ATTACHMENT A – GEOTECHNICAL

The following Geotechnical Structure Subsurface Investigations are for information only and are not a part of this contract. This information is for investigation only and no accuracy is implied or guaranteed. No claim will be allowed as a result of the use of this information.

		the transmission of the second s	T REFERENCE NO. SHEET SHEET (BD-5111V) 1 15
		14.0. 43337.1.20	
	STATE OF NORTH DEPARTMENT OF TRANS DIVISION OF HIGH GEOTECHNICAL ENGINEE	PORTATION VAYS	
	STRUCTU		
	SUBSURFACE INV		N
	PROJ. REFERENCE NO. <u>45357.1.26 (BD-5111V)</u> COUNTY SURRY		
	PROJECT DESCRIPTION <u>REPLACE BRIDGE</u>	NO. 244 ON -L-	
	(SR 1809, OLD WESTFIELD RD) OVER		
	SITE DESCRIPTION		
CONTI SHEET	ENTS DESCRIPTION		PERSONNEL C. NORVILLE
1	TITLE SHEET		J. R. HAMM
2-2A 3-5	NCDOT DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT SOIL AND ROCK LEGEND, TERMS, SYMBOLS AND ABBREVIATI FOUNDATION RECOMMENDATIONS, PLAN NOTES AND PAY ITEM QU		T. E. EVANS
6	SITE PLAN		TRIGON EXP.
7-14	BORING LOGS, CORE LOGS AND CORE PHOTOS		
	APPENDIX - STRUCTURE FOUNDATION LOADS		
			BY <u>T. E. EVANS</u>
			J. R. HAMM
		SUBMITTED BY	FALCON ENG MARCH 2013
	CAUTION NOT	DATE	
THE VAR	ISURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF ROUS FELD BORNOL LOGS, ROCK CORES, AND SOL TEST DATA AVALABLE MAY BE REVEWED OR INSPECTED IN RALECH MOLL ENDIRECTING UNIT AT (1992) 260-008. NETHER THE SUBSURFACE PLANS AND DERPORTS, NOR THE FELD BORNOL U	STUDY, PLANNING, AND DESIGN, AND NOT FOR CONSTRUCTION OR P BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION,	
REFLECT RELIED (INVESTIG	L SOIL AND ROCK STRATA DESCRPTIONS AND INDICATED BOLNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION T THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORNOS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD, THE OBSERVED WATER LEVELS O ATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION THESE WATER LEVELS OR SOL MOISTURE CONDITIONS ATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION THESE WATER LEVELS OR SOL MOISTURE CONDITIONS ATURES, PRECIPITATION, AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.	LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DA OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE	TA CAN BE
AND CO OR ACC CONTRA CONTRA	DER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN NSTRUCTOR PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON TH URACY OF THE WINSTRUCTION MORE, NOR THE INTERPRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE T CTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY CTOR SHALL HAVE NO CLAM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESUL MOCATED N THE SUBSURFACE INFORMATION.	S PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTE YPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BI HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THIS PROJE	E THE SUFFICIENCY DDER OR CT. THE
			CARO
OF 1	INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N.C. DEPARTMENT TRANSPORTATION AS BEING ACCURATE NOR IT IS CONSIDERED TO BE PART OF THE PLANS, JFICATIONS, OR CONTRACT FOR THE PROJECT.	A Start Carlos	SSIONA
FOR	HAVING REQUESTED THIS INFORMATION THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE ITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.	Juff 3	19779 29/13
WN BY: T.E.	EVANS	CHILEM	R. HAMIN

							PROJECT REFERENCE NO. SHEET N 45357,1,26 (BD-5111V) 2
so	DIL AN		GI	E COTEC	NVISION HNIC	ON O AL E	MENT OF TRANSPORTATION F HIGHWAYS NGINEERING UNIT S, SYMBOLS, AND ABBREVIATIONS
	SOIL D	ESCRIP'	TION				GRADATION
SOIL IS CONSIDERED TO BE THE UNC THAT CAN BE PERETRATED WITH A CI 1908 BLOWS PER FOOT ACCORDING TO CLASSIFICATION IS BASED ON THE AR CONSISTENCY, COLOR, TEXTURE, MOISTL AS MINERALOGICAL COMPOSITION, ANG WR STFF, RW, SIT	Intinuous flig Standard Pene ISHTO System, I RE, AASHTO CLA ILARITY, STRUCT	IT POWER A TRATION TE BASIC DESC SSIFICATION URE, PLASTI	NUGER, AND YIELI ST (AASHTO T26 RIPTIONS GENER N, AND OTHER PE	d less than 16, astm d-1: Ally Shall RTINENT FAC PLE:	i 586), soil Include:		Nell braned - indicates a codo representation of particle sizes from fine to coarse, landown - indicates that soil particles are all approximately the same size, (also porly graned) Carly Graned) - indicates a mixture of uniform particles of two or more sizes, ANGUL ARTY OF GRAINS ANGULARITY OF ROLINDESS OF SOIL GRAINS - indicates a mountees of soil grains is designated by the terms <u>angular</u> , <u>subangular</u> , <u>subrounded</u> , or <u>rounded</u> .
SOIL LEGE				CATION			MINERALOGICAL COMPOSITION MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAQLIN, ETC, ARE USED IN DESCRIPTIONS
GENERAL GRANULAR MATE CLASS. (≤ 35% PASSING	*299)	(> 35%	AY MATERIALS PASSING •200)		NIC MATER	IALS	WHENEVER THEY ARE CONSIDERED OF SIGNIFICANCE.
GROUP A-1 A-3 CLASS. A-1-e A-1-b A-2-4 SYMBOL A-2-4 A-2-4	A-2 A-2-5 A-2-6 A-2	-7	A-5 A-6 A-7 A-7-8 A-7-8 A-7-8	0-3	A-4, A-5 A-6, A-7		COMPRESSIBILITY SLIGHTLY COMPRESSIBLE LIQUID LIMIT LESS THAN 31 MODERATELY COMPRESSIBLE LIQUID LIMIT EQUAL TO 31-50 HIGHLY COMPRESSIBLE LIQUID LIMIT GRAFTER THAN 50
PASSING 10 58 MX				GRANULAR	SILT- CLAY	MUCK,	
• 48 39 HX 59 HX 51 HN • 289 15 HX 25 HX 18 HX 35 HX 19000 LIMIT 48 HX	41 MN 40 MX 41 M	W 48 MX 41	1 MN 48 MX 41 M		SOILS	PEAT	ORGANIC MATERIAL SULS DTHER MATERIAL TRACE OF ORGANIC MATTER 2 - 32 3 - 52 TRACE 1 - 182 LITTLE ORGANIC MATTER 3 - 52 5 - 122 LITTLE 18 - 282 MODEPATELY ORGANIC MATTER 3 - 52 5 - 122 LITTLE 18 - 282 MODEPATELY ORGANIC 5 - 184 12 - 284 SOME 28 - 352
LASTIC INDEX 6 MX NP 18 MX ROUP INDEX 8 8 8	18 MOX 11 MN 11 M	N 18 MX 18	8 MX 11 MN 11 MN 2 MX 16 MX No M		E OR	HIGHLY ORGANIC	HIGHLY ORGANIC >18% >28% HIGHLY 35% AND ABOVE
F MAJOR GHATEL, ANU COND CRA	y or clayey Vel and sand	SILT		ORGAN	IIC	SOILS	✓ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING ✓ STATIC WATER LEVEL AFTER 24 HOURS
HATERIALS SAND UNITED CAN . EN.RATING AS A EXCELLENT TO SUBGRADE	GOOD	FAI	IR TO POOR	FAIR TO POOR	POOR	UNSULTABLE	VPM PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
PI OF A-7-5 SUBGROUP				Roup is >	LL - 30		
	TNESS OR	RANGE	DENSENESS OF STANDARD ON RESISTENCE		OF LINCONF		
GENERALLY VERY GRANULAR LOO MATERIAL DEN INNO-COLESTVE) DEN	M DENSE	(N) 4 16	-VALUE) <4 TO 18 TO 38 TO 50 >58	(TONS/FT ²)			
MATERIAL STI	T M STIFF FF STIFF	4 8 15	(2 TO 4 TO 8 TO 15 TO 38 >30		<pre><0.25 0.25 TO 0.1 0.5 TO 1.0 1 TO 2 2 TO 4 >4</pre>		INFERRED SOIL BOUNDARY MONITORING WELL PIEZOMETER INSTALLATION SOIL BOUNDARY SOIL BOUNDARY
a los comos as tenentes de como los es	EXTURE (and the second	and	a contract	2.2		
J.S. STD. SIEVE SIZE OPENING (MM)	4 18		10000 0000	state and state and state			SOUNDING ROD ABBREVIATIONS
BOULDER (BLDR.) COBBLE (COB.) GRAIN MM 365 75 SIZE INL 12 3	GRAVEL (GR.) 2.8		5AN D.1 (F 5 Ø.25	0 0.05 0.05	SILT (SL.) 6.005	CLAY (CL.)	AR Auger REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT BORING TERMINATED MICA MICACEOUS VEA VEATHERED CL CLAY MICA MUCATELY γ' - UNIT WEIGHT CPT COMP PORTATION TEST NP - NON PLASTIC γ'_d - DRY UNIT WEIGHT CSE CLARSE ORG TORGANIC γ'_d - DRY UNIT WEIGHT CSE CLARSE ORG TORGANIC γ'_d - DRY UNIT WEIGHT SAMPLE ABBREVIATION
SOIL MOIS SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MO DESCRIF	DISTURE PTION		FIELD MOI			DPT - DYNAMIC PENETRATION TEST SAP SAPPOLITIC S - BULK e - YOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SLIT, SLITY ST - SHELBY TUBE
	- Satur (Sat,		FROM BEL	L10UID; VER' OW THE GR	ound wate	er table	FOSSFOSSILIFEROUS SLISLIGHTLY RS - ROCK FRACFRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAX FRAGEFRAGMENTS W - MOISTURE CONTENT CBR - CALIFORNIA BEARIN HLHIGHLY V - VERY RATIO
RANGE ((PI) PL PLASTIC LIMIT	- WET	- (W));requires Ptimum moi		U	EQUIPMENT USED ON SUBJECT PROJECT ORILL UNITS: ADVANCING TOOLS: HAMMER TYPE;
om _ Optimum moisture SL _ Shrinkage limit							
	- DRY - (D) REDUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE PLASTICITY						
NONPLASTIC		ry index (DRY ST			CME-45C L HARD FACED FINGER BITS X→N_02 TUNG-CARBIDE INSERTS
NONTLASTIC LOW PLASTICITY MED. PLASTICITY HIGH PLASTICITY	6- 16-:	15		SLIG MEDI HIC	HT WM		
DESCRIPTIONS MAY INCLUDE COLO MODIFIERS SUCH AS LIGHT, DA	R OR COLOR (GRAY).	Image: marked bit in the second se
						0	REVISED 09/23/09

					PROJECT REFERENCE NO.	SHEET NO.					
					45357.1.26 (BD-5IIIV)	2A					
		1	NORTH CAROLINA DEPARTM	ENT OF TRANS	PORTATION						
			DIVISION OF	HIGHWAYS							
			GEOTECHNICAL ENG	INEERING UNIT	•						
	SI		D ROCK LEGEND, TERMS,								
	50	JL AN	D ROCK LEGEND, IERMS,	SIMBOLS, ANI	ABBREVIATIONS						
HARD ROCK	IS NON-COASTAL PLAT		DESCRIPTION IF TESTED, WOULD YIELD SPT REFUSAL, AN INFERRED		TERMS AND DEFINITIONS						
ROCK LINE	INDICATES THE LEVEL	AT WHICH NON-CI	DASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SAMPLER EDUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.	AQUIFER - A WATER BEARING FO	i have been transported by water. Irmation or strata.						
	ASTAL PLAIN MATERIAL		n Between Soll and Rock is often represented by a zone		S THAT HAVE BEEN DERIVED FROM SAND OR THAT C	ONTAIN SAND.					
ROCK MATE	RIALS ARE TYPICALLY	or contractor and	an a		L ROCKS OR SUBSTANCES COMPOSED OF CLAY MINER/ ION OF CLAY IN THEIR COMPOSITION, AS SHALE, SLATI						
Weathered Rock (WR)		NON-COASTAL PL BLOWS PER FOOT	ain material that would yield SPT N values > 100 IF tested.	ARTESIAN - GROUND WATER THA	T IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE T	HE LEVEL					
CRYSTALLINE	22		GRAIN IGNEOUS AND METAMORPHIC ROCK THAT T REFUSAL IF TESTED, ROCK TYPE INCLUDES GRANITE,	AT WHICH IT IS ENCOUNTERED, I GROUND SURFACE.	BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOV	ETHE					
ROCK (CR)	XeXe	GNEISS, GABBRO,	CHIST, ETC. GRAIN METAMORPHIC AND NON-COASTAL PLAIN	CALCAREOUS (CALC.) - SOILS TH	AT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARE	JONATE.					
Non-Crystall Rock (NCR)	.INE	SEDIMENTARY RO	THAT WOULD YELLO SPT REFUSAL IF TESTED, ROCK TYPE TE, SLATE, SANDSTONE, ETC.	<u>Colluvium</u> - Rock Fragments of Slope.	MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE	OR AT BOTTOM					
COASTAL PLAI SEDIMENTARY		COASTAL PLAIN S	EDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YJELD CK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED	CORE RECOVERY (REC.) - TOTAL LI	ENGTH OF ALL MATERIAL RECOVERED IN THE CORE BAR	REL DIVIDED BY TOTA					
CP)		SHELL BEDS, ETC.	prote that the protocological and the protocological and the second second second second second second second s	LENGTH OF CORE RUN AND EXPRE		AD LACENT					
Carlos and an		and the state of the second	THERING	ROCKS OR CUTS MASSIVE ROCK.	IEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF						
FRESH	ROCK FRESH, CRYSTALL		INTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER	DIP - THE ANGLE AT WHICH A S HORIZONTAL.	TRATUM OR ANY PLANAR FEATURE IS INCLINED FROM	I THE					
	ROCK GENERALLY FRE	SH, JOINTS STAINE	D, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN,	DIP DIRECTION (DIP AZIMUTH) -	THE DIRECTION OR BEARING OF THE HORIZONTAL TRA	CE OF					
(V SLL)	OF A CRYSTALLINE N	ATURE.	e Shine Brightly, rock rings under hanner blows if	THE LINE OF DIP, MEASURED CLOCKWISE FROM WORTH. FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE							
SLIGHT (SL1.)			20 AND DISCOLORATION EXTENDS INTO ROCK UP TO Y. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR	FALL - A FRACIONE OF FRACIDAE ZONE ALUNG WHICH HERE HAS BEEN DISPLALEMENT OF THE SIDES RELATIVE TO DAE ANOTHER PARALLEL TO THE FRACTURE, FISSLE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM							
	CRYSTALS ARE DULL	AND DISCOLORED.	CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.								
MODERATE (MOD_)	GRANITOID ROCKS, MOS	ST FELDSPARS ARE	Discoloration and weathering effects, in 5 Dull and Discolored, some show clay, rock has	FLOAT - ROCK FRAGMENTS ON S	URFACE NEAR THEIR ORIGINAL POSITION AND DISLOD	JED FROM					
	WITH FRESH ROCK.	Ammer Blows And	I Shows significant loss of strength as compared		RING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY						
MODERATELY			or stained. In granitoid rocks, all feldspars dull / Kaolinization, rock shows severe loss of strength	THE STREAM.	GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACE	ED IN					
MOD. SEV.)	AND CAN BE EXCAVAT	ED WITH A GEOLO	SIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK.	THE FIELD.							
SEVERE	IF TESTED, WOULD YIE		OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED		NG WHICH NO APPRECIABLE MOVEMENT HAS DCCURRED						
(SEV.)	IN STRENGTH TO STR	ONG SOIL. IN GRAM	NITOID ROCKS ALL FELOSPARS ARE KAOLINIZED TO SOME ROCK USUALLY REMAIN.	LEDGE - A SHELF-LIKE RIDGE C	r projection of rock whose thickness is small	COMPARED TO					
	IF TESTED. YIELDS S				CK THAT THINS OUT IN ONE OR MORE DIRECTIONS.						
VERY SEVERE			OR STAINED, ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK		MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLIN R AERATION AND LACK OF GOOD DRAINAGE.	NIG					
	REMAINING. SAPROLITE	IS AN EXAMPLE	of rock weathered to a degree such that only minor IC Remain, <u>IF TESTED, YIELDS SPT N VALUES < 100 BPF</u>	PERCHED WATER - WATER MAINT	AINED ABOVE THE NORMAL GROUND WATER LEVEL BY	THE PRESENCE OF A					
COMPLETE	ROCK REDUCED TO SO	IL. ROCK FABRIC I	IOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND	al an his and a state of the analysis and an an a	RMED IN PLACE BY THE WEATHERING OF ROCK.						
	SCATTERED CONCENTR ALSO AN EXAMPLE.	ATIONS, QUARTZ M	AY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS		D) - A MEASURE OF ROCK QUALITY DESCRIBED BY TO BREATER THAN 4 INCHES DIVIDED BY THE TOTAL LEN						
		ROCK	HARDNESS	EXPRESSED AS A PERCENTAGE.	INCHIER INHE & INCHES DIVIDED BY THE TOTAL LER	GIN OF LORE RUN H					
VERY HARD			SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES	SAPROLITE (SAP.) - RESIDUAL SO PARENT ROCK.	IL THAT RETAINS THE RELIC STRUCTURE OR FABRIC	OF THE					
HARD	Several Hard Blow		IST'S PICK. ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED	SILL - AN INTRUSIVE BODY OF	IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNES						
	to detach hand sp	ECIMEN.		RELATIVELY THIN COMPARED WI TO THE BEDDING OR SCHISTOSI	TH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED IY OF THE INTRUDED ROCKS.	ratallel					
MODERATELY HARD			. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE OGIST'S PICK. HAND SPECIMENS CAN BE DETACHED	SLICKENSIDE - POLISHED AND S	TRIATED SURFACE THAT RESULTS FROM FRICTION AL	ONG A FAULT OR					
MEDIUM	BY MODERATE BLOWS		HES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT.	STANDARD PENETRATION TEST O	PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS						
HARD	CAN BE EXCAVATED	IN SMALL CHIPS 1	O PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE	A 148 LB. HAMMER FALLING 38	INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 PLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION	FOOT INTO SOIL WITH					
SOFT		GOUGED READILY	By KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS	THAN 0.1 FOOT PER 60 BLOWS.							
	FROM CHIPS TO SEV PIECES CAN BE BRO		IZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN ESSURE.	GF STRATUM AND EXPRESSED AS	TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIL A PERCENTAGE.	JEU BT TUTAL LENGTH					
VERY			Excavated readily with point of pick. Pieces 1 Inch		on (Srod) - A measure of rock quality described e 's within a stratum equal to or greater than 4 i						
SOFT	FINGERNAIL.	ISS CAN BE BROKE	N BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY	TOTAL LENGTH OF STRATA AND E	XPRESSED AS A PERCENTAGE.						
	RACTURE SPACE		BEDDING IERM THICKNESS	IUPSUL ITSJ - SURFACE SOILS	USUALLY CONTAINING ORGANIC MATTER.						
TERM VERY WID	No. Strategicture	ACING IAN 18 FEET	VERY THICKLY BEODED > 4 FEET	BENCH MARK:							
WIDE	3 TO 10 LY CLOSE 1 TO 3 I	FEET	THICKLY BEDDED 1.5 - 4 FEET THINLY BEDDED 0.16 - 1.5 FEET		ELEVATIO	N: FT					
CLOSE	8.16 TO		VERY THINLY BEDDED 0.03 - 0.16 FEET THICKLY LAMINATED 0.038 - 0.03 FEET	NOTES:							
VERY CLO	ar LESS TH		THINLY LAMINATED (0.000 FEET	F.I.A.D FILLED IMME	DIATELY AFTER DRILLING						
OR SEDIMENT	ARY ROCKS. INFURATION		JRATION NG OF THE MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.	-							
	IABLE		WITH FINGER FREES NUMEROUS GRAINS;								
FR	INDLE	GENTLE E	LOW BY HAMMER DISINTEGRATES SAMPLE.								
MOL	BRATELY INDURATED		AN BE SEPARATED FROM SAMPLE WITH STEEL PROBE: ASILY WHEN HIT WITH HAMMER.								
IND	URATED		RE DIFFICULT TO SEPARATE WITH STEEL PROBE;								
		DIFFICUL	T TO BREAK WITH HAMMER.								
EXT	REMELY INDURATED		NAMER BLOWS REQUIRED TO BREAK SAMPLE; BREAKS ACROSS GRAINS,								

REVISED 09/23/09

WBS #	4535	57.1.26		DESCRIPTION	Replace Bridge No. 244 on SR 1809			
T.I.P. NO.	BD-	5111V	N.C. DEPT. OF	TRANSPORTATION				
COUNTY	S	urry		OF HIGHWAYS L ENG. UNIT-WRO				
STATION	11+7-	4.28 -L-	ACI	CEPTED	SEAL			
				CEPTED AS NOTED	A TO ROFESSION A THE			
					SEAL OCONTO			
DESIGN	INITIALS JRH	DATE 3/29/20	-		3/29/13			
CHECK	CVN	3/29/20	3		EMY R. HAMIN			
APPROVAL			BY: DATE: 04/1	Hardister, PE	SIGNATURE			
			DATE: 04/1	2/2010				
	STATI		FOUNDATION TYPE	FACTORED RESISTANCE	MISCELLANEOUS DETAI			
END BENT NO. 1	12+93.65 -L-		Cap on HP 12x53 Steel Piles	90 tons/pile	Bottom of Cap Elev. = 1139.84 ft Length of Pile = 60 ft Number of Vertical Piles = 7 Pile Spacing = 8 feet 6 inches			
BENT NO. 1	13+33.6	5 -L-	Footing on HP 14x73 Steel Piles	110 tons/pile	Bottom of Footing Elev. = 1113.5 to 1115.0 Length of Pile = 55 ft Footing Size = 7 ft x 7 ft Number of Vertical Piles Per Footing = 4 Number of Footings = 3			
BENT NO. 2	13+83.6	5 -L-	Footing on HP 14x73 Steel Piles	100 tons/pile	Bottom of Footing Elev. = 1116.0 to 1117.5 ft Length of Pile = 60 ft Footing Size = 7 ft x 7 ft Number of Vertical Piles Per Footing = 4 Number of Footings = 3 Bottom of Cap Elev. = 1136.13 ft Length of Pile = 70 ft Number of Vertical Piles = 7 Pile Spacing = 8 feet 6 inches			
END BENT NO. 2	14+23.6	5 -L-	Cap on HP 12x53 Steel Piles	85 tons/pile				

SHEET 4

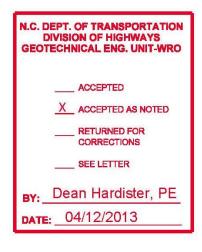
TIP# BD-5111V

County Surry

FOUNDATION RECOMMENDATION NOTES ON PLANS

- 1. For piles, see Section 450 of the Standard Specifications.
- 2. Piles at End Bent No. 1 are designed for a factored resistance of 90 tons per pile.
- 3. Drive piles at End Bent No. 1 to a required driving resistance of 150 tons per pile.
- 4. Piles at Bent No. 1 are designed for a factored resistance of 110 tons per pile.
- 5. Drive piles at Bent No. 1 to a required driving resistance of 1987 tons per pile. 185
- 6. Piles at Bent No. 2 are designed for a factored resistance of 100 tons per pile.
- 7. Drive piles at Bent No. 2 to a required driving resistance of 165 tons per pile. 170
- 8. Steel H-pile points are required for steel H-piles at Bont No. 2. For steel pile points, see Section 450 of the Standard Specifications.
- 9. Piles at End Bent No. 2 are designed for a factored resistance of 85 tons per pile.
- 10. Drive piles at End Bent No. 2 to a required driving resistance of 140 tons per pile. 145
- 11. Steel H pile points are required for steel H piles at End Bon. No. 2. For steel pile points, see Section 450 of the Standard Specifications.
- 12. Testing piles with the PDA during driving, restriking, or redriving may be required. The Engineer will determine the need for PDA testing. For PDA see Section 450 of the Standard Specifications.

END BENT NO. 1, BENT NO. 1, BENT NO. 2, & END BENT NO. 2



FOUNDATION RECOMMENDATION COMMENTS

- 1. No waiting period is required.
- 2. Pile lengths shown are based on excavation for the footings prior to pile driving.

				DIVISIO		3		SHEET 5
WBS ELEMENT		45357.1.26		<u> </u>	ACCEPTED AS NOT	ED	DATE	3/29/2013
TIP NO.		BD-5111V			RETURNED FOR	DES	GNED BY	JRH
COUNTY		Surry		\$	SEE LETTER	CHE	CKED BY	CVN
		11+74.28 -L	-	BY: Dear	n Hardister,	PE		
2				DATE: 04	/12/2013			
DESCRIPTION	-	Re	place	e Bridge No over SR		1809		
NUM NUMBER OF	BER OF F END BEN	TS WITH PILES PILES PER BENT TS WITH PILES PER END BENT			for Excav	Piles" & vation" p	"Predrilling : "Pile ay items	
		P.	ILE	PAY ITEM	QUANTIT	1		
Bent # or	Steel Pile Points	Pipe Pile Plates	F	ed rilling or Piles	Pile Redrives	Exc (per In	Pile avation linear ft) Not In	PDA Testing
End Bent #	(yes/no)	(yes/no/maybe)	(per	r linear ft)	(per each)	Soil	Soil	(per each)
End Bent No. 1 Bent No. 1	YES 🔊							Λ Λ
Bent No. 2	yes							
End Bent No. 2	yes							$ \setminus / $

Notes:

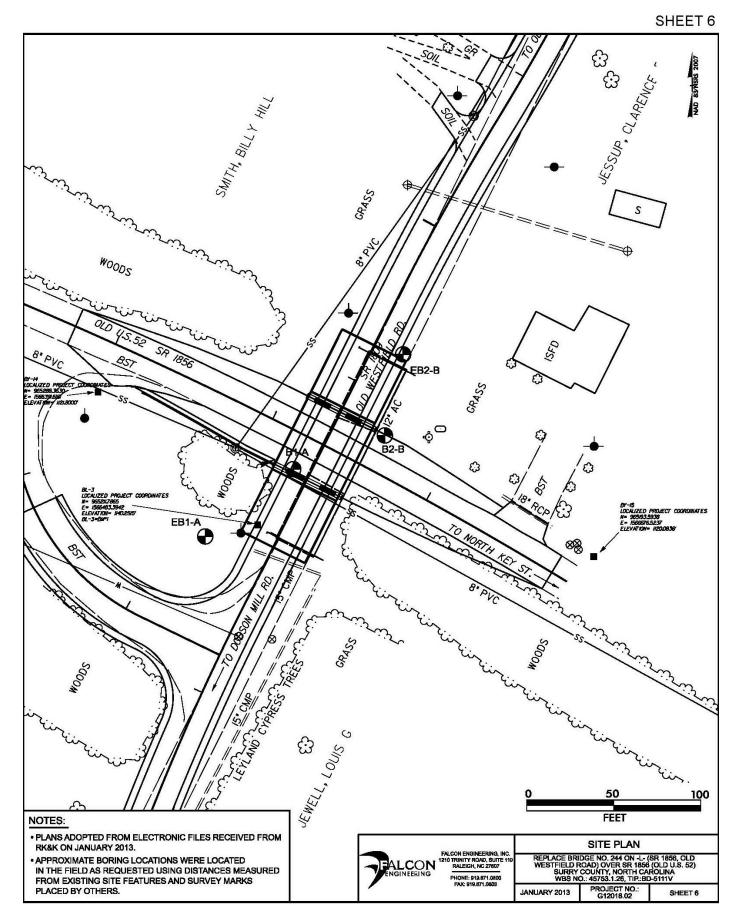
Blanks or "no" represent quantity of zero.

If steel pile points are required, calculate quantity of "Steel Pile Points" as equal to the number of steel piles.

If pipe pile plates are or may be required, calculate the quantity of "Pipe Pile Plates" as equal to the number of pipe piles.

Show quantity of "PDA Testing" on the plans as total only.

If quantity of "PDA Testing" is 3 or less, reference "Pile Driving Criteria" provision in PDA notes on plans and include "Pile Driving Criteria" provision in the contract.



NCDOT GEOTECHNICAL ENGINEERING UNIT 5)(17

WBS	45357	.1.26			Т	IP B	D-511	1V	C	OUNT	Y SURRY	1				GEOLOGIST T. Eva	าร		
SITE	DESCR	IPTION	REF	PLACE	EBRI	DGE	NO. 24	4 ON -L	(SI	R 1809	, OLD WE	STFIE	LD F	RD) O	VER	SR 1856 (US 52)		GROUN	ND WTR (f
	NG NO.	and a balance	- and the second s				ON 12				OFFSET					ALIGNMENT -L-		0 HR.	34.
	AR ELE			ft				H 58.6	S ft		NORTHIN			15		EASTING 1,566,453		24 HR.	34.
	RIG/HAI									111	NORTHIN	Contract Constants		0.5039	<u>ъ п</u>	S. Augers	LIAM	MER TYPE	
		entre anna anna anna ann						1.00 90.000 0.000				-			у п.,		Constantion of		Automatic
	LER S. DRIVE						DATE	08/20			COMP. D	11	1		L	SURFACE WATER DE	PIN	N/A	
ELEV (ft)	ELEV (ft)	DEPTH (ft)	0.5ft	0.5ft	1	0	2	BLOW	50	R FOOT	75 10	SAN D NO	10000	моі	O G	SOIL AND R ELEV. (ft)	OCK DE	SCRIPTION	DEPTH
1140		_							15		<i>X</i>				-	- 1,137.5 GROL	NDSUR	FACE	1
1135	1,136.0	1.5	4	4	6		1::			• • • •					- III	RED, F. SAND	ESIDUAI		CE
1100	1,134.0	3.5	5	5	6		¶10							M	1	-	MICA	1	
	1,131.5	6.0		19862		i	•11 · •							M	s -				
1130	1,129.0	8.5	2	4	4		8		-			-		D		1,130.3 1.129.5	ESIDUA		
	1,129.0	0.0	3	5	5		• 10		•					М	<u>s</u> t	ORANGE AND TA		F. SAND (A	A-2-4)
1125	-					-			•			1				BROWN RED			=
3	1,124.0	13.5	3	4	5		• • •	* * *	•	: : : :				м	8	SANDY SILT (A-4) W/ LIT*		
	3 -	_					N: :								s t	1,120.5	AYERS		1
1120	1,119.0	18.5			L .		$\frac{1}{1}$									- R			
	8- 	_	6	10	9		: : •) 	•					M	Ľ	PURPLE BROWN (A-2-4) SAPRO	ITIC, W	LITTLE MIC	
1115	-	_					: 1:		•							. RC -	CK FRAG	GS	
5	1,114.0	23.5	5	6	6	:	12.		•					w	Ľ				
	-	-					J												
110	1,109.0	28.5					1::	111	:		1.1.1.1					-			
	-	-	6	6	4	-	•10 -		•					W					
1105		-					1.1	22.2	2							-1,104.5			3
	1,104.0	33.5	2	5	8	•	1 3		•					V		R	ESIDUA		
	-	-												W	s -	PURPLE BLACK SILT (A-4) SAPR			
1100	1,099.0	38.5					1.		-							- 1,099.5			3
2	-	-	2	4	6		•10							W	-		ESIDUAI /N, SILT		2-4)
095	-	-				-	N: 1								-			195	
	1,094.0	43.5	3	7	11		111							Cat	_				
	-	-				-	· ·••18		:					Sat.					
1090	1.089.0	40.5				-	<u>::</u> j:	***	•	· · · ·		-				- 			4
8	1,069.0_	46.5	6	8	12		: :	0		::::				w		BLACK TAN AND	GRAY S		(A-4)
085	-	-				-	· · · ·			 					88 -	W/ SAND LAYE	RS, TRA	CE TO LITT	
1000	1,084.0	53.5	10	- 24	70		• • •							_	880 -	MICA, V	ERT GR	AVELL Y	F
8	1,082.5	55.0	10 100/0.2	21	70		:::		•	::::	- 100/0.3	•		D	111		HERED		5
080	-		100000000000000000000000000000000000000						•			1			Stor.	GRAY AND WHI	E GNE	SS, W/ QUA	
2	1,079.0	- 58.5	60/0.1	-	-						60/0.	•	-			1,078.9 Boring Term		th Standard	5
	-	-														Penetration Te 1,078.9 ft in C			
	-	-															yoranno	Rook. Onoic	
		-																	
	-														_	-			
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	-																		
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	2. 8-	F													F				
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	-	-													-	•			

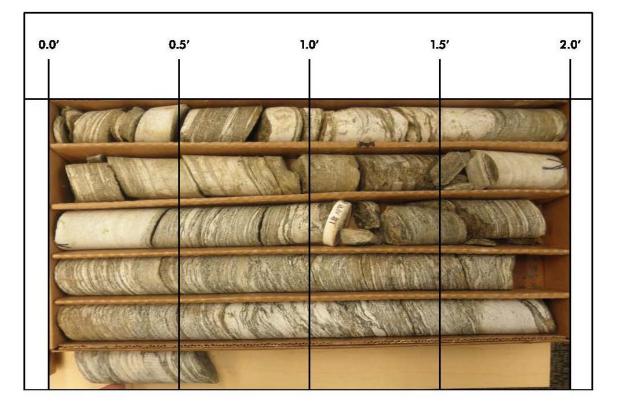
		26			TI		The second secon				GEOLOGIST T. Evans				
SITE	DESCRIPT	TION	REP	LACE	BRID)GE NO. 244 ON -L- (SR 1809	OLD WES	TFIELD	RD) OV	/ER SR 1856 (US 52)	GROUND WTR			
	NG NO. E	1				TATION 13+40		OFFSET		, = •	ALIGNMENT -L-	0 HR. 2			
	and the second second		04 5 4							4.4					
	AR ELEV.					DTAL DEPTH 70.5 ft		NORTHING	1		EASTING 1,566,504	24 HR. 1			
		-		IE TR	UNITED STORE	MOBILE B-57 93% 12/08	enconcrete pro-				HSA/Rotary Wash	HAMMER TYPE Automa			
DRILI	LER S.G	ower	•		ST	TART DATE 08/20/1	2	COMP. DA	TE 08/2	21/12	SURFACE WATER DEI	PTH N/A			
ELEV	ELEV I	РТН	BLO	W COL	JNT	BLOWS F	PER FOOT		SAMP.		L SOIL AND RC	CK DESCRIPTION			
(ft)	(ft) ((ft)	0.5ft	0.5ft	0.5ft	0 25 €	50	75 100	NO.		G ELEV. (ft)	DEP1			
1125											1,124.5 GROUN	ID SURFACE			
	1,123.0	1.5				1					RE RE	SIDUAL			
	Ţ	3.5	2	2	3					D		ID BROWN, F. SANDY LITIC, W/ LITTLE MICA			
1120		and a	3	4	6	1 0			6	D	E.				
-	1,118.5	5.0	2	2	3					M	- 1,118.5 RE	SIDUAL			
1115	1,116.0 8	3.5	3	F	6	N::::	1:1:				1,116.5 TAN, ORANGE AN	ND DK. BROWN, SILTY			
1110	+		3	5	0	11			8	M		MICA /			
						1 : <u>1</u> : <u>1</u> : : : :	::::					D AND GRAY, SILTY F. PROLITIC, W/ TRACE			
1110	1,111.0 1	3.5	5	7	9		• • • •		0	м		MICA			
	Ŧ					.						ND TAN, F. SANDY SILT FIC, W/ LITTLE MICA			
	1,106.0 1	8.5				- 7 - 1									
1105			2	3	6	•• 9 • • • • • • •	<u> : : : :</u>			M	×F				
	Ŧ										× F				
1100	1,101.0 2	3.5	3	5	7		::::				WHITE ORANG	E AND BLACK, SILTY			
. 100	‡		5	3		12	1			W	SAND (A-2-4) SA	PROLITIC, W/ LITTLE			
						$ $: : λ : $ $: : : :	: : : :	••••							
1095	1,096.0 2	8.5	5	12	9	· · · · • • • • • • • • • • • • • • • •				w					
	±					::: []::::									
	1,091.0 3	3.5				···· · · · ·									
1090	-		7	6	11	•17	· · · ·			М					
	Ŧ					::: \ ::::	::::								
1085	1,086.0 3	8.5	12	12	14			••••							
	‡		14	14	14	<u> </u>				D					
	±					:::: / ::::									
1080	1,081.0 4	3.5	4	8	12	20				D					
	±														
	1,076.0 4	8.5				N					-				
1075	+		20	18	20	38-				М					
	‡					:::: :: \ :									
1070	1.071.0 5	3.5	23	22	22	:::: :: \ :	::::								
	‡		20	22	22	· · · · · · · • 4	~			D	-				
	1 and 1 -						1.1.								
1065		8.5	21	79/0.3				100/0.8			- 1,065.5 - 1,064.0 WEATH	ERED ROCK			
F	1,064.0 6	0.5	60/0.0	2			::::	60/0.0			DK. GRAY AND W	HITE, GNEISS, W/ MICA			
	±									1.6		ALLINE ROCK RAY, MOD. SEV. TO SLI.			
1060	Ŧ								k.	1.	WEATHERED, MC	D. HARD, V. CLOSELY			
	Ŧ										TO CLOSELY F	RACTURED GNEISS			
1055	Ŧ						::::	::::			₫.				
										-	1,054.0 Boring Torminated	at Elevation 1 054 0 ft in			
	‡										- Boring Terminated Crystalline	at Elevation 1,054.0 ft in e Rock: Gneiss			
	+														
	Ŧ										_				
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	+										É				
	‡										F				
	‡										F				
	1.000														

NCDOT GEOTECHNICAL ENGINEERING UNIT

SHEEL 9

NBS	4535	7.1.26			TIP	BD-5	111V	C	OUNT	Y S	IRRY	GEOLOGIST T. Evan	s		
SITE	DESCR		RE	PLACE B	RIDGI	E NO.	244 ON -	L- (SF	R 1809	, OL	WESTFIELD RD) OVER	R SR 1856 (US 52)		GROUN	D WTR (ft
	ING NO	white the second			and our states of	and a state	13+40	-			SET 10 ft LT	ALIGNMENT -L-		0 HR.	20.2
OL	LAR EL	EV . 1,	124.5	ft	тот	AL DE	PTH 70	.5 ft		NO	THING 965,244	EASTING 1,566,504		24 HR.	18.4
RILI	. RIG/HA	MMER E	FF./DA		16 MO	BILE B-	57 93% 1	2/08/20	11		DRILL METHOD	SA/Rotary Wash	HAMM		Automatic
RIL	LER S	. Gowe	er		STA	RT DA	TE 08/2	0/12		co	IP. DATE 08/21/12	SURFACE WATER DE	PTH N	/A	
OR	E SIZE	NQ2			тот	AL RU	N 10.0 f	t							
LEV	RUN	DEPTH	RUN	DRILL	REC.	JN RQD	SAMP.	STR REC.	ATA	L			<i>(</i> 0		
(ft)	ELEV (ft)	(ft)	(ft)	RATE (Min/ft)	(ft)	(1)	NO.	(ft)	(ft) %	O G	ELEV. (ft)	DESCRIPTION AND REMARK	(S		DEPTH
064												Begin Coring @ 60.5 ft			
	1,064.0	60.5	5.0	3:00/1.0 2:55/1.0	(5.0)	(2.0) 40%		(10.0) 100%	(5.9) 59%	S.	1,064.0 DK. GRAY AND G	CRYSTALLINE ROCK RAY, MOD. SEV. TO SLI. WEA	THERED	, MOD. HA	60 ARD,
060	1,059.0	65.5		2:40/1.0 2:45/1.0 2:22/1.0		-		1.6		S.	V. CLO	SELY TO CLOSELY FRACTUR	RED GNE	ISS	
	1,005.0	- 00.0	5.0	2:22/1.0	(5.0)	(3.9)				S					
055		‡		2:21/1.0 2:33/1.0 2:10/1.0 2:13/1.0 2:44/1.0	100%	78%				S					
	1,054.0	70.5		2:44/1.0	-		a			C)	1,054.0 Boring Terminat	ed at Elevation 1,054.0 ft in Cry	etalline R	lock: Gneis	70
		ŧ													
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SHEET 10

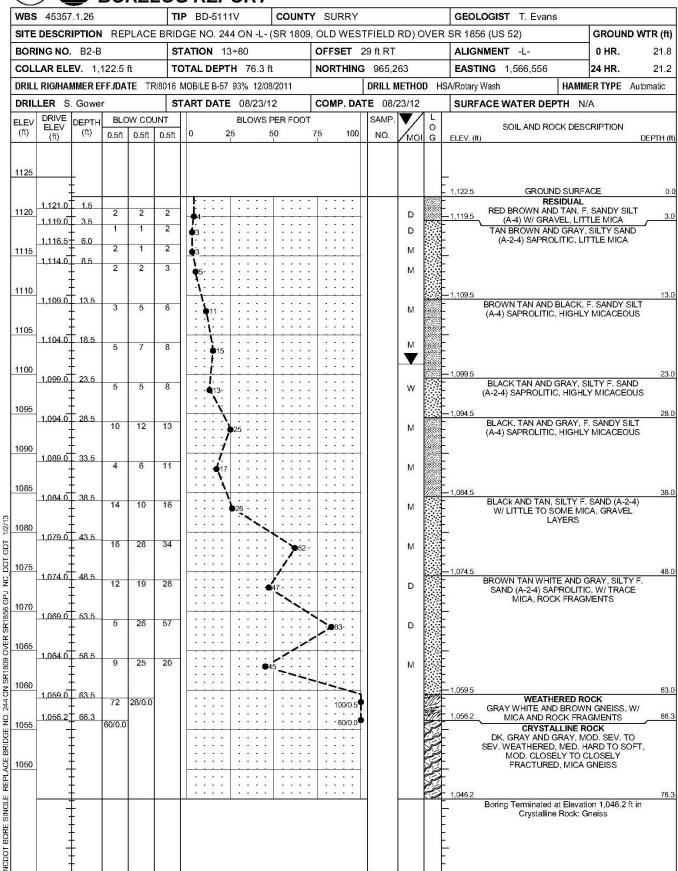


BORING B1-A, BOX 1 OF 1, 60.5 FEET TO 70.5 FEET.



Page A12

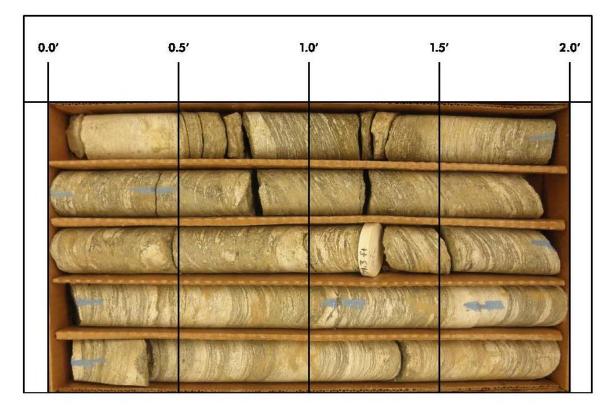
RCDOT GEOTECHNICAL ENGINEERING UNIT BORELOG REPORT



NCDOT GEOTECHNICAL ENGINEERING UNIT

WBS	45357	.1.26			TIP	BD-51	111V	C	OUNT	GEOLOGIST T. Evans	
SITE D	DESCR	IPTION	REF	PLACE B	RIDGE	E NO.	244 ON -	L- (SF	R 1809	OLD WESTFIELD RD) OVER SR 1856 (US 52) GROUND WT	R (f
ORIN	IG NO.	B2-B			STAT	TION	13+80			OFFSET 29 ft RT ALIGNMENT -L- 0 HR.	21.
OLL	AR ELE	V. 1,1	22.5	ft	тот	AL DE	PTH 76.	3 ft		NORTHING 965,263 EASTING 1,566,556 24 HR.	21.
RILL F	RIGIHAN	AMER E	F.IDA	TE TRI80	16 MO	BILE B-	57 93% 1	2/08/20	11	DRILL METHOD HSA/Rotary Wash HAMMER TYPE Autom	natic
RILL	ER S.	Gowe	r		STAR	RT DA	TE 08/2	3/12		COMP. DATE 08/23/12 SURFACE WATER DEPTH N/A	
ORE	SIZE	NQ2			TOTA		N 10.0 f	t			
LEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN SAMP. REC. ROD (ft) (ft) NO. (ft) (ft)					L DESCRIPTION AND REMARKS G ELEV. (ft) DEFINITION AND REMARKS	PTH
56.2	0									Begin Coring @ 66.3 ft	
	1,056.2 - - 1,051.2	-	5.0	8:47/1.0 4:36/1.0 5:13/1.0 6:03/1.0 5:29/1.0 3:42/1.0	(4.8) 96% (4.6)	(4.0) 80% (4.3)		(9.4) 94%	(8.3) 83%	1,056.2 CRYSTALLINE ROCK DK. GRAY AND GRAY, MOD. SEV. TO SEV. WEATHERED, MED. HARD TO SOFT, MOD. CLOSELY TO CLOSELY FRACTURED, MICA GNEISS	6
	1,046.2	- - - 76.3	0.0	3:42/1.0 4:39/1.0 4:45/1.0 4:20/1.0 4:10/1.0	92%	86%				1,046.2	71

SHEET 13



BORING B2-B, BOX 1 OF 1, 66.3 FEET TO 76.3 FEET.



NCDOT GEOTECHNICAL ENGINEERING UNIT

WBS	45357	7.1.26			TI	P BD-5111V	COUNT	Y SURRY				GEOLOGIST T. Evans			
SITE	DESCR	IPTION	REF	PLACE	BRID	GE NO. 244 ON -L-	SR 1809	, OLD WES	TFIELD	RD) OV	'ER	SR 1856 (US 52)		GROUN	D WTR (
	NG NO.					TATION 14+26		OFFSET		1		ALIGNMENT -L-		0 HR.	39.
	AREL	C. Strangerates		ft		DTAL DEPTH 74.4 f		NORTHING		10		EASTING 1,566,567		24 HR.	FIA
	concerning in the second							NORTHING	1						
						MOBILE B-57 93% 12/08			DRILL N		H.S	<u> </u>			Automatic
DRIL	LER S	. Gowe	-			TART DATE 08/22/1		COMP. DA		22/12		SURFACE WATER DEPT	TH N/A	۱	
ELEV	DRIVE ELEV	DEPTH	1000				PER FOOT	land and a	SAMP.	▼/	ŏ	SOIL AND ROC	K DESC	RIPTION	
(ft)	(ft)	(ft)	0.5ft	0.5ft	0.5ft	0 25	50	75 100	NO.			ELEV. (ft)			DEPTH
1140															
								1					MENT	DETE	
	1,137.0		2	3	2					M		1,137.5 6" BITUMINOU 6" AGGREGATE			
1135	1,135.0	3.5	2	1	2		1.00.0.0		6	M		RED-BROWN, MED			
	1,132.5	6.0				•3 • • • • • • • •						SANDY CLAY (A-6)			
1130	1,130.0	95	WOH	WOH	2	• 2	1::::			M		GRAVEL LAYEF	R @ 12.0	-13.0 FT	
	1,130.0	- 0.0	WOH	1	2	• 3 • • • • • • •	1212		8	ML					
	-	ŧ					::::			L					
1125	1,125.0	13.5	3						0		H	1,125.5 RED-BROWN AND T	AN, SILT	Y CLAY	(A-7)
	1	t i	3	1	1	4 2 · · · · · · · · · · · · · · · · · · ·	::::			W			RAVEL	I OD II I	(,,,)
		t				X ::: : : : : : : : : : : : : : : : : :	::::			L	1				
1120	1,120.0	18.5	4	4	7	11.			6	M	1	1,119.0			
	10- 11-	F	2.46	2368			1.1.1					RED-BROWN, SA		AV (A-6) V	N//
1115	1,115.0	23.5										1,115.5 TRAC	E MICA		
			4	4	4		1.1.1			м	8-	BLACK AND TAN, F SAPROLITIC, \			-4)
	-	ŧ					::::				81 -	0/11/10/21/10/1			
1110	1,110.0	28.5	6	5	6	· · · · · · · · · · · · · · · · · · ·			4		۳t				
	-	ŧ	0	5	0	: ∲ 11 : : : : :	::::			D	۳ţ				
						: <u>!</u> : : : : : :				000 P		1,106.5 TAN BLACK AND (CDAV C		3
1105	1,105.0	33.5	3	3	3				6	w		(A-2-4) SAPROLITI			
	-	-									÷				
1100	1,100.0	38.5								∇	F				
		F	6	9	14	• • • • • • • 23• • • •				w	F				
	-	ŧ				· · · · · · · · · · · · · · · · · · ·					-				
1095	1,095.0	43.5	7	8	13	····			e.		Ŀ				
	-	ŧ		0	10	1 · · · • • • • • • • • • • • • • • • •	::::			W	-				
		-				No.					t				
1090	1,090.0	48.5	12	19	25		4		6	w	Ŀ				
	-	E .				:::: ::: T	1::::				t				
1085	1.085.0	53.5		2 2							-	1,085.5		CANDY	
	- 2		11	21	24		5			M	<u>-</u>	BROWN BLACK ANE (A-4) SAPROLITI			
	-	f.					N				×F.	1 090 5			
1080	1,080.0	58.5	36	26	34		1		6	NA NA		1,080.5 GRAY BROWN AND			
	-	ŧ		20			1 60 :			М	-	(A-2-4) SAPROLI			
1075	1,075.0	- co.r					1.111	::::				1,075.5	0000000000000		6
1010	1,075.0	63.5	10	17	26		3		8	D	Ø.	BROWN GRAY AN SILT (A-4) SAPROL			
	-	ŧ					1				Ø1				
1070	1,070.0	68.5							8		8£	-1,069.5			
			27	69	31/0.2			100/0.7		D	12	WEATHEI			0
	-	ŀ						a sea another a sea		100	1	BROWN TAN AND E	SLACK M	IICA GNE	ISS
1065	1,065.0	73.5	33	67/0.4							-	1,064.1			1
	0	F	00	0				100/0.9		ľ	Ŧ	Boring Terminated at			
	-	ŧ									F	Weathered	ROCK: Gr	HSS	
		ŧ									-				
	-	ŧ.									E				
	-	t									-				
	-	+									F				

