F-89005 REFERENCE

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**DESCRIPTION** 

BORING LOGS & CORE LOGS

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LEGEND (SOIL AND ROCK) SUPPLEMENTAL LEGEND (GSI)

TITLE SHEET

SITE PLAN PROFILE CROSS SECTIONS

SHEET NO.

2A

# P10.R003 B

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

## **STRUCTURE** SUBSURFACE INVESTIGATION

COUNTY \_**UNION** 

PROJECT DESCRIPTION BRIDGE #52 OVER

MANESS BRANCH ON SR 1725 (DEEP SPRINGS RD)

STATE PROJECT REFERENCE NO. SF-890052 14

#### **CAUTION NOTICE**

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1999 707-6805. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS MEDICATED DESCRIPTIONS AND ASSECTIONS OF THE INVESTIGATION. THE STANDARD TEST METHOD. INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR CUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS FOR ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

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  OR CONTRACT FOR THE PROJECT.
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  FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE
  CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

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DATE . **JUNE 2022** 





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PROJECT REFERENCE NO. SHEET NO.

SF-890052

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# NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

# SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

COLL DECEDIATION	CDADATION	DOCK DECEDIATION	TERMS AND DEFINITIONS
SOIL DESCRIPTION  SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN	GRADATION  WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.	ROCK DESCRIPTION  HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED	TERMS AND DEFINITIONS  ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.
BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION	UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.  GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.	ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL.  SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60	AQUIFER - A WATER BEARING FORMATION OR STRATA.
IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH	ANGULARITY OF GRAINS	BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.	ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.
AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,	THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS:	ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:	ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING
VERY STIFF,GRAY, SILTY CLAY, MOIST WITH INTERBEDOED FINE SAID LAYERS, HIGHLY PLASTIC, A-7-6 SOIL LEGEND AND AASHTO CLASSIFICATION	ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.	WEATHERED VIOLEN NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > ROCK (WR) 100 BLOWS PER FOOT IF TESTED.	A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.  ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT
GENERAL GRANULAR MATERIALS SILT-CLAY MATERIALS  ORGANIC MATERIALS  ORGANIC MATERIALS  ORGANIC MATERIALS	MINERALOGICAL COMPOSITION	CRYSTALLINE FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT	WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND
CLASS. (≤ 35% PASSING "200) (> 35% PASSING "200)	MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC.  ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.	ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	SURFACE.  CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.
CROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 CLASS. A-1-0 A-1-b A-2-4 A-2-5 A-2-6 A-2-7 A-2-6 A-2-7 A-3-4 A-3 A-6, A-7	COMPRESSIBILITY	NON-CRYSTALLINE FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN	COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM
SYMBOL 000000000000000000000000000000000000	SLIGHTLY COMPRESSIBLE LL < 31	ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	OF SLOPE.
Z PASSING	MODERATELY COMPRESSIBLE LL = 31 - 50 HIGHLY COMPRESSIBLE LL > 50	SEDIMENTARY ROCK SPT REFUSAL, ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
■10 50 MX GRANULAR SILT-	PERCENTAGE OF MATERIAL	(CP) SHELL BEDS, ETC. WEATHERING	DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT
*40 38 MX 58 MX 51 MN	GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL	FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER	ROCKS OR CUTS MASSIVE ROCK.
MATERIAL DAGGER A CO	TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10%  LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20%	HAMMER IF CRYSTALLINE.	DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.
PASSING *40 40 MX 41 MN LITTLE OR	MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35%	VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF	DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE
PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 MN MODERATE OPCOME	HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE  GROUND WATER	OF A CRYSTALLINE NATURE.	LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.  FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE
GROUP INDEX 0 0 0 4 MX 8 MX 12 MX 16 MX NO MX AMOUNTS OF ORGANIC USUAL TYPES STONE FRAGS. FINE CHAPTER COLLEGE OF CANEER		SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO  (SLI.) I INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.
OF MAJOR GRAVEL, AND FINE SILIT OR CLATET SILIT CLATET MITTER	▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING  STATIC WATER LEVEL AFTER 24 HOURS  ■ CONTROL OF THE PROPERTY OF TH	CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.
MATERIALS SANU	STATIC WATER LEVEL AFTER 24 HOURS  \[ \sum_{PW}\]  PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA	MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN (MOD.) GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS	FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.
GEN.RATING AS SUBGRADE EXCELLENT TO GOOD FAIR TO POOR POOR UNSUITABLE		DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.
PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30	SPRING OR SEEP	MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL	FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE
CONSISTENCY OR DENSENESS	MISCELLANEOUS SYMBOLS	SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION, ROCK SHOWS SEVERE LOSS OF STRENGTH (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK,	FIELD.   <u>JOINT</u> - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD RANGE OF UNCONFINED PENETRATION RESISTENCE COMPRESSIVE STRENGTH	ROADWAY EMBANKMENT (RE) 25/025 DIP & DIP DIRECTION	IF TESTED, WOULD YIELD SPT REFUSAL	LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO
(N-VALUE) (TUNS/FT-)	WITH SOIL DESCRIPTION → OF ROCK STRUCTURES	SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED	ITS LATERAL EXTENT.
GENERALLY VERY LOOSE 4 TO 10	SOIL SYMBOL  Opt omt test boring  SLOPE INDICATOR INSTALLATION	TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN.	LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.
GRANULAR MEDIUM DENSE 10 TO 30 N/A MATERIAL DENSE 30 TO 50	ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER	IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF  VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE	MOTILED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.
(NON-COHESIVE) VERY DENSE > 50	THAN ROADWAY EMBANKMENT TEST	SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK	PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE
VERY SOFT         < 2         < 0.25           GENERALLY         SOFT         2 TO 4         0.25 TO 0.5	— INFERRED SOIL BOUNDARY — CORE BORING SOUNDING ROD	(V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, WOULD YIELD SPT N VALUES &lt; 100 BPF</i>	OF AN INTERVENING IMPERVIOUS STRATUM.  RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.
SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0	MW MONITORING WELL TEST BORING WITH CORE	COMPLETE ROCK REDUCED TO SOIL, ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF
MATERIAL         STIFF         8 TO 15         1 TO 2           (COHESIVE)         VERY STIFF         15 TO 30         2 TO 4	TTTTT ALLUVIAL SOIL BOUNDARY A PIEZOMETER INSTALLATION - SPT N-VALUE	SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.
HARD > 30 > 4  TEXTURE OR GRAIN SIZE	RECOMMENDATION SYMBOLS	ROCK HARDNESS	SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT
	THE INCLOSE FOR EVEN A TION OF THE PROPERTY OF	VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES	ROCK.
U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053	□ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □ □	SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.  HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO
BOULDER COBBLE GRAVEL COARSE FINE SILT CLAY	SHALLOW UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - UNCLASSIFIED EXCAVATION - EMBANKMENT OR BACKFILL	TO DETACH HAND SPECIMEN.	THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.
(BLDR.) (COB.) (GR.) (CSE. SD.) (F SD.) (SL.) (CL.)	ABBREVIATIONS	MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.
GRAIN MM 305 75 2.0 0.25 0.05 0.005	AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST	BY MODERATE BLOWS.	STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF
SIZE IN. 12 3	BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED CL CLAY MOD MODERATELY 7 - UNIT WEIGHT	MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.  HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES I INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL
SOIL MOISTURE - CORRELATION OF TERMS  SOIL MOISTURE SCALE FIELD MOISTURE COMES FOR FIELD MOISTURE OF SCALED MOISTURE	CPT - CONE PENETRATION TEST NP - NON PLASTIC $\gamma_{ m d}$ - DRY UNIT WEIGHT CSE COARSE ORG ORGANIC	POINT OF A GEOLOGIST'S PICK.	TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.
(ATTERBERG LIMITS)  (ATTERBERG LIMITS)  DESCRIPTION  GUIDE FOR FIELD MOISTURE DESCRIPTION	DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS	SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN	STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.
- SATURATED - USUALLY LIQUID; VERY WET, USUALLY	DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON	PIECES CAN BE BROKEN BY FINGER PRESSURE.	STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY
(SAT,) FROM BELOW THE GROUND WATER TABLE  LL _ LIQUID LIMIT	F - FINE SL SILT, SILTY ST - SHELBY TUBE	VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.
PLASTIC   SEMISOLID: REQUIRES DRYING TO	FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL	FINGERNAIL.	TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.
RANGE < - WET - (W) ATTAIN OPTIMUM MOISTURE  (PI) PL PLASTIC LIMIT	FRAGS FRAGMENTS	FRACTURE SPACING BEDDING  TERM SPACING TERM THICKNESS	BENCH MARK; BM#2; RR SPIKE IN 42" OAK AT N469031.599, E1610120.487
	EQUIPMENT USED ON SUBJECT PROJECT	VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET	ELEVATION: 454.06 FEET
OM OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL SHRINKAGE LIMIT	DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE:	WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET MODERATELY CLOSE 1 TO 3 FEET THINLY BEDDED 0.16 - 1.5 FEET	
REQUIRES ADDITIONAL WATER TO	CME-45C CLAY BITS X AUTOMATIC MANUAL	CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET	NOTES:  EXISTING GROUND SURFACE SHOWN WAS TAKEN FROM
- DRY - (D) ATTAIN OPTIMUM MOISTURE	CME-55 6° CONTINUOUS FLIGHT AUGER CORE SIZE:	THINLY LAMINATED < 0.008 FEET	ELECTRONIC TIN FILE (DATED JULY 2021).
PLASTICITY	8" HOLLOW AUGERS	INDURATION	-
PLASTICITY INDEX (PI) DRY STRENGTH	X CME-550 HARD FACED FINGER BITS X-N Q	FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	FIAD: FILLED IMMEDIATELY AFTER DRILLING
NON PLASTIC 0-5 VERY LOW SLIGHTLY PLASTIC 6-15 SLIGHT	VANE SHEAR TEST TUNGCARBIDE INSERTS	FRIABLE RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY PLASTIC 16-25 MEDIUM HIGHLY PLASTIC 26 OR MORE HIGH	CASING W/ ADVANCER POST HOLE DIGGER	MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE;	
COLOR	PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER	BREAKS EASILY WHEN HIT WITH HAMMER.	
	X TRICONE 'TUNGCARB. SOUNDING ROD	INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY).  MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.	CORE BIT VANE SHEAR TEST	EXTREMELY INDURATED SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE;	
	PDC BIT <u>2-15/16</u> 0.D.	SAMPLE BREAKS ACROSS GRAINS.	DATE: 8-15-14

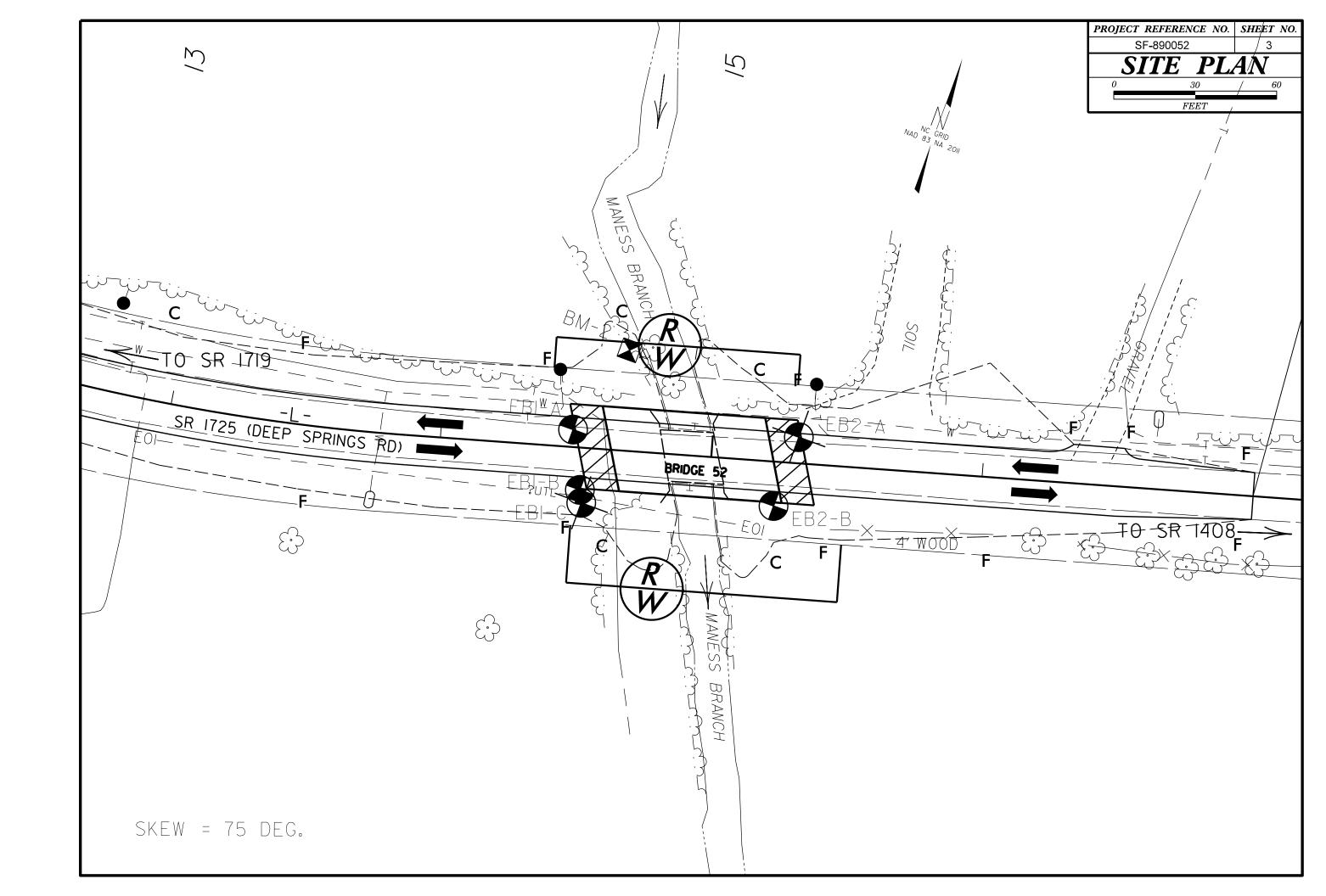
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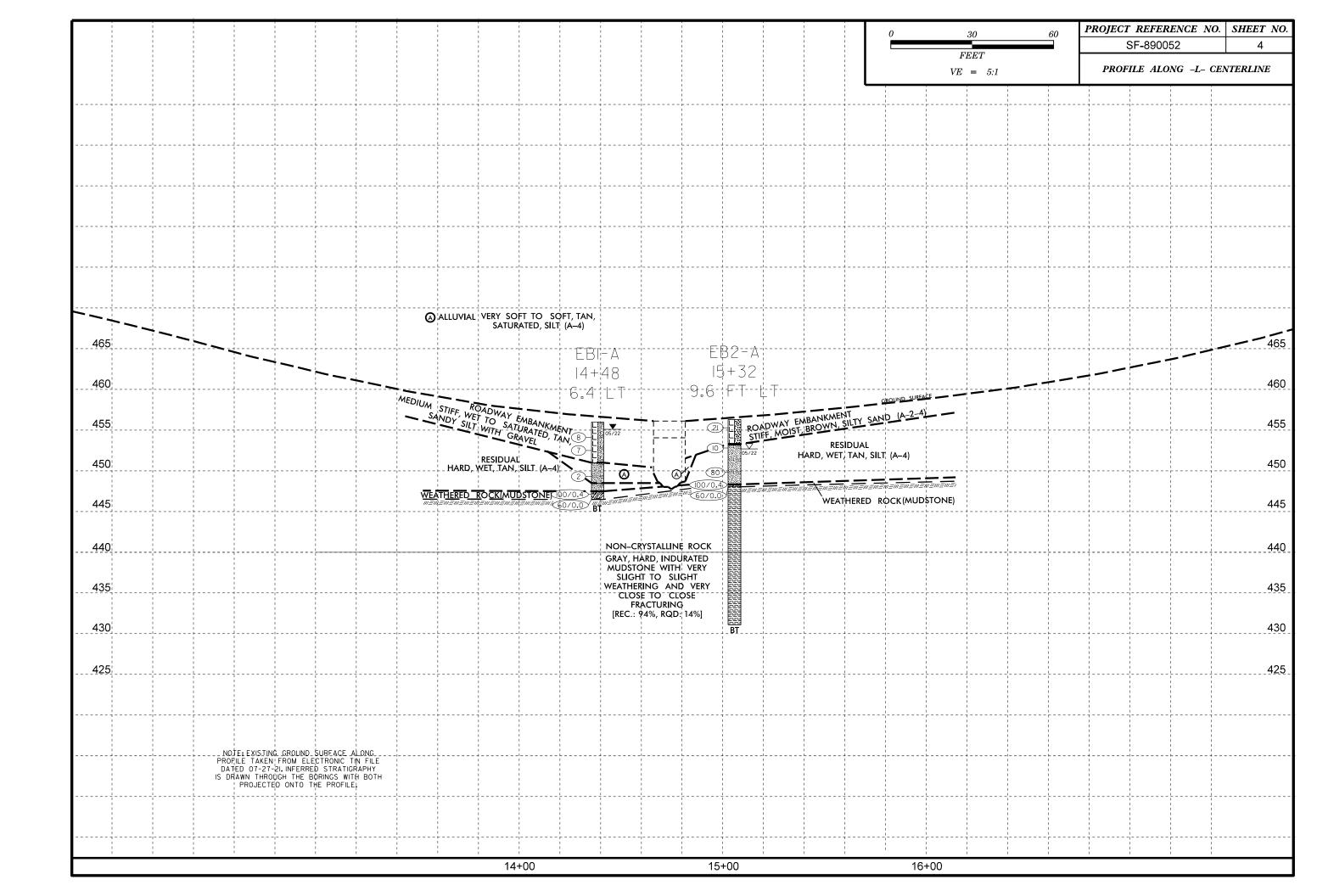
#### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

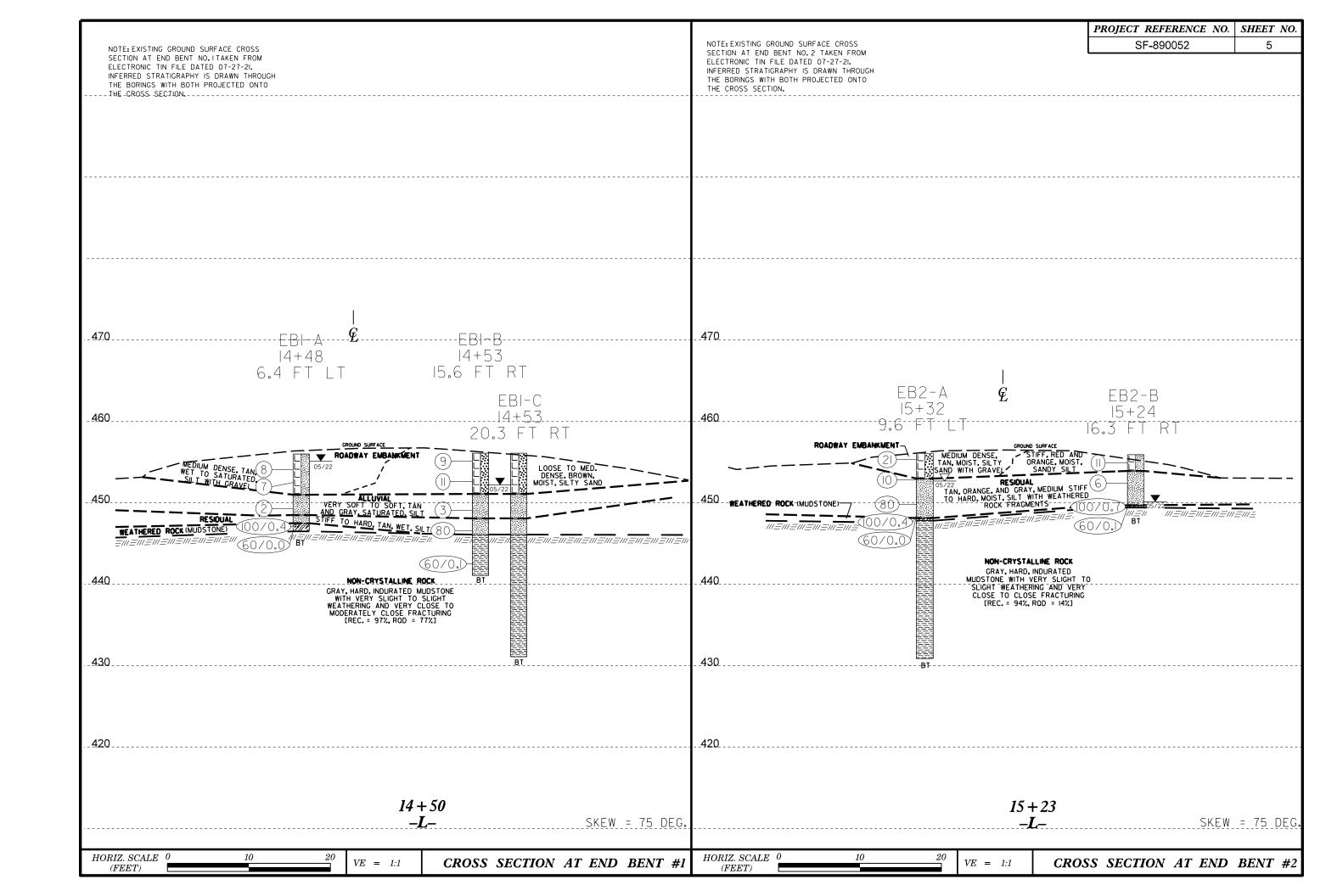
# SUBSURFACE INVESTIGATION

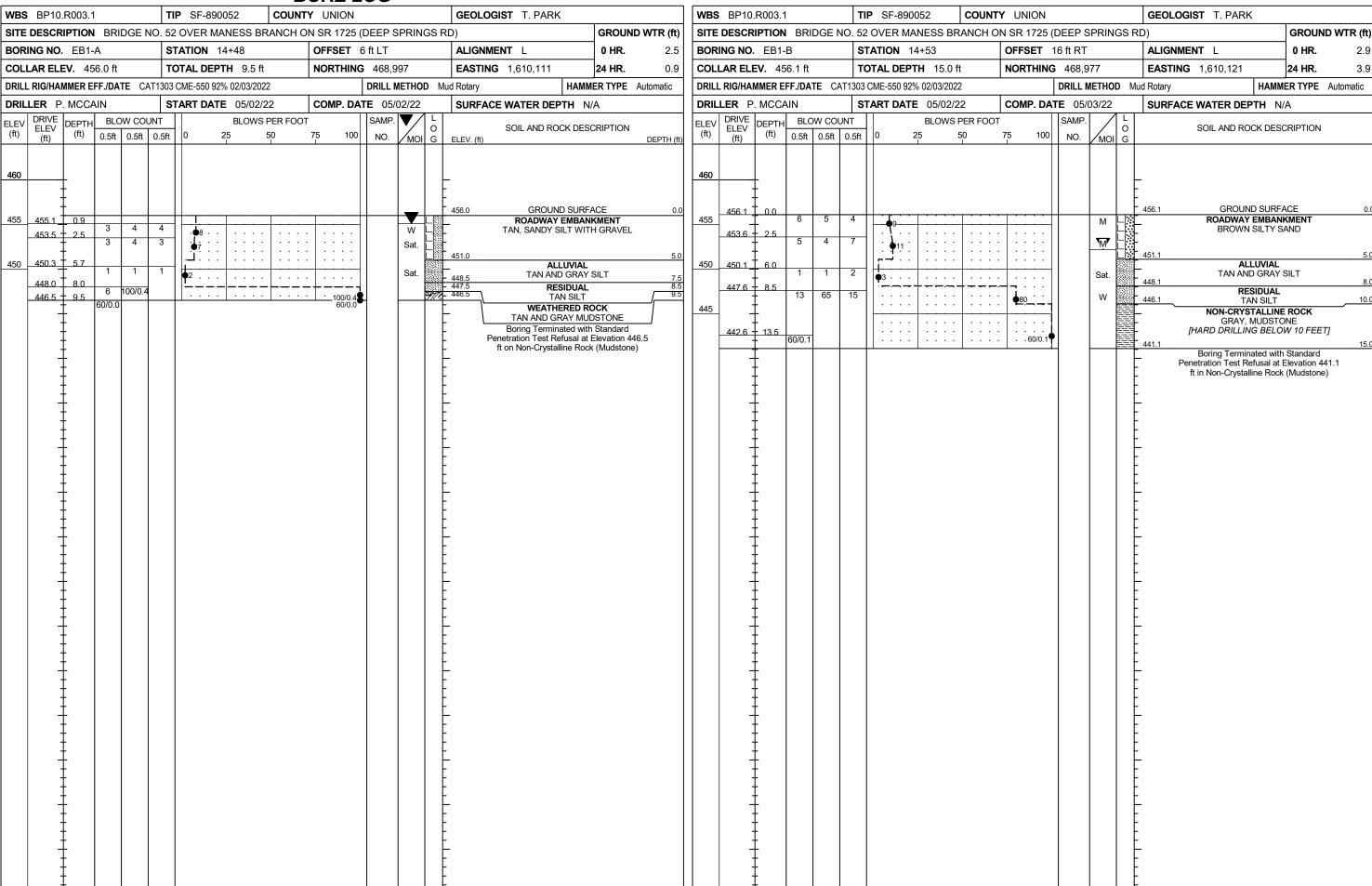
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

		SUPPLEMENT FROM	TAL LE 1 AASF	GEND, GI HTO LRF	EOLOGIC FD BRID	AL STRENGTH INDEX (GSI) TABLES GE DESIGN SPECIFICATIONS
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Joi	nted Rock Mass (Ma	rinos and Hoek,2000	<b>2</b> ))			AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)
GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)	aces	p		s e o	s e o	GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)
From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.	SURFACE CONDITIONS  VERY GOOD  Very rough, fresh unweathered surface	GOOD Rough, slightly weathered, iron stained surfaces	<b>FAIR</b> Smooth, moderately weathered and altered surfaces	POOR Slickensided, highly weathered surfacturith compact coatings or fillings or angular fragments	<b>VERY POOR</b> Slickensided, highly weathered surface with soft clay coatings or fillings	Surface conditions (barticularly of the pedding planes), choose a pox in the chart. Focate the bosition in the pox that corresponds to the condition of the discontinuities and estimate the average value of QSI from the contours. Do not attempt to be too because of Controlled failures. Mere and assess is reduced by the bresence of groundwater and this can be allowed for by a slight shift to the right in the columns for the rock masses or the poor and very book as slight shift to the right in the columns for the rock masses and the poor and very book as a slight shift to the right in the columns for the rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very book a slight shift to the right in the columns for fair, poor and very book as a slight shift to the right in the columns for change the value of QSI and it is dealt with by using effective stress analysis.  NERY GOOD - Rough and it is dealt with by using effective stress analysis.
STRUCTURE	D	ECREASING SURF			'_	COMPOSITION AND STRUCTURE
INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities  BLOCKY - well interlocked un-	PIECES 06 06	70		N/A	N/A	A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability.  70  A
disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets  VERY BLOCKY - interlocked,	G OF ROCK	60				8. Sand- stone with thin inter- layers of siltstone amounts  C. Sand- stone and stitution or silty shale with sand- stone layers or clayey stone layers solution or clayey stone layers solution amounts  E. Weak siltstone or clayey stone layers shale with sandstone
partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets	RLOCKING	50	40			Table 1 July 1 J
BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity	ASING INTERL		3	30		C.D. E. and G - may be more or less folded than illustrated but this does not change the strength. Tectonic deformation, faulting and loss of continuity moves these categories to F and H.  F. Tectonically deformed, intensively folded/faulted, sheared clayey shale or siltstone with broken and deformed sandstone layers forming an almost chaotic structure
DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces	DECREASING			20		G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers  H. Tectonically deformed silty or clayey shale with por clayey shale forming a chaotic structure with pockets of clay. Thin layers of
LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes	N/A	N/A			10	sandstone are transformed into small rock pieces.  Means deformation after tectonic disturbance









												D	<u> УК</u>		_(	JG				
/BS	BP10	.R003.	1		TI	IP S	SF-8	9005	52		COL	JNTY	' UN	IION					GEOLOGIST T. PARK	
ITE	DESCF	RIPTION	<b>I</b> BRI	IDGE I	NO. 52	2 OV	/ER I	ИAN	IESS	BRA	NCI	10 h	I SR	1725	(D	EEP S	PRIN	GS R	RD)	GROUND WTR (ft
ORII	NG NO	. EB1-	·C		S	TAT	ION	14+	+53				OFF	SET	20	ft RT			ALIGNMENT L	<b>0 HR.</b> N/A
OLL	AR EL	<b>EV</b> . 45	6.1 ft		T	ОТА	L DE	PTH	<b>l</b> 25	.0 ft			NOR	THIN	G	468,9	73		<b>EASTING</b> 1,610,123	24 HR. FIAD
RILL	RIG/HA	MMER E	FF./DA	TE C	AT1303	CME	E-550	92%	02/03/	2022						ORILL M	IETHOL	) Mu	ud Rotary HAMN	MER TYPE Automatic
RILL	<b>.ER</b> F	. MCC	AIN		S	TAR	T DA	TE	05/0	)3/22	2		COM	P. D	ATE	E 05/0	03/22		SURFACE WATER DEPTH N	/A
EV ft)	DRIVE ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft		0		25	BLOV	NS PI 5(			75 	100	$\perp$	SAMP. NO.	MOI	L O G	SOIL AND ROCK DES	CRIPTION DEPTH (I
						<u>.</u>		•		<del></del> .									456.1 GROUND SURF	
55	-	ł				<u>                                     </u>		_					<del>                                     </del>		+				ROADWAY EMBAN BROWN SILTY S	
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)	_	Ŧ				lĿ						٠.							451.1 ALLUVIAL	
		Ŧ				-		-										-	TAN AND GRAY	8.
		Ŧ				:							: :					F	RESIDUAL 446.1 TAN SILT	10
5	-	Ŧ				ĬĿ		-					<u> </u>	· ·	+				NON-CRYSTALLIN	
		Ŧ				:		:					: :						GRAY, HARD, INDURATE WITH VERY SLIGHT 1	O SLIGHT
,		‡				:		:			: :	: :	: :						<ul> <li>WEATHERING AND VER</li> <li>MODERATELY CLOSE F</li> </ul>	RACTURING
	-	‡				-		-					: :		11				= [REC. = 97%, RQD	= 77%]
		‡				:		:		: :			: :					雪	•	
<u>.                                    </u>	_	‡				╽┟╴		-					<u>  : :</u>		41				• <del>-</del>	
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		‡				Щ·		•	• •		• •	• •		• •	Щ				431.1	25
	=	‡																-	Boring Terminated at Eleva Non-Crystalline Rock (	Mudstone)
	-	+ + + + + + + + + + + + + + + + + + +																	OFFSET BORING FOR EI WAS ADVANCED TO RO SPT. SOIL LITHOLOGY T BORING EB1- BORI	CK WITHOUT TAKEN FROM
	-																			

_									C	OI	RE L	.OG					
	WBS	BP10.R003.	1		TIP	SF-89	0052	C	OUNT	<b>Y</b> U	NION			GEOLOGIST T. PARK	(		
		DESCRIPTION		DGE NO				BRAN	ICH O	N SF	R 1725 (	GROUN	D WTR (ft)				
	BOR	ING NO. EB1	-C		STATION 14+53					OFFSET 20 ft RT				ALIGNMENT L	0 HR.	N/A	
	COLI	LAR ELEV. 4	56.1 ft		TOT	AL DE	<b>PTH</b> 25.	0 ft		NO	RTHING	468,973		<b>EASTING</b> 1,610,123		24 HR.	FIAD
	DRILL	. RIG/HAMMER E	FF./DA	TE CAT1	303 CM	E-550 9	2% 02/03/2	2022				DRILL METHO	<b>D</b> Muc	d Rotary	HAMM	ER TYPE	Automatic
	DRIL	LER P. MCC	AIN		STAF	RT DA	<b>TE</b> 05/0	3/22		СО	MP. DA	<b>TE</b> 05/03/22		SURFACE WATER DE	PTH N	/A	
	COR	E SIZE NQ			TOTA	AL RUI	<b>1</b> 15.0 ft										
	ELEV (ft)	RUN ELEV (ft) DEPTH	RUN (ft)	DRILL RATE (Min/ft)	RI REC. (ft) %	JN RQD (ft) %	SAMP. NO.	STR REC. (ft) %	ATA RQD (ft) %	L O G			D	ESCRIPTION AND REMARK	(S		
	446.1													Begin Coring @ 10.0 ft			
)	445	440.1 16.0 435.1 21.0 431.1 25.0	1.0 5.0 5.0	1.08/1.0 1.17/1.0 1.19/1.0 1.15/1.0 1/1.0 1.09/1.0 1.06/1.0 1.32/1.0 3.59/1.0 1.15/1.0 1.08/1.0 1.18/1.0	(0.5) 50% / (5.0) 100% (5.0) 100% (4.0) 100%	(3.1) 62% (3.4)		(14.5) 97%	(11.5) 77%		446.1	WEATH	ERING A	NON-CRYSTALLINE ROC PATED MUDSTONE WITH VE AND VERY CLOSE TO MOD FRACTURING  Elevation 431.1 ft in Non-Cry	K ERY SLIG ERATEL	Y CLOSE	25.0
		****															

	NTY UNION GEOLOGIST T. PARK	WDC DD40 D002 4 TID SE 900052 COUNTY	CEOLOGIST T DADIC
			V UNION GEOLOGIST T. PARK
SITE DESCRIPTION BRIDGE NO. 52 OVER MANESS BRANCH C		SITE DESCRIPTION BRIDGE NO. 52 OVER MANESS BRANCH ON	
BORING NO. EB2-A STATION 15+32	OFFSET 10 ft LT ALIGNMENT L 0 HR. 3.6		OFFSET 10 ft LT ALIGNMENT L 0 HR. 3.6
COLLAR ELEV. 456.3 ft TOTAL DEPTH 25.2 ft	NORTHING         469,023         EASTING         1,610,190         24 HR.         FIAD	l l	NORTHING 469,023
DRILL RIG/HAMMER EFF./DATE CAT1303 CME-550 92% 02/03/2022	DRILL METHOD Mud Rotary HAMMER TYPE Automatic	DRILL RIG/HAMMER EFF./DATE CAT1303 CME-550 92% 02/03/2022	DRILL METHOD Mud Rotary HAMMER TYPE Automatic
DRILLER P. MCCAIN START DATE 05/02/22	COMP. DATE 05/03/22 SURFACE WATER DEPTH N/A		COMP. DATE 05/03/22 SURFACE WATER DEPTH N/A
ELEV   DRIVE   DEPTH   BLOW COUNT   BLOWS PER FOOT   BLOWS PER FOOT	SOIL AND ROCK DESCRIPTION	CORE SIZE NQ TOTAL RUN 17.2 ft	
(ft) (ft) 0.5ft 0.5ft 0.5ft 0 25 50	75 100 NO. MOI G ELEV. (ft) DEPTH (ft)		O DESCRIPTION AND REMARKS
		(it) (ft) (it) (Min/ft) (ii) (iii) (Min/ft) (iii) (iii	G
460		448.3 + 8.0 1.0 N=60/0.0 (0.0) (0.0) (16.2) (2.4)	Begin Coring @ 8.0 ft  NON-CRYSTALLINE ROCK 8.0
		1 040/ 1 440/ F	GRAY, HARD, INDURATED MUDSTONE WITH VERY SLIGHT TO SLIGHT WEATHERING AND VERY CLOSE TO CLOSE FRACTURING
456.3 7 0.0 9 13 8	456.3 GROUND SURFACE 0.0   M L	445 5.1 2.35/1.1 (5.1) (0.8) 2.35/1.0 (1.23/1.0 100% 16% 14.7) 442.2 14.1 1.47/1.0 1.47/1.0	WE WILLIAM DESCRIPTION OF SECTION
453.8 + 2.5	TAN, SILTY SAND WITH GRAVEL	442.2 † 14.1   1.23/1.0   1.47/1.0   5.1   1.59/1.1   (5.1)   (1.0)	
	TAN ORANGE AND GRAY SILT WITH	440	
450 450.8 + 5.5	WEATHERED ROCK FRAGMENTS	2.09/1.0 1.35/1.0 437.1 19.2 1.18/1.0	
448.3 7 8.0 32 100/0.4	. 100/0.4	5.0 1.24/1.0 (5.0) (0.0)	
445			
<b>T</b>	GRAY, HARD, INDURATED MUDSTONE WITH VERY SLIGHT TO SLIGHT	432.1 24.2 1.16/1.0 1.16/1.0 (1.0) (0.6)	
	WEATHERING AND VERY CLOSE TO	431.1 25.2 1.0 1.15/1.0 (1.0) (0.6) 100% 60%	431.1 25.2 Boring Terminated at Elevation 431.1 ft on Non-Crystalline Rock (Mudstone)
440	[REC. = 94%, RQD = 14%]		<u> </u>
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435			[-
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	Boring Terminated at Elevation 431.1 ft on		F
	Non-Crystalline Rock (Mudstone)		-
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<b>WBS</b> BP10.R003.1	TIP SF-890052 COUNT	Y UNION	GEOLOGIST T. PARK	
SITE DESCRIPTION BRIDGE NO	. 52 OVER MANESS BRANCH C	,	D)	GROUND WTR (ft
BORING NO. EB2-B	STATION 15+24	OFFSET 16 ft RT	ALIGNMENT L	<b>0 HR.</b> 5.3
COLLAR ELEV. 455.9 ft	TOTAL DEPTH 6.5 ft	NORTHING 468,996	<b>EASTING</b> 1,610,190	<b>24 HR.</b> 5.8
DRILL RIG/HAMMER EFF./DATE CAT1	303 CME-550 92% 02/03/2022	DRILL METHOD Mu	id Rotary HAMM	ER TYPE Automatic
DRILLER P. MCCAIN	<b>START DATE</b> 05/02/22	COMP. DATE 05/03/22	SURFACE WATER DEPTH N/	Α
DRIVE   DEPTH   BLOW COUNT     (ft)   (ft)   0.5ft   0.5ft   0.	T BLOWS PER FOO <sup>-1</sup> .5ft 0 25 50	75 100 NO. MOI G	SOIL AND ROCK DESC ELEV. (ft)	CRIPTION DEPTH (
453.4 + 2.5   3   3   3	4	1000.7 60/0.1	455.9 GROUND SURFA ROADWAY EMBANI 453.9 RED AND ORANGE, SA RESIDUAL TAN, GRAY, AND RE  449.7  449.4 WEATHERED RC GRAY, MUDSTO Boring Terminated with Penetration Test Refusal at 8 ft in Non-Crystalline Rock	ED, SILT  COCK STANDARD STANDA

SHEET 9

PROJECT REPERENCE NO. SHEET NO.

SF-890052

10

# CORE PHOTOGRAPHS

# BRIDGE NO. 52 ON SR 1725 (DEEP SPRINGS RD) OVER MANESS BRANCH



END OF RUN #2 16.0 FT

BORING EBI-C STA. 14+53 -L-, 16 FT RT DEPTH: 10.0 FT TO 16.0 FT

BORING EB1-C STA. 14+53 -L-, 16 FT RT DEPTH: 16.0 FT TO 21.0 FT



END O RUN # 21.0 F

0

0.5

FEET

# CORE PHOTOGRAPHS

# BRIDGE NO. 52 ON SR 1725 (DEEP SPRINGS RD) OVER MANESS BRANCH

START OF RUN #4 @ 21.0 FT

BORING EB1-C STA. 14+53 -L-, 16 FT RT DEPTH: 21.0 FT TO 25.0 FT



END OF CORING 25.0 FT



START OF RUN #2\* 9.0 FT

BORING EB2-A STA. 15+32 -L-, 10 FT LT DEPTH: 8.0 FT TO ~15.5 FT



RUN #3 CONT'D ON NEXT SHEET

0

0.5

FEET

START OF RUN #3 13.1 FT

PROJECT REPERENCE NO. SHEET NO. 12

# CORE PHOTOGRAPHS

# BRIDGE NO. 52 ON SR 1725 (DEEP SPRINGS RD) OVER MANESS BRANCH

RUN #3 CONT'D FROM PREV SHEET



END OF RUN #3 AT 19-2 FT

BORING EB2-A STA. 15+32 -L-, 10 FT LT DEPTH: ~16.5 FT TO 25.2 FT

START OF RUN #4 19.2 FT



START OF RUN #5 24.2 FT

FEET

BORING EB2-A STA. 15+32 -L-, 10 FT LT DEPTH: 24.2 FT TO 25.2 FT

PROJECT REPERENCE NO. SHEET NO. 13

# SITE PHOTOGRAPH

# BRIDGE NO. 52 ON SR 1725 (DEEP SPRINGS RD) OVER MANESS BRANCH



**VIEW LOOKING EAST**