



**CULVERT FOUNDATION RECOMMENDATIONS REPORT
REPLACEMENT OF BRIDGE 890366 ON SR 1153
(FLOYD MOORE ROAD) OVER BEAVER DAM CREEK (WEST)**

**WBS No.: 17BP.10.R.19
Tip No.: NA
County: UNION**

Prepared by:

**AMEC Environment and Infrastructure, Inc.
4021 Stirrup Creek Drive, Suite 100
Durham, North Carolina 27703
(Project No. 6469-12-1040)**

Prepared for:

NCDOT

July 25, 2012



July 25, 2012

Division Bridge Program Manager
NCDOT Division 10 Office
716 W. Main Street
Albemarle, North Carolina 28001

Attention: Mr. James Wally, E.I.:

Subject: **Culvert Foundation Recommendation Report
Replacement of Bridge No. 890366 on SR 1153 (Floyd Moore Road) over
Beaver Dam Creek (West)
WBS No.: 17BP.10.R.19
TIP No.: NA
County: Union
AMEC Project Number: 6469-12-1040**

Dear Mr. Wally:

AMEC Environment and Infrastructure, Inc. (AMEC) is pleased to transmit the Culvert Foundation Recommendations Report in association with the replacement of Bridge No. 366 on SR 1153 (Floyd Moore Road) over Beaver Dam Creek (West). The recommended structure type is two Reinforced Concrete Box Culverts (2 @ 12' x 5'). The Structure Subsurface Investigation Report provided by NCDOT and the additional Structure Subsurface Report performed by AMEC are provided in the Appendix of this report. The Foundation Recommendations Report has been prepared using boring data obtained by AMEC and others.

INCLUDE AS NOTE ON PLANS

RECOMMENDATIONS

- Excavate 1 foot below culvert and footings and replace with foundation conditioning material in accordance with Article 414 of the Standard Specifications.
- Recommend including 25 cubic yards of undercut of soft foundation soils as a contingency item to be used at the discretion of the Engineer.
- Recommend including 25 cubic yards of Select Material Class VI to be used as backfill as a contingency item to be used at the discretion of the Engineer.

Correspondence:
AMEC E&I, Inc.
4021 Stirrup Creek Drive, Suite 100
Durham, North Carolina 27703
Tel (919) 381-9900
Fax (919) 381-9901
Licensure: NC Engineering F-1253 NC Geology C-247

**N.C. DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENG. UNIT-WRO**

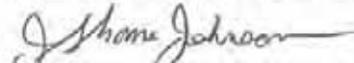
- ACCEPTED
- ACCEPTED AS NOTED
- RETURNED FOR CORRECTIONS
- SEE LETTER

BY: Dean Hardister, PE

DATE: 08/20/2012

If you have any questions regarding recommendations contained in this report, please contact us at 919-381-9900.

Sincerely,
AMEC Environment and Infrastructure, INC.


J. Shane Johnson, P.E., P.G.
Senior Geotechnical Engineer
Registered, North Carolina 037422


Gary R. Taylor, P.E.
Geotechnical Department Manager
Registered, North Carolina 18580



**N.C. DEPT. OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENG. UNIT-WRO**

- ACCEPTED
 ACCEPTED AS NOTED
 RETURNED FOR
CORRECTIONS
 SEE LETTER

BY: Dean Hardister, PE

DATE: 08/20/2012

APPENDIX

PROVIDED INFORMATION

SITE DATA

Source FEMA FIS
 Character RURAL
 Frout, High Quality Water, etc.) WS-IV
 ber deck on timber joists (low water type) ERTS; Timber caps,
 st & sills, with Timber post & sill (crutched) 1 @ 18'-6"

rate..... X High
 n Stream Upstream: 199, 2 lines 12"x75" CM Pipe Arch: 50'-9" along
 centerline of pipe, Downstream: Non-comparable.

Period of Records N/A
 c.f.s Date N/A Frequency N/A

BRETT HILDRETH*
 NCDOT BRIDGE MAINTENANCE Period of Knowledge 15 Yr,
 it, Freq. Source Period of Knowledge

Normal Water Surface Elev. ~609'
 Channel 0.045 Right O.B. 0.12 Obtained From FEMA FIS
 Limited Detail Study Floodway Established? NO

MOA TYPE 2B (1.32' DECREASE HEC-RAS SECTION 34869)
 Natural Channel Velocity (V10) 5.1 ft/s
 CLASS II RIP RAP

FORMATION TO BE SHOWN ON PLANS
 25 yr. Flow 613.2'

CULVERT SURVEY & HYDRAULIC DESIGN REPORT

FEMA LIMITED DETAILED STUDY
 ZONE AE
 YADKIN RIVER BASIN
 RALEIGH, N. C.

N. C. DEPARTMENT OF TRANSPORTATION
 DIVISION OF HIGHWAYS
 HYDRAULICS UNIT
 RALEIGH, N. C.

I.D. No. SF-890366 Project No. 17BP10.R.19 Proj. Station Sta. 12+27.1-

County Union Stream Branch Beaverdam Creek (West) Stru. No. 890366
 On Highway SR153 (Floyd Moore Rd) Between (S.Rocky River Rd) and Dead End

Recommended Structure 2'-12" x 5' BOX CULVERT BARREL SECTION

Recommended Width of Roadway 20' (EOP to EOP) Skew 80 DEGREES
 Recommended Location is (Up) Down Stream from Existing Crossing.

Bench Mark is Monument BL-2 -Elev. Sta. 11+78.53 10.24' Right
 Bl. 3: N: 433407.073 E: 151511.036 Elev. 617.27' Datum: NAVD 88

Temporary Crossing Dead end road, existing bridge, to remain open during phased construction.



Beaverdam Struc. Inv. No. 890366 I.D. No. SF-890366 Project No. 17BP10.R.19
 ek (West) Struc. Inv. No. 890366 I.D. No. SF-890366 Project No. 17BP10.R.19
 PDF File 890366 2012 SF890366 Branch Beaverdam Creek West SMI53.pd

Inlet Control	Outlet Control				Remarks
	H.W.	dc	dc+D/2	H.W.	
COMPUTATIONS FROM HEC-RAS					

MOA TYPE 2B (1.32' DECREASE HEC-RAS SECTION 34869)
 Natural Channel Velocity (V10) 5.1 ft/s
 CLASS II RIP RAP
 FORMATION TO BE SHOWN ON PLANS
 25 yr. Flow 613.2'

Designed by: AMEC, E&I, INC.
 Date: 07/20/2012

10 + 00

11 + 00

12 + 00

13 + 00

BEGIN CONSTRUCTION
-L- POT STA. 10+16.50
EL. 617.76

BEGIN GRADE
-L- POT STA. 10+66.50
EL. 615.26

PI = 12+12.00
EL = 610.18'
VC = 250'
K = 37

2 - 12' x 5'
BOX CULVERT
BARREL SECTION

EXIST. BRIDGE
TO BE REMOVED

NG @ PROP. ALIGNMENT
EXISTING GROUND
DOWNSREAM
100 YEAR ELEV. = 613.80
25 YEAR ELEV. = 613.2
ROCK ELEV. = 604.7
ROCK' ELEV. = 605.7
FLOOR = 605.25
BURY BOTH BARRELS 2.0'

-13.4897%

+13.794%

WS ELEV. = 608
DATE: 4-13-12

ROCK LINE FROM BORING LOG
6ft ± 604'

END G
-L- P1
EL. 615.

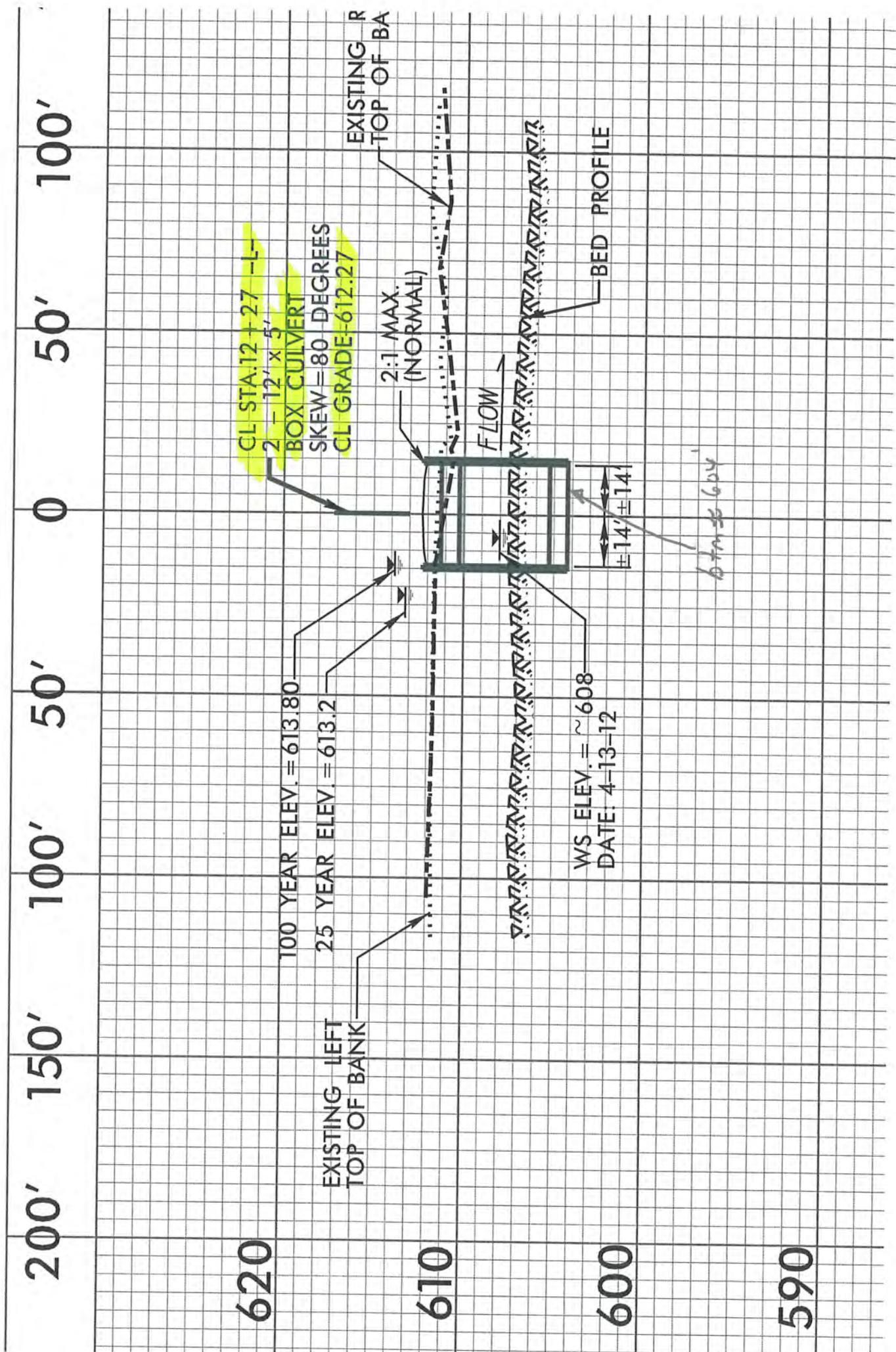
620

610

600

590

DATE



CL STA: 12 + 27 - L -
 2 - 12' x 5'
 BOX CULVERT
 SKEW = 80 DEGREES
 CL GRADE = 612.27

100 YEAR ELEV. = 613.80
 25 YEAR ELEV. = 613.2

EXISTING LEFT
 TOP OF BANK

EXISTING R
 TOP OF BA

2:1 MAX.
 (NORMAL)

FLOW

WS ELEV. ≈ 608
 DATE: 4-13-12

BED PROFILE

14' ± 14'

67m ≈ 604'

200' 150' 100' 50' 0 50' 100' 150'

620

610

600

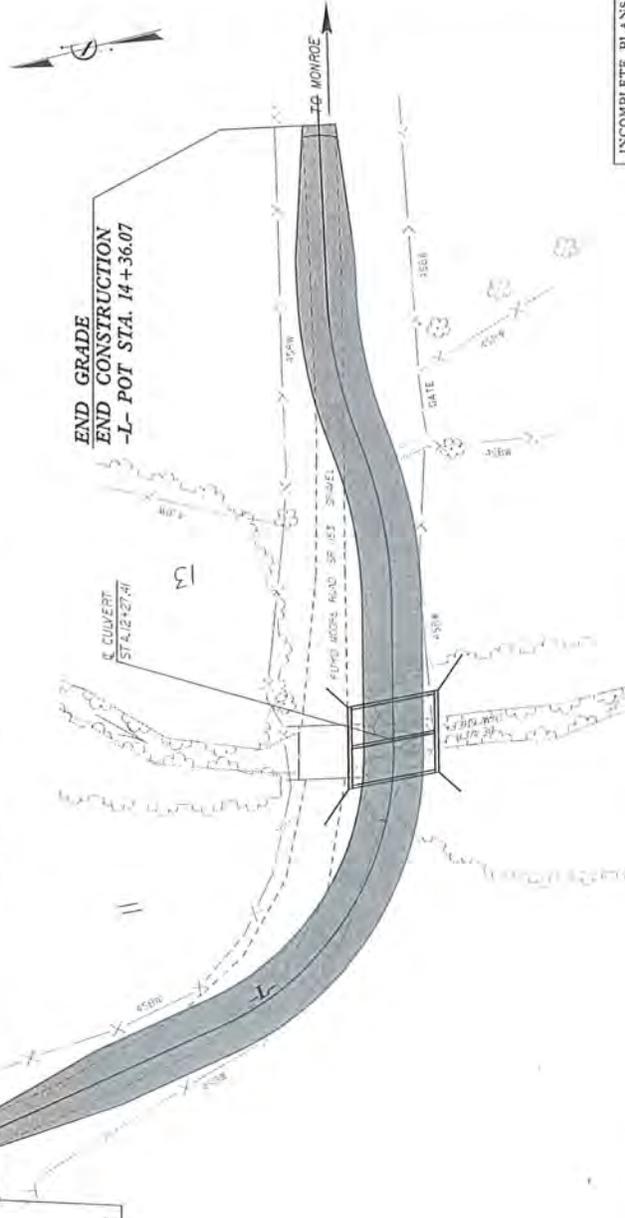
590

STATE PROJECT REFERENCE NO.	17BP.10.R.19	DATE	1
STATE ROAD NO.	366	PROJECT NAME	ROW/UTIL. CONST.
PROJECT NO.	17BP.10.R.19	DESIGNER	AMEC

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS
UNION COUNTY

LOCATION: BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE ROAD)
OVER BEAVER DAM CREEK

TYPE OF WORK: GRADING, PAVING, DRAINAGE, STRUCTURES AND
TRAFFIC CONTROL



INCOMPLETE PLANS
DO NOT USE FOR CONSTRUCTION
PRELIMINARY PLANS
DO NOT USE FOR CONSTRUCTION

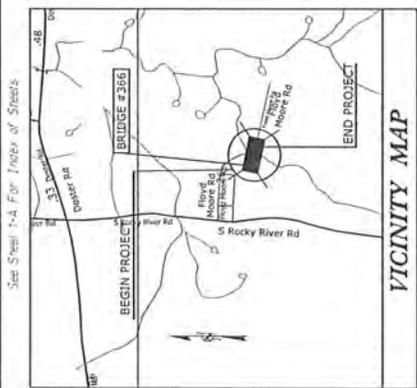
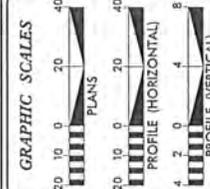


HYDRAULICS ENGINEER
STRUCTURE: _____ PE
ROADWAY DESIGN ENGINEER
_____ PE

PROJECTED IN THE OFFICE OF:
AMEC Environmental & Infrastructure
15500 E. 15th Avenue, Suite 100
Denver, Colorado 80232
Tel: 303.750.3600
Fax: 303.750.3601
www.amec.com
FOR THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
SR STANDARD SPECIFICATIONS
PROJECT ENGINEER
PROJECT DESIGN ENGINEER

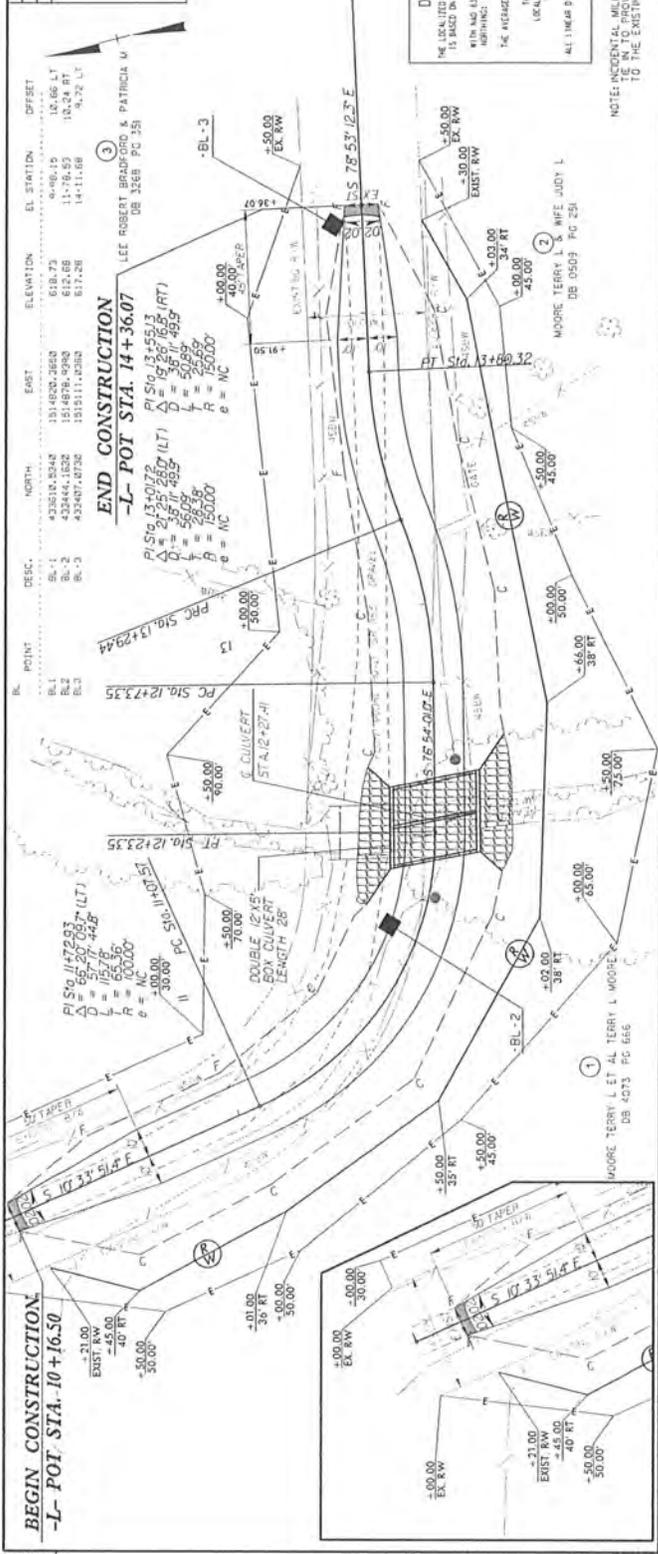
RIGHT OF WAY DATE: _____
LETTING DATE: _____
PROJECT LENGTH
LENGTH OF ROADWAY T.I.P. PROJECT 17BP.10.R.5 = 0.079 MI.
LENGTH OF STRUCTURE T.I.P. PROJECT 17BP.10.R.5 = 0.000 MI
TOTAL LENGTH OF T.I.P. PROJECT 17BP.10.R.5 = 0.079 MI
NCDOT CONTACT: LOUIS L. MITCHELL, PE
DISTRICT CONSTRUCTION MANAGER

DESIGN DATA
ADT 2000 = 70
ADT 2020 = 115
DHV = NA %
D = NA %
T = NA %
V = 15 MPH
FUNC CLASS = _____
LOCAL
SUB-REGIONAL TIER



PROJECT: WBS 17BP.10.R.19

CONTRACT:



DATUM DESCRIPTION
 THE LOCKED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE POINT FOR MONUMENT '81-21' (ELEVATION 102.24 FT) WITH AN ELEVATION OF 102.24 FT. THE POINT IS LOCATED AT THE INTERSECTION OF THE CENTERLINE OF THE PROJECT AND THE CENTERLINE OF THE EXISTING ASPHALT PAVEMENT. THE N.C. LIQUIDITY GRID BEARING AND LOCALITY (NAD 83) COORDINATE SYSTEM IS USED FOR ALL DISTANCE MEASUREMENTS. THE N.C. LIQUIDITY GRID BEARING AND LOCALITY (NAD 83) COORDINATE SYSTEM IS USED FOR ALL DISTANCE MEASUREMENTS. ALL LINEAR DISTANCE MEASUREMENTS ARE IN FEET.

NOTE: MONUMENTS, M.B.L. APPROX. 38' AT EACH END, TO PROVIDE A SMOOTH TRANSITION TO THE EXISTING ASPHALT PAVEMENT.

624	BEGIN CONSTRUCTION -L- POT STA. 10+16.50 EL 617.76	10
620	BEGIN GRADE -L- POT STA. 10+16.50 EL 615.26	11
616		12
612		13
608		14
604		15
600		
596		
592		

STRUCTURE HYDRAULIC DATA

DESIGN FREQUENCY	= 25 YRS
DESIGN DISCHARGE	= 850 CFS
DESIGN HW ELEVATION	= 613.2 FT
100 YEAR DISCHARGE	= 1200 CFS
100 YEAR HW ELEVATION	= 613.8 FT
OVERTOPPING FREQUENCY	= 25 YRS
OVERTOPPING DISCHARGE	= 850 CFS
OVERTOPPING ELEVATION	= 613.2 FT

-L-

BEGIN CONSTRUCTION
-L- POT STA. 10+16.50
EL 617.76

BEGIN GRADE
-L- POT STA. 10+16.50
EL 615.26

EXIST BRIDGE TO BE REMOVED
PROPOSED GRADE
EL 613.7942

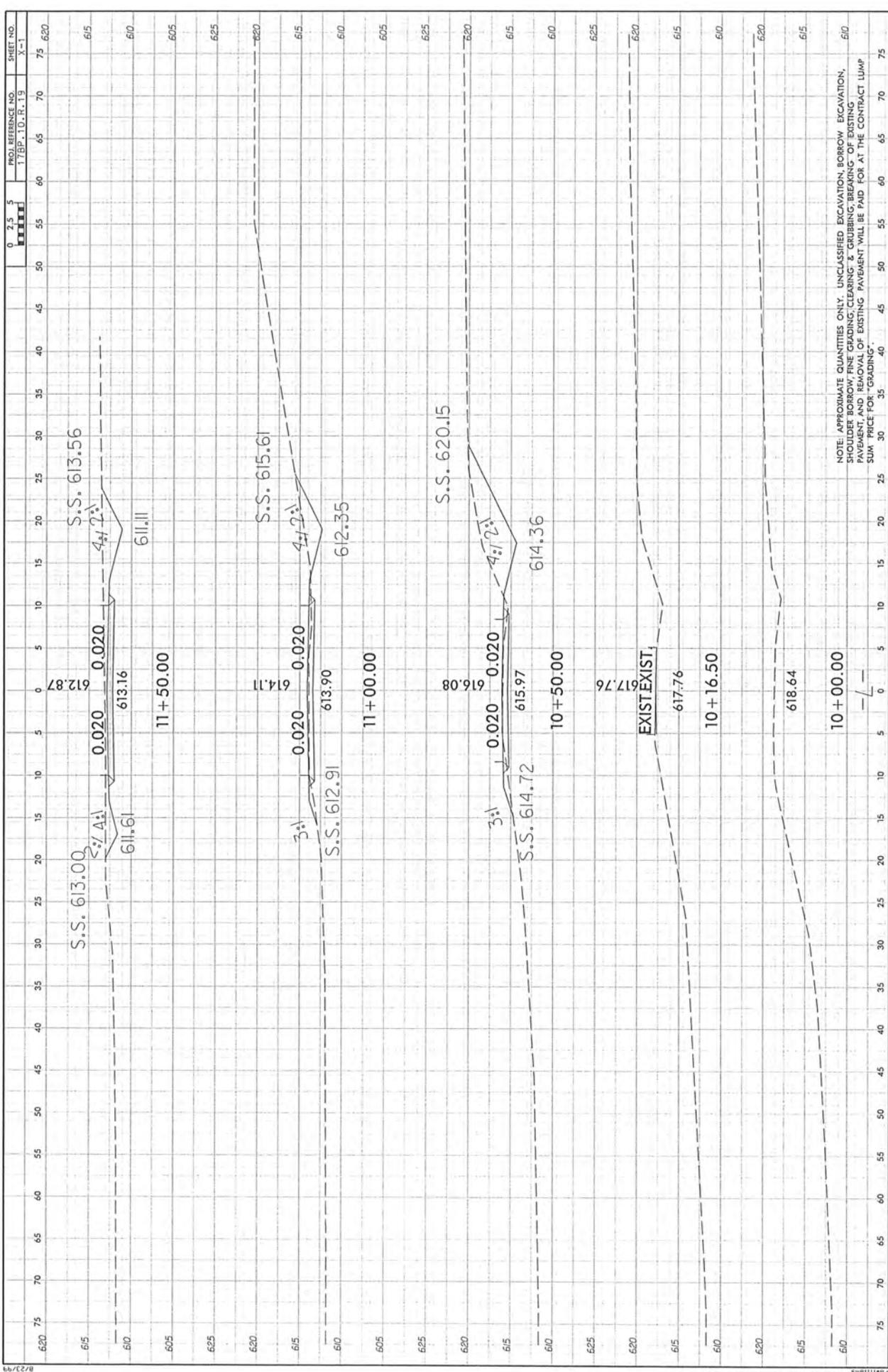
EXIST BRIDGE TO BE REMOVED
PROPOSED GRADE
EL 613.7942

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PROPOSED GRADE
EL 613.7942

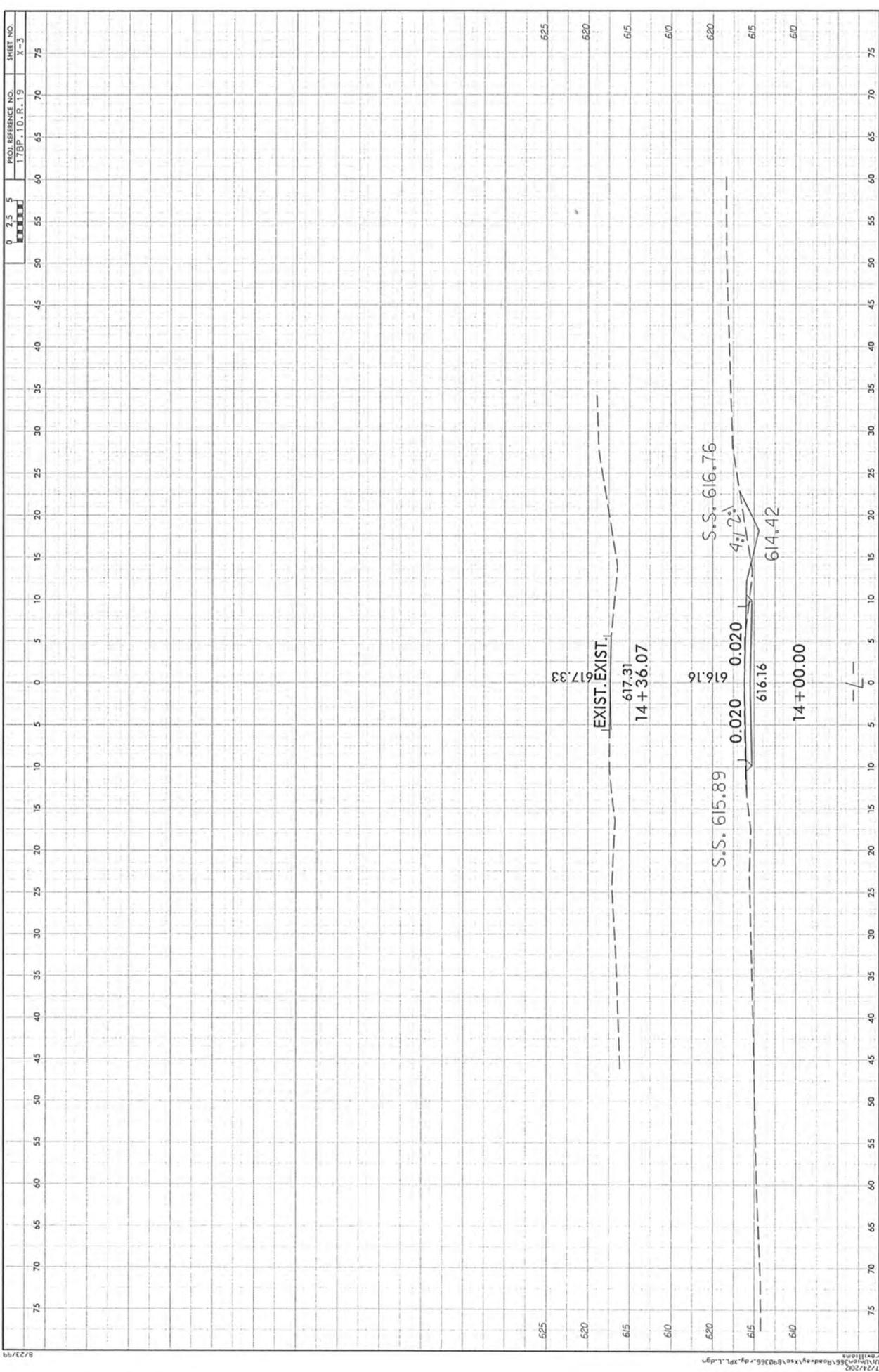
EXIST BRIDGE TO BE REMOVED
PROPOSED GRADE
EL 613.7942

EXIST BRIDGE TO BE REMOVED
PROPOSED GRADE
EL 613.7942

DOUBLE 12x15 BOX CULVERT
BURY BOTH BARRELS 2.0'



NOTE: APPROXIMATE QUANTITIES ONLY. UNCLASSIFIED EXCAVATION, BORROW, EXCAVATION, SHOULDER BORROW, FINE GRADING, CLEARING, & GRUBBING, BREAKING OF EXISTING PAVEMENT, AND REMOVAL OF EXISTING PAVEMENT WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR "GRADING".



FOUNDATION CALCULATIONS



AMEC E&I, Inc.
4021 Stirrup Creek Drive, Suite 100
Durham, NC 27703

JOB NO. 6469-12-1040 SHEET 1 OF 15 X
PHASE GROUP N BRIDGE TASK _____ X
JOB NAME UNION Co. BR. 366 _____ X
BY JSJ DATE 7/25/12 _____ X
CHECKED BY SG DATE 7/26/12 _____ X

BR. 366 UNION Co.

- Recommended Structure = 2 @ 12' X 5' RCBC
 - Culvert Invert = 605.25 ft ✓
 - Btm of structure = \approx 604 ft ✓
 - Existing Btm of creek = \approx 607' ✓
 - PROPOSED GRADE = @ \approx 612.27' ✓
 - Existing Grade near creek Banks "abutments" = 611' - 612' ✓
 - Amount of fill on top of culvert \approx 1' ✓
 - Amount of fill at abutments = $<$ 2' (minimal change) ✓
- preliminary CSR
+
preliminary
roadway
drawings
- NCDOT Provided 2 borings to Refusal on each side of the Creek (4 total borings by NCDOT) at the existing Bridge location. AMEC Provided 1 - Boring w/ Rock curving on each side of the creek (2 - Borings) at the existing bridge location. Due to the 1 new culvert being planned adjacent the existing structure, AMEC went back to site and perform 4 Hand augurs / Rod soundings to confirm rock elevations in the footprint of the proposed culvert.
- Collar Elevations of Borings were determined by using NCDOT Benchmarks.



AMEC E&I, Inc.
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JOB NO. 6469-12-1040 SHEET 2 OF 15 X
 PHASE Group N Bridges TASK _____ X
 JOB NAME UNION CO. BR. 366 _____ X
 BY JSJ DATE 7/25/12 _____ X
 CHECKED BY SG DATE 7/26/12 _____ X

BR. 366

END BENT 1 SIDE OF CULVERT (Down station of creek)

<u>BORING</u>	<u>Collar Elev.</u>	<u>WR Elev.</u>	<u>HR Elev.</u>	<u>Btm of culvert elev.</u>	<u>Depth to WR/HR</u>
B-3	613.0'	605.3'	604.5'	≈ 604'	-1.3'
B-4	612.6'	604.9'	604.5'	≈ 604'	-0.9'
AMEC B-6	612.9'	604.7'	604.1'	≈ 604'	-0.7'
SR-1	611.7'	605.7'	-	≈ 604'	-1.7'
SR-2	611.1'	606.6'	-	≈ 604'	-2.4'

END BENT 2 side of culvert (up station of creek)

B-1	612.6'	606.4'	606.4	≈ 604'	-2.4'
B-2	612.9'	607.2'	606.9	≈ 604'	-3.2'
AMEC B-5	612.4'	605.7'	605.7	≈ 604'	-1.7'
SR-3	607.9'	604.9'	-	≈ 604'	-0.9'
SR-4	606.8'	603.3'	-	≈ 604'	0.7'

Summary

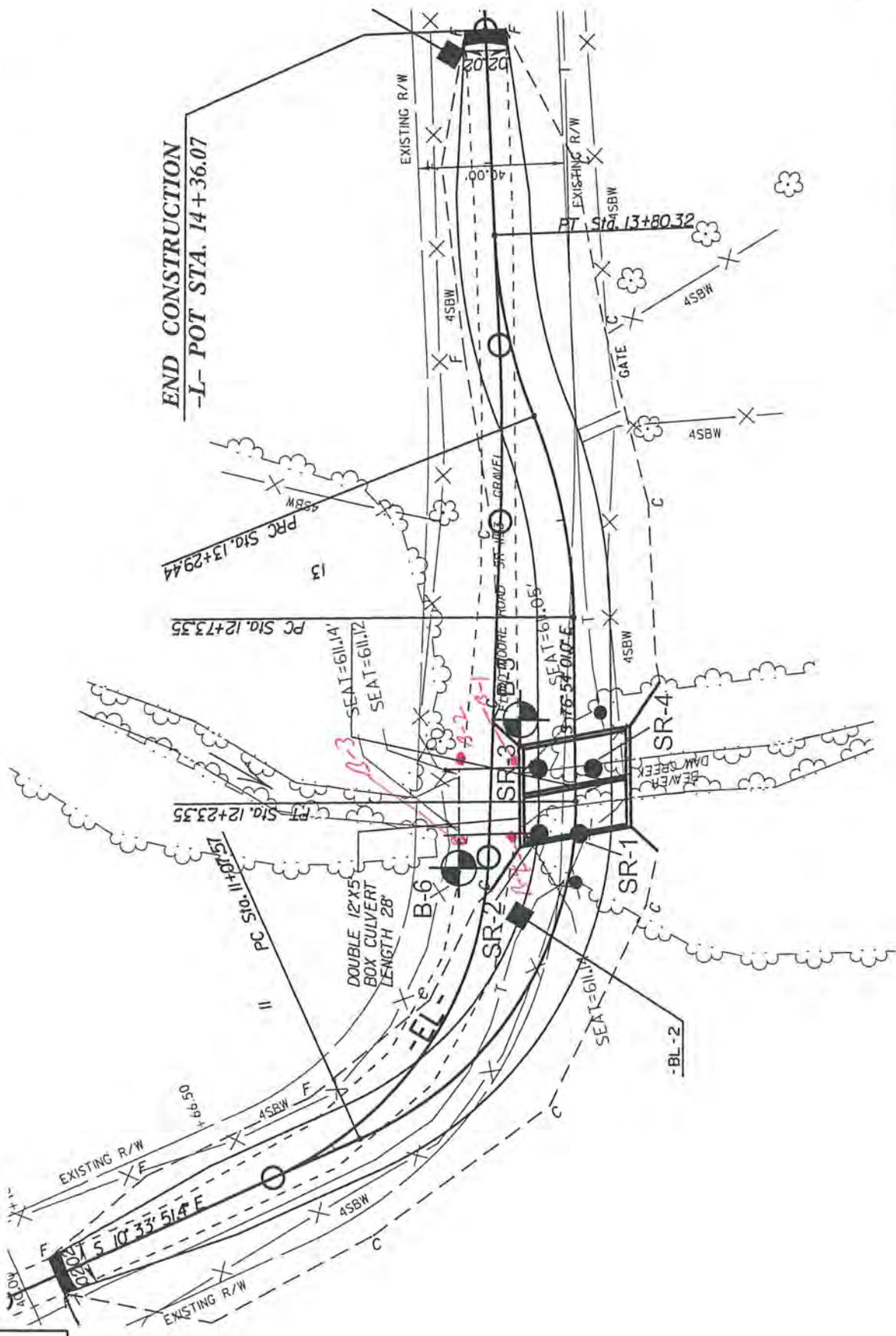
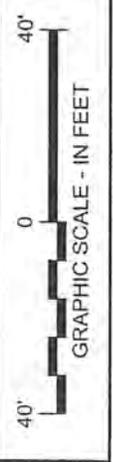
- The Btm of culvert on the ERI side is Approx. 1-2 ft, into WR/HR
- The Btm of culvert on the EAR side is Approx. 1 ft above WR/HR OR approx. 1-3' ft, into WR/HR.
- Based on the Boring / Rod soundings, The proposed btm of culvert is in WR/HR OR near WR/HR.

SHEET NO. 3
 W.B.S. NO.: 17BP.10.R.19
 T.I.P. NO.: N/A
 COUNTY: UNION



DESCRIPTION:
 REPLACE BRIDGE 890366 ON SR 1153
 (FLOYD MOORE ROAD) OVER BEAVER
 DAM CREEK WEST

4/15



END CONSTRUCTION
-L- POT STA. 14+36.07



PROJECT REFERENCE NO. SHEET
 17BP10.R.19 (17BP10R19) 3

SITE PLAN



DATUM DESCRIPTION

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE NAD 83/NSRS 2007 DATUM. THE LOCALIZED HORIZONTAL DISTANCES ARE BASED ON THE NAD 83/NSRS 2007 DATUM. THE VERTICAL DISTANCES ARE BASED ON THE NAD 83/NSRS 2007 DATUM. THE LOCALIZED HORIZONTAL DISTANCES ARE BASED ON THE NAD 83/NSRS 2007 DATUM. THE VERTICAL DISTANCES ARE BASED ON THE NAD 83/NSRS 2007 DATUM.



BL POINT	DESC.	NORTH	EAST	ELEVATION	EL STATION	OFFSET
BL-1		433610.5340	1514820.3650	618.73	9+98.15	10.66 LT
BL-2		433444.1630	1514878.9390	612.68	11+78.53	10.24 RT
BL-3		433407.0730	1515111.0360	617.28	14+11.68	9.72 LT



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

6/15

WBS 17BP.10.R.19		TIP 17BP10R19		COUNTY UNION		GEOLOGIST Stickney, J. K.								
SITE DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST							GROUND WTR (ft)							
BORING NO. B3		STATION 11+95		OFFSET 8 ft LT		ALIGNMENT -EL-								
COLLAR ELEV. 613.0 ft		TOTAL DEPTH 8.5 ft		NORTHING 433,456		EASTING 1,514,901								
DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550 89% 09/02/2009		DRILL METHOD H.S. Augers		HAMMER TYPE Automatic										
DRILLER Smith, C. L.		START DATE 10/24/11		COMP. DATE 10/24/11		SURFACE WATER DEPTH N/A								
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT				SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)
			0.5ft	0.5ft	0.5ft	0	25	50	75					
615													GROUND SURFACE	0.0
610	609.0	4.0											ROADWAY EMBANKMENT RED-BRN SOFT TO MED. STIFF MOIST SILTY CLAY (A-6)	3.0
			1	2	3								ALLUVIAL TAN-GRAY MED. STIFF LOW (PI=15) PLASTIC SANDY SILTY CLAY (A-6)	6.5
605										SS-3	M		RESIDUAL TAN-BRN-GRAY STIFF TO V. STIFF MOIST CLAYEY SANDY SILT (A-4)	7.7
													WEATHERED ROCK SEV. WEATH. NON-CRYSTALLINE ROCK	8.5
													Boring Terminated BY AUGER REFUSAL at Elevation 604.5 ft ON NON-CRYSTALLINE ROCK	

Btm. of culvert
= 604'



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

8/15

WBS 17BP.10.R.19	TIP 17BP.10.R.19	COUNTY UNION	GEOLOGIST R. Clark
SITE DESCRIPTION Replace Bridge 890366 on SR 1153 (Floyd Moore Road) over Beaver Dam Creek			GROUND WTR (ft)
BORING NO. B-6	STATION 11+97	OFFSET 30 ft LT	ALIGNMENT -L-
COLLAR ELEV. 612.9 ft	TOTAL DEPTH 19.8 ft	NORTHING 433,457	EASTING 1,514,895
DRILL RIG/HAMMER EFF./DATE MAC9354 CME-45C 81% 03/01/11		DRILL METHOD SPT Core Boring	HAMMER TYPE Automatic
DRILLER F. Cox	START DATE 05/15/12	COMP. DATE 05/15/12	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
615																
	612.9	0.0	4	5	4									612.9	0.0	GROUND SURFACE
610	609.4	3.5	2	2	5									610.4	2.5	ROADWAY EMBANKMENT Brownish red, stiff, moist, fine sandy SILT (A-4) with little gravel
																ALLUVIAL Yellowish brown, medium stiff, wet, silty CLAY (A-6) with trace coarse sand and gravel
605	604.7	8.2	100	0	3									604.7	8.2	
	604.1	8.8	60	0	0									604.1	8.8	WEATHERED ROCK Gray-tan, METAVOLCANIC ROCK
600																NON-CRYSTALLINE ROCK Greenish gray, METAVOLCANIC ROCK
595																
														593.1	19.8	Boring Terminated at Elevation 593.1 ft in Non-Crystalline Rock: METAVOLCANIC ROCK Auger refusal at 8.8 feet.

Btm of Culvert ≈ 604'

NCDOT BORE SINGLE BRIDGE 366 LOGS.GPJ NC_DOT_GDT 8/21/12



NCDOT GEOTECHNICAL ENGINEERING UNIT
FIELD PENETROMETER LOG (ENGLISH)

9/15

PROJECT NUMBER	17BP.10.R.19	ID	17BP.10.R.19	CO	Union	GEO	C. Baldwin
SITE DESC	Bridge No. 366 on SR 1153 over Beaver Dam Creek West						
BORING NUMBER	SR-1	STA	12+15	OFFSET	1 FT RT	ALIGNMENT	L
ELEVATION	611.7 FT	TOTAL DEPTH	6.0 FT	NORTH	433,423	EAST	1,514,897
DRILL METHOD	Sounding Rod/Hand Auger					DRILLER	N/A
START DATE	07/09/12	COMP DATE	07/09/12	SURFACE WTR DEPTH	N/A FT	DEPTH TO ROCK	6.0 FT

DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMPLE NO. & INTERVAL	MOI	ORIGIN	SOIL & ROCK DESCRIPTION <small>(w/ color, density/consistency, texture, plasticity, organics, other)</small>
	0.5 ft	0.5 ft	TOTAL	0	25	50	75	100				
0.0			17	X						S-1 0.0 - 4.0 ft	D	Residual Brown, fine sandy SILT (A-4) Elev. 611.7 ft
			15	X								
			17	X								
			28		X							
			25		X							
			31			X						Elev. 605.7 ft
6.0												Sounding Rod refusal at 6.0 feet

↑
Btm of culvert
≈ 604'

NOTES _____

SIGNATURE _____ DATE _____
 NOTES _____

DECK TO DATUM DISTANCE N/A FT



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

11/15

WBS 17BP.10.R.19	TIP 17BP10R19	COUNTY UNION	GEOLOGIST Stickney, J. K.
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SITE DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST			GROUND WTR (ft)
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BORING NO. B1	STATION 12+18	OFFSET 10 ft RT	ALIGNMENT -EL-	0 HR. Dry
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COLLAR ELEV. 612.6 ft	TOTAL DEPTH 6.2 ft	NORTHING 433,433	EASTING 1,514,918	24 HR. NM
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DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550 89% 09/02/2009	DRILL METHOD H.S. Augers	HAMMER TYPE Automatic
---	--------------------------	-----------------------

DRILLER Smith, C. L.	START DATE 10/24/11	COMP. DATE 10/24/11	SURFACE WATER DEPTH N/A
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ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)	
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					ELEV. (ft)
615															
610	608.7	3.9	2	2	3									612.6 GROUND SURFACE 0.0	
														609.6 ROADWAY EMBANKMENT RED-BRN SOFT TO MED. STIFF MOIST SILTY CLAY (A-6) 3.0	
														607.3 ALLUVIAL TAN-GRAY MED. STIFF MOIST MED. (PI=20) PLASTIC SILTY SANDY CLAY (A-6) 5.3	
														606.4 RESIDUAL TAN-BRN-GRAY STIFF TO V. STIFF MOIST CLAYEY SANDY SILT (A-4) 6.2	
														Boring Terminated BY AUGER REFUSAL at Elevation 606.4 ft ON NON-CRYSTALLINE ROCK	

btm of culvert
= 604'



NCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT

12/15

WBS 17BP.10.R.19 TIP 17BP10R19 COUNTY UNION GEOLOGIST Stickney, J. K.

SITE DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST GROUND WTR (ft)

BORING NO. B2 STATION 12+33 OFFSET 6 ft LT ALIGNMENT -EL- 0 HR. Dry

COLLAR ELEV. 612.9 ft TOTAL DEPTH 6.0 ft NORTHING 433,444 EASTING 1,514,937 24 HR. NM

DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550 89% 09/02/2009 DRILL METHOD H.S. Augers HAMMER TYPE Automatic

DRILLER Smith, C. L. START DATE 10/24/11 COMP. DATE 10/24/11 SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	LOG	SOIL AND ROCK DESCRIPTION	DEPTH (ft)		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100					ELEV. (ft)	DEPTH (ft)
615																
610	609.0	3.9	14	13	7									612.9	0.0	GROUND SURFACE
														609.9	3.0	ROADWAY EMBANKMENT RED-BRN SOFT TO MED. STIFF MOIST SILTY CLAY (A-6)
														607.2	5.7	RESIDUAL TAN-BRN-GRAY V. STIFF MOIST CLAYEY SANDY SILT (A-4)
														606.9	6.0	WEATHERED ROCK SEV. WEATH. NON-CRYSTALLINE ROCK
																Boring Terminated BY AUGER REFUSAL at Elevation 606.9 ft ON NON-CRYSTALLINE ROCK

Star of Calcut
≈ 604'



NCDOT GEOTECHNICAL ENGINEERING UNIT
FIELD PENETROMETER LOG (ENGLISH)

15/15

PROJECT NUMBER	17BP.10.R.19	ID	17BP.10.R.19	CO	Union	GEO	C. Baldwin
SITE DESC	Bridge No. 366 on SR 1153 over Beaver Dam Creek West						
BORING NUMBER	SR-4	STA	12+32	OFFSET	5 FT	RT	ALIGNMENT L
ELEVATION	606.8 FT	TOTAL DEPTH	3.5 FT	NORTH	433,415	EAST	1,514,913
DRILL METHOD	Sounding Rod					DRILLER	N/A
START DATE	07/09/12	COMP DATE	07/09/12	SURFACE WTR DEPTH	N/A	FT	DEPTH TO ROCK 3.5 FT

DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT				SAMPLE NO. & INTERVAL	MOI	ORIGIN	SOIL & ROCK DESCRIPTION <small>(SOIL or ROCK NAME (w/ color, density/consistency, texture, plasticity, organics, other))</small>
	0.5 ft	0.5 ft	TOTAL	0	25	50	75				
0.0			5	X						M	Residual Brown, fine sandy SILT (A-4) Elev. 606.8ft
			15	X							
			26								
			20	X							Elev. 603.3 ft
3.5											Sounding Rod refusal at 3.5 feet

Btm. of culvert
x 604'

NOTES

SIGNATURE _____ DATE _____

NOTES

DECK TO DATUM DISTANCE N/A FT

**STRUCTURE SUBSURFACE
INVESTIGATION PROVIDED BY
NCDOT**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	17BP.10.R.19	1	8

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 17BP.10.R.19 (17BP10R19) F.A. PROJ. _____

COUNTY UNION

PROJECT DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153
(FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST

SITE DESCRIPTION _____

CONTENTS

<u>SHEET</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2	LEGEND
3	SITE PLAN
4-7	BORE LOGS
8	SAMPLES

PERSONNEL

J.K. STICKNEY

C.L. SMITH

INVESTIGATED BY J.R. BEVERLY

CHECKED BY C.B. LITTLE

SUBMITTED BY C.B. LITTLE

DATE NOVEMBER 2011

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 250-4088. NEITHER THE SUBSURFACE PLANS AND REPORTS, NOR THE FIELD BORING LOGS, ROCK CORES, OR SOIL TEST DATA ARE PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT. FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IS IT CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.



DRAWN BY: J.K. McCLURE

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION				GRADATION			
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (AASHTO T206, ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, DRY, SILTY CLAY, MOST WITH INTERBEDDED FINE SAND LAYERS, NGHT PLASTIC, A-1-6</i>				WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. ALSO POORLY GRADED. GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.			
SOIL LEGEND AND AASHTO CLASSIFICATION				ANGULARITY OF GRAINS			
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.				THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS ANGULAR , SUBANGULAR , SUBROUNDED , OR ROUNDED .			
MINERALOGICAL COMPOSITION				COMPRESSIBILITY			
SLIGHTLY COMPRESSIBLE MODERATELY COMPRESSIBLE HIGHLY COMPRESSIBLE				LIQUID LIMIT LESS THAN 31 LIQUID LIMIT EQUAL TO 31-50 LIQUID LIMIT GREATER THAN 50			
PERCENTAGE OF MATERIAL				GROUND WATER			
ORGANIC MATERIAL GRANULAR SOILS SILT - CLAY SOILS OTHER MATERIAL				WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING STATIC WATER LEVEL AFTER 24 HOURS PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA SPRING OR SEEP			
TRACE OF ORGANIC MATTER 2 - 3% LITTLE ORGANIC MATTER 3 - 5% MODERATELY ORGANIC 5 - 10% HIGHLY ORGANIC >10%				GRANULAR SOILS 3 - 5% SILT - CLAY SOILS 5 - 12% 12 - 20% >20%			
TRACE 1 - 10% LITTLE 10 - 20% SOME 20 - 35% HIGHLY 35% AND ABOVE				MISCELLANEOUS SYMBOLS			
CONSISTENCY OR DENSENESS				ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION			
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)				SPT OPT DMT VST PHT TEST BORING W/ CORE AUGER BORING CORE BORING MONITORING WELL PIEZOMETER INSTALLATION SLOPE INDICATOR INSTALLATION CONE PENETROMETER TEST SOUNDING ROD			
GENERALLY GRANULAR MATERIAL (NON-COHESIVE) VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE				<4 4 TO 10 10 TO 30 30 TO 50 >50			
GENERALLY SILT-CLAY MATERIAL (COHESIVE) VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD				<0.25 0.25 TO 0.50 0.5 TO 1.0 1 TO 2 2 TO 4 >4			
TEXTURE OR GRAIN SIZE				ABBREVIATIONS			
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.76 2.00 0.42 0.25 0.075 0.053				AR - AUGER REFUSAL BT - BORING TERMINATED CL - CLAY CPT - CONE PENETRATION TEST CSE - COARSE DMT - DILATOMETER TEST DPT - DYNAMIC PENETRATION TEST e - VOID RATIO F - FINE FOSS. - FOSSILIFEROUS FRAC. - FRACTURED, FRACTURES FRAGS. - FRAGMENTS HL - HIGHLY			
BOULDER (BLR) COBBLE (COB) GRAVEL (GRV) COARSE SAND (CSE, SD) FINE SAND (F, SOJ) SILT (SLJ) CLAY (CLJ)				MED. - MEDIUM MICA - MICACEOUS MOD. - MODERATELY NP - NON PLASTIC ORG. - ORGANIC PHT - PRESSUREMETER TEST SAP. - SAPROLITIC SD. - SAND, SANDY SL. - SILT, SILTY SLI. - SLIGHTLY TCR - TRICONE REFUSAL w - MOISTURE CONTENT V - VERY			
GRAIN SIZE MM 305 75 2.0 0.25 0.05 0.005 IN. 12 3				SAMPLE ABBREVIATIONS			
SOIL MOISTURE - CORRELATION OF TERMS				S - BULK SS - SPLIT SPOON ST - SHELBY TUBE RS - ROCK RT - RECOMPACTED TRIAXIAL CBR - CALIFORNIA BEARING RATIO			
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION				EQUIPMENT USED ON SUBJECT PROJECT			
LL - LIQUID LIMIT - SATURATED - (SAT) USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE PL - PLASTIC LIMIT - WET - (W) SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE OM - OPTIMUM MOISTURE - MOIST - (M) SOLID; AT OR NEAR OPTIMUM MOISTURE SL - SHRINKAGE LIMIT - DRY - (D) REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE				DRILL UNITS: <input type="checkbox"/> MOBILE B- <input type="checkbox"/> BK-51 <input type="checkbox"/> CME-45C <input checked="" type="checkbox"/> CME-550 <input type="checkbox"/> PORTABLE MOIST <input type="checkbox"/> <input type="checkbox"/>			
PLASTICITY				ADVANCING TOOLS: <input type="checkbox"/> CLAY BITS <input type="checkbox"/> 6" CONTINUOUS FLIGHT AUGER <input checked="" type="checkbox"/> 8" HOLLOW AUGERS <input type="checkbox"/> HARD FACED FINGER BITS <input checked="" type="checkbox"/> TUNG-CARBIDE INSERTS <input type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER <input type="checkbox"/> TRICONE _____ * STEEL TEETH <input type="checkbox"/> TRICONE _____ * TUNG-CARB. <input type="checkbox"/> CORE BIT <input type="checkbox"/>			
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY				HAMMER TYPE: <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL CORE SIZE: <input type="checkbox"/> B <input type="checkbox"/> H <input type="checkbox"/> H HAND TOOLS: <input type="checkbox"/> POST HOLE DICER <input type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST			
COLOR				DESCRIPTIONS			
PLASTICITY INDEX (PI) DRY STRENGTH				DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.			
0-5 VERY LOW 6-15 SLIGHT 16-25 MEDIUM 26 OR MORE HIGH							

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

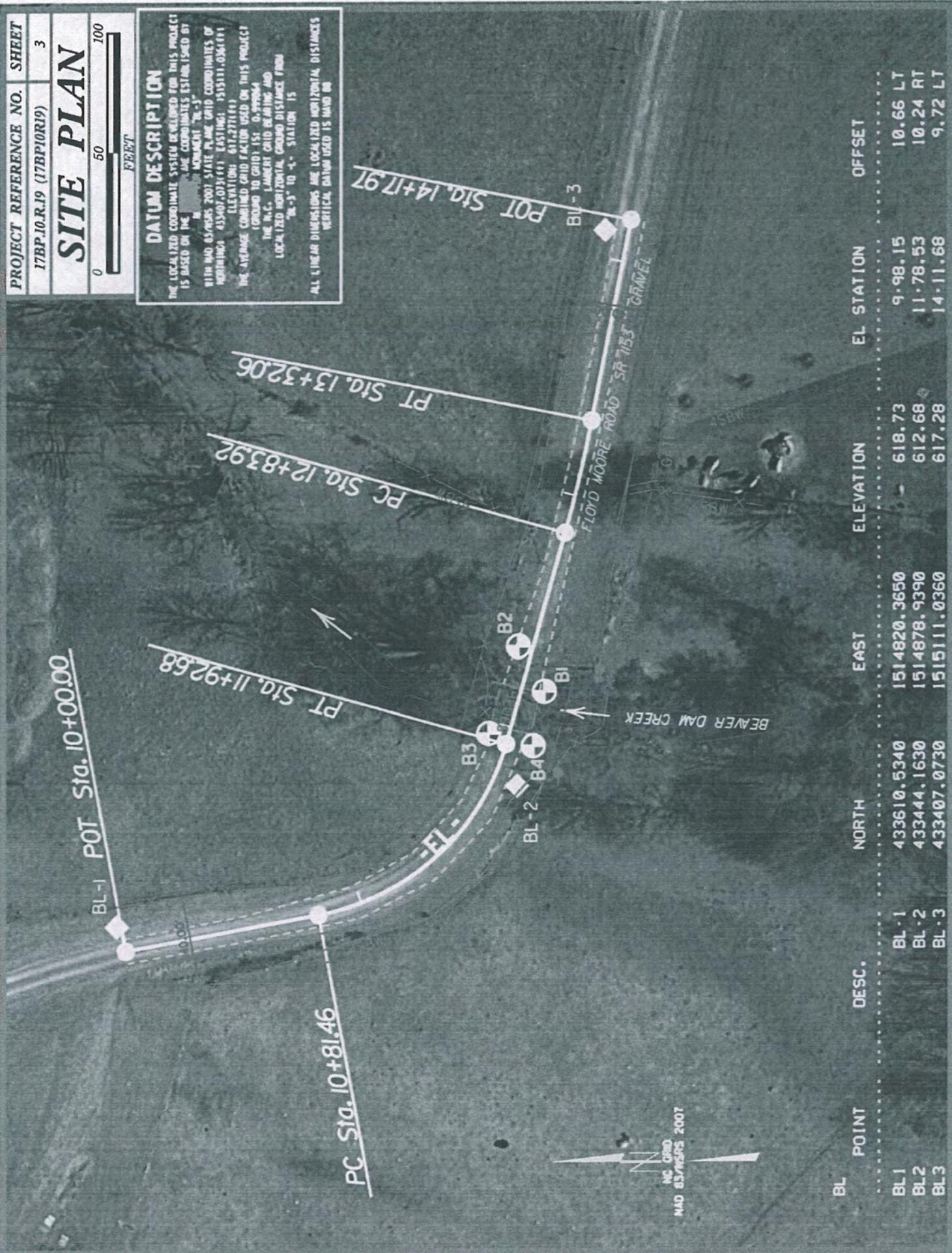
ROCK DESCRIPTION		TERMS AND DEFINITIONS	
<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>		<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS ON OR BPF OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>	
WEATHERED ROCK (WR)		NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.	
CRYSTALLINE ROCK (CR)		FINE TO COARSE GRAIN (IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.	
NON-CRYSTALLINE ROCK (NCR)		FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.	
COASTAL PLAIN SEDIMENTARY ROCK (CP)		COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.	
WEATHERING			
FRESH		ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.	
VERY SLIGHT (V SLT)		ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN. CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.	
SLIGHT (SLT)		ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.	
MODERATE (MOD)		SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.	
MODERATELY SEVERE (MOD. SEV)		ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <i>IF TESTED, WOULD YIELD SPT REFUSAL.</i>	
SEVERE (SEV)		ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <i>IF TESTED, YIELDS SPT N VALUES > 100 BPF.</i>	
VERY SEVERE (V SEV)		ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <i>IF TESTED, YIELDS SPT N VALUES < 100 BPF.</i>	
COMPLETE		ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIXES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.	
ROCK HARDNESS			
VERY HARD		CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.	
HARD		CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.	
MODERATELY HARD		CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.	
MEDIUM HARD		CAN BE GROOVED OR GOUGED 0.25 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PIECES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.	
SOFT		CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.	
VERY SOFT		CAN BE CARYED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.	
FRACTURE SPACING		BEDDING	
TERM	SPACING	TERM	THICKNESS
VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED	> 4 FEET
WIDE	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET
MODERATELY CLOSE	1 TO 3 FEET	THINLY BEDDED	0.16 - 1.5 FEET
CLOSE	0.16 TO 1 FEET	VERY THINLY BEDDED	0.03 - 0.16 FEET
VERY CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED	0.008 - 0.03 FEET
		THINLY LAMINATED	< 0.008 FEET
INDURATION			
FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.			
FRIBLE		RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.	
MODERATELY INDURATED		GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.	
INDURATED		GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.	
EXTREMELY INDURATED		SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.	
BENCH MARK: BL-1		STA. 9+98.15 -EL- 10.66 LT.	
N 433610.5340		E 1514820.3650	
		ELEVATION: 618.73 FT.	
NOTES:			

PROJECT REFERENCE NO. SHEET
 17BP.10.R.19 (17BP10R19) 3

SITE PLAN



DATUM DESCRIPTION
 THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE NAD 83 DATUM. THE COORDINATES ESTABLISHED BY THIS DATUM ARE: NAD 83 DATUM GRID COORDINATES OF NORTHING: 433407.0730 (E) EASTING: 151511.0361 (E) ELEVATION: 617.271 (E) THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (ROUND TO GRID) IS: 0.999964 THE N.C. LATENT GRID BEARING AND LOCALIZED HORIZONTAL GRID DISTANCE FROM "N-5" TO "S-1" STATION IS ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES VERTICAL DATUM USED IS NAVD 83



BL POINT	DESC.	NORTH	EAST	ELEVATION	EL STATION	OFFSET
BL-1	BL-1	433610.5340	1514820.3650	618.73	9+98.15	10.66 LT
BL-2	BL-2	433444.1630	1514878.9390	612.68	11+78.53	10.24 RT
BL-3	BL-3	433407.0730	151511.0360	617.28	14+11.68	9.72 LT



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 17BP.10.R.19	TIP 17BP10R19	COUNTY UNION	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST			GROUND WTR (ft)
BORING NO. B1	STATION 12+18	OFFSET 10 ft RT	ALIGNMENT -EL-
COLLAR ELEV. 612.6 ft	TOTAL DEPTH 6.2 ft	NORTHING 433,433	EASTING 1,514,918
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009			DRILL METHOD H.S. Augers
DRILLER Smith, C. L.			HAMMER TYPE Automatic
START DATE 10/24/11		COMP. DATE 10/24/11	SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
615																
610	608.7	3.9	2	2	3	•••••	•••••	•••••	•••••	•••••				612.6	0.0	GROUND SURFACE
						•••••	•••••	•••••	•••••	•••••				609.6	3.0	ROADWAY EMBANKMENT RED-BRN SOFT TO MED. STIFF MOIST SILTY CLAY (A-6)
						•••••	•••••	•••••	•••••	•••••	SS-1	M		607.3	5.3	ALLUVIAL TAN-GRAY MED. STIFF MOIST MED. (PI=20) PLASTIC SILTY SANDY CLAY (A-6)
						•••••	•••••	•••••	•••••	•••••				606.4	6.2	RESIDUAL TAN-BRN-GRAY STIFF TO V. STIFF MOIST CLAYEY SANDY SILT (A-4)
																Boring Terminated BY AUGER REFUSAL at Elevation 606.4 ft ON NON-CRYSTALLINE ROCK

NCDOT BORE SINGLE 0908.000_GEO_BH_BRDG0366_UNION.GPJ NC_DOT.GDT 11/22/11



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 17BP.10.R.19	TIP 17BP10R19	COUNTY UNION	GEOLOGIST Stickney, J. K.
SITE DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST			GROUND WTR (ft)
BORING NO. B2	STATION 12+33	OFFSET 6 ft LT	ALIGNMENT -EL-
COLLAR ELEV. 612.9 ft	TOTAL DEPTH 6.0 ft	NORTHING 433,444	EASTING 1,514,937
DRILL RIG/HAMMER EFF./DATE HFO0072 CME-550 89% 09/02/2009			DRILL METHOD H.S. Augers
DRILLER Smith, C. L.		START DATE 10/24/11	COMP. DATE 10/24/11
			HAMMER TYPE Automatic
			SURFACE WATER DEPTH N/A

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION		
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)	
615																
															612.9	0.0
610															609.9	3.0
	609.0	3.9														
			14	13	7											
															607.2	5.7
															606.9	6.0

NCDOT BORE SINGLE 09080000_GEO_BH_BRDGG0386_UNION.GPJ_NC_DOT.GDT 11/22/11

ROADWAY EMBANKMENT
 RED-BRN SOFT TO MED. STIFF MOIST
 SILTY CLAY (A-6)
 RESIDUAL
 TAN-BRN-GRAY V. STIFF MOIST
 CLAYEY SANDY SILT (A-4)
 WEATHERED ROCK
 SEV. WEATH. NON-CRYSTALLINE ROCK
 Boring Terminated BY AUGER REFUSAL
 at Elevation 606.9 ft ON
 NON-CRYSTALLINE ROCK



NCDOT GEOTECHNICAL ENGINEERING UNIT

BORELOG REPORT

WBS 17BP.10.R.19		TIP 17BP10R19		COUNTY UNION		GEOLOGIST Stickney, J. K.											
SITE DESCRIPTION REPLACE BRIDGE NO. 366 ON SR 1153 (FLOYD MOORE RD.) OVER BEAVER DAM CREEK WEST							GROUND WTR (ft)										
BORING NO. B3		STATION 11+95		OFFSET 8 ft LT		ALIGNMENT -EL-											
COLLAR ELEV. 613.0 ft		TOTAL DEPTH 8.5 ft		NORTHING 433,456		EASTING 1,514,901											
DRILL RIG/HAMMER EFF./DATE HFC0072 CME-550 89% 09/02/2009				DRILL METHOD H.S. Augers		HAMMER TYPE Automatic											
DRILLER Smith, C. L.		START DATE 10/24/11		COMP. DATE 10/24/11		SURFACE WATER DEPTH N/A											
ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION			
			0.5ft	0.5ft	0.5ft	0	25	50	75	100				ELEV. (ft)	DEPTH (ft)		
615															613.0	GROUND SURFACE	0.0
610															610.0	ROADWAY EMBANKMENT RED-BRN SOFT TO MED. STIFF MOIST SILTY CLAY (A-6)	3.0
	609.0	4.0		1	2	3									606.5	ALLUVIAL TAN-GRAY MED. STIFF LOW (PI=15) PLASTIC SANDY SILTY CLAY (A-6)	6.5
605															605.3	RESIDUAL	7.7
															604.5	TAN-BRN-GRAY STIFF TO V. STIFF MOIST CLAYEY SANDY SILT (A-4)	8.5
																WEATHERED ROCK SEV. WEATH. NON-CRYSTALLINE ROCK	
																Boring Terminated BY AUGER REFUSAL at Elevation 604.5 ft ON NON-CRYSTALLINE ROCK	

NCDOT BORE SINGLE 0808000_GEO_BH_BRDGG0386_UNION.GPJ NC_DOT.GDT 11/22/11

**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAY
MATERIALS & TESTS UNIT
SOILS LABORATORY**

T. I. P. No. 17BP10R19**REPORT ON SAMPLES OF SOILS FOR QUALITY**

Project 17BP.10.R.19 County UNION Owner _____
 Date: Sampled 10/26/11 Received 11/1/11 Reported 11/3/11
 Sampled from BRIDGE By J E BEVERLY
 Submitted by N WAINAINA 1995 Standard Specifications

775235 TO 775237
11/22/11

TEST RESULTS

Proj. Sample No.		SS-1	SS-2	SS-3		
Lab. Sample No.		775235	775236	775237		
Retained #4 Sieve	%	-	-	-		
Passing #10 Sieve	%	96	98	99		
Passing #40 Sieve	%	81	83	96		
Passing #200 Sieve	%	61	51	87		

MINUS NO. 10 FRACTION

SOIL MORTAR - 100%						
Coarse Sand Ret - #60	%	24.0	30.7	5.2		
Fine Sand Ret - #270	%	16.1	20.6	10.7		
Silt 0.05 - 0.005 mm	%	23.5	20.5	39.7		
Clay < 0.005 mm	%	36.3	28.3	44.4		
Passing #40 Sieve	%	-	-	-		
Passing #200 Sieve	%	-	-	-		

L. L.		37	22	34		
P. I.		20	7	15		
AASHTO Classification		A-6(9)	A-4(1)	A-6(12)		
Station		12+18	12+33	11+95		
Offset		9.5 RT.	6.0 LT.	8.2 LT.		
Alignment		EL	EL	EL		
Location		B1	B2	B3		
Depth (Ft)		4.40	4.40	4.50		
	to	5.40	5.40	5.50		

cc: J E BEVERLY

Soils Engineer

**SUPPLEMENTAL STRUCTURE
SUBSURFACE INVESTIGATION
PROVIDED BY AMEC**

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	17BP.10.R.19	1	14

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE
SUBSURFACE INVESTIGATION

PROJ. REFERENCE NO. 17BP.10.R.19 F.A. PROJ. N/A
COUNTY UNION
PROJECT DESCRIPTION DIVISION 10 GROUP N BRIDGE
REPLACEMENT
SITE DESCRIPTION REPLACE BRIDGE 890366 ON SR 1153 (FLOYD
MOORE ROAD) OVER BEAVER DAM CREEK WEST

CONTENTS

<u>SHEET</u>	<u>DESCRIPTION</u>
1	TITLE SHEET
2-2A	LEGEND SHEETS
3	SITE PLAN
4-9	BORING LOGS
10-13	SOUNDING ROD LOGS

PERSONNEL

F. Cox

D. Rhodes

R. Clark

INVESTIGATED BY AMEC E&I, Inc.

CHECKED BY S. Johnson, P.G. P.E.

SUBMITTED BY M. Lear, P.G.

DATE July 2012

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF PREPARING THE SCOPE OF WORK TO BE INCLUDED IN THE REQUEST FOR PROPOSAL. THE VARIOUS FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N.C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES, AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

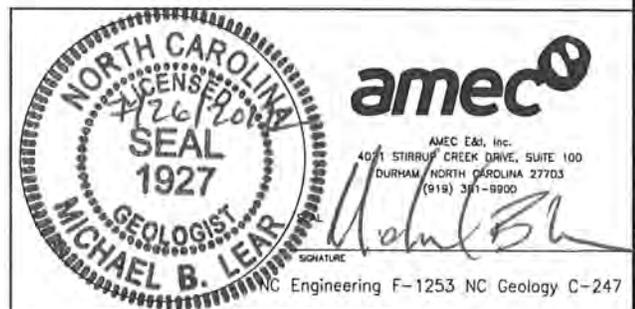
SOIL AND ROCK BOUNDARIES WITHIN A BOREHOLE ARE BASED ON GEOTECHNICAL INTERPRETATION UNLESS ENCOUNTERED IN A SAMPLE. INTERPRETED BOUNDARIES MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN SAMPLED STRATA, AND BOREHOLE INFORMATION MAY NOT NECESSARILY REFLECT ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

NOTE - THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT OF TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, SPECIFICATIONS, OR CONTRACT FOR THE PROJECT.

NOTE - BY HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

DRAWN BY: R. Rahie



**NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS**

SOIL DESCRIPTION										GRADATION									
SOIL IS CONSIDERED TO BE THE UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER, AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO STANDARD PENETRATION TEST (ASTM D-1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY SHALL INCLUDE: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. EXAMPLE: <i>VERY STIFF, GRAN. SILTY CLAY, MOIST WITH INTERLOCKED FINE SAND LIVERS, HIGH PLASTIC, A-7-6</i>										WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. UNIFORM - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. (ALSO POORLY GRADED) GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLES OF TWO OR MORE SIZES.									
SOIL LEGEND AND AASHTO CLASSIFICATION										ANGULARITY OF GRAINS									
GENERAL CLASS. GRANULAR MATERIALS (<= 35% PASSING #200) SILT-CLAY MATERIALS (> 35% PASSING #200) ORGANIC MATERIALS										THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.									
MINERALOGICAL COMPOSITION										COMPRESSIONIBILITY									
MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.										SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GREATER THAN 50									
PERCENTAGE OF MATERIAL										GROUND WATER									
ORGANIC MATERIAL GRANULAR SILT - CLAY OTHER MATERIAL																			
TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% HIGHLY ORGANIC >10% >20% HIGHLY 35% AND ABOVE																			
CONSISTENCY OR DENSENESS										MISCELLANEOUS SYMBOLS									
PRIMARY SOIL TYPE COMPACTNESS OR CONSISTENCY RANGE OF STANDARD PENETRATION RESISTANCE (N-VALUE) RANGE OF UNCONFINED COMPRESSIVE STRENGTH (TONS/FT ²)																			
TEXTURE OR GRAIN SIZE										ABBREVIATIONS									
U.S. STD. SIEVE SIZE OPENING (MM) 4 10 40 60 200 270 4.75 2.00 0.42 0.25 0.075 0.053										AR - AUGER REFUSAL MED. - MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA - MICACEOUS WEA. - WEATHERED CL - CLAY MOD. - MODERATELY WGT. - UNIT WEIGHT CPT - CONE PENETRATION TEST NP - NON PLASTIC W _g - DRY UNIT WEIGHT CSE. - COARSE DRG. - ORGANIC DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST DPT - DYNAMIC PENETRATION TEST SAP. - SAPROLITIC e - VOID RATIO SD. - SAND, SANDY F - FINE SL. - SILT, SILTY FOSS. - FOSSILIFEROUS SLI. - SLIGHTLY FRAC. - FRACTURED, FRACTURES TCR - TRICONE REFUSAL FRAGS. - FRAGMENTS w - MOISTURE CONTENT HL - HIGHLY V - VERY									
SOIL MOISTURE - CORRELATION OF TERMS										EQUIPMENT USED ON SUBJECT PROJECT									
SOIL MOISTURE SCALE (ATTERBERG LIMITS) FIELD MOISTURE DESCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION										DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: <input type="checkbox"/> MOBILE B- <input type="checkbox"/> CLAY BITS <input checked="" type="checkbox"/> AUTOMATIC <input type="checkbox"/> MANUAL <input type="checkbox"/> BK-51 <input type="checkbox"/> 6' CONTINUOUS FLIGHT AUGER <input checked="" type="checkbox"/> CME-45C <input type="checkbox"/> 8' HOLLOW AUGERS <input type="checkbox"/> CME-550 <input type="checkbox"/> HARD FACED FINGER BITS <input type="checkbox"/> PORTABLE HOIST <input type="checkbox"/> TUNG-CARBIDE INSERTS <input type="checkbox"/> <input type="checkbox"/> CASING <input type="checkbox"/> W/ ADVANCER <input type="checkbox"/> <input checked="" type="checkbox"/> TRICONE 2 7/8" STEEL TEETH <input type="checkbox"/> <input type="checkbox"/> TRICONE " TUNG.-CARB. <input type="checkbox"/> <input checked="" type="checkbox"/> CORE BIT <input type="checkbox"/> <input type="checkbox"/> CORE BIT									
PLASTICITY										HAND TOOLS:									
NONPLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY PLASTICITY INDEX (PI) DRY STRENGTH VERY LOW SLIGHT MEDIUM HIGH										<input type="checkbox"/> POST HOLE DIGGER <input type="checkbox"/> HAND AUGER <input type="checkbox"/> SOUNDING ROD <input type="checkbox"/> VANE SHEAR TEST									
COLOR										DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.									

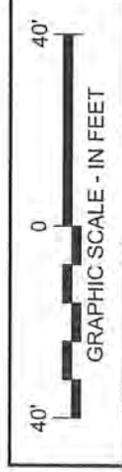
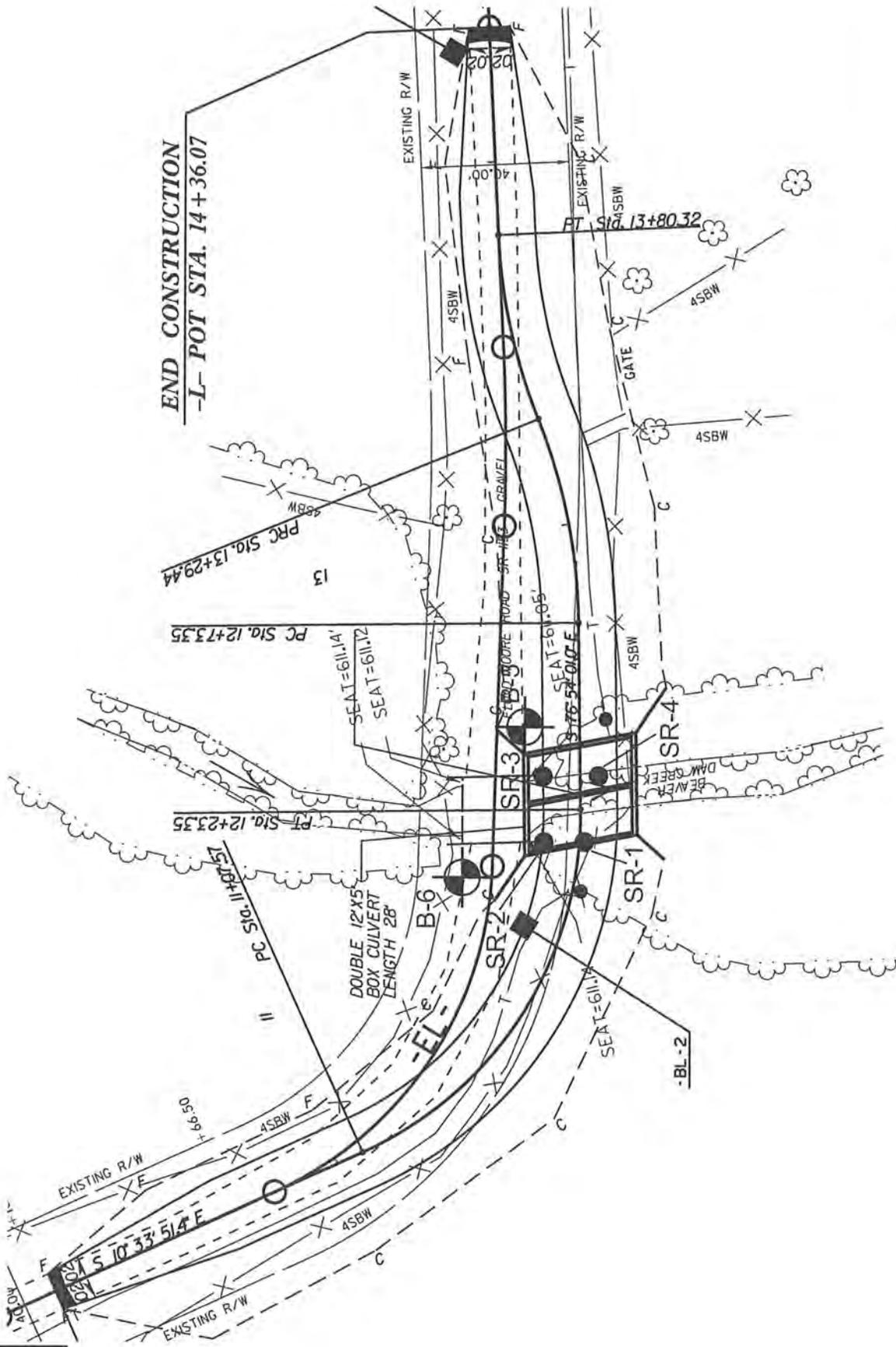
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT
SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

ROCK DESCRIPTION		TERMS AND DEFINITIONS	
<p>HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:</p>		<p>ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. AQUIFER - A WATER BEARING FORMATION OR STRATA. ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATE, ETC. ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE. CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK. DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLOGGED FROM PARENT MATERIAL. FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD. JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT. LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM. RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK. SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE. STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS IN OR BPF) OF A 148 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.</p>	
<p>WEATHERED ROCK (WR)</p> 	<p>NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.</p>		
<p>CRYSTALLINE ROCK (CR)</p> 	<p>FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.</p>		
<p>NON-CRYSTALLINE ROCK (NCR)</p> 	<p>FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.</p>		
<p>COASTAL PLAIN SEDIMENTARY ROCK (CPS)</p> 	<p>COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.</p>		
WEATHERING			
FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER HAMMER IF CRYSTALLINE.		
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.		
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.		
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.		
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. <u>IF TESTED, WOULD YIELD SPT REFUSAL</u>		
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT, SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. <u>IF TESTED, YIELDS SPT N VALUES > 100 BPF</u>		
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT THE MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE SUCH THAT ONLY MINOR VESTIGES OF THE ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, YIELDS SPT N VALUES < 100 BPF</u>		
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.		
ROCK HARDNESS			
VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.		
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.		
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.		
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.		
SOFT	CAN BE GROOVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.		
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.		
FRACTURE SPACING		BEDDING	
TERM	SPACING	TERM	THICKNESS
VERY WIDE	MORE THAN 10 FEET	VERY THICKLY BEDDED	> 4 FEET
WIDE	3 TO 10 FEET	THICKLY BEDDED	1.5 - 4 FEET
MODERATELY CLOSE	1 TO 3 FEET	THINLY BEDDED	0.16 - 1.5 FEET
CLOSE	0.16 TO 1 FEET	VERY THINLY BEDDED	0.03 - 0.16 FEET
VERY CLOSE	LESS THAN 0.16 FEET	THICKLY LAMINATED	0.008 - 0.03 FEET
		THINLY LAMINATED	< 0.008 FEET
INDURATION			
FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.			
FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.		
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.		
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.		
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.		
		<p>BENCH MARK: NCDOT REBAR & CAP STAMPED BL-3 LOCATED AT STATION 14+11.68 (-EL-), 9.72 LT ELEVATION: 617.28 FT.</p>	
NOTES:			

SHEET NO. 3
 W.B.S. NO.: 17BP.10.R.19
 T.I.P. NO.: N/A
 COUNTY: UNION



DESCRIPTION:
 REPLACE BRIDGE 890366 ON SR 1153
 (FLOYD MOORE ROAD) OVER BEAVER
 DAM CREEK WEST





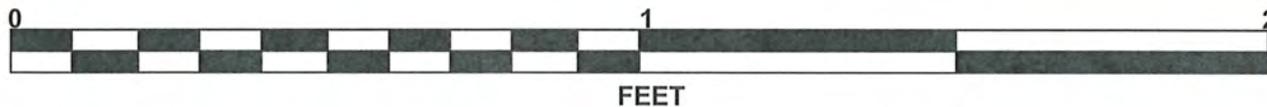
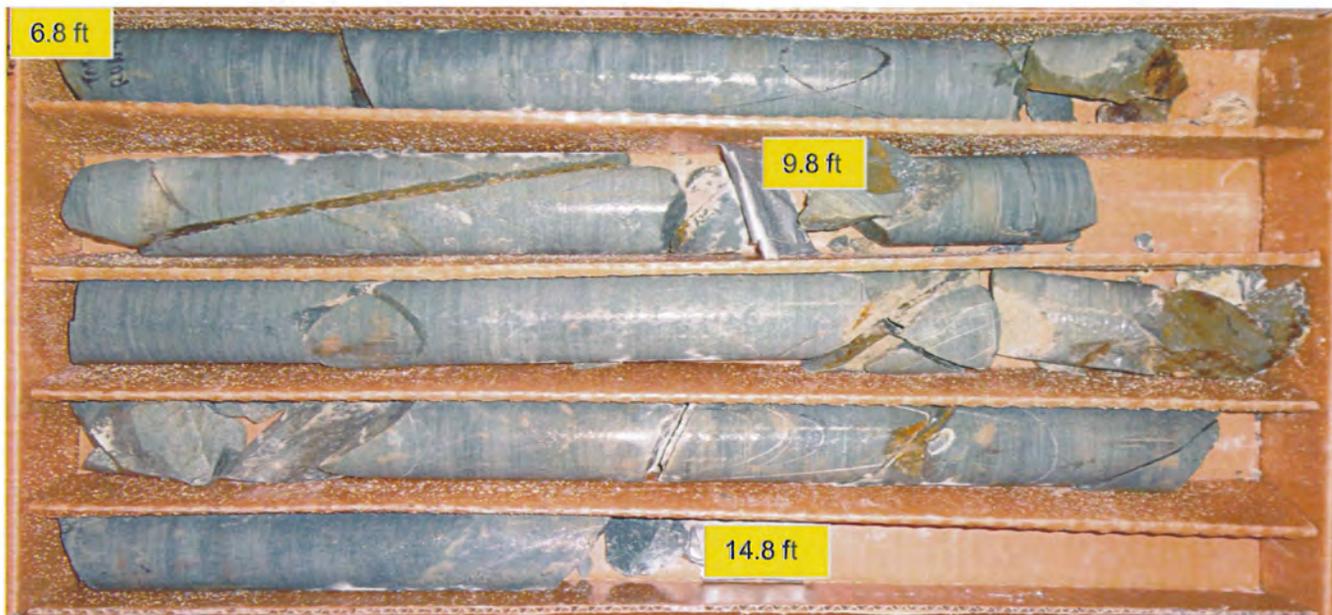
NCDOT GEOTECHNICAL ENGINEERING UNIT CORE BORING REPORT

WBS 17BP.10.R.19		TIP 17BP.10.R.19		COUNTY UNION		GEOLOGIST R. Clark						
SITE DESCRIPTION Replace Bridge 890366 on SR 1153 (Floyd Moore Road) over Beaver Dam Creek									GROUND WTR (ft)			
BORING NO. B-5		STATION 12+46		OFFSET 15 ft LT		ALIGNMENT -L-		0 HR. 1.2				
COLLAR ELEV. 612.4 ft		TOTAL DEPTH 14.8 ft		NORTHING 433,431		EASTING 1,514,931		24 HR. 2.5				
DRILL RIG/HAMMER EFF./DATE MAC9354 CME-45C 81% 03/01/11					DRILL METHOD SPT Core Boring			HAMMER TYPE Automatic				
DRILLER F. Cox		START DATE 05/15/12		COMP. DATE 05/16/12		SURFACE WATER DEPTH N/A						
CORE SIZE NQ		TOTAL RUN 8.0 ft										
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		SAMP. NO.	STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (ft) %	RQD (ft) %		REC. (ft) %	RQD (ft) %			
605.6											Begin Coring @ 6.8 ft	
	605.6	6.8	3.0	3:31	(2.9)	(1.4)		(7.8)	(3.5)		605.6 Greenish gray, moderately to slightly weathered, moderately hard to hard, close fracture spacing, METAVOLCANIC ROCK Joint sets at 45° - 65°	6.8
	602.6	9.8		3:48 4:05	97%	47%						
600			5.0	3:30 4:25 3:45	(4.9) 98%	(2.1) 42%						
	597.6	14.8		4:15 4:55							Boring Terminated at Elevation 597.6 ft in Non-Crystalline Rock: METAVOLCANIC ROCK Auger refusal at 6.7 feet.	14.8

CORE PHOTOGRAPHS

B-5

BOX 1: 6.8 - 14.8 FEET





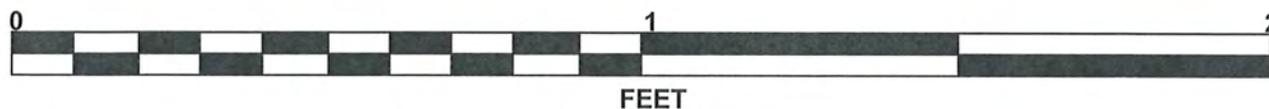
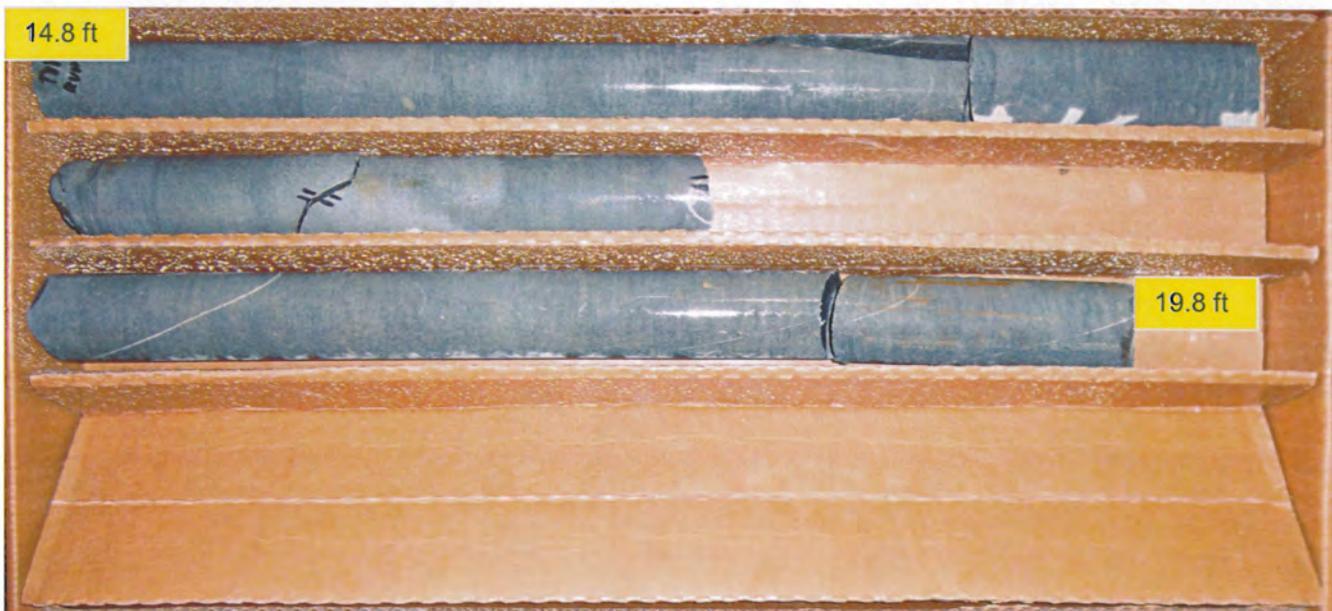
NCDOT GEOTECHNICAL ENGINEERING UNIT CORE BORING REPORT

WBS 17BP.10.R.19		TIP 17BP.10.R.19		COUNTY UNION		GEOLOGIST R. Clark					
SITE DESCRIPTION Replace Bridge 890366 on SR 1153 (Floyd Moore Road) over Beaver Dam Creek							GROUND WTR (ft)				
BORING NO. B-6		STATION 11+97		OFFSET 30 ft LT		ALIGNMENT -L-					
COLLAR ELEV. 612.9 ft		TOTAL DEPTH 19.8 ft		NORTHING 433,457		EASTING 1,514,895					
DRILL RIG/HAMMER EFF./DATE MAC9354 CME-45C 81% 03/01/11				DRILL METHOD SPT Core Boring		HAMMER TYPE Automatic					
DRILLER F. Cox		START DATE 05/15/12		COMP. DATE 05/15/12		SURFACE WATER DEPTH N/A					
CORE SIZE NQ		TOTAL RUN 11.0 ft									
ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN		STRATA		LOG	DESCRIPTION AND REMARKS	DEPTH (ft)
					REC. (%)	RQD (%)	REC. (%)	ROD (%)			
604.1											
	604.1	8.8	1.0	N=60/0.0	(0.9)	(0.0)	(10.9)	(7.1)		Begin Coring @ 8.8 ft	
	603.1	9.8	5.0	3:50	90%	0%	99%	65%		NON-CRYSTALLINE ROCK	8.8
				3:13						Greenish gray, moderately to slightly weathered, moderately hard to hard, very close to moderately close fracture spacing, METAVOLCANIC ROCK	
				2:47	(5.0)	(2.3)				Joint sets at 45° - 65° with some staining in fractures	
600				3:18	100%	46%					
	598.1	14.8		3:05							
			5.0	3:31							
				3:27	(5.0)	(4.8)					
595				3:38	100%	96%					
				3:51							
	593.1	19.8		3:41							
				4:02							
										Boring Terminated at Elevation 593.1 ft in Non-Crystalline Rock: METAVOLCANIC ROCK	19.8
										Auger refusal at 8.8 feet.	

CORE PHOTOGRAPHS

B-6

BOXES 1 & 2: 8.8 - 19.8 FEET





NCDOT GEOTECHNICAL ENGINEERING UNIT
FIELD PENETROMETER LOG (ENGLISH)

PROJECT NUMBER	17BP.10.R.19	ID	17BP.10.R.19	CO	Union	GEO	C. Baldwin
SITE DESC	Bridge No. 366 on SR 1153 over Beaver Dam Creek West						
BORING NUMBER	SR-1	STA	12+15	OFFSET	1 FT	RT	ALIGNMENT L
ELEVATION	611.7 FT	TOTAL DEPTH	6.0 FT	NORTH	433,423	EAST	1,514,897
DRILL METHOD	Sounding Rod/Hand Auger					DRILLER	N/A
START DATE	07/09/12	COMP DATE	07/09/12	SURFACE WTR DEPTH	N/A	FT	DEPTH TO ROCK 6.0 FT

DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT				SAMPLE NO. & INTERVAL	MOI	ORIGIN	SOIL & ROCK DESCRIPTION	
	0.5 ft	0.5 ft	TOTAL	0	25	50	75				100	SOIL or ROCK NAME (w/ color, density/consistency, texture, plasticity, organics, other)
0.0			17		X					S-1 0.0 - 4.0 ft	D	Residual Brown, fine sandy SILT (A-4) Elev. 611.7 ft
			15		X							
			17		X							
			28			X						
			25		X							
			31			X						
6.0												Sounding Rod refusal at 6.0 feet

NOTES _____

SIGNATURE _____ DATE _____
 NOTES _____

DECK TO DATUM DISTANCE N/A FT



NCDOT GEOTECHNICAL ENGINEERING UNIT
FIELD PENETROMETER LOG (ENGLISH)

PROJECT NUMBER	17BP.10.R.19	ID	17BP.10.R.19	CO	Union	GEO	C. Baldwin
SITE DESC	Bridge No. 366 on SR 1153 over Beaver Dam Creek West						
BORING NUMBER	SR-2	STA	12+14	OFFSET	10 FT	LT	ALIGNMENT L
ELEVATION	611.1 FT	TOTAL DEPTH	4.5 FT	NORTH	433,434	EAST	1,514,899
DRILL METHOD	Sounding Rod					DRILLER	N/A
START DATE	07/09/12	COMP DATE	07/09/12	SURFACE WTR DEPTH	N/A	FT	DEPTH TO ROCK 4.5 FT

DEPTH (ft)	BLOW COUNT			BLOWS PER FOOT					SAMPLE NO. & INTERVAL	MOI	ORIGIN	SOIL & ROCK DESCRIPTION <small>(w/ color, density/consistency, texture, plasticity, organics, other)</small>
	0.5 ft	0.5 ft	TOTAL	0	25	50	75	100				
0.0			8	X						D-M	Residual	Brown, fine sandy SILT (A-4) Elev. 611.1 ft
			11	X								
			16	X								
			27	X								Elev. 606.6 ft
			28	X								
4.5												Sounding Rod refusal at 4.5 feet

NOTES _____

SIGNATURE _____ DATE _____
 NOTES _____

DECK TO DATUM DISTANCE N/A FT

