

OVERTOPPING OCCURS AT STA. 15+67.85 -L-TOTAL STRUCTURE QUANTITIES REMOVAL OF EXISTING STRUCTURE LUMP SUM LUMP SUM ASBESTOS ASSESSMENT CULVERT EXCAVATION @ STA. 15+28.50 -L-LUMP SUM FOUNDATION CONDITIONING MATERIAL TOTAL: 152 TONS CLASS A CONCRETE 228.9 C.Y. BARREL @ 4.577 C.Y./FT. 39.5 C.Y. WINGS, ETC. 268.4 C.Y. TOTAL REINFORCING STEEL 27,991 LBS. BARREL

HYDRAULIC DATA

OVERTOPPING DATA



1,975 LBS.



	LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR REINFORCED CONCRETE BOX CULVERTS															
										STRENGTH	I LIM	IT SI	TATE			
										MOMENT				SHEAR		1
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#)	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (Y _{LL})	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (f+)	RATING FACTOR	BOX NO.	ELEMENT TYPE	DISTANCE FROM LEFT END OF ELEMENT (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N⁄A		1.35		1.75	1.35	1 & 3	FLOOR SLAB	12.83′	2.10	1 & 3	FLOOR SLAB	11.99′	
DESIGN		HL-93 (OPERATING)	N⁄A		1.75		1.35	1.75	1 & 3	FLOOR SLAB	12.83′	2.73	1 & 3	FLOOR SLAB	11.99′	
RATING		HS-20 (INVENTORY)	36.000	2	1.66	59.760	1.75	1.66	1 & 3	FLOOR SLAB	12.83′	2.22	1 & 3	FLOOR SLAB	11.99′	
		HS-20 (OPERATING)	36.000		2.15	77.400	1.35	2.15	1 & 3	FLOOR SLAB	12.83′	2.87	1 & 3	FLOOR SLAB	11.99′	
		SNSH	13.500		3.01	40.635	1.40	3.01	1 & 3	FLOOR SLAB	12.83′	4.03	1 & 3	FLOOR SLAB	11.99′	
		SNGARBS2	20.000		2.82	56.400	1.40	2.82	1 & 3	FLOOR SLAB	12.83′	3.77	1 & 3	FLOOR SLAB	11.99′	
	ICLE	SNAGRIS2	22.000		3.01	66.220	1.40	3.01	1 & 3	FLOOR SLAB	12.83′	4.03	1 & 3	FLOOR SLAB	11.99′	
	VEH.	SNCOTTS3	27.250	3	1.82	49.595	1.40	1.82	1 & 3	FLOOR SLAB	12.83′	2.85	1 & 3	FLOOR SLAB	11.99′	
	(S	SNAGGRS4	34.925		2.14	74.740	1.40	2.14	1 & 3	FLOOR SLAB	12.83′	4.32	1 & 3	FLOOR SLAB	11.99′	
	ING	SNS5A	35.550		2.05	72.878	1.40	2.05	1 & 3	FLOOR SLAB	12.83′	4.58	1 & 3	FLOOR SLAB	11.99′	
		SNS6A	39.950		2.05	81.898	1.40	2.05	1 & 3	FLOOR SLAB	12.83′	4.61	1 & 3	FLOOR SLAB	11.99′	
LEGAL		SNS7B	42.000		2.12	89.040	1.40	2.12	1 & 3	FLOOR SLAB	12.83′	4.98	1 & 3	FLOOR SLAB	11.99′	
RATING	ER	TNAGRIT3	33.000		3.01	99.330	1.40	3.01	1 & 3	FLOOR SLAB	12.83′	4.03	1 & 3	FLOOR SLAB	11.99′	
	RAIL	TNT4A	33.075		2.16	71.442	1.40	2.16	1 & 3	FLOOR SLAB	12.83′	3.39	1 & 3	FLOOR SLAB	11.99′	
	1-I/	TNT6A	41.600		2.17	90.272	1.40	2.17	1 & 3	FLOOR SLAB	12.83′	3.65	1 & 3	FLOOR SLAB	11.99′	
	SEN ST)	ΤΝΤ7Α	42.000		2.16	90.720	1.40	2.16	1 & 3	FLOOR SLAB	12.83′	3.50	1 & 3	FLOOR SLAB	11.99′	
)TOR (TT)	TNT7B	42.000		2.08	87.360	1.40	2.08	1 & 3	FLOOR SLAB	12.83′	3.69	1 & 3	FLOOR SLAB	11.99′	
	TRAC	TNAGRIT4	43.000		2.16	92.880	1.40	2.16	1 & 3	FLOOR SLAB	12.83′	3.39	1 & 3	FLOOR SLAB	11.99′	
	JCK	TNAGT5A	45.000		2.42	108.900	1.40	2.42	1 & 3	FLOOR SLAB	12.83′	3.82	1 & 3	FLOOR SLAB	11.99′	
	TRL	TNAGT5B	45.000		2.16	97.200	1.40	2.16	1 & 3	FLOOR SLAB	12.83′	3.39	1 & 3	FLOOR SLAB	11.99′	



LRFR SUMMARY

(LOOKING DOWNSTREAM)

<u>e</u>	DRAWN BY :	LEM	_ DATE : _	3-20
÷	CHECKED BY :	JWJ	_ DATE : _	1-21
<u>e</u>	DESIGN ENGINEER	OF RECORD : LEM	DATE : _	2-21

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BOX 3



LOAD FACTORS:

LOAD TYPE	MAX FACTOR	MIN FACTOR
DC	1.25	0.90
DW	1.50	0.65
EV	1.30	0.90
EH	1.35	0.90
ES	1.35	0.90
LS	1.75	
WA	1.00	

DESIGN LOAD RATING FACTORS

NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

COMMENTS:

1.	
2.	
3.	

4.

(#) CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93)

- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **
- * * SEE CHART FOR VEHICLE TYPE

	PROJEC	CT NO.	17B	P . 10.PE	.4
		CABAR	RUS	CO	UNTY
	STATI	ON:	15+28	.50 -L·	-
Hoodes Mon	DEPA	stat RTMENT	e of north car OF TRAI RALEIGH	OLINA NSPORTA	TION
0463 14 0463 14 NGINEE A. E. ME	LR REI	FR S NFOR BOX	UMMA CED (CULV	RY F CONCF ERTS	OR RETE
,100 STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991	(NON	N-INTE	RSTATE	E TRAF	FIC)
		REVIS	SIONS	DATE	SHEET NO. C-2
INAL UNLESS ALL IATURES COMPLETED	1 2	DATE:	8 3 4	DATE	TOTAL SHEETS 7

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PES	В	ILL	OF FOR	REIN BAR	IFORCI REL	NG	
	MARK	NO.	SIZE	TYPE	LENGTH	WEIGHT	
	Λ1	200	#⊿		5'-6"	735	
	Δ2	200	#4		5'-6"	735	
	A3	200	#4		5'-6"	735	
	A100	100	#5	STR	38'-10"	4,050	
	A200	100	#4	STR	29'-9"	1,987	
	A201	100	#4	STR	11'-6″	768	
	A300	100	#5	STR	38′-10″	4,050	
	A400	100	#6	STR	30′-1″	4,519	
2'-1	A401	100	#6	STR	11′-6″	1,727	
	B1	200	#4	STR	10′-1″	1.347	
	B2	200	#4	STR	7'-4"	980	-
31/2	B3	200	#4	STR	10'-1"	1,347	
31/2	C1	264	#4	STR	26'-1"	4,600	
	G1	8	#5	STR	38′-10″	324	
	D1	16	#6	STR	2'-9"	66	
	D2	8	#6	STR	1'-9"	21	
	TOTAL	BARR	EL REI		ING STEE	 _ =	
RE OUT TO OUT.					27	,991 LBS.	
							-
CONST. JT.	C01	NST.JT	.S				CONST. JT.
CONST. JT.	00	NST.JT					CONST. JT.

POUR 1. PHASE 1 FLOOR SLAB (INCLUDING WING FOOTINGS) WITH 4" OF VERTICAL WALLS/WINGS. POUR 2. REMAINING PORTIONS OF PHASE 1 WALLS/WINGS TO FULL HEIGHT.

POUR 1. PHASE 2 FLOOR SLAB (INCLUDING WING FOOTINGS) WITH 4" OF VERTICAL WALL/WINGS. POUR 2. REMAINING PORTIONS OF PHASE 2 WALL/WINGS TO FULL HEIGHT. POUR 3. ENTIRE ROOF SLAB AND HEADWALLS.

T CONSTRUCTION	SEQUE	ENCE			
LOOKING DOWNSTREAM	PROJEC	CT NO.	17B	P.10.P	E.4
		CABAR	RUS	C(
	STATI	ON:	15+28	.50 -L	
EIDSEAGABE 46C SEAL 0463 14	DEPA	RTMENT	E OF NORTH CAR OF TRA RALEIGH	NSPORTA	TION
A E. MELTIN 2/23/2021	TRI CONC	PLE Rete	12'-0 BOX	" × 8 CUL	′-0″ VERT
STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991	9	0 - 0	0′-00)" SKE	. W
MENT NOT CONSTDERED	NO. BY:	REVI:	SIONS	DATE:	C-4
FINAL UNLESS ALL GNATURES COMPLETED	1		3 4		TOTAL SHEETS 7



	PROJECT NO. 17BP.10.PE.4
	CABARRUS COUNTY
	STATION: 15+28.50 -L-
EIDSEA6A88E245C SEAL 046314	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
A E. MERTIN 2/23/2021	WINGS FOR CONCRETE BOX CULVERT
100 ears STV ENGINEERS, INC. 900 West Trade St., Suite 715 Charlotte, NC 28202 NC License Number F-0991	H = 8'-0" SLOPE = 2:1 90° SKEW
IENT NOT CONSTDERED	REVISIONS SHEET NO.
INAL UNLESS ALL NATURES COMPLETED	1 3 TOTAL SHEETS 2 4 7

DIMENSIONS	ARE	OUT	ΤO	OUT.	

$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	Ň		2'-0'				1,-0"
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	-3″		1'-	8¾″		•	
8'-8\/2" 7'-8\/2" 6'-5\/2" 5'-1\/2" 3'-10\/2"		N1	N2	N3	N4	N5	
01/2		8'-81/2"	7'-81/2"	6'-51/2"	5'-11/2"	3'-101/2"	<u>,</u>

BILL OF MATERIAL							
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT		
H1	18	#4	STR	10'-10″	130		
H2	6	#4	STR	7′-8″	31		
H3	6	#4	STR	4'-1"	16		
H4	36	#4	1	3′-3″	78		
H5	6	#4	STR	11′-9″	47		
N1	6	#5	2	10'-2"	64		
N2	9	#5	2	9'-2"	86		
N3	9	#4	2	7'-11″	48		
N4	9	#4	2	6′-7″	40		
N5	9	#4	2	5′-4″	32		
S1	9	#6	STR	6'-0″	81		
T1	9	#5	STR	12′-9″	120		
V1	6	#4	STR	8'-1"	32		
V2	9	#4	STR	7'-1″	43		
٧3	9	#4	STR	5′-10″	35		
V4	9	#4	STR	4'-7"	28		
V5	9	#4	STR	3'-4"	20		
Z1	6	#5	3	6'-0"	38		
Z2	9	#5	3	5′-5″	51		
Z3	9	#4	3	4'-7"	28		
Z4	9	#4	3	3'-10"	23		
Z5	9	#4	3	3'-1"	19		
REINFORCING STEEL FOR 3 WINGS 1,090 LBS							

FOR LOCATION OF W1 WINGS, SEE

FOR CLASS A CONCRETE, SEE

NOTES:

SHEET C-3

SHEET C-6.





TYPICAL WING SECTION W2

7

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS SHALL CONSIST OF THE FOLLOWING

A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 21/2".

B. 4 - 1" \emptyset X 2¹/₄" BOLTS WITH WASHERS, BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1" Ø X 2 1/4" GALVANIZED BOLTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

C. WIRE STRUTS SHOWN IN THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS DETAIL ARE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 P.S.I. AS AN OPTION, A $\frac{\gamma_{16}}{\omega}$ wire strut with a minimum tensile STRENGTH OF 90,000 PSI IS ACCEPTABLE.

GUARDRAIL ANCHOR ASSEMBLY WITH BOLTS SHALL BE ASSEMBLED IN THE SHOP. BOLT THREADS MAY BE RECUT AS NECESSARY TO INSURE FIT.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS COMPLETE IN PLACE. SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CLASS "A" CONCRETE. FERRULES TO BE PLUGGED DURING POURING OF SLAB AS RECOMMENDED BY THE

AT THE CONTRACTOR'S OPTION, FERRULES WITH OPEN OR CLOSED ENDS MAY BE USED. PAYMENT FOR GUARDRAIL, POSTS, AND POST BASE PLATES IS INCLUDED IN ROADWAY PAY

SLAB REINFORCING STEEL MAY BE SHIFTED AS NECESSARY TO CLEAR GUARDRAIL ANCHOR ASSEMBLY. CARE SHOULD BE TAKEN TO KEEP THE SHIFTING OF REINFORCING STEEL TO

THE CONTRACTOR MAY USE ADHESIVELY ANCHORED ANCHOR BOLTS IN PLACE OF GUARDRAIL ANCHOR ASSEMBLY. LEVEL TWO FIELD TESTING IS REQUIRED, AND THE YIELD LOAD OF THE 1"Ø BOLT IS 21.8 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE

PROJECT NO	. <u>178P.10.PE.4</u>
CABA	RRUS COUNTY
STATION:	15+28.50 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

ANCHORAGE DETAILS FOR GUARDRAIL ANCHOR ASSEMBLY FOR CULVERTS

REVISIONS						SHEET NO.
NO.	BY:	DATE:	N0.	BY:	DATE:	C-7
1			හු			TOTAL SHEETS
2			4			7

	_
MENT NOT CONSIDERED	
FINAL UNLESS ALL	
GNATURES COMPLETED	

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{1}{8}$ " Ø SHEAR STUDS FOR THE 3/4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 – $\frac{7}{8}$ "ø studs for 4 – $\frac{3}{4}$ "ø studs, and stud spacing changes SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" & STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{1}{16}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAÍNTING. GALVANIZING. OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

