.25	0.502	2.02	A	EL	7.85	0.80	0.273	1.28	A	EL	39.25	
			TNAGT5A	45.000		1.207	54.337	1.4	0.273	2.25	A	EL	39.25	0.502	2	A	EL	7.85	0.80	0.273	1.21	A	EL	39.25	
			TNAGT5B	45.000	3	1.194	53.739	1.4	0.273	2.22	A	EL	39.25	0.502	1.92	Α	EL	7.85	0.80	0.273	1.19	Α	EL	39.25	
								·		$\begin{pmatrix} 1 \\ 2 \\ \hline 3 \end{pmatrix}$												·			
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LOAD FACTORS:

DESIGN	LIMIT STATE	γ _{DC}	γ _{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

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MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
3 LEGAL LOAD RATING **
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER
EL - EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER

PROJECT N	o. <u>17BP.1</u>	0.R.64
	ISON	
STATION:	12+29.00	D -L-
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ASSEMBLED BY : N.D'AIUTO

CHECKED BY : M.E.GILES

DRAWN BY : DGE 8/II

CHECKED BY : TMG II/II

DATE : 6-27-14

MAA/TMG

DATE : 7-7-14

REV. 8/14

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

	PROJECT NO. <u>17BP.10.R.64</u> <u>ANSON</u> COUNTY STATION: <u>12+29.00</u> -L-
SEAL 23371 BOCUSIGNED DOCUSIGNED 10157DE15D464AA 1/20/2016	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-0" X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT
	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: 5-4 1 3 TOTAL SHEETS 14
	STD.NO.STD.33PCBB1_30



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CHECKED BY : TMG II/II

	PROJECT NO. <u>17BP.10.R.64</u> <u>ANSON</u> COUNTY STATION: <u>12+29.00</u> -L-
	SHEET 3 OF 5
Bocusigned by: vipul - p-tel 1/20/2016	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD 3'-O"X 2'-9" PRESTRESSED CONCRETE BOX BEAM UNIT
	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: S-6 1 3 TOTAL SHEETS 14
	STD.NO.33PCBB4_90S_80L



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DOUBLE DIAPHRAGM DETAILS

#4 ``S'' BARS NOT SHOWN. #4 ``S'' BARS MAY BE SHIFTED SLIGHTLY TO CLEAR $2^{1}/_{2}$ " Ø HOLE.



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PART SECTION AT RECESS

GR	ROUTED RECESS DETAIL AT
END (OF POST-TENSIONED STRANDS
	OF EXTERIOR BOX BEAM

DEAD LOAD DEFLECTION AN 75' & 80' BOX BEAM UNIT (NC) CAMBER (SLAB ALONE IN PLACE) DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	ND CAMBER $3'-0'' \times 2'-9''$ $0.6'' \oslash L.R.$ STRAND $1^{3}/_{4}''$ $1'/_{2}''$		PROJECT NO. <u>17BP.10.R</u> <u>ANSON</u> CC STATION: <u>12+29.00</u> - SHEET 4 OF 5	<u>2.64</u>)UNTY ∙L –
** INCLUDES FUTURE WEARING SURF	ACE	BEAL 23371 BOCUSIGNED DocuSigned by: vipul - p-tel 1/20/2016	DEPARTMENT OF TRANSPORTA RALEIGH 3'-0"X 2'-9" PRESTRESSED CONC BOX BEAM UNI	TION RETE T
	DC	CUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	REVISIONS NO. BY: DATE: NO. BY: DATE: 1 3 4 4 4	SHEET NO. S-7 total sheets 14
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BOX BEAM UNITS REQUIRED											
NUMBER LENGTH LENGTH											
EXTERIOR B.B.	2	80'-0"	160'-0"								
INTERIOR B.B.	8	80'-0"	640'-0"								
TOTAL	10		800'-0"								

	BILL OF MATERIAL FOR VERTICAL CONCRETE	BARI	RIER	RAIL	
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	80' UNIT				
* B8	72	# 5	STR	26'-3"	1971
* S6	222	# 5	1	7'-2″	1659
★ EPOX	Y COATED REINFORCING STEEL		LBS.		3630
CLASS	AA CONCRETE		CU.YDS.		21.5
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LIN.FT	•	160.00





PHALT THICKNESS & RA	IL HEIGHT
ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
2 ¹ /4″	3′-8 ¹ /4″

	PROJECT NO. <u>17BP.10.R</u> ANSON	<u>.64</u>			
	STATION: 12+29.00 -				
SEAL 23371 NONEER 23371 Docusigned by: vipul - patel 1C157DE15D464AA 1/20/2016	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTAT RALEIGH 3'-0" X 2'-9" PRESTRESSED CONCI BOX BEAM UNIT	RETE			
	REVISIONS	SHEET NO.			
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	NO. BY: DATE: NO. BY: DATE: 1 3 4 3 1	TOTAL SHEETS 14			
STD. NO. 33PCBB8_90S					

€ 1	€ GUARDRAIL ANCHOR ASSEMBLY
	PLAN
	V/4" HOLD-DOWN P
	GUARDRAIL ANCHOR ASSEMBLY DETAILS
ASSEMBLED BY : CHECKED BY : DRAWN BY : MAA CHECKED BY : GM	/.A.PATEL DATE : 6/17/14 A.E.GILES DATE : 7-7-14 5/10 REV. 10/1/11 MAA/GM 5/10 REV. 12/5/11 MAA/GM REV. 6/13 MAA/GM

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		REV	ISIONS		SHEET NO
NO.	BY:	DATE:	NO. BY:	DATE:	S-9
1			3		TOTAL SHEETS
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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

PROJECT NO. 178P.10.R.64 ANSON COUNTY STATION: 12+29.00 -L-SHEET 1 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TH CARO RALEIGH OFESSION SEAL 23371 SUBSTRUCTURE NCINEER " A. PATELIN END BENT 1 vípul a patel 1/20/2016 SHEET NO. REVISIONS S-10 NO. BY: DATE: DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 14

STD. NO. EB_30_90S4_33BB

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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

PROJECT NO. 178P.10.R.64 ANSON COUNTY STATION: 12+29.00 -L-SHEET 2 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION TH CARO RALEIGH FESSION SEAL 23371 SUBSTRUCTURE ACINEER A. PATELIN END BENT 2 vipul a patel 1/20/2016 SHEET NO. REVISIONS S-11 NO. BY: DATE: DATE: BY: DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TOTAL SHEETS 14

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	<u>SECTION Y-Y</u>
	PROJECT NO. 17BP.10.R.64
	ANSON COUNTY
	STATION: 12+29.00 -L-
	SHEET 3 OF 4
TH CAROLANT	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL	SUBSTRUCTURE
23371	END BENT
DocuSigned by:	WING DETAILS
vípul a patel 10157DE15D464AA 1/20/2016	
DOCUMENT NOT CONSTDERED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-12
FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 14
	STD.NO.EB_30_90S4_33BB

<u>2" CL</u>

FILL FACE

—#4 V1

CONST.JT.

<u>SPA. @</u> 8″CTS.

SPA. @ 8"CTS.

5 SPA. @ 8"CTS.

FACE)

(EACH

12-#

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YPES		ΒI	LL O	F MA	ATERIA	L
		FO	R ON	E EN	ND BEN	Т
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
$ 4^{\prime} 2^{\prime \prime} 2^{\prime \prime} - 5^{\prime \prime} 4^{\prime} 2^{\prime \prime} $	B1	8	#9	1	38'-0"	1,034
	B2	28	#4	STR	19'-1"	357
НК.	B3	9	#4	STR	2'-5"	15
(4)						
	D1	20	# 8	STR	2'-3"	120
·──1′-3′′ I AP						
	H1	48	# 5	2	11'-4"	567
\mathbf{x}					.	
	K1	12	#4	SIR	2'-11"	23
	K2	12	#4	SIR	19'-1"	153
$\left(\begin{array}{c} \overline{5} \end{array}\right)$	<u> </u>	10	#1	7	101-5"	320
	51	40	⁺⁺ 4 ±∕	<u> </u>	31-2"	320
	52 53	40	*4 #1	4	<u> </u>	87
	- 3-5	20		5	0 0	01
1'-8"Ø	111	30	#4	6	3'-7"	72
	01		· ·	Ŭ	<u> </u>	
	V1	60	#4	STR	7'-2"	287
0″	V2	60	#4	STR	5'-3"	210
	REINF	ORCIN	NG STE	EL		
	(FOR	ONE E	ND BEN	IT)	L	.BS. 3,342
	CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT)					
	POUR #1 CAP,LOWER PART C.Y. 18.5 OF WINGS & COLLARS					
ARE OUT TO OUT.	POUR	#2 B P	ACKWAL ART OF	L & U WING	IPPER S	C.Y. 5.2
END BENT 2						
HP 12 X 53 STEEL PILES						
5 LIN.FT.= 75	TOTAL	CLAS	SS A C	ONCRE	ΓE	C.Y. 23.8
E EXCAVATION						
SOIL LIN. FT.= 22						
F FXCAVATION						
IN SUIL LIN. FI.= 25						

	PROJECT NO. <u>17BP.10.R.64</u> ANSON COUNTY
_ ⊻	STATION: 12+29.00 -L-
WITH CAROLAND	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
SEAL 23371	SUBSTRUCTURE
DocuSigned by: vípul ~ patel 10157DE15D464AA	DETAILS
DOCUMENT NOT CONSTDERED	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-13
FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 TOTAL SHEETS 2 4 14
	STD.NO.EB_30_90S4_33BB

SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
# 5	2'-6"	2'-2"
#6	3'-10"	2'-7"

STD. NO. BAS_BB_30_90S

¹⁷⁻JUL-2014 14:02 S:\DPG3\DivisionLets\Div10\178P.10.R.64\Plans\178P10R64_SD_AS_01.dgn ndoluto

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ. IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ. IN.
COMPRESSION PERPENDICULAR TO GRAIN	
OF LIMBER	STS LBS. PER SU. IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS. ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS. SETTLEMENT OF FALSEWORK. AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE

INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ "Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER

SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING. GALVANIZING. OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS. BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:

