

PI = 11+90.00 -L-EL = 3225.17' VC = 50.00 FT. (+)0.8343% (-)2.9500% -L- GRADE DATA

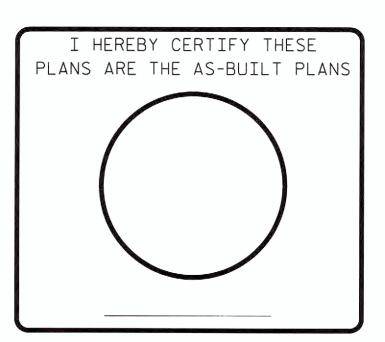
APPROXIMATE EXISTING GROUND LINE

## HYDRAULIC DATA

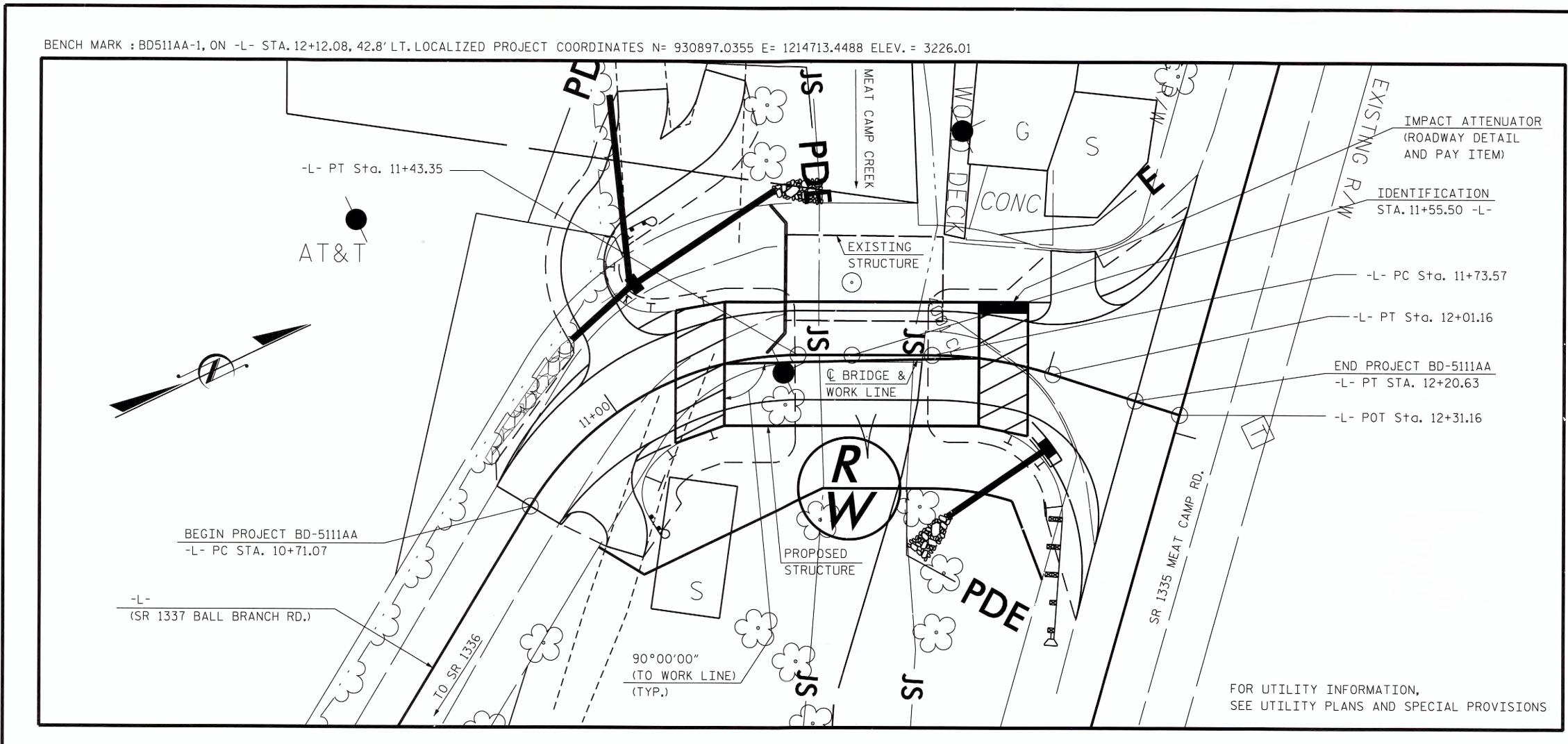
DESIGN DISCHARGE
FREQUENCY OF DESIGN FLOOD25 YR.
DESIGN HIGH WATER ELEVATION3225.4
DRAINAGE AREA6.8 SQ. MI.
BASE DISCHARGE (Q100)2,300 C.F.S.
BASE HIGH WATER ELEVATION3227.54

#### OVERTOPPING FLOOD DATA

OVERTOPPING DISCHARGE------1,748 C.F.S. FREQUENCY OF OVERTOPPING FLOOD-----25 YR. + OVERTOPPING FLOOD ELEVATION------3225.7



5	PR	ROJECT	NO	BC	)-5111A	Α
		N	ATAU	GA		ΤY
	ST	TATION:	STA.	11+55.50	) -L- P	.0.T
		SHEET 1 C			S BRIDGE	
CH RD.)	-	DEPA		OF NORTH CA		ΓΙΟΝ
	DWG.NO.1		GEN	ERAL DRA	AWING	
	HITH CAROLINI			OVER ME. 7 (BALL		
	SEAL 10966			1335 (MI 336 (LEE		
K	CINER HILLING			ISIONS		SHEET NO.
KAHL, LLP E SUITE 350	1 Million Maine	NO. BY:	DATE:	NO. BY:	DATE:	1
E 3011E 350 IA 27609–3960 ER: F–0112	7/2/2013	1		3 4		total sheets 22



			τοτα	L BILL	OF M	ATERIAL	S						
	REMOVAL OF EXISTING STRUCTURE	FOUNDATION EXCAVATION	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	APPROACH STEEL S		12 X 53 El PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	ELASTOMERIC BEARINGS	PRES CO	)″X 1'-9″ STRESSED NCRETE ED SLABS
	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	EA.	LIN.FT.	LUMP SUM	NO.	LIN.FT
SUPERSTRUCTURE					LUMP SUM					110.25		10	550.0
END BENT NO.1		LUMP SUM	LUMP SUM	49.1		5,314							
END BENT NO.2	60 C		LUMP SUM	22.0		2,535	6	90.0	6				
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	71.1	LUMP SUM	7,849	6	90.0	6	110.25	LUMP SUM	10	550.0

DRAWN BY :	F.D. WEEDEN	DATE : JUNE 2013
CHECKED BY : _	R.V. KEITH	DATE : JUNE 2013

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LOCATION SKETCH



	PR	OJECT	NO	B	D-5111A	Α			
			STA.		_ <b>COUN</b>				
		SHEET 2	OF 4						
		DEPA	RTMENT	ATE OF NORTH C OF TRA RALEIGH	NSPORTA	TION			
	DWG. NO. 2	ON	RIDGE O SR 1337	VER ME	AT CAMF BRANCH	RD.)			
	SEAL 10966	BETWEEN SR 1335 (MEAT CAMP RD.) AND SR 1336 (LEE SOUTH RD.)							
	V KEITHIN	NO. BY:	REVI DATE:	SIONS	DATE:	SHEET NO. 2			
0	7/2/2013	1		3 4		total sheets 22			

### NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF 1 SPAN @ 35'-6" WITH AN ASPHALT WEARING SURFACE OVER A TIMBER FLOOR ON I-BEAM SUPERSTRUCTURE AND A CLEAR ROADWAY WIDTH OF 19.1' ON A SUBSTRUCTURE CONSISTING OF TIMBER CAPS/TIMBER POST & SILLS AND LOCATED UPSTREAM OF THE PROPOSED STRUCTURE LOCATION SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. SEE SPECIAL PROVISION FOR "REMOVAL OF EXISTING STRUCTURE AT STA. 11+55.50 -L-''.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FEET EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY, 2001.

DRAWN BY :	F.D. WEEDEN	DATE : JUNE 2013
CHECKED BY		DATE : JUNE 2013

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INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD. THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 11+55.50 -L-".

FOR SUBMITTAL OF WORKING DRAWINGS. SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK. SEE SPECIAL PROVISIONS. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. FOR IMPACT ATTENUATOR. SEE ROADWAY SPECIAL PROVISIONS. FOR CONSTRUCTION STAGING, SEE TRAFFIC CONTROL PLANS.

## FOUNDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 85 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 142 TONS PER PILE.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO. 2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR END BENT NO.1 IS THE BOTTOM OF FOOTING ELEVATION. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

THE SPREAD FOOTING AT END BENT NO.1 IS DESIGNED FOR A FACTORED RESISTANCE OF 6 TSF. CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OF 14 TSF JUST BEFORE PLACING CONCRETE.

KEY IN SPREAD FOOTING AT END BENT NO.1 AT LEAST 12" INTO ROCK WITH MINIMUM THICKNESS AS SHOWN ON THE PLANS.

FOR BLASTING ADJACENT TO HIGHWAY STRUCTURES, SEE ARTICLE 410-9 OF THE STANDARD SPECIFICATIONS.



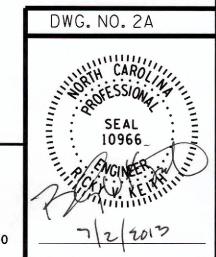
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$\boldsymbol{\omega}$	$\boldsymbol{\nabla}$	-	1	Τ.	1	$\sim$	$\sim$

#### WATAUGA COUNTY

SHEET 3 OF 4

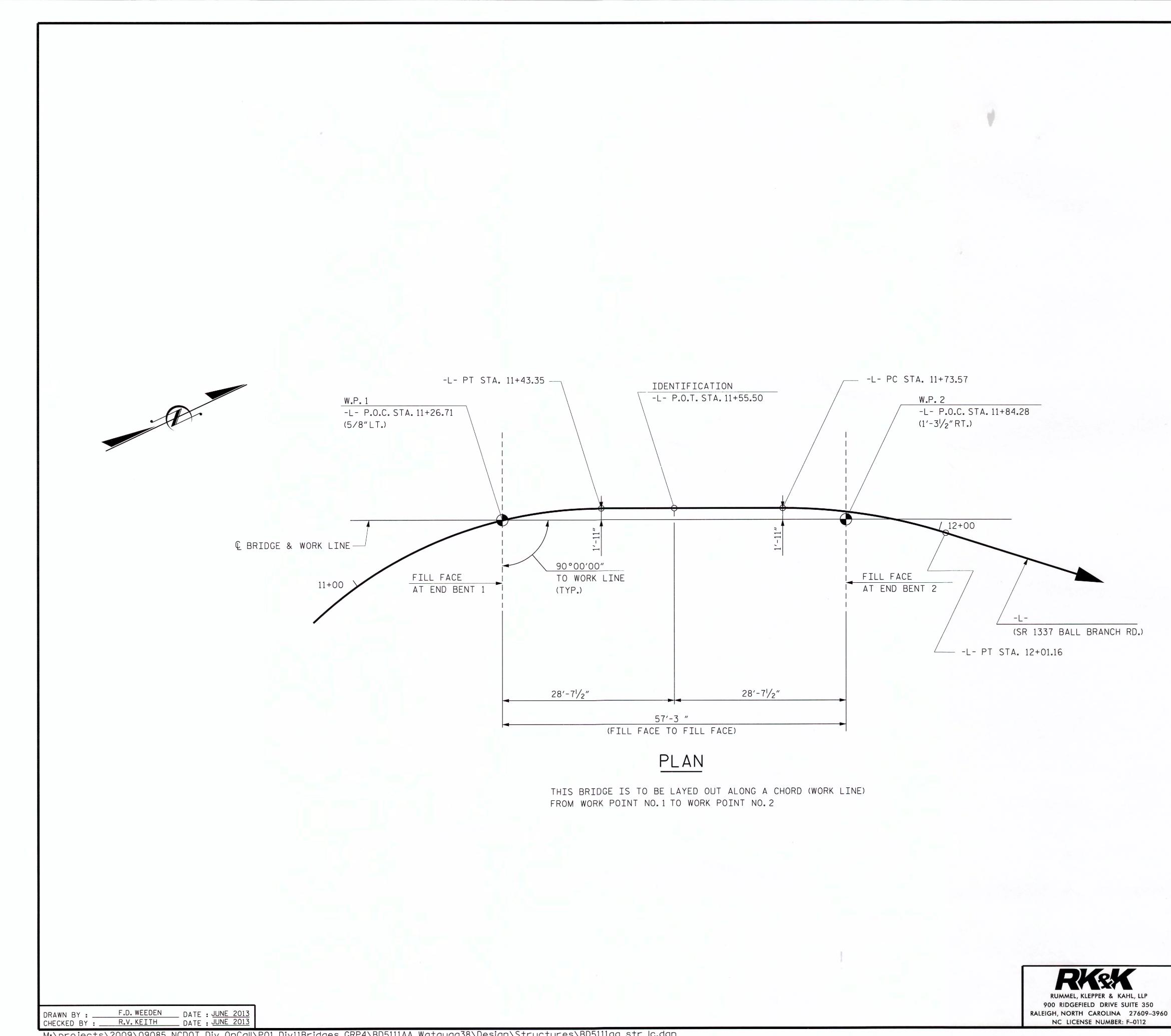
PROJECT NO. \_\_\_\_

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH



GENERAL DRAWING FOR BRIDGE OVER MEAT CAMP CREEK ON SR 1337 (BALL BRANCH RD.) BETWEEN SR 1335 (MEAT CAMP RD.) AND SR 1336 (LEE SOUTH RD.)

		REN	ISIONS/			SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATE:	2A
1			3			TOTAL SHEETS
2			4			22



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#### -L- HORIZONTAL CURVE DATA

PI Sta 11+10.80 △ = 59° 09′ 55.3″(RT) D = 81° 51′ 04.0″ L = 72.28' T = 39.74′ R = 70.00'

PI Sta 11+87.48 △ = 17° 33′ 45.4″(RT) D = 63° 39′ 43.1″ L = 27.59′ T = 13.90' R = 90.00'

PROJECT NO. \_\_\_\_BD-5111AA WATAUGA COUNTY STATION: STA. 11+55.50 -L- P.O.T SHEET 4 OF 4 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH DWG.NO.3 LONG CHORD LAYOUT FESSION SEAL 10966 SHEET NO. REVISIONS

DATE:

BY:

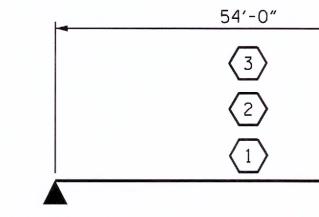
3

total sheets 22

NO. BY: DATE:

7/2/2013

							STRENGTH I LIMIT STATE								SERVICE III LIMIT STATE								
												SHEAR					MOMENT						
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING (#) LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	L I VEL 0 A D F A C T O R S	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF
	0	HL-93(Inv)	N/A	1	1.11		1.75	0.276	1.15	А	EL	27	0.523	1.19	А	EL	5.4	0.80	0.276	1.11	А	EL	27.00
ESIGN		HL-93(0pr)	N/A		1.49		1.35	0.276	1.49	А	EL	27	0.523	1.54	А	EL	5.4	N/A					
OAD ATING		HS-20(Inv)	36.000	2	1.39	50.184	1.75	0.276	1.44	А	EL	27	0.523	1.42	А	EL	5.4	0.80	0.276	1.39	А	EL	27.00
		HS-20(0pr)	36.000		1.84	66.310	1.35	0.276	1.86	А	EL	27	0.523	1.84	А	EL	5.4	N/A					
		SNSH	13.500		2.93	39.523	1.4	0.276	3.78	А	EL	27	0.523	4.04	А	EL	5.4	0.80	0.276	2.93	А	EL	27.00
		SNGARBS2	20.000		2.27	45.449	1.4	0.276	2.93	А	EL	27	0.523	2.93	А	EL	5.4	0.80	0.276	2.27	А	EL	27.00
		SNAGRIS2	22.000		2.19	48.232	1.4	0.276	2.83	А	EL	27	0.523	2.74	А	EL	5.4	0.80	0.276	2.19	Α	EL	27.00
		SNCOTTS3	27.250		1.46	39.768	1.4	0.276	1.88	А	EL	27	0.523	2.02	А	EL	5.4	0.80	0.276	1.46	Α	EL	27.00
	l s	SNAGGRS4	34.925		1.25	43.795	1.4	0.276	1.62	А	EL	27	0.523	1.72	А	EL	5.4	0.80	0.276	1.25	А	EL	27.00
		SNS5A	35.550		1.22	43.508	1.4	0.276	1.58	А	EL	27	0.523	1.76	А	EL	5.4	0.80	0.276	1.22	Α	EL	27.00
		SNS6A	39.950		1.14	45.456	1.4	0.276	1.47	А	EL	27	0.523	1.63	Α	EL	5.4	0.80	0.276	1.14	Α	EL	27.00
EGAL		SNS7B	42.000		1.08	45.533	1.4	0.276	1.40	А	EL	27	0.523	1.62	Α	EL	5.4	0.80	0.276	1.08	Α	EL	27.00
OAD		TNAGRIT3	33.000		1.39	45.936	1.4	0.276	1.80	Α	EL	27	0.523	1.92	А	EL	5.4	0.80	0.276	1.39	Α	EL	27.00
ATING		TNT4A	33.075		1.40	46.381	1.4	0.276	1.81	Α	EL	27	0.523	1.85	А	EL	5.4	0.80	0.276	1.40	Α	EL	27.00
		TNT6A	41.600		1.16	48.314	1.4	0.276	1.50	Α	EL	27	0.523	1.77	А	EL	5.4	0.80	0.276	1.16	Α	EL	27.00
	ST	TNT7A	42.000		1.18	49.361	1.4	0.276	1.52	Α	EL	27	0.523	1.66	А	EL	5.4	0.80	0.276	1.18	Α	EL	27.00
		TNT7B	42.000		1.23	51.517	1.4	0.276	1.58	А	EL	27	0.523	1.57	А	EL	5.4	0.80	0.276	1.23	A	EL	27.00
		TNAGRIT4	43.000		1.16	49.916	1.4	0.276	1.50	Α	EL	27	0.523	1.51	А	EL	5.4	0.80	0.276	1.16	A	EL	27.00
		TNAGT5A	45.000		1.09	48.939	1.4	0.276	1.40	А	EL	27	0.523	1.53	Α	EL	5.4	0.80	0.276	1.09	A	EL	27.00
		TNAGT5B	45.000	3	1.07	48.072	1.4	0.276	1.38	Α	EL	27	0.523	1.43	Α	EL	5.4	0.80	0.276	1.07	Α	EL	27.00



END BENT 1

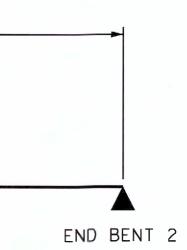
LRFR SUMMARY

FOR SPAN `A'

ASSEMBLED BY : B.A.DUKE CHECKED BY : R.Z.DEAN	DATE : DATE :	
	REV. 11/12/08RR REV. 10/1/11	MAA/GM MAA/GM

+

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#### LOAD FACTORS:

DESIGN LOAD RATING	LIMIT STATE	$\gamma_{DC}$	γ <sub>Dw</sub>
	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

#### NOTES:

NUMBER

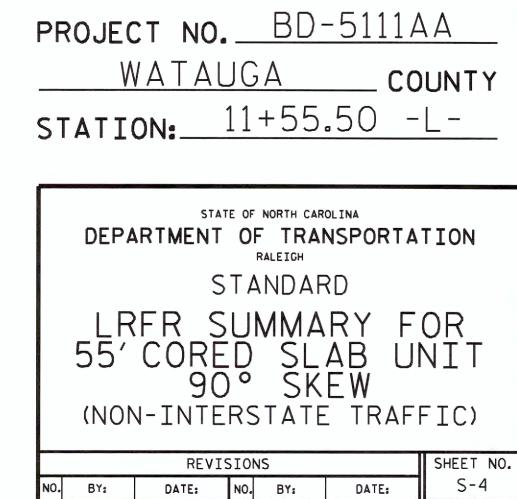
COMMENT

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

#### COMMENTS:

2.

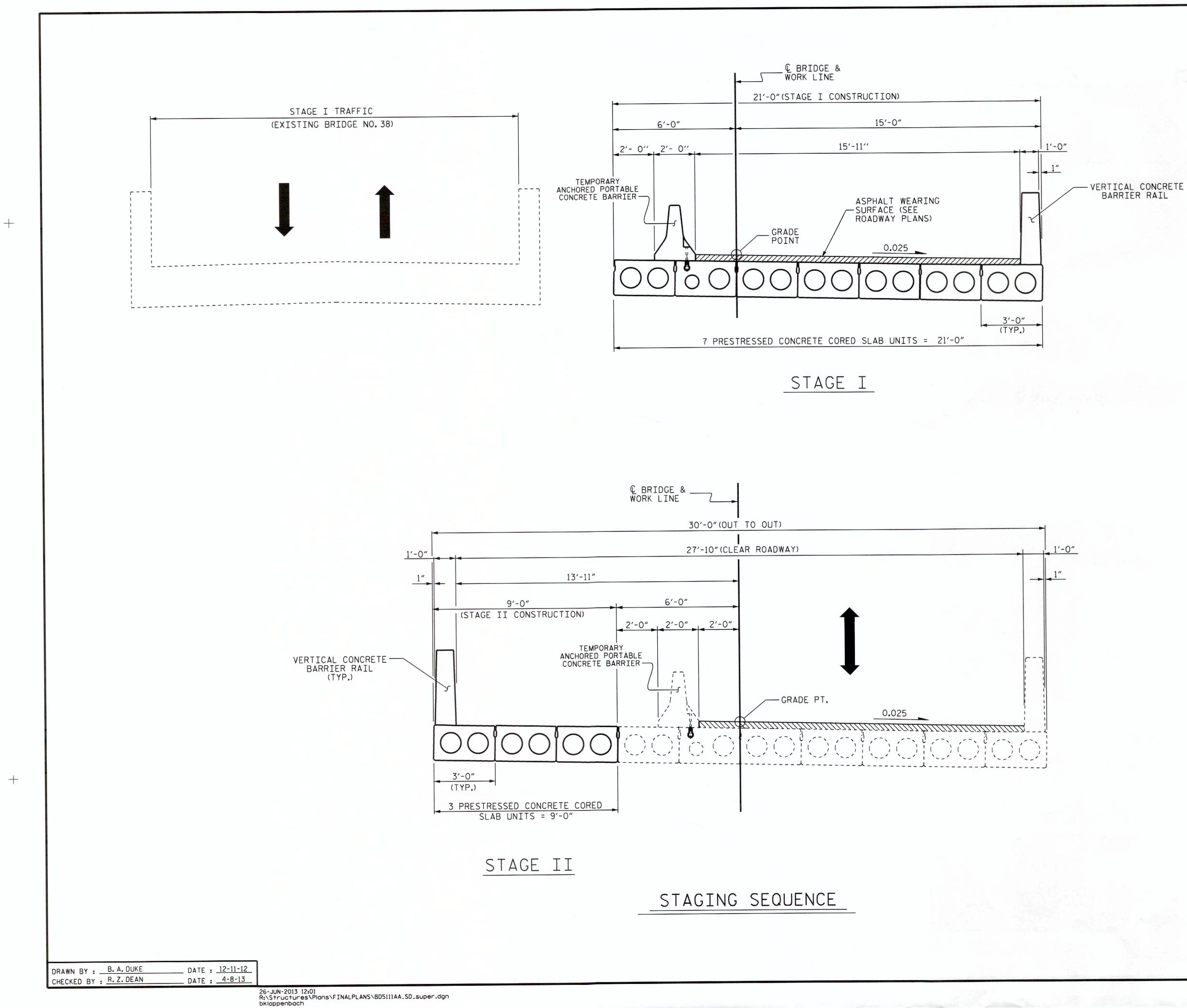
	(#) CONTROLLING LOAD RATING
The second se	1 DESIGN LOAD RATING (HL-93)
	2 DESIGN LOAD RATING (HS-20)
	3 LEGAL LOAD RATING **
and the second se	** SEE CHART FOR VEHICLE TYPE
	GIRDER LOCATION
	I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER



STD. NO. LRFR1

total sheets 22





NOTES

FOR TEMPORARY ANCHORED PORTABLE CONCRETE BARRIER, SEE TRAFFIC CONTROL PLANS.

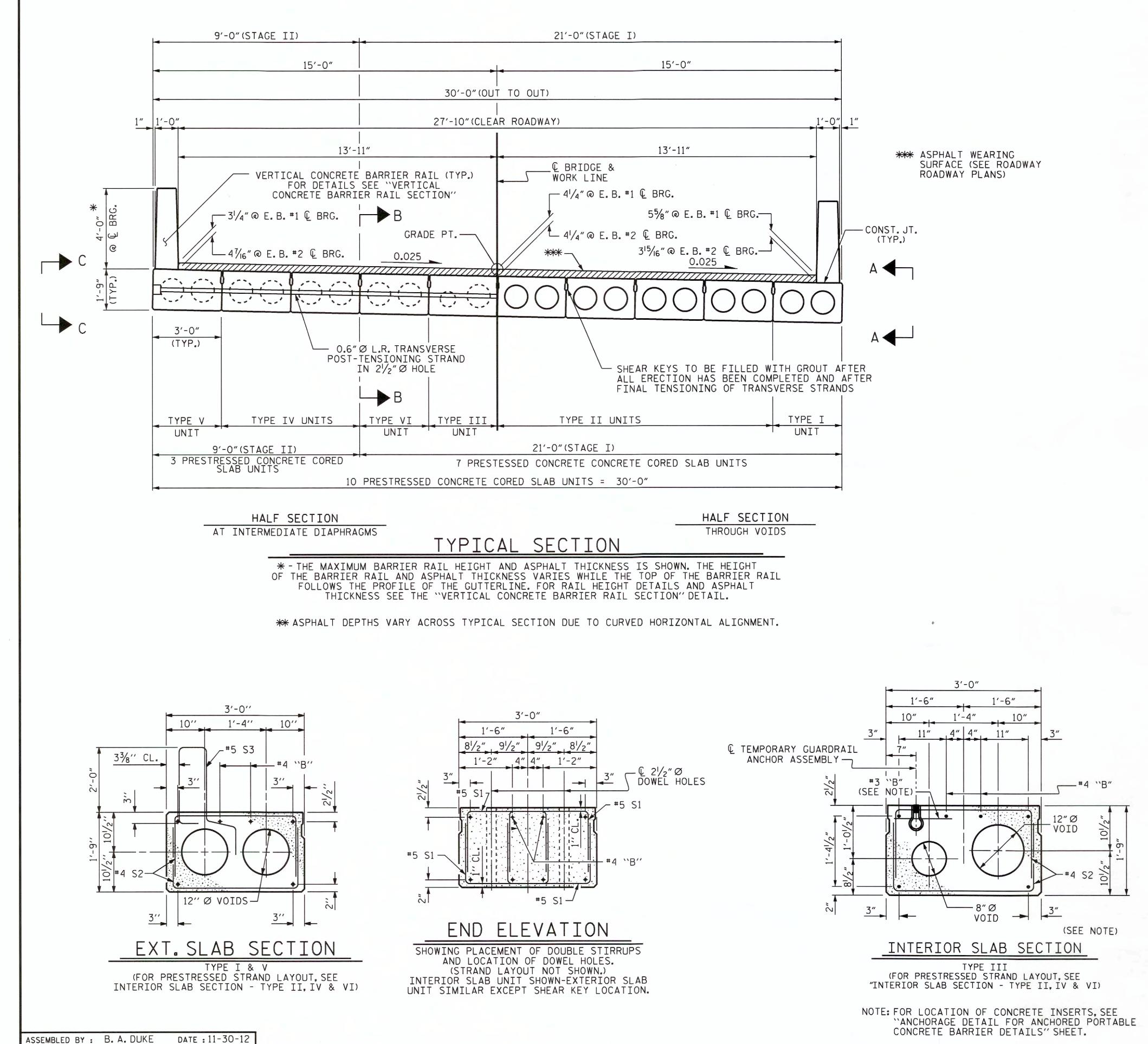
### PROJECT NO. \_\_\_\_BD-5111AA WATAUGA \_\_ COUNTY STATION: 11+55.50 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

# CONSTRUCTION STAGING

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-5
1			3			TOTAL SHEETS
2			4			22
				NCBD	9	





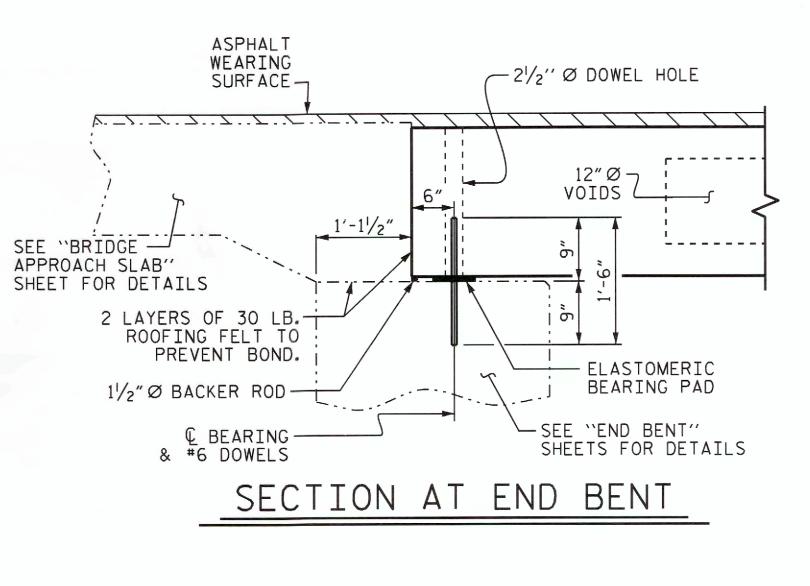
DRAWN BY : DGE CHECKED BY : BCH	5/09 6/09	REV. 12/11	MAA/AAC	DESIGN ENGINEER OF RECORD: B. A. DUKE	DATE :	4-22-13	
ASSEMBLED BY : CHECKED BY :	B. A. DU R. Z. DE		11-30-12 4-8-13				

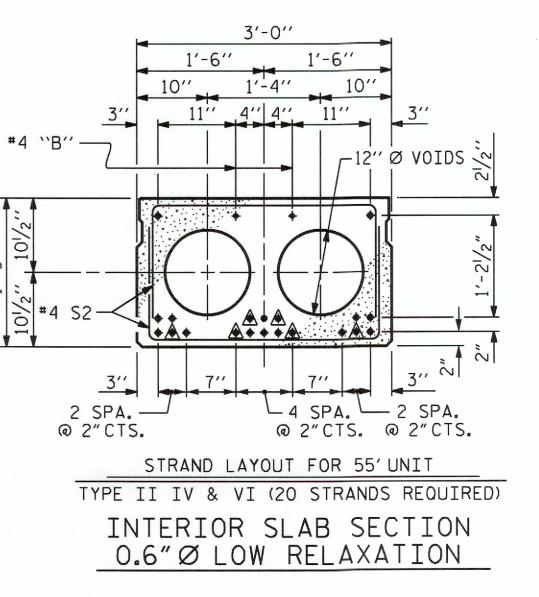
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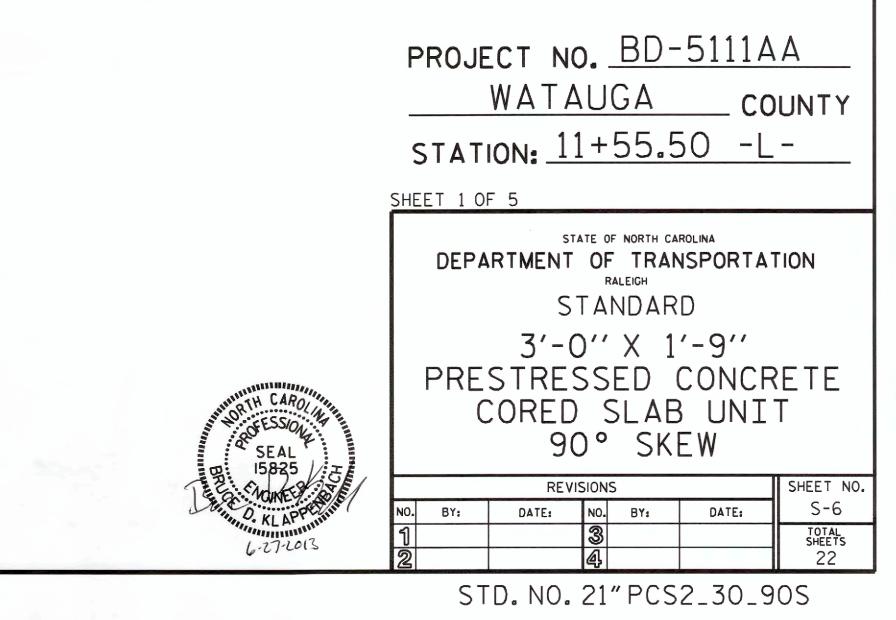
26-JUN-2013 12:01 R:\Structures\Plans\FINALPLANS\BD5111AA\_SD\_super.dgn bklappenbach

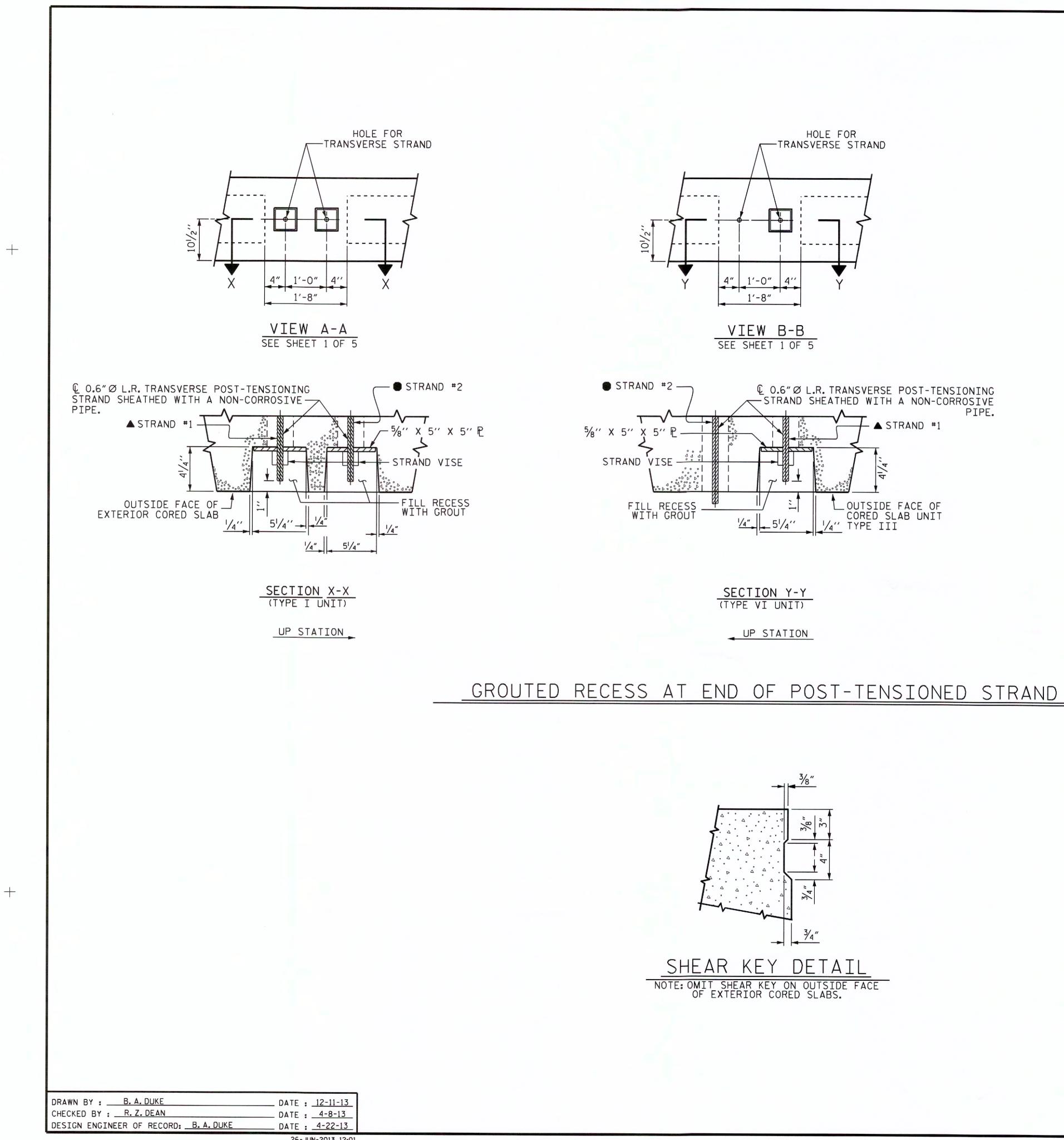
FIXED END



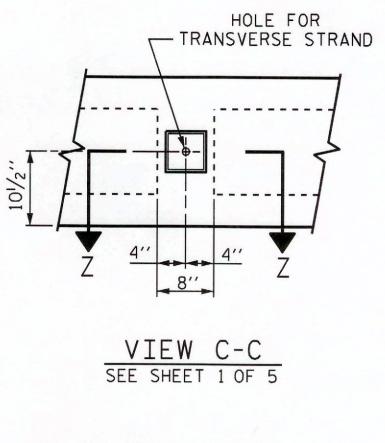


BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 8'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7. DEBONDING LEGEND

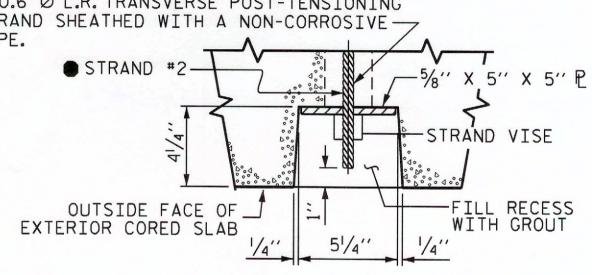




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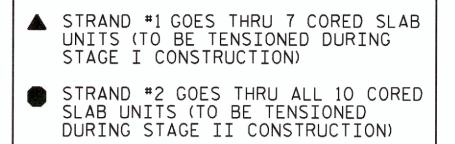


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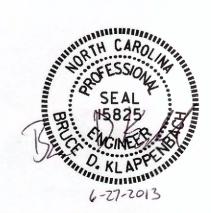


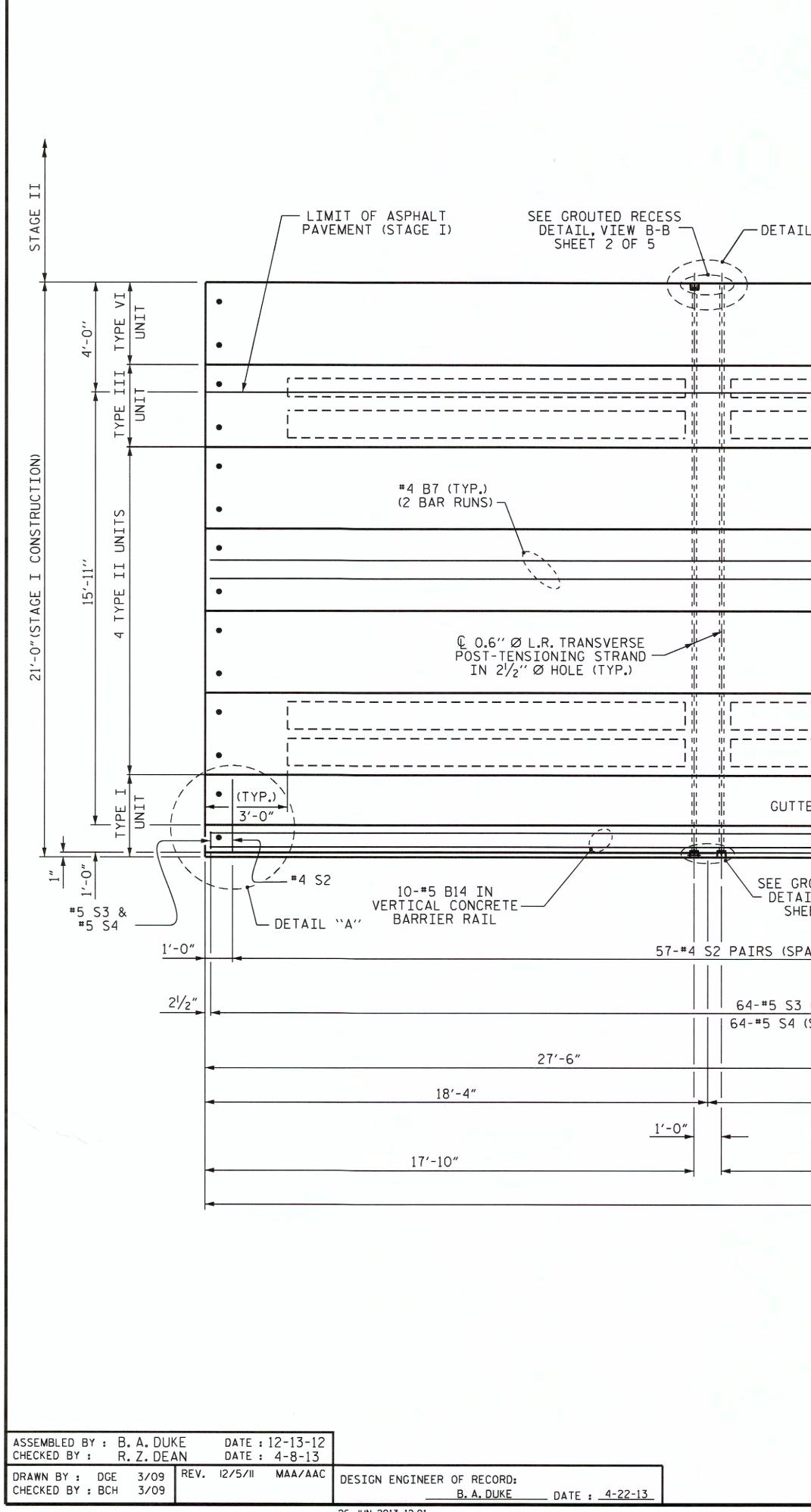
SECTION Z-Z (TYPE V UNIT)

UP STATION



#### PROJECT NO. BD- 5111AA WATAGUA \_ COUNTY STATION: 11+55.50 -L-SHEET 2 OF 5 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUPERSTRUCTURE 3'-0"X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT SHEET NO. REVISIONS NO. BY: S-7 BY: DATE: DATE: TOTAL SHEETS 22





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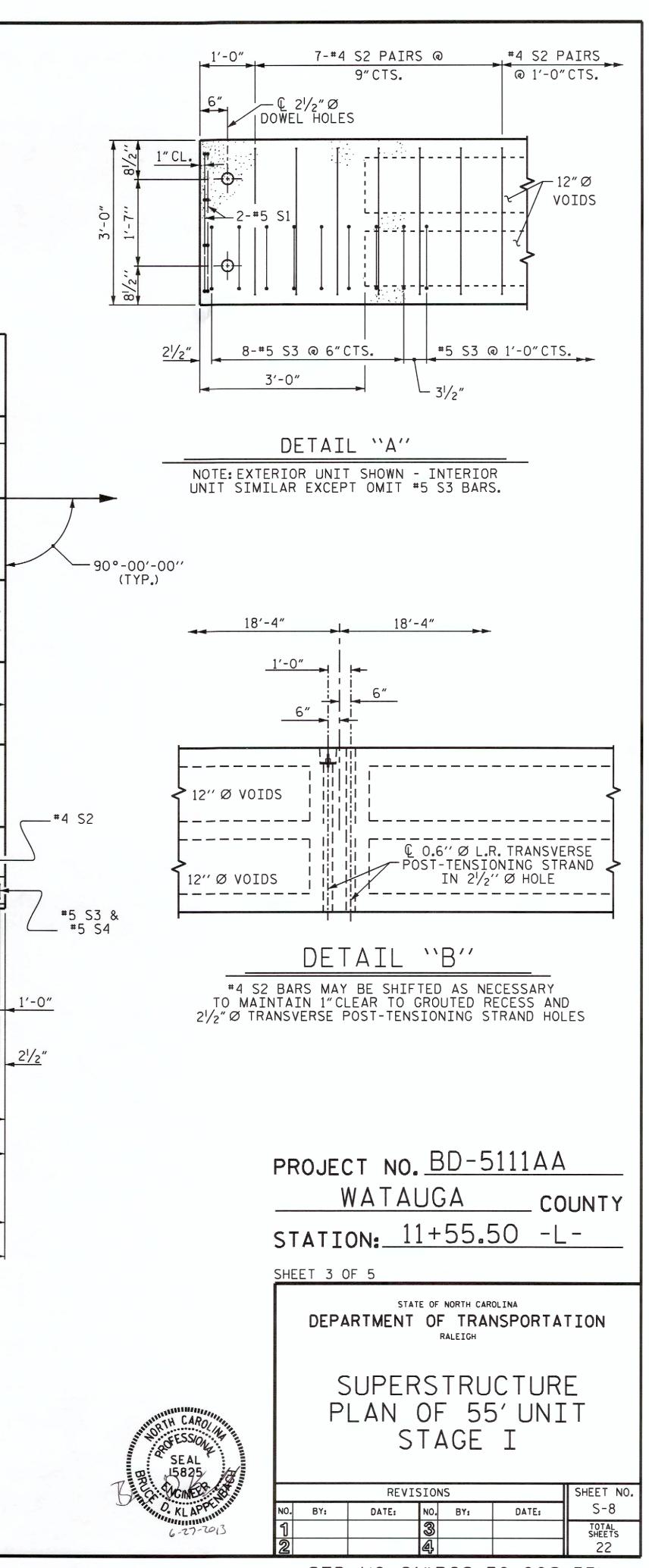
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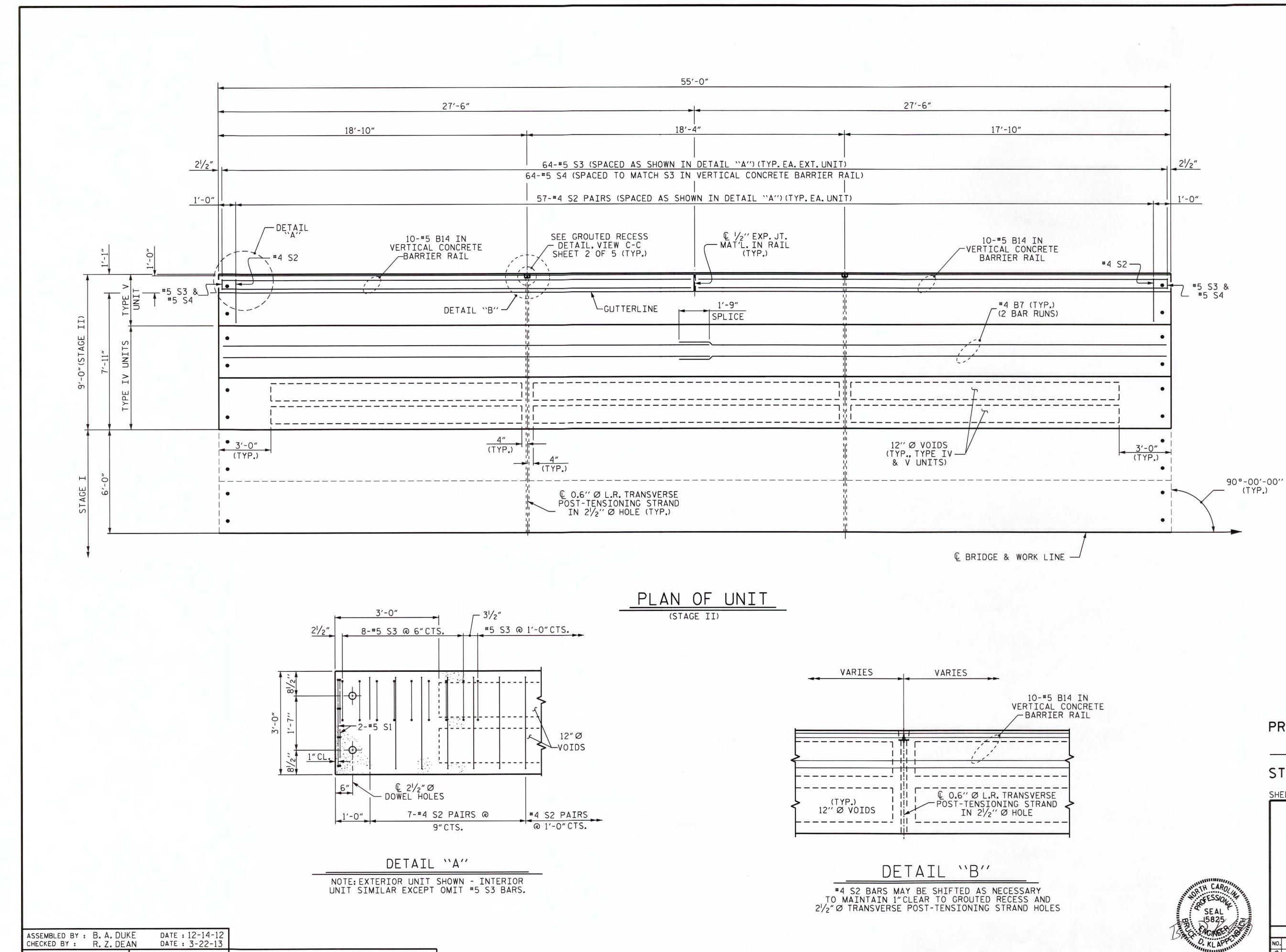
SEE GROUTED RECESS DETAIL, VIEW B-B - CONST.JTSHEET 2 OF 5		DETAIL "B"
8″Ø VOID ~		
-12"Ø VOID 1'-9" SPLICE		C BRIDGE & WORK LINE
	4 4 4 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	
12" Ø VOIDS (TYP., TYPE I & II UNITS)		4" (TYP.)
OUTED RECESS IL, VIEW A-A ET 2 OF 5 (TYP.)		SEE GROUTED RECESS 10-#5 B14 IN DETAIL, VIEW A-A VERTICAL CONCRETE SHEET 2 OF 5 BARRIER RAIL
ACED AS SHOWN IN DETAIL ``A'' PLAN SECTION)(TYP.	EA.	UNIT)
(SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA. UNIT) SPACED TO MATCH S3 IN VERTICAL BARRIER RAIL)		
	<u>   </u> 	27'-6"
18'-4"		18'-4"
		<u>1'-0"</u>
17'-4"		17'-10"
55'-0"	-	

PLAN OF UNIT

(STAGE I)



STD. NO. 21" PCS\_30\_90S\_55L



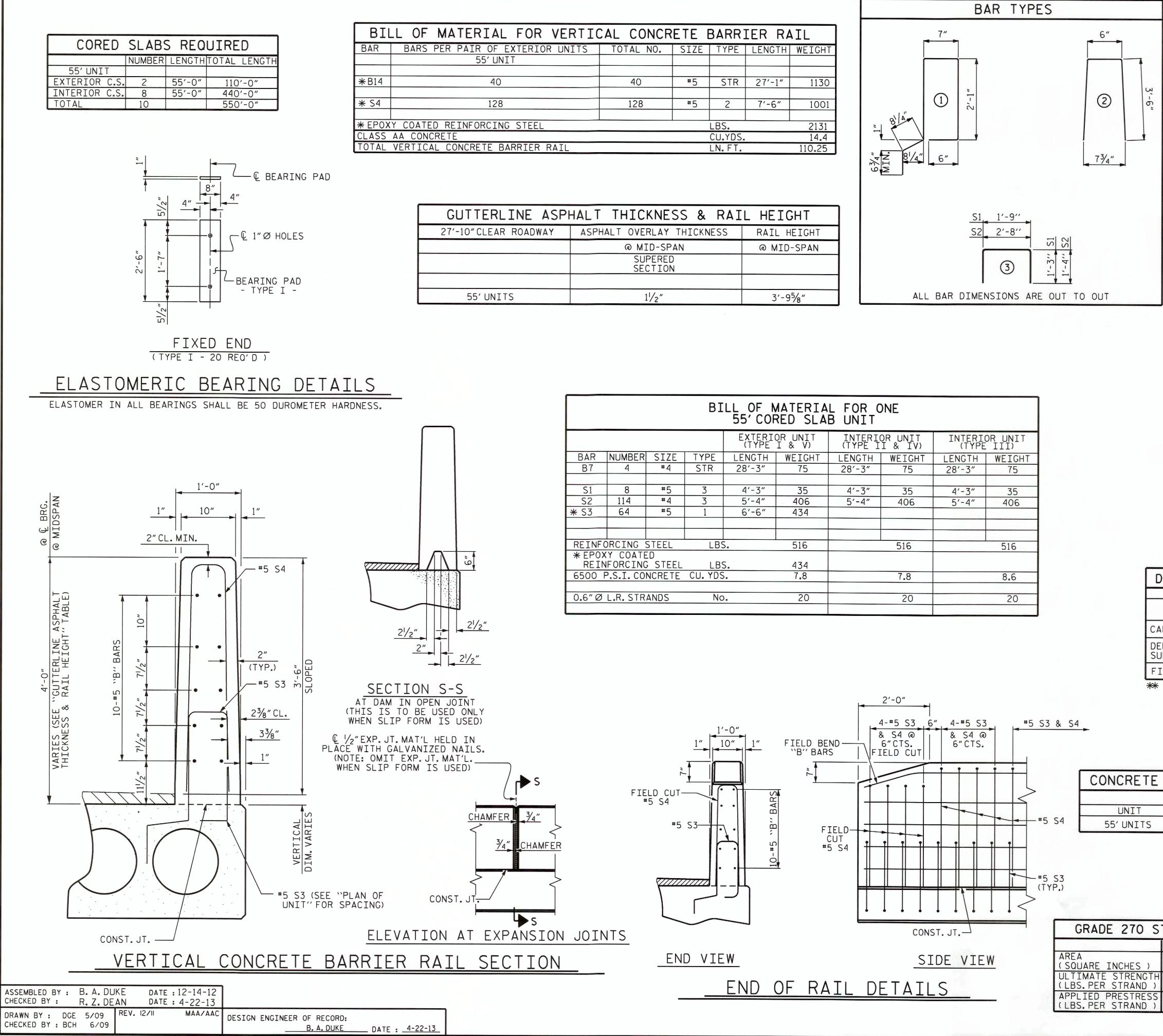
ASSEMBLED BY : B. A. DU CHECKED BY : R. Z. DE		
DRAWN BY : DGE 3/09 CHECKED BY : BCH 3/09	REV. 12/5/11 MAA/AAC	DESIGN ENGINEER OF RECORD: 
		05-JUN-2013 11:17 R:\Structures\Plans\FINALPLANS\BD5111AA_SD_super.dgn bklappenbach

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	PROJECT NO. <u>BD-5111AA</u> <u>WATAUGA</u> COUNTY STATION: <u>11+55.50</u> -L-								
SHEET 4 OF 5 STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATIO RALEIGH SUPERSTRUCTURE PLAN OF 55' UNIT STAGE II									
NCINC ST	REVISIONS SHEET NO. NO. BY: DATE: NO. BY: DATE: S-9								
6-5-2013	NO.     BY:     DATE:     NO.     BY:     DATE:     S-9       1     3     TOTAL SHEETS     TOTAL SHEETS     22								
	STD. NO. 21" PCS_30_90S_55L								



<sup>18-</sup>JUL-2013 09:08 R:\Structures\Plans\FINALPLANS\BD5111AA\_SD\_super.dgn

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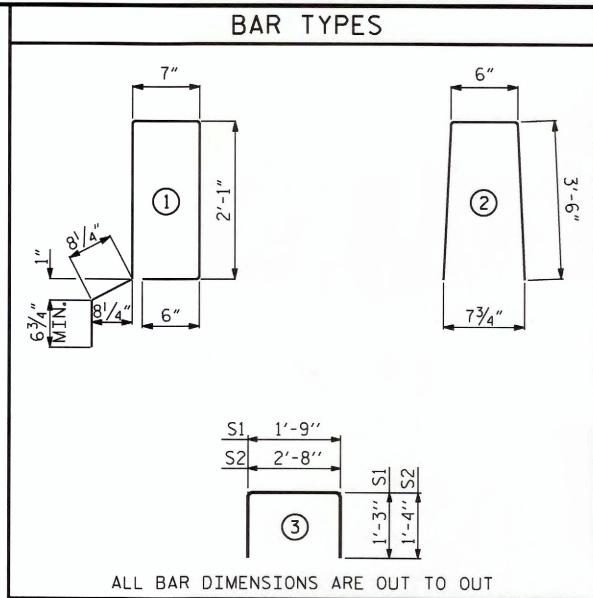
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bklappenbach

VERTICAL CONCRETE BARRIER RAIL								
R UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT			
	40	#5	STR	27'-1"	1130			
	100			71.0%	10.01			
	128	#5	2	7'-6″	1001			
			LBS.		2131			
			CU.YDS.					
DATI		and the second second second second			14.4			
RAIL			LN.FT.		110.25			

SPI	HALT THICKNESS & RAI	L HEIGHT
١Y	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
<u> </u>	@ MID-SPAN	@ MID-SPAN
	SUPERED SECTION	
	11/2″	3′-9 <sup>5</sup> ⁄8″



~~ 0	-		BI	LL OF N 55'COF	MATERIA RED SLA	L FOR C B UNIT	)NE		
				EXTERI (TYPE	OR UNIT I & V)	INTERI( (TYPE I	OR UNIT I & IV)	INTERI( (TYPE	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	LENGTH	WEIGHT
B7	4	#4	STR	28'-3"	75	28'-3"	75	28'-3"	75
S1	8	#5	3	4'-3"	35	4'-3"	35	4'-3"	35
S2	114	#4	3	5'-4"	406	5'-4"	406	5'-4"	406
* S3	64	#5	1	6′-6″	434			-	
REINF	ORCING S	STEEL	LBS		516		516		516
	(Y COATE NFORCINO		LBS		434				010
6500	P.S.I.CO	NCRETE	CU.YDS		7.8		7.8		8.6
0.6″Ø	L.R. STR	ANDS	No	a	20		20		20

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0"× 1'-9"
55' CORED SLAB UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2 <b>7⁄8″ ∳</b>
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	<sup>1</sup> ∕2″ ♦
FINAL CAMBER	2 <b>³⁄</b> 8″ 🕴
** INCLUDES FUTURE WEARING SURF	ACE

## NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

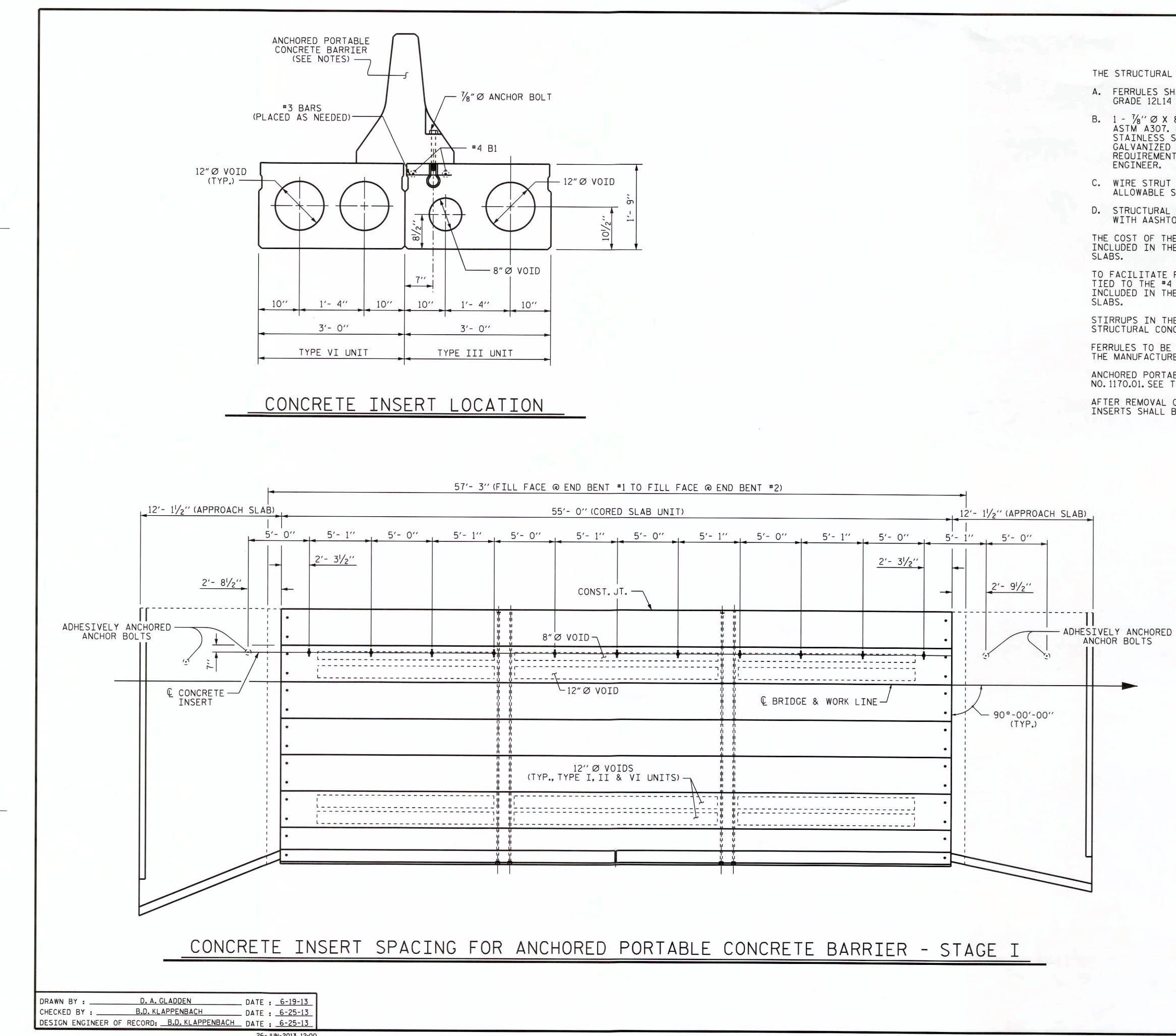
TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

CLUDES FUTURE WEARING SURFACE

RELEASE	STRENGTH PSI 4900	PROJECT NO. <u>BD-5111AA</u> <u>WATAUGA</u> COUNTY STATION: <u>11+55.50</u> -L-
<b>RANDS</b> 0.6″ØL.R. 0.217 58,600	AND THE CAROLINA THE THE CAROLINA THE THE THE CAROLINA THE THE CAROLINA THE	DEPARTMENT OF TRANSPORTATION RALEIGH 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW
43,950	15825 J.T	REVISIONS SHEET NO.
15	KLAPPINI	NO.     BY:     DATE:     NO.     BY:     DATE:     S-10       1     3     3     TOTAL SHEETS 22
<b>(</b>		STD.NO.21"PCS3_30_90S



<sup>26-</sup>JUN-2013 12:00 R:\Structures\Plans\FINALPLANS\BD5111AA\_SD\_super.dgn bklappenbach

- WITH AASHTO M111.

- THE MANUFACTURER.

#### NOTES

THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS: A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS\_OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF 15/8".

B. 1 - 7/8" Ø X 81/2" BOLT WITH WASHER. BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307. BOLT AND WASHER SHALL BE GALVANIZED. AT THE CONTRACTORS OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø X 81/2" GALVANIZED BOLT AND WASHER. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE

C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. D. STRUCTURAL CONCRETE INSERT ASSEMBLIES SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE

THE COST OF THE STRUCTURAL CONCRETE INSERT ASSEMBLY, COMPLETE IN PLACE, SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-O" × 1'-9" PRESTRESSED CONCRETE CORED

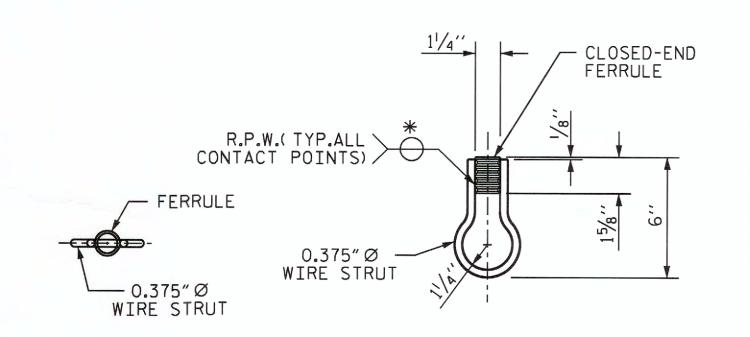
TO FACILITATE PLACEMENT OF STRUCTURAL CONCRETE INSERT ASSEMBLIES, #3 BARS MAY BE TIED TO THE #4 B1 BARS IN THE CORED SLAB UNITS. THE COST OF THE #3 BARS SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR 3'-O" × 1'-9" PRESTRESSED CONCRETE CORED

STIRRUPS IN THE CORED SLAB UNITS MAY BE SHIFTED SLIGHTLY AS NECESSARY TO CLEAR STRUCTURAL CONCRETE INSERT ASSEMBLIES.

FERRULES TO BE PLUGGED DURING CASTING OF THE CORED SLAB UNITS AS RECOMMENDED BY

ANCHORED PORTABLE CONCRETE BARRIER SHALL BE AS SPECIFIED IN ROADWAY STANDARD NO. 1170.01. SEE TRAFFIC CONTROL PLANS.

AFTER REMOVAL OF ANCHORED PORTABLE CONCRETE BARRIER, THE STRUCTURAL CONCRETE INSERTS SHALL BE FILLED WITH GROUT.



PLAN

ELEVATION

STRUCTURAL CONCRETE INSERT

\* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

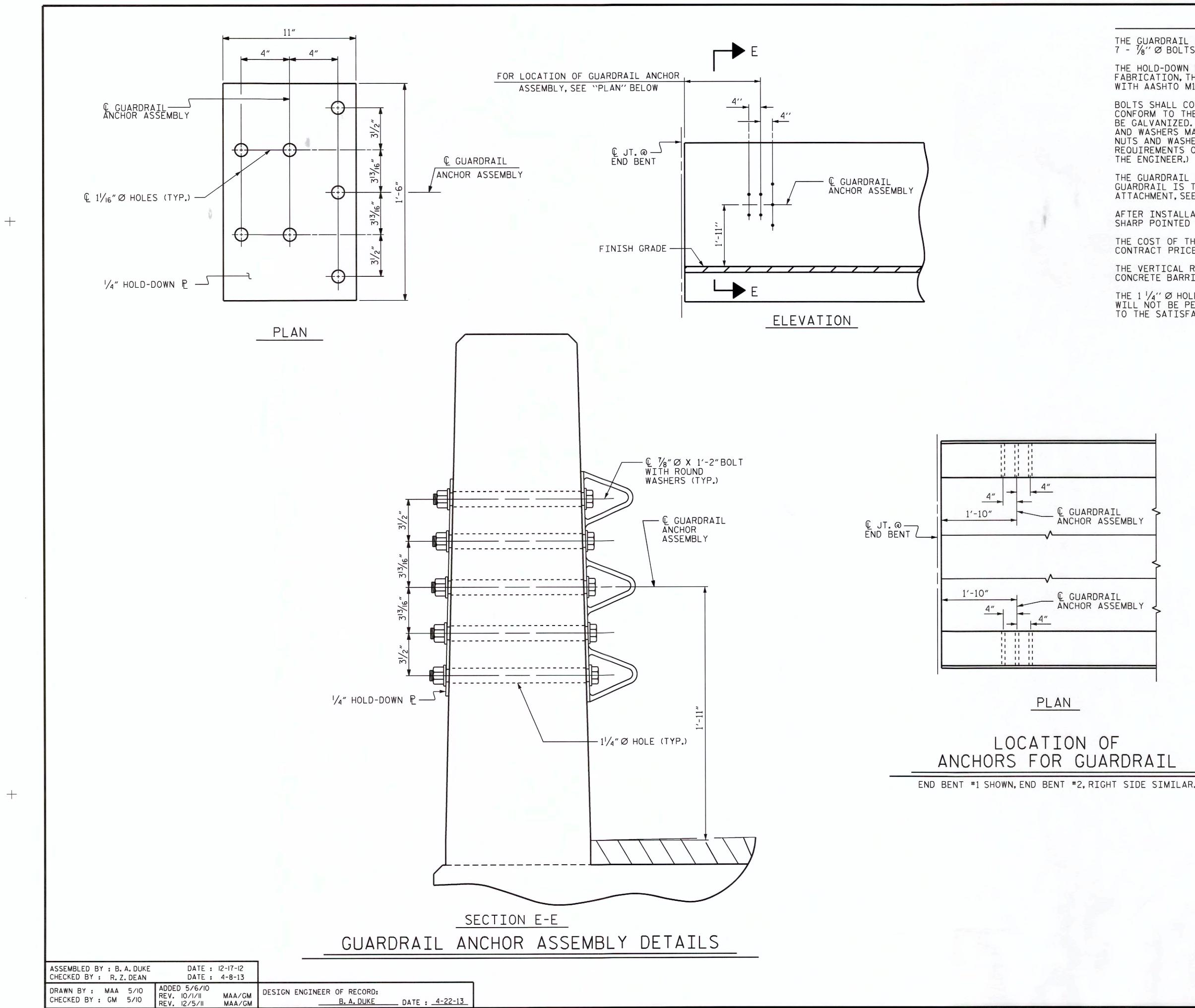
PROJECT NO. BD-5111AA WATAUGA \_ COUNTY STATION: 11+55.50 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

ANCHORAGE DETAIL FOR ANCHORED PORTABLE CONCRETE BARRIER DETAILS

	SHEET NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-11
1			3			TOTAL SHEETS
2			4			22





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#### NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $1/4^{\prime\prime}$  HOLD DOWN PLATE AND 7 -  $7/8^{\prime\prime}$  Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS.THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307.THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

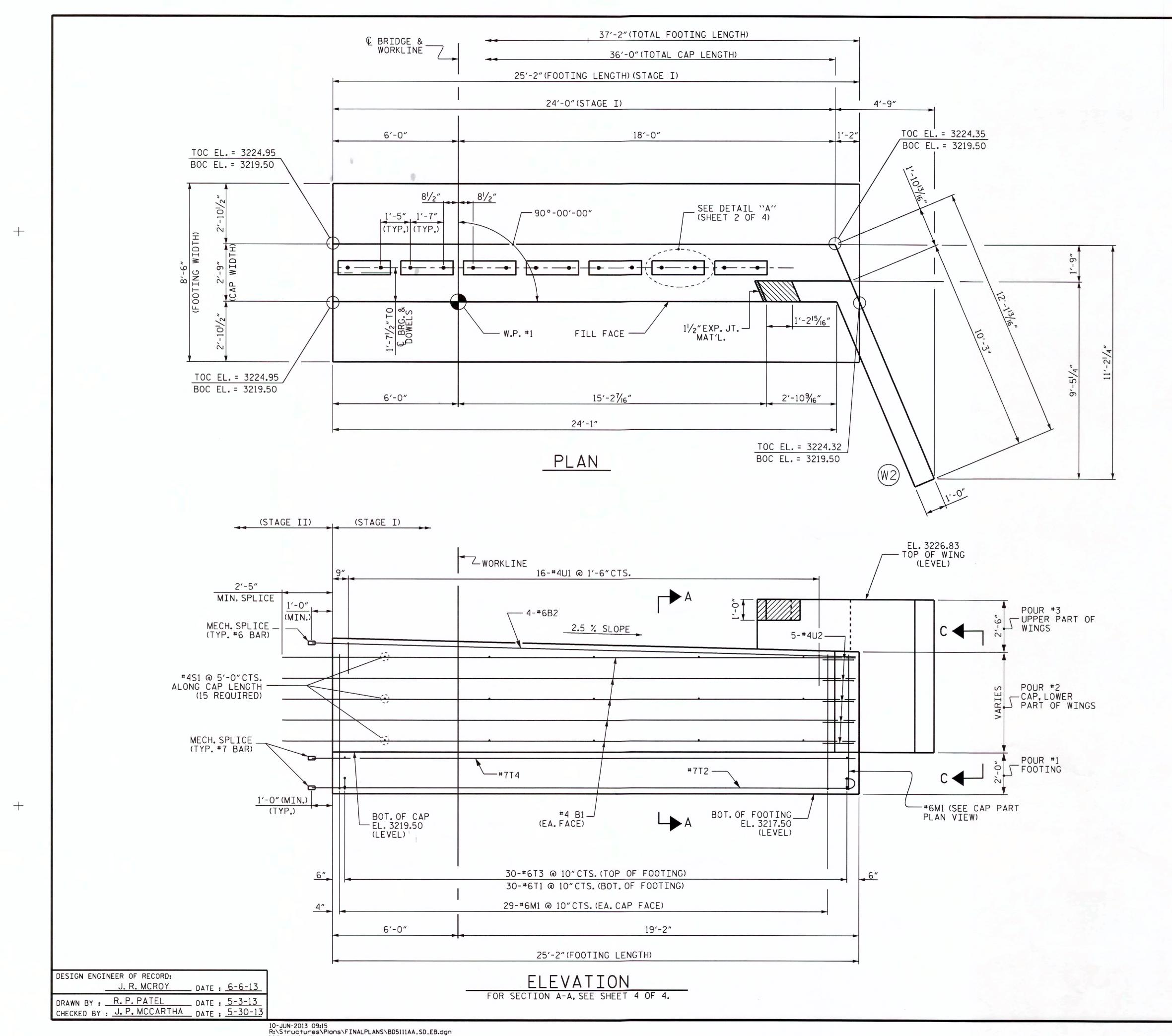
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

*	Q JT.@ END BENT #1 ** *
POINT * DENOTES ** LOCATION	ETCH SHOWING S OF ATTACHMENT S GUARDRAIL ANCHOR ASSEMBLY ON OF IMPACT ATTENUATOUR ATTACHMENT ADWAY PLANS AND SPECIAL PROVISIONS TAIL AND PAY ITEM.
	PROJECT NO. <u>BD-5111AA</u> <u>WATAUGA</u> county station: <u>11+55.50</u> -L-
OR SEAL HILL	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD GUARDRAIL ANCHORAGE FOR VERTICAL CONCRETE BARRIER RAIL
CAT GINET	REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:S-1213

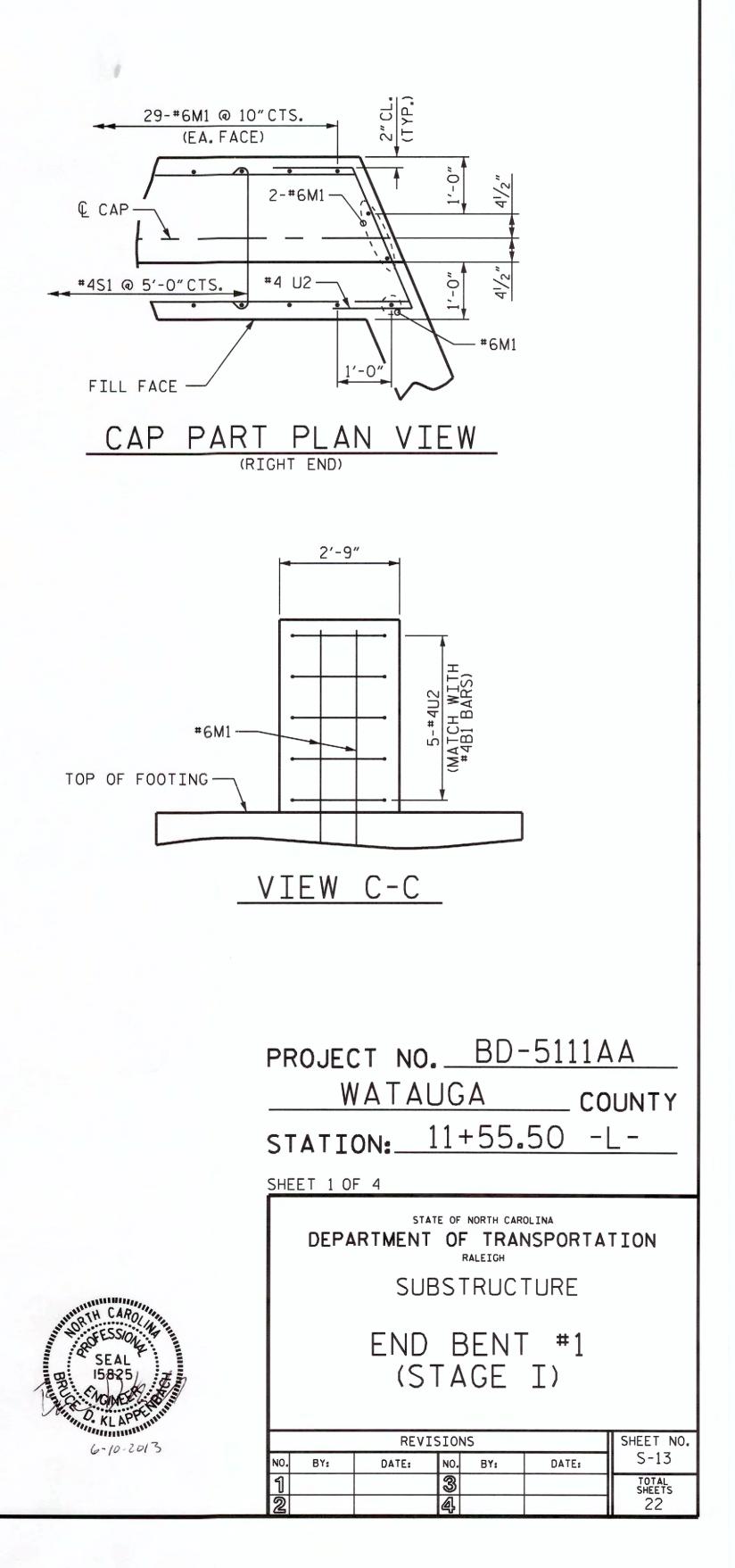


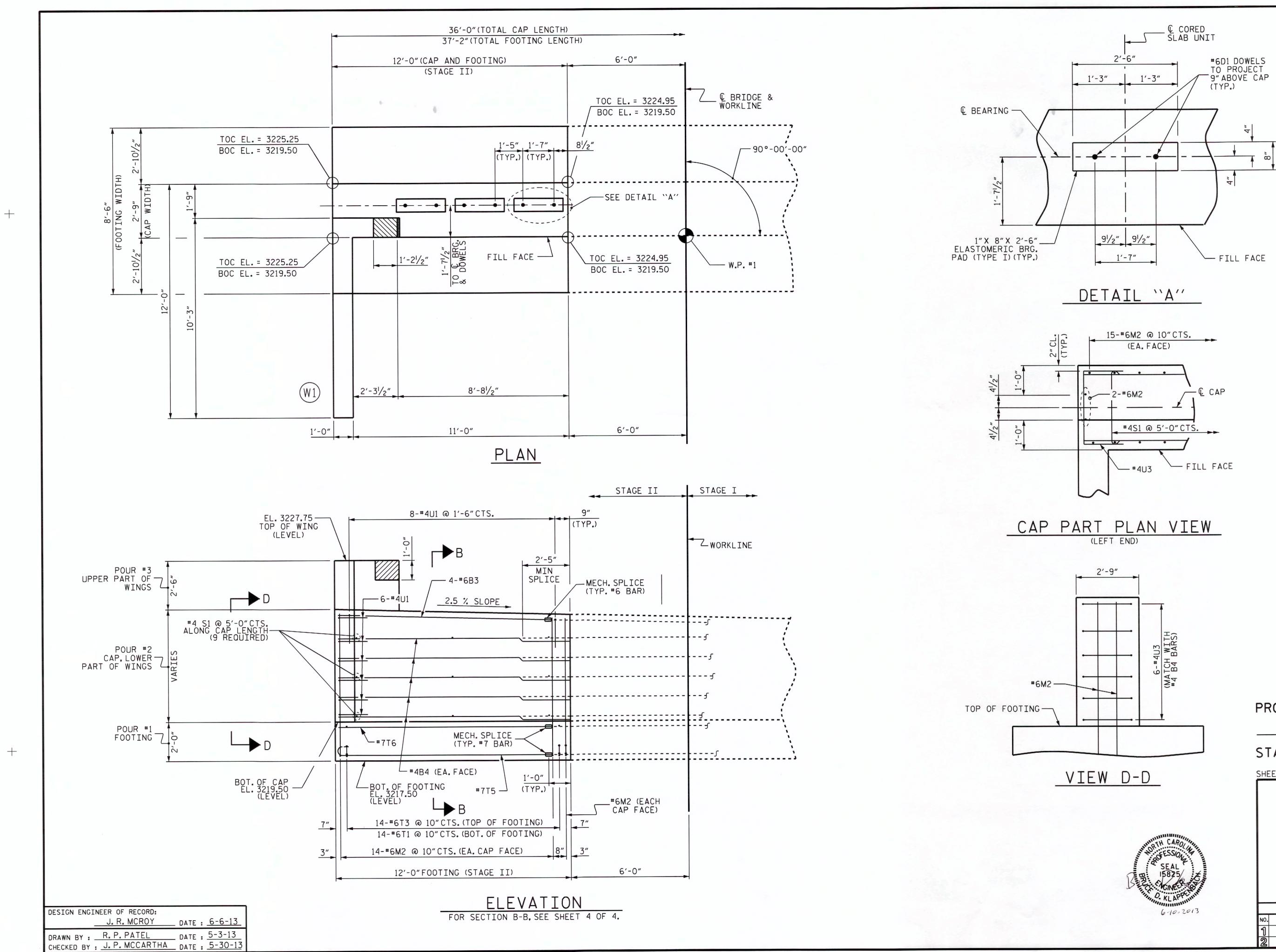
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#### NOTES

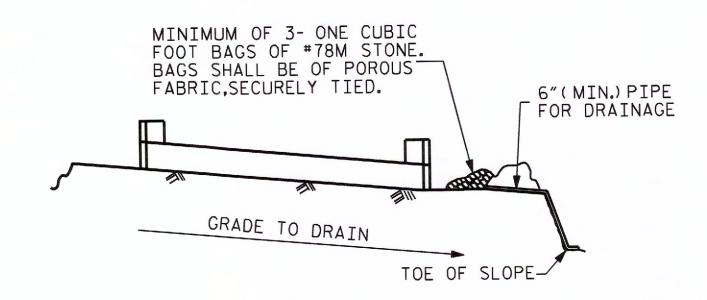
THE TOP SURFACE AREAS OF THE END BENT CAPS SHALL BE CURED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS EXCEPT THAT THE MEMBRANE CURING COMPOUND METHOD SHALL NOT BE USED.

THE CONTRACTOR SHALL PROVIDE FOR INSTALLATION OF THE 4"DIAMETER DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE. THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED. FOR WING DETAILS. SEE SHEET 3 OF 4.





PROJECT NO. <u>BD-5111AA</u> <u>WATAUGA</u> county STATION: <u>11+55.50</u> -L- SHEET 2 OF 4							
DEPARTMENT OF TRANSPORTATION RALEIGH							
SUBSTRUCTURE							
END BENT #1 (STAGE II)							
REVISIONS SHEET NO							
NO. BY: DATE: NO. BY: DATE: S-14							
1 3 TOTAL 2 4 2 22							



BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

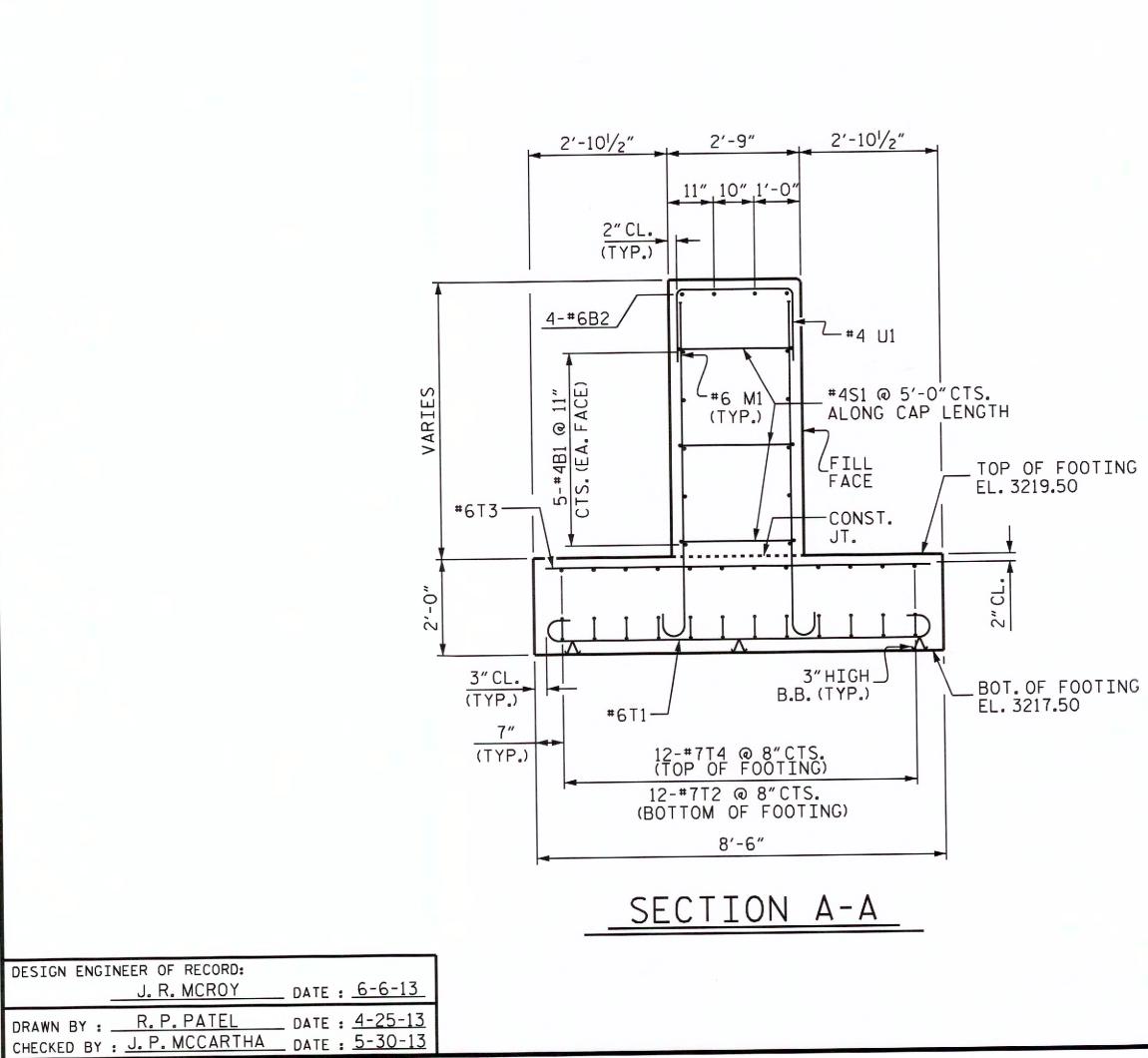
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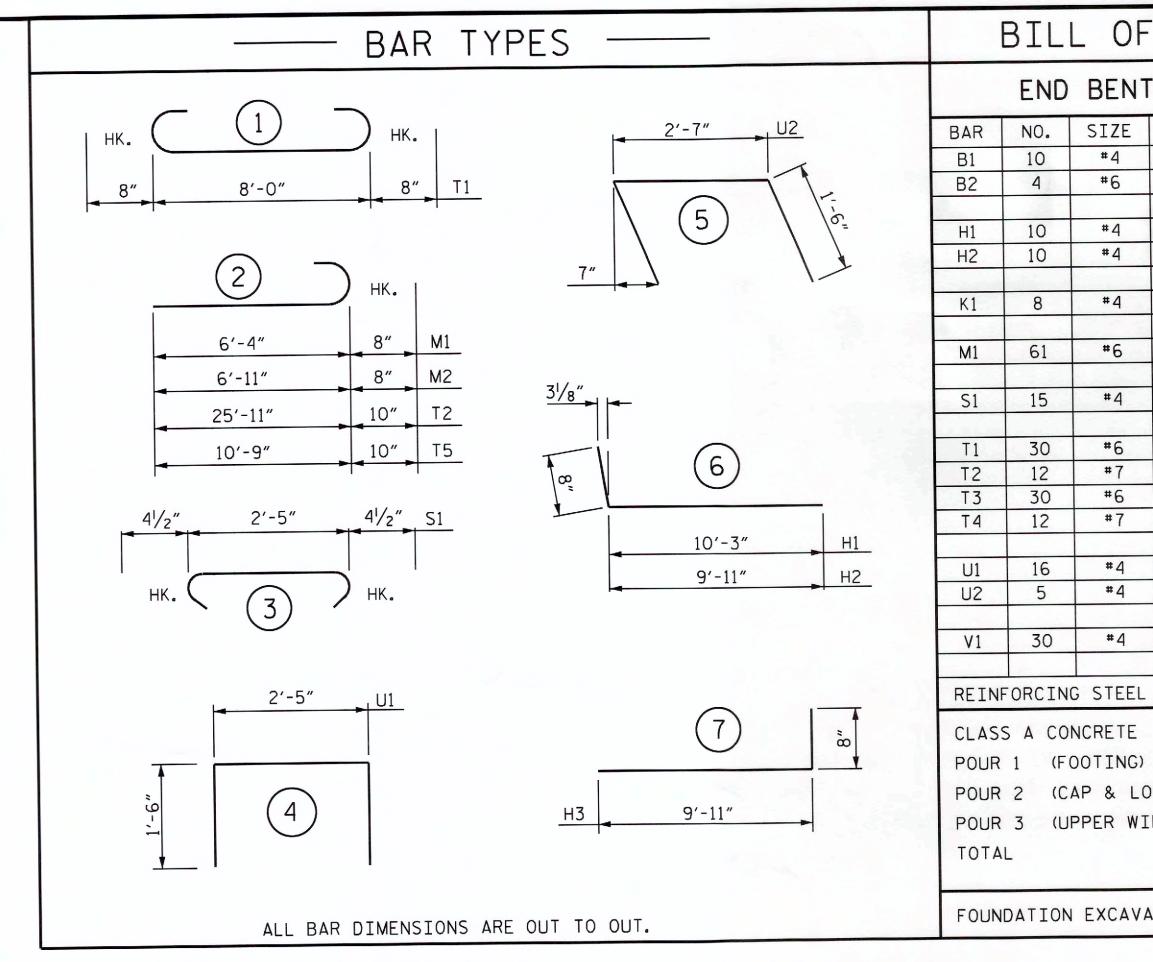
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

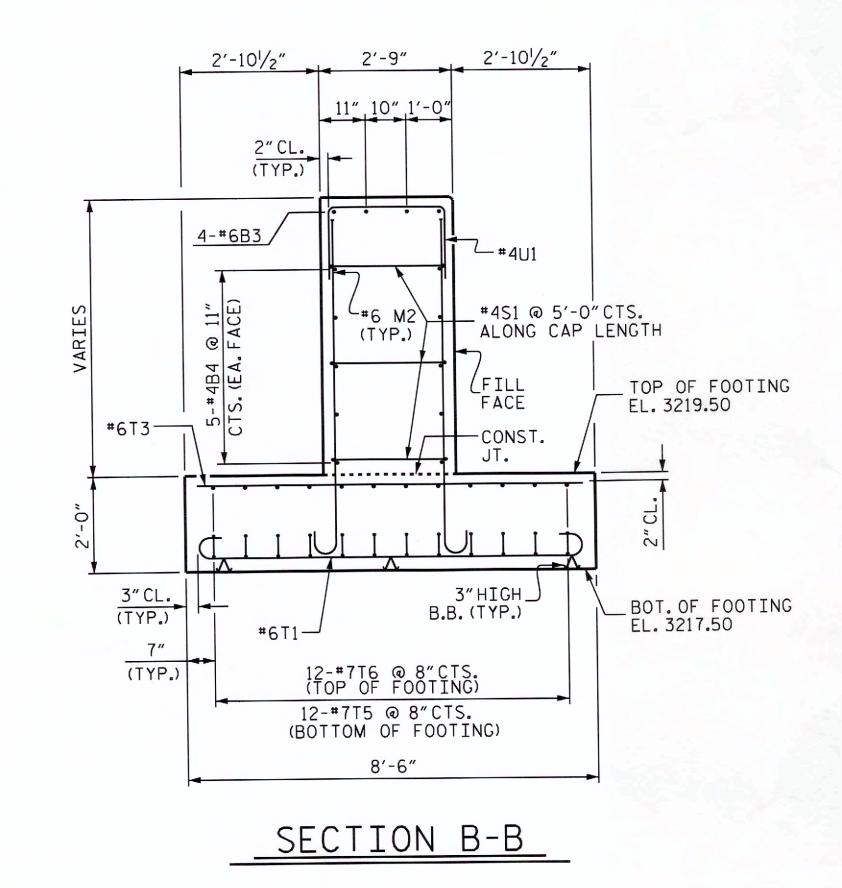
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

## TEMPORARY DRAINAGE AT END BENT



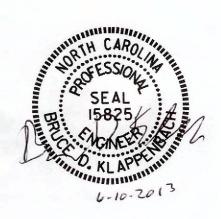
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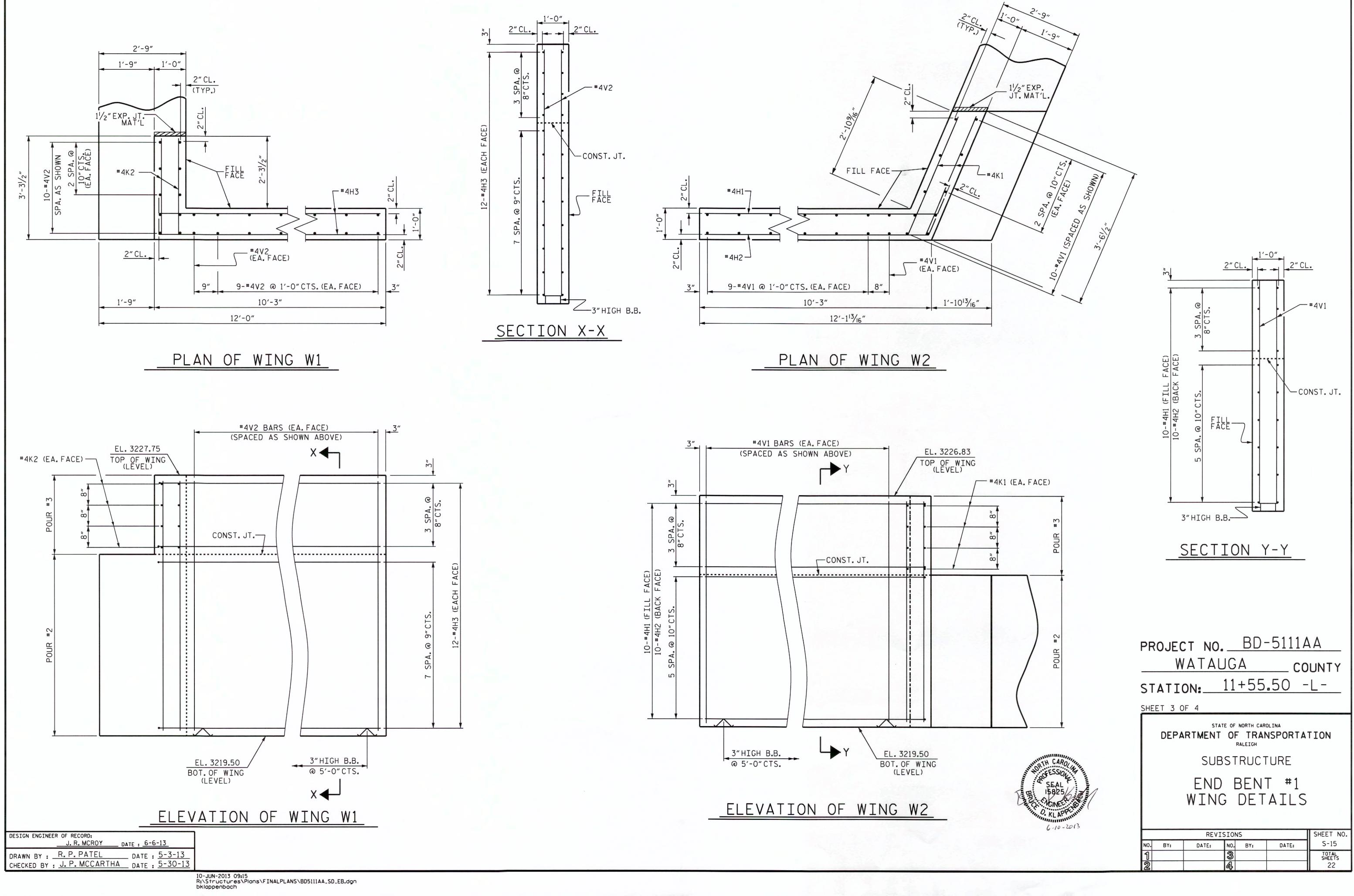


F	MA	TERIA	AL		BIL	L OF	MA	TERIA	L
T #1 (STAGE I)					END	BENT	#1 (S	TAGE I	I)
Т	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
+	STR	27'-5"	183	B3	4	#6	STR	10'-10"	65
	STR	26'-0"	156	B4	10	#4	STR	11'-10"	79
1									
	6	10'-11"	73	Н3	24	#4	7	10'-7"	170
	6	10'-7"	71						
1				К2	8	#4	STR	2'-11"	16
	STR	3'-2"	17						
				M2	32	#6	2	7'-7"	364
	2	7'-0"	641	16.00					10
				S1	9	#4	3	3'-2"	19
	3	3'-2"	32						100
				Τ1	14	#6	1	9'-4"	196
	1	9'-4"	421	Т3	14	*6	STR	8'-2"	172
	2	26'-9"	656	Т5	12	#7	2	11'-7"	284
	STR	8'-2"	368	Т6	12	#7	STR	10'-10"	266
	STR	26'-0"	638					<b>F</b> / <b>F</b> //	E 1
				U1	14	#4	4	5′-5″	51
1	4	5′-5″	58				GTD	7/ 11//	150
	5	5'-7"	19	V2	30	#4	STR	7'-11"	159
	STR	7'-0"	140						
									10.41
L		LBS.	3473	REIN	FORCIN	G STEEL		LBS.	1841
				CLAS	S A CO	NCRETE			
						DOTING)			7.6 C.Y.
;)			15.8 C.Y.	POUR					
0	WER WI	NGS)	14.5 C.Y.	POUR	2 (C	AP & LO	WER WI	NGS)	8.8 C.Y.
III	NGS)		1.2 C.Y.	POUR	3 (U	PPER WI	NGS)		1.2 C.Y.
			31.5 C.Y.	TOTAL					17.6 C.Y.
/ A	TION		LUMP SUM	FOUN	DATION	EXCAVA	TION		LUMP SUM

PROJEC				-51		
	NATA		Aر	(		UNTY
STATIC	)N:	<u>11</u> .	+55.	50	-	L -
SHEET 4 OF	- 4					
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	END DI		SENT AIL			
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NO. BY:	DATE:	NO.	BY:	DATE		
1		3 4			_	TOTAL SHEETS 22

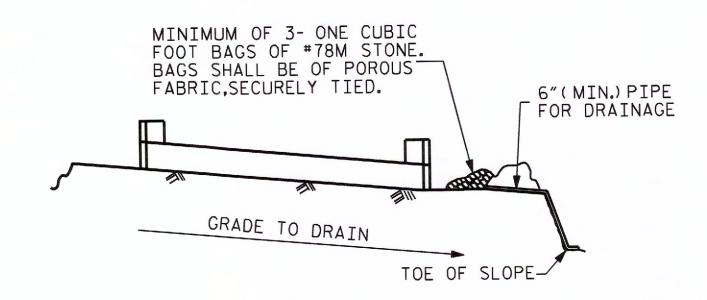


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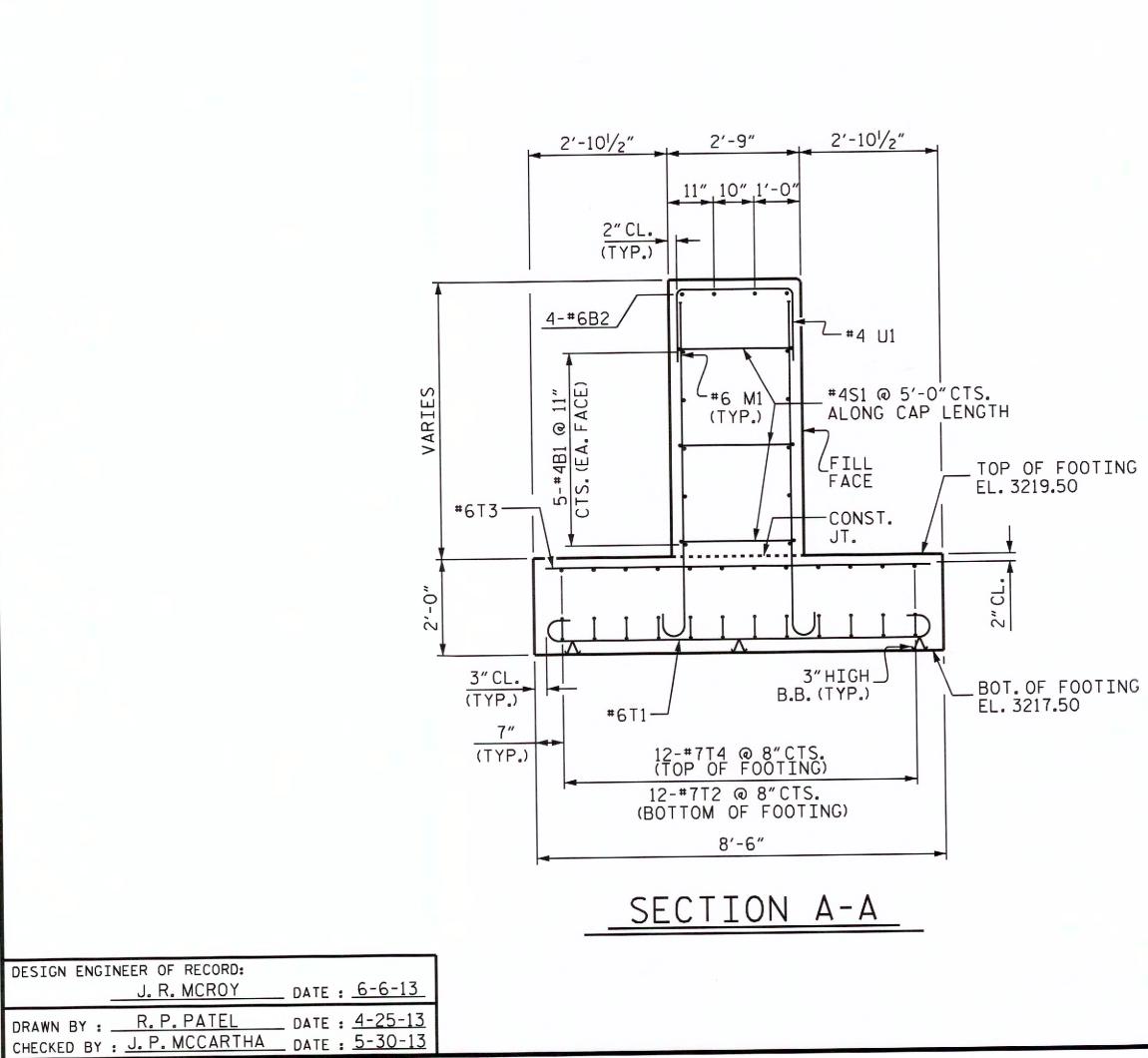
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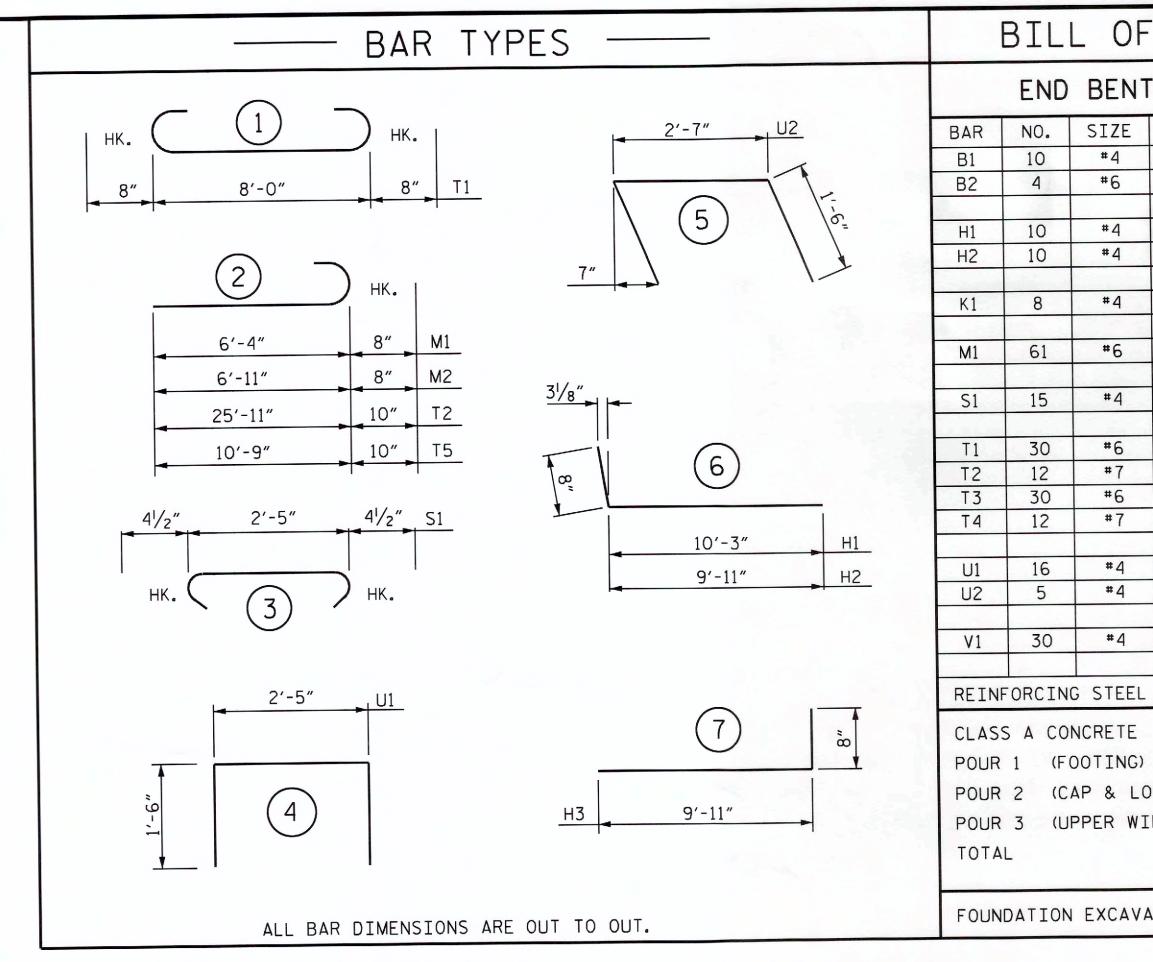
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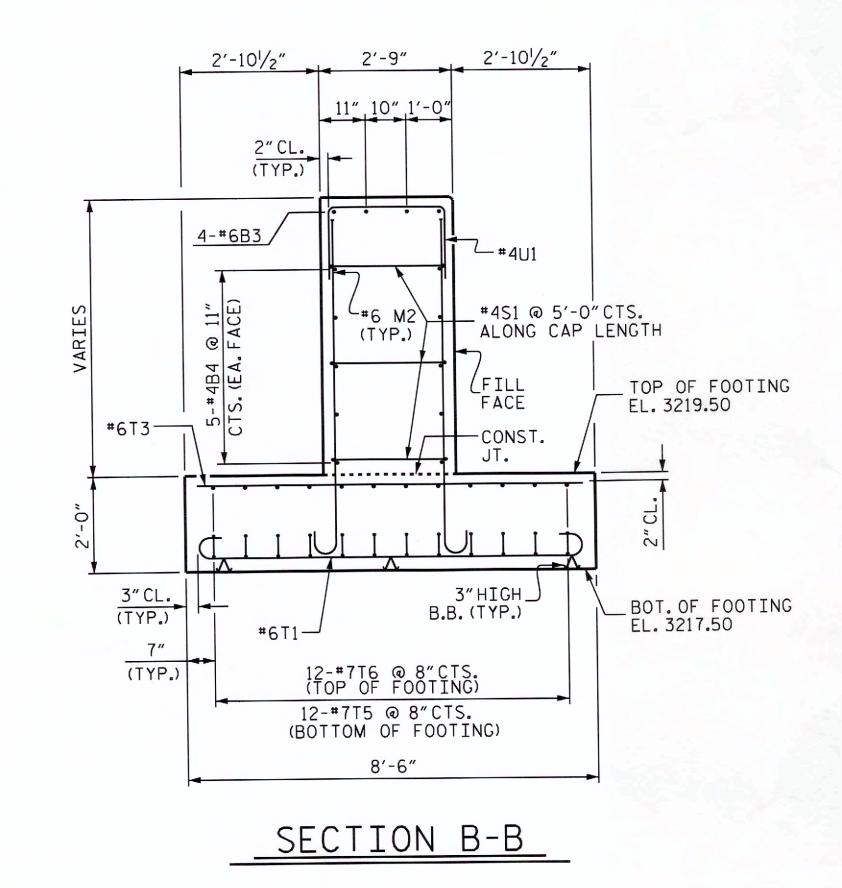
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## TEMPORARY DRAINAGE AT END BENT



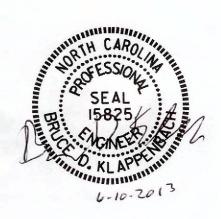
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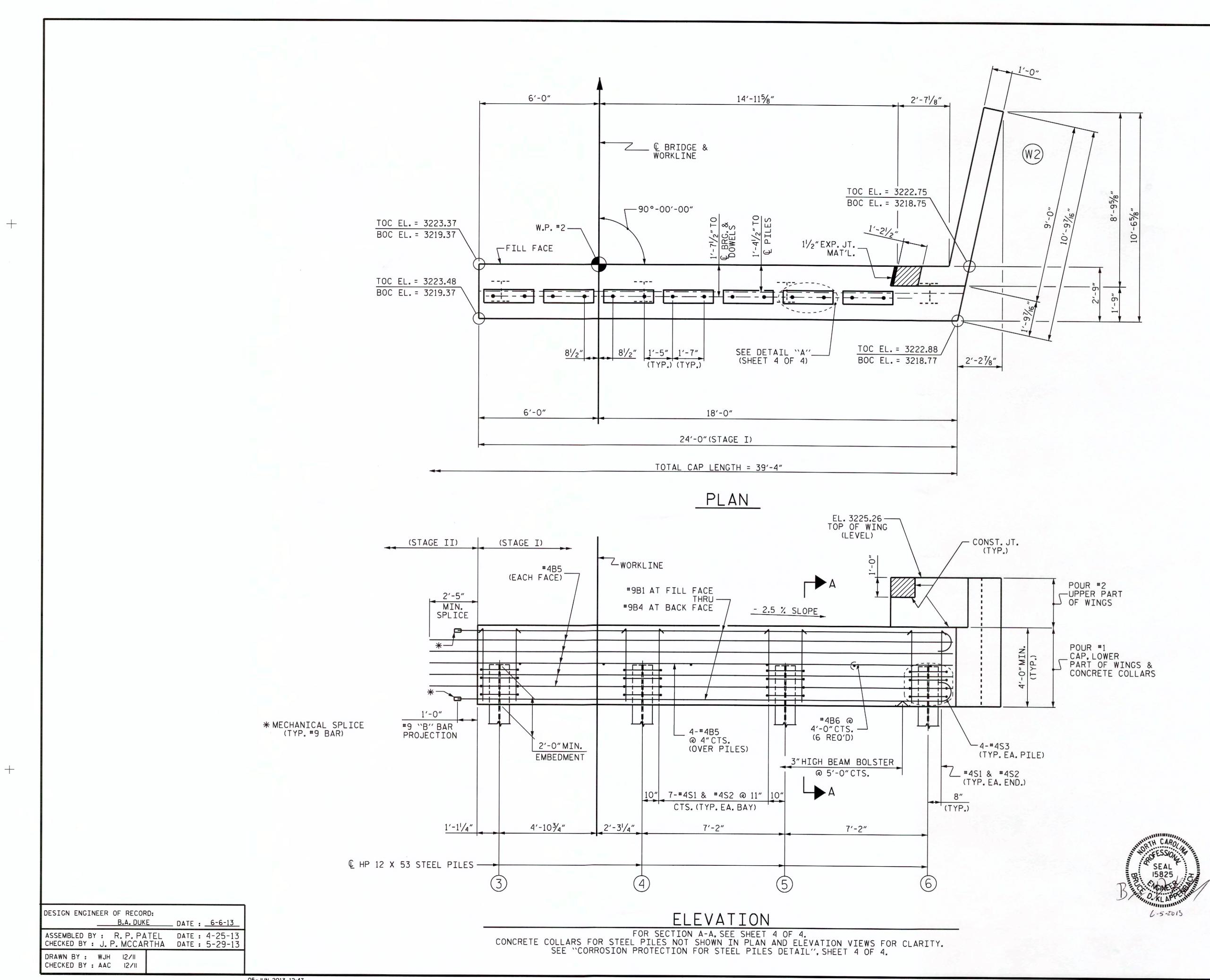


F	MA	TERIA	AL		BIL	L OF	MA	TERIA	L
T #1 (STAGE I)					END	BENT	#1 (S	TAGE I	I)
Т	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
+	STR	27'-5"	183	B3	4	#6	STR	10'-10"	65
	STR	26'-0"	156	B4	10	#4	STR	11'-10"	79
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	STR	3'-2"	17						
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	2	7'-0"	641	16.00					10
				S1	9	#4	3	3'-2"	19
	3	3'-2"	32						100
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	1	9'-4"	421	Т3	14	*6	STR	8'-2"	172
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	STR	8'-2"	368	Т6	12	#7	STR	10'-10"	266
	STR	26'-0"	638					<b>F</b> / <b>F</b> //	E 1
				U1	14	#4	4	5′-5″	51
1	4	5′-5″	58				GTD	7/ 11//	150
	5	5'-7"	19	V2	30	#4	STR	7'-11"	159
	STR	7'-0"	140						
									10.41
L		LBS.	3473	REIN	FORCIN	G STEEL		LBS.	1841
				CLAS	S A CO	NCRETE			
						DOTING)			7.6 C.Y.
;)			15.8 C.Y.	POUR					
0	WER WI	NGS)	14.5 C.Y.	POUR	2 (C	AP & LO	WER WI	NGS)	8.8 C.Y.
III	NGS)		1.2 C.Y.	POUR	3 (U	PPER WI	NGS)		1.2 C.Y.
			31.5 C.Y.	TOTAL					17.6 C.Y.
/ A	TION		LUMP SUM	FOUN	DATION	EXCAVA	TION		LUMP SUM

PROJEC				-51		
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SHEET 4 OF	- 4					
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	SUB	STI	RUCT	URE		
	END DI		BENT AIL			
	REVI	SION	IS			SHEET NO S-16
NO. BY:	DATE:	NO.	BY:	DATE		
1		3 4			_	TOTAL SHEETS 22



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## NOTES

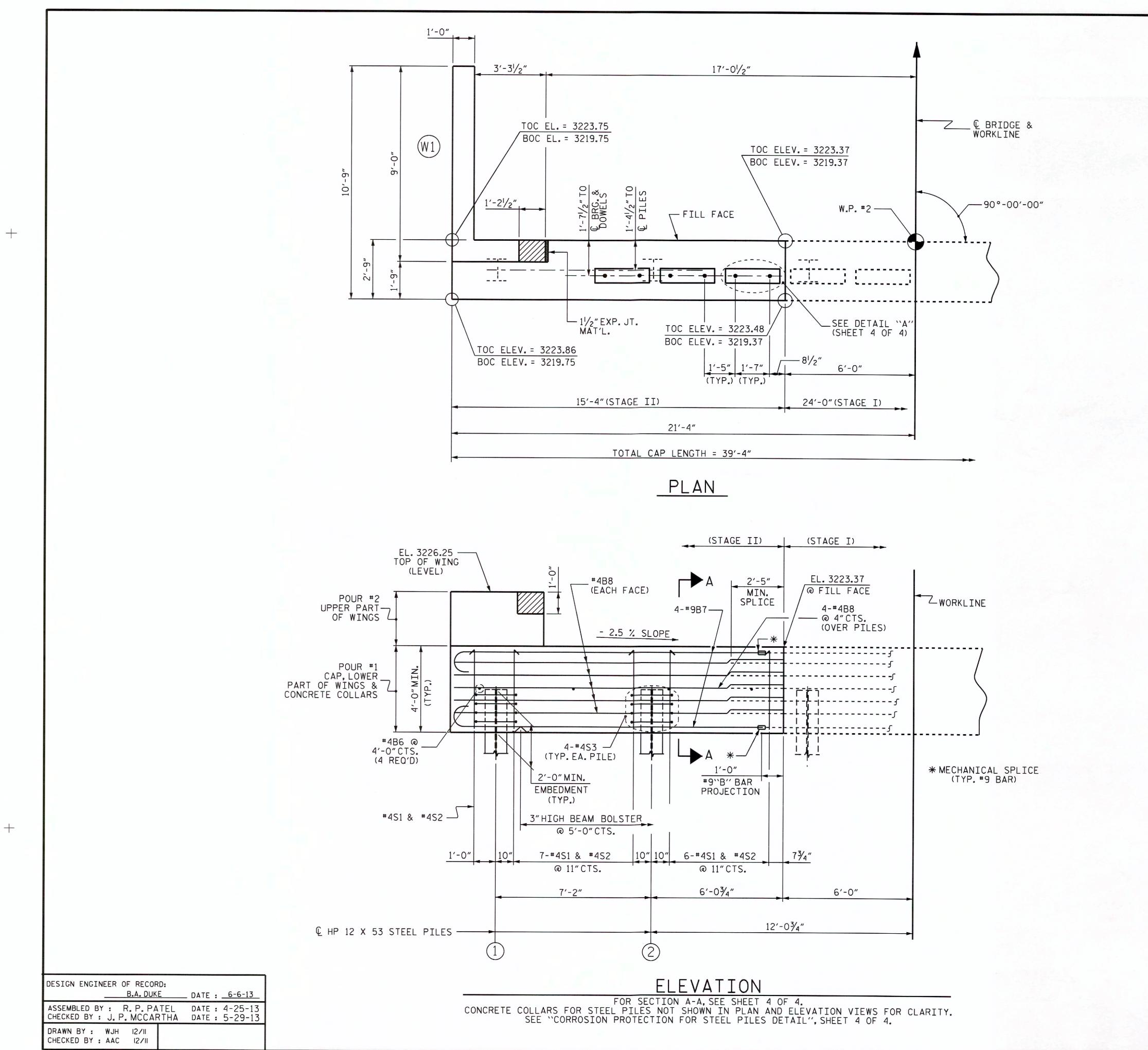
STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED. FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

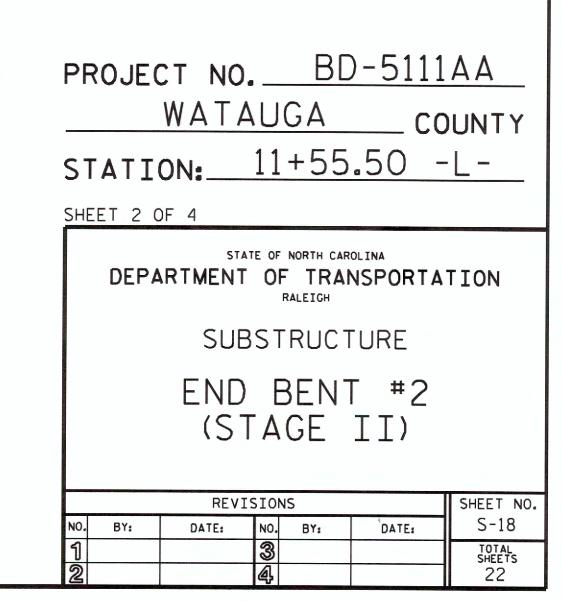
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3	3221.34					
4	3221.16					
5	3220.98					
6	3220.80					

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DEPA	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH								
	SUB	STRUC	TURE						
	END BENT #2 (STAGE I)								
NO. BY:	REVISIONS								
1 2		3 4		total sheets 22					

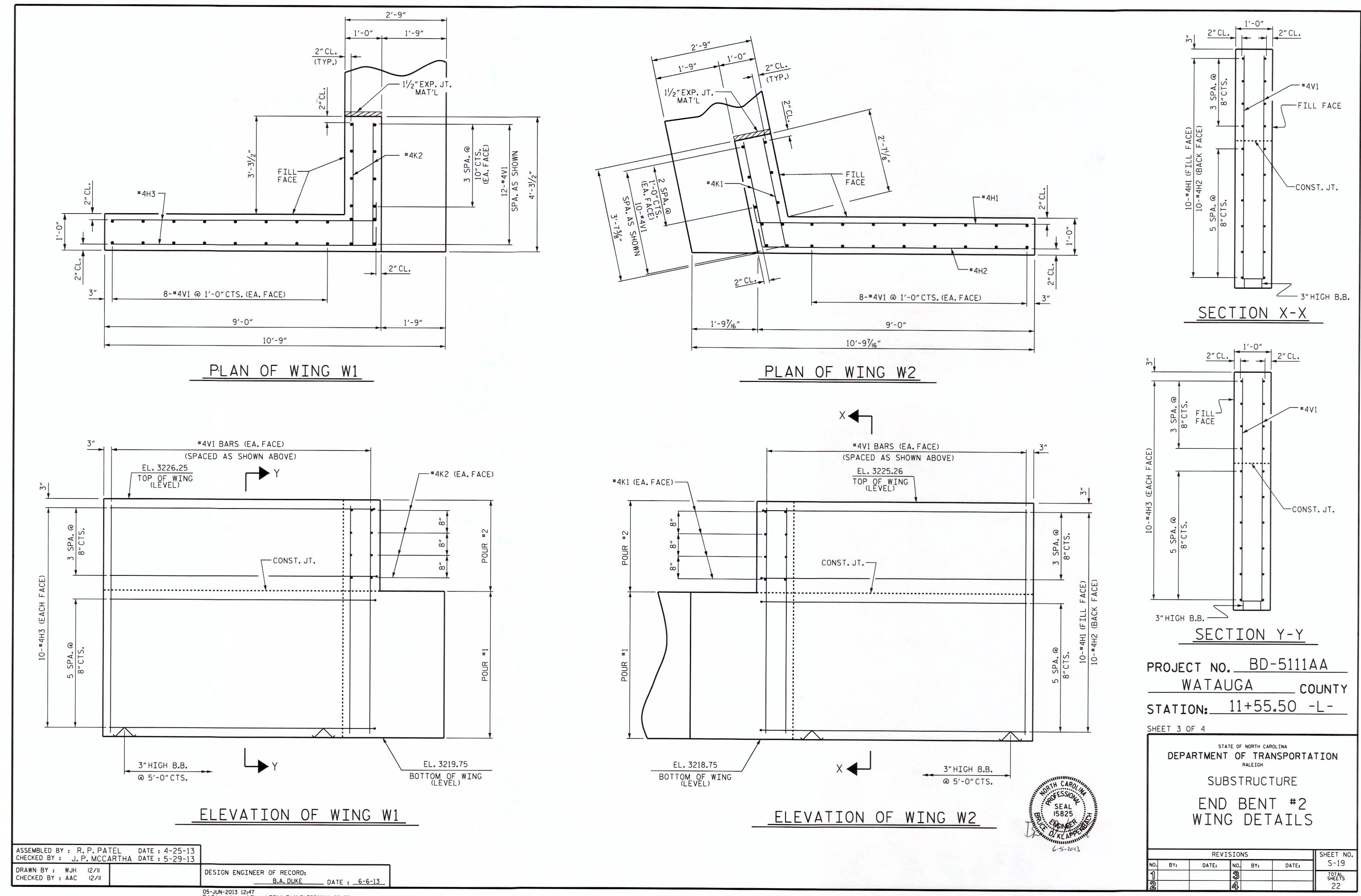


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TOP	OF PILE VATIONS
	3221.70
2	3221.52



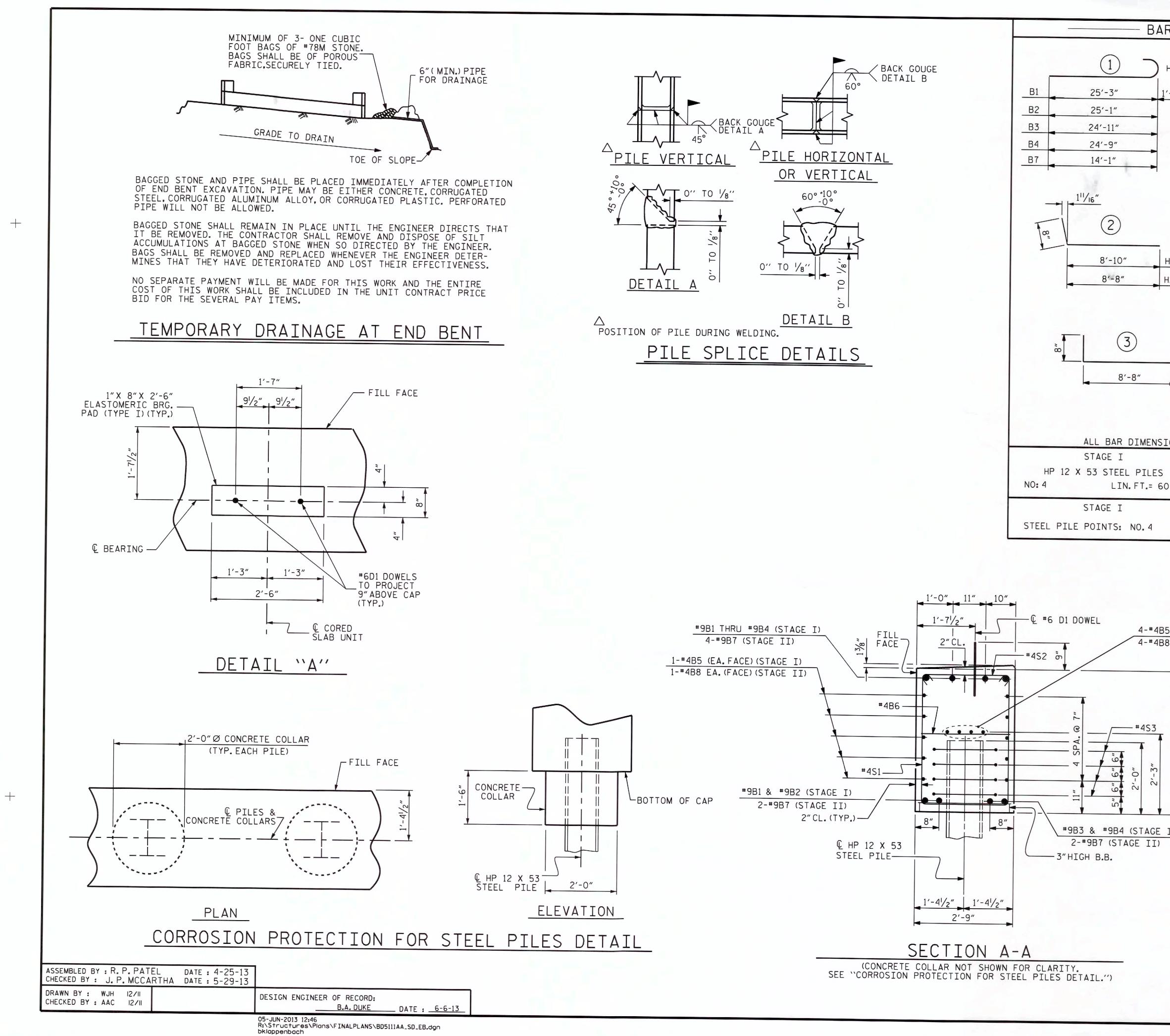




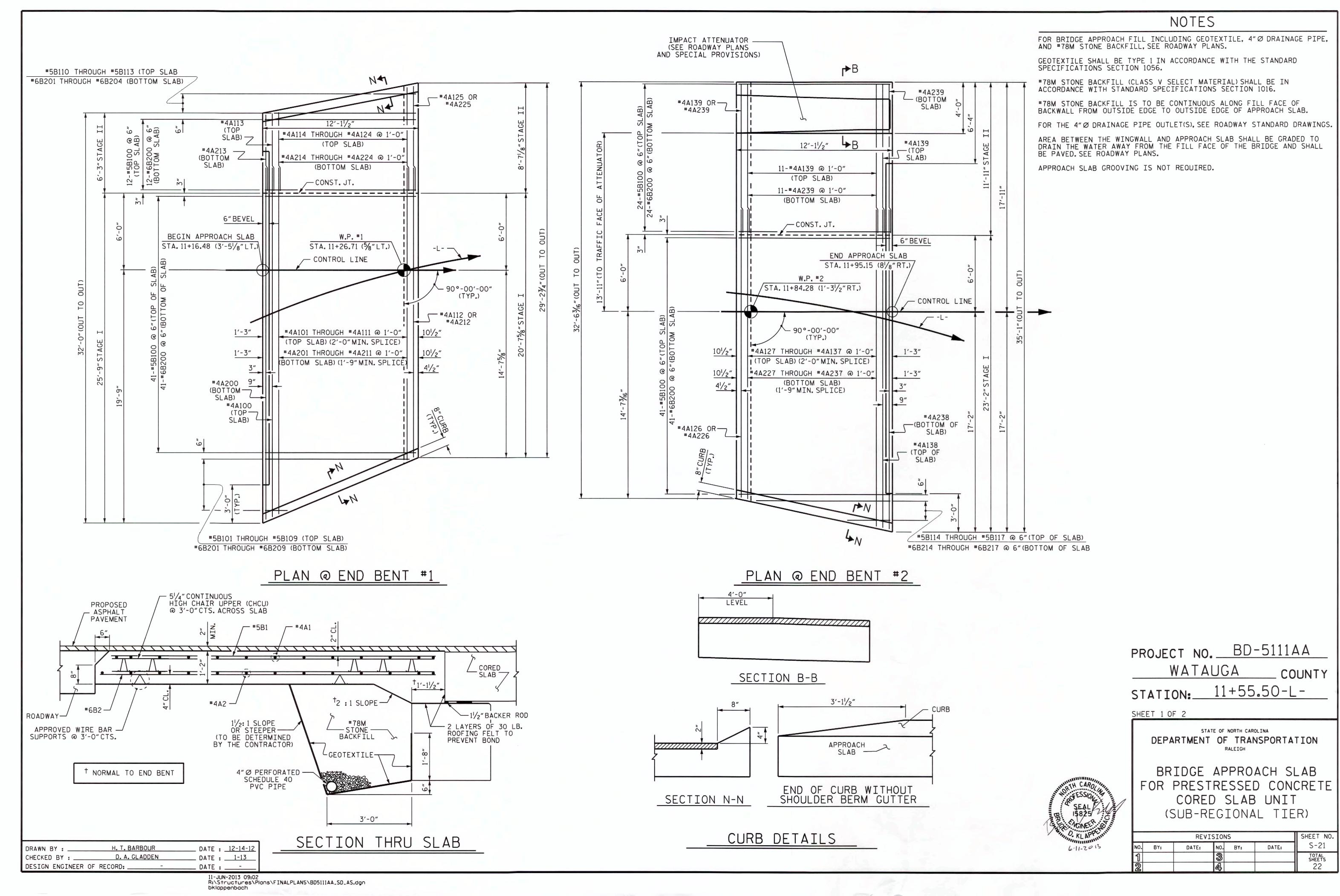
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R TYPES	BILL OF MATERIAL					
	STAGE I					
HK.	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
HK. <sup>*</sup> ℃ ±	B1	2	#9	1	26'-6"	180
-3"	B2 B3	2	#9 #9	1	26'-4"	179
	B4	2	#9		26'-2" 26-0"	178
	B5	14	#4	STR	26'-9"	250
<u>3</u> ,-71/2"	B6	6	#4	STR	2'-5"	10
		1.4		CTD.	11.0 "	
	D1	14	#6	STR	1'-6"	32
2′-5″	H1	10	#4	2	9′-6″	63
	H2	10	#4	2	9'-4"	62
			7			
41/2" 2'-5" 41/2"	K1	8	#4	STR	3'-3"	17
	S1	23	#4	4	10′-5″	160
н1 нк нк.	S2	23	#4	5	3'-2"	49
$\frac{H}{5} \rightarrow HK.$	S3	16	#4	6	6'-6"	69
12						
/1'-3'' LAP	V1	26	#4	STR	6'-2"	107
	REINF	ORCIN	NG STE	EL	1,	533 LBS.
			NCRET		KDOWN	
			TONEL			
$\left(\begin{array}{c} \end{array}\right)$	POUR	#1 C	AP, LOW	ER PA	RT	11.9 C.Y.
- ( $(6)$ )					OLLARS	
H3	POUR		PPER P INGS	ART O	F	1.1 C.Y.
	TOTAL			ONCRET	E	13.0 C.Y.
1'-8"Ø						
	BAR	NO		AGE		WETOUT
ONS ARE OUT TO OUT.	B6	NO. 4	SIZE #4	TYPE STR	LENGTH 2'-5"	WEIGHT 6
STAGE II	B7	8	#9	1	15'-4"	417
HP 12 X 53 STEEL PILES	B8	14	#4	STR	15'-2"	142
NO: 2 LIN. FT.= 30		6				
	D1	6	#6	STR	1'-6"	14
STAGE II	НЗ	20	#4	3	9'-4"	125
STEEL PILE POINTS: NO.2					5	120
	К2	8	#4	STR	3'-11"	21
	S1	14	#4	4	10/ 5/	07
	S2	14	#4 #4	4	10'-5" 3'-2"	97 30
	S3	8	#4	6	6'-6"	35
	V1	28	#4	STR	6'-2″	115
5 @ 4"CTS.OVER PILES (STAGE I)	REINFO	ORCIN	G STEE	L	1,C	02 LBS.
3 @ 4"CTS.OVER PILES (STAGE II)						
	CLASS	A CO	NCRETE	BREA	KDOWN	
4	POUR *	‡1 CA	P, LOW	ER PAP	RT	7.9 C.Y.
		OF	WINGS	S & C(	OLLARS	
	POUR *		PER PA	ari OF		1.1 C.Y.
	TOTAL			NCRET	E	9.0 C.Y.
4'-0" (MIN_)	L					
-, <del>[</del> <del>4</del> , -						
PRO	DJECT		1	BD.	-5111A	
					0111/	
	W	AIA	AUG	A	COL	JNTY
			11+	55		_
	ATION	1:	TTL	JJ.	JU -L	
I) SHEE	T 4 OF 4	4				
					Thia	
	DEPART		ATE OF NOF		SPORTATI	
			RALE			1
		SUE	BSTR	υςτι	JRE	
NORTH CAROLAND						
NORTH CAROLINA I						
BEAL 15825	E	END	BE	INT	#2	
BEAL IS825	E				*2 S	
KLAPPTININ (	E		BE E T A			1
BESSION AND SEAL 15825 CNCNED TO KLAPTCHING 6-5-2013	E					
6-5-2013	[	D			S	SHEET NO.
6-5-2013		D	ETA		S	SHEET NO. S-20 TOTAL SHEETS



+

+

STATION:	11+55.50-L-

		SHEET NO.				
NO.	BY:	DATE:	NO.	BY:	DATE:	S-21
1			3			TOTAL SHEETS
2			4			22

#### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SO.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.
	(MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

### STANDARD NOTES

#### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB. THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### **REINFORCING STEEL:**

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS FQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

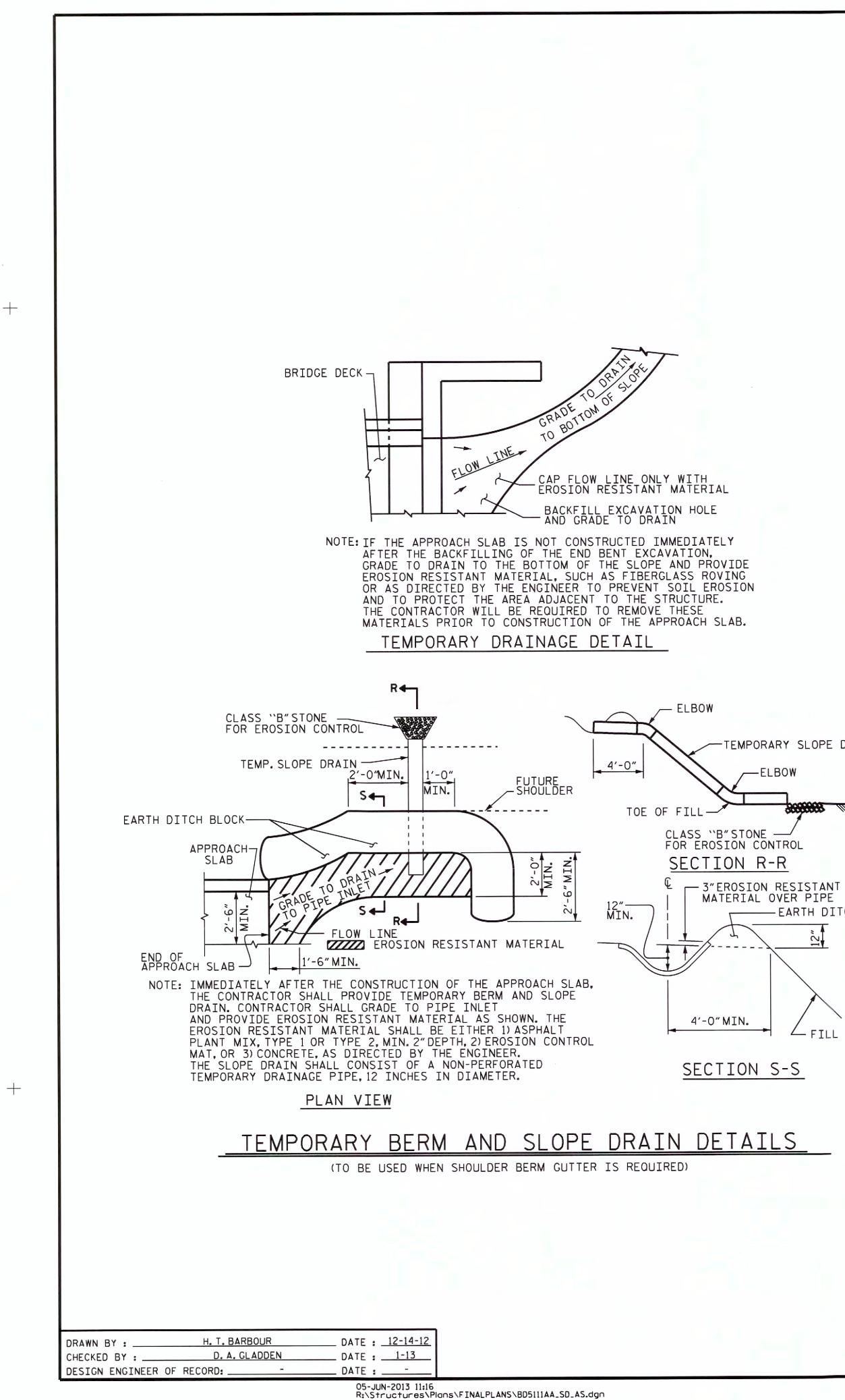
METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

## ENGLISH JANUARY, 1990

STD. NO. SN



bklappenbach

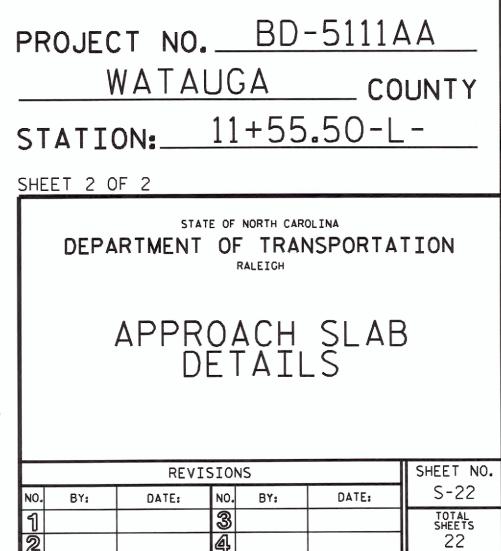
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AP	PRO	ACH	SLAE	3 AT EI	B #1	А	PPR	ОАСН	SLA	Β ΑΤ Ε	B #1	AF	PR	ОАСН	SLAE	β ΑΤ ΕΕ	3 #2	AF	PRC	АСН	SLAE	3 AT E	B #2
		S	TAGE	I				S	TAGE	II				S	TAGE	I				ST	AGE		
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	BAR	NO.	SIZE	TYPE		WEIGHT	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
<b>*</b> A100	1	#4	STR	27'-5"		<b>*</b> A113		#4	STR	6'-0"		*A126	1	#4	STR	22'-9"		<b>*</b> A139	13	#4	STR	11'-7"	101
* A101	1	#4 #4	STR STR	27'-3"		₩ A114 ₩ A115		#4	STR STR	6'-1" 6'-4"		* A127 * A128	1	#4	STR STR	22'-10" 23'-0"	15	A239	13	#4	STR	11'-7"	101
* A102 * A103	1	#4	STR	26'-10" 26'-5"		* A115 * A116		#4	STR	6'-6"	4	* A120	1	#4	STR	23'-3"	15	ALUU	-15		0111		
* A103	1	#4	STR	26'-0"		* A117		#4	STR	6'-8"	4	*A130	1	#4	STR	23'-5'		<b>∗</b> B100	24	#5	STR	11'-4"	284
<b>*</b> A105	1	#4	STR	25'-7"		<b>*</b> A118		#4	STR	6'-11″	5	<b>₩</b> A131	1	#4	STR	23'-8'	16	B200	24	<b>#</b> 6	STR	11'-9"	424
<b>*</b> A106	1	#4	STR	25′-2″	17	<b>*</b> A119	1	#4	STR	7'-1"		<b>₩</b> A132	1	#4	STR	23'-11"	16	-		IG STEE	EL	LBS.	525
<b>*</b> A107	1	#4	STR	24'-9"		*A120		#4	STR	7'-3"		*A133	1	#4	STR	24'-1"	16	* EPO		ATED ING ST	FFI	LBS.	385
* A108	1	#4	STR	24'-4"		* A121	-	#4	STR	7'-6"		* A134	1	#4	STR	24'-4"	16 16			1110 01		2001	
* A109 * A110	1	#4 #4	STR STR	23'-11"		₩ A122 ₩ A123		#4	STR STR	7'-8" 7'-10"	_	₩A135 ₩A136	1	#4	STR STR	24'-6" 24'-9"	10	CLASS	S AA C	ONCRET	ΓE	С.Ү.	6.9
* A110 * A111	1	#4	STR	23'-0"		* A12		#4	STR	8'-1"		*A130	1	#4	STR	24'-11"	17						
* A112	1	#4	STR	22'-10"		* A125		#4	STR	8'-2"		*A138	1	#4	STR	25'-0"	17	1					
	-																	1					
A200	1	#4	STR	27'-5″	18	A213	5 1	#4	STR	5'-11"	4	A226	1	#4	STR	22'-6"	15						
A201	1	#4	STR	27'-0"	18	A214		#4	STR	6'-1"	4	A227	1	#4	STR	22'-7"	15						
A202	1	#4	STR	26'-7"	18			#4	STR	6'-4"	4	A228	1	#4	STR	22'-9"	15	-					
A203	1	#4	STR	26'-2"	17	A216		#4	STR	6'-6"	4	A229	1	#4	STR	23'-0"	15	-					
A204	1	#4 #4	STR	25'-9"	11	A217		#4	STR	6'-8"	4	A230	1	#4	STR STR	23'-2" 23'-5"	15	1					
A205 A206	1	#4	STR STR	25'-4" 24'-11"	17	A218 A219		#4	STR STR	6'-11" 7'-1"	5	A231 A232	1	#4	STR	23'-8"	16	1					
A200	1	#4	STR	24'-6"	16			#4	STR	7'-3"	5	A233	1	#4	STR	23'-10"	16	1				ENCTU	ิล
A208	1	#4	STR	24'-1"	16			#4	STR	7'-6"	5	A234	1	#4	STR	24'-1"	16	1		SPLI	CE L	ENGTH	의
A209	1	#4	STR	23′-8″	16			#4	STR	7'-8"	5	A235	1	#4	STR	24'-3"	16	]		BAR SIZE	EPOX COATE		red
A210	1	#4	STR	23'-2″	15	A223	3 1	#4	STR	7'-10"	5	A236	1	#4	STR	24'-6"	16	1	⊨				
A211	1	#4	STR	22′-9″	15			#4	STR	8'-1"	5	A237	1	#4	STR	24'-8"	16	4	L	#4	2'-0		
A212	1	#4	STR	22'-7″	15	A225	5 1	#4	STR	8'-2"	5	A238	1	#4	STR	24'-11"	17	-		#5	2'-6	"   2′-2	"
¥ B100	41	#5	STR	11'-4"	105	<del>*</del> B10	0 12	#5	STR	11'-4"	1/12	<b>₩</b> B100	41	#5	STR	11'-4"	485	-		#6	3'-1(	D″ 2′-7	
* B100 * B101	41 1	*5	STR	11'-4		* B10		#5	STR	11'-1"		* B100		#5	STR	10'-5"	11	1	L	<u> </u>	<u> </u>	·	
* B101 * B102	1	#5	STR	10'-1"		* B111		#5	STR	8'-5"		* B115		#5	STR	8'-0"	8	1					
<b>*</b> B103	1	#5	STR	8'-10"		<b>*</b> B112		#5	STR	5'-10"		<b>*</b> B116		*5	STR	5'-8"	6						
<del>*</del> B104	1	*5	STR	7′-8″	8	<b>*</b> B113	3 1	#5	STR	3'-3"	3	<b>*</b> B117	1	*5	STR	3'-4"	3						
<del>*</del> B105	1	#5	STR	6'-6"	7			1			0.000			-				_					
<b>*</b> B116	1	#5	STR	5'-4"	6	B200		-	STR	11'-9"	212	B200	41	#6	STR	11'-9"	724	-					
* B107	1	#5	STR	4'-1"	4	B210	1	*6	STR	11'-1"	17	B214	1	#6	STR	10'-5"	16	-					
* B108 * B109	1	#5 #5	STR STR	2'-11" 1'-9"	3	B211 B212		#6 #6	STR STR	8'-5" 5'-10"	13	B215 B216	1	#6 #6	STR STR	8'-0" 5'-8"	9	-					
T 0103	1			1 5	<u> </u>	B212		#6	STR	3'-3"	5	B210	1	#6	STR	3'-4"	5						
B200	41	#6	STR	11'-9"	724	1							1			1							
B201	1	#6	STR	11'-3"	17	REIN	FORCI	NG STE	EL	LBS.	316	REINF	ORCI	NG STE	EL	LBS.	970						
B202	1	#6	STR	10'-1"	15		OXY C		TEE			* EPC			T.C.C.		701	1					
B203	1	#6	STR	8'-10"	13	-	INFOR	CING S	IEEL	LBS.	232	REI	NFOR	CING S	IEEL	LBS.	721						
B204	1	#6	STR	7'-8"	12		S A A	CONCRE	TF	C. Y.	1 2	CLASS	5		TF	С. Ү.	12.4						
B205	1	#6 #C	STR	6'-6"	10	ULAS	J AA	CONCINE	16	Us Is	C.F	ULAJ.	JAA	SONGILE	16	U. 14	LLaT						
B206 B207	1	#6 #6	STR STR	5'-4" 4'-1"	8 C	-																	
B207	1	*6	STR	2'-11"	<u>ه</u>																		
B208	1	#6	STR	1'-9"	3																		
	-			·	-1																		
REINF	ORCIN	NG STEE	EL	LBS.	1027																		
* EPO> REIN		ATED	FEL	LBS.	765																		
CLASS	AA C	CONCRE	TE	C.Y.	13.2																		

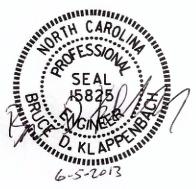
TEMPORARY SLOPE DRAIN

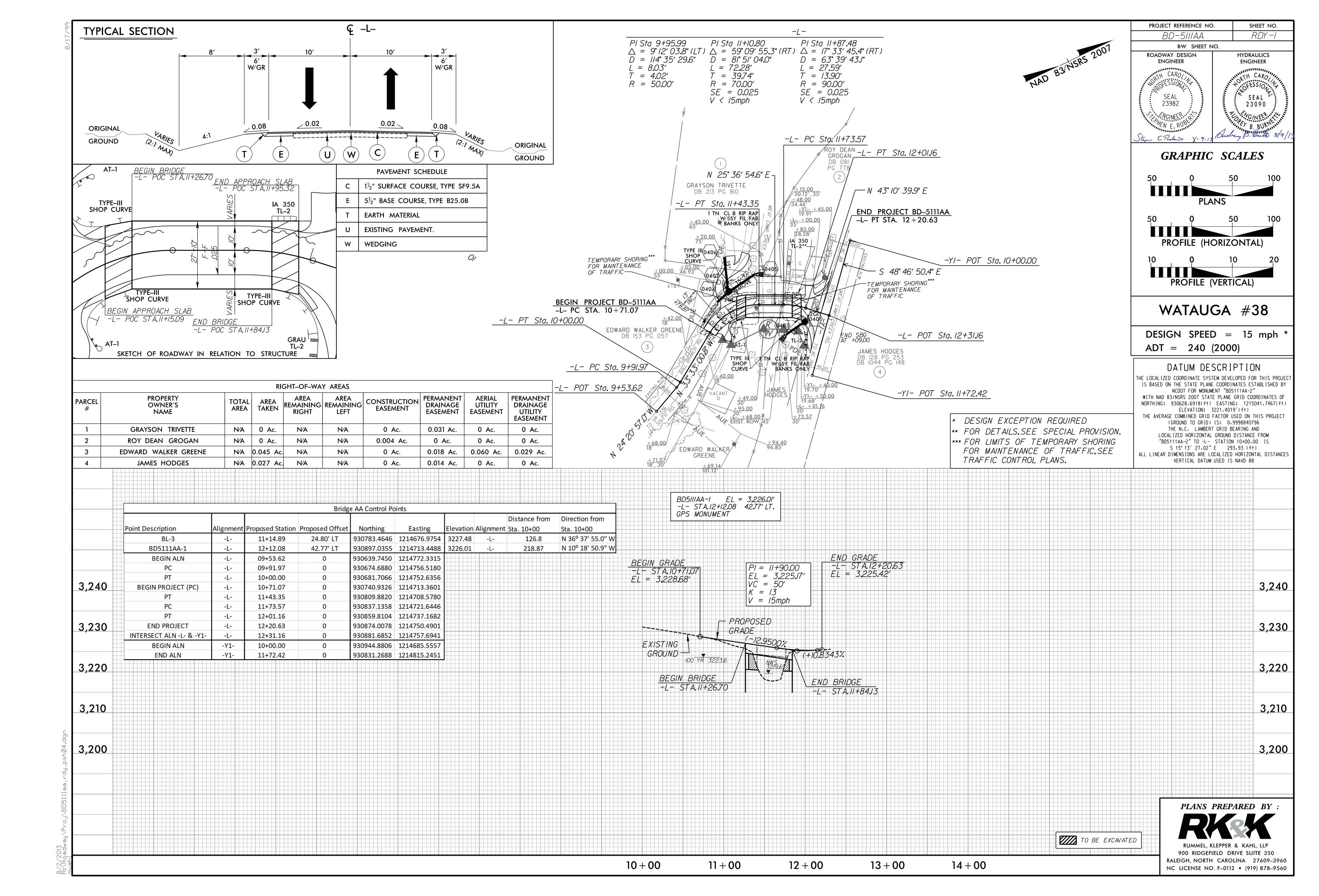
-ELBOW

- EARTH DITCH BLOCK

∠ FILL SLOPE





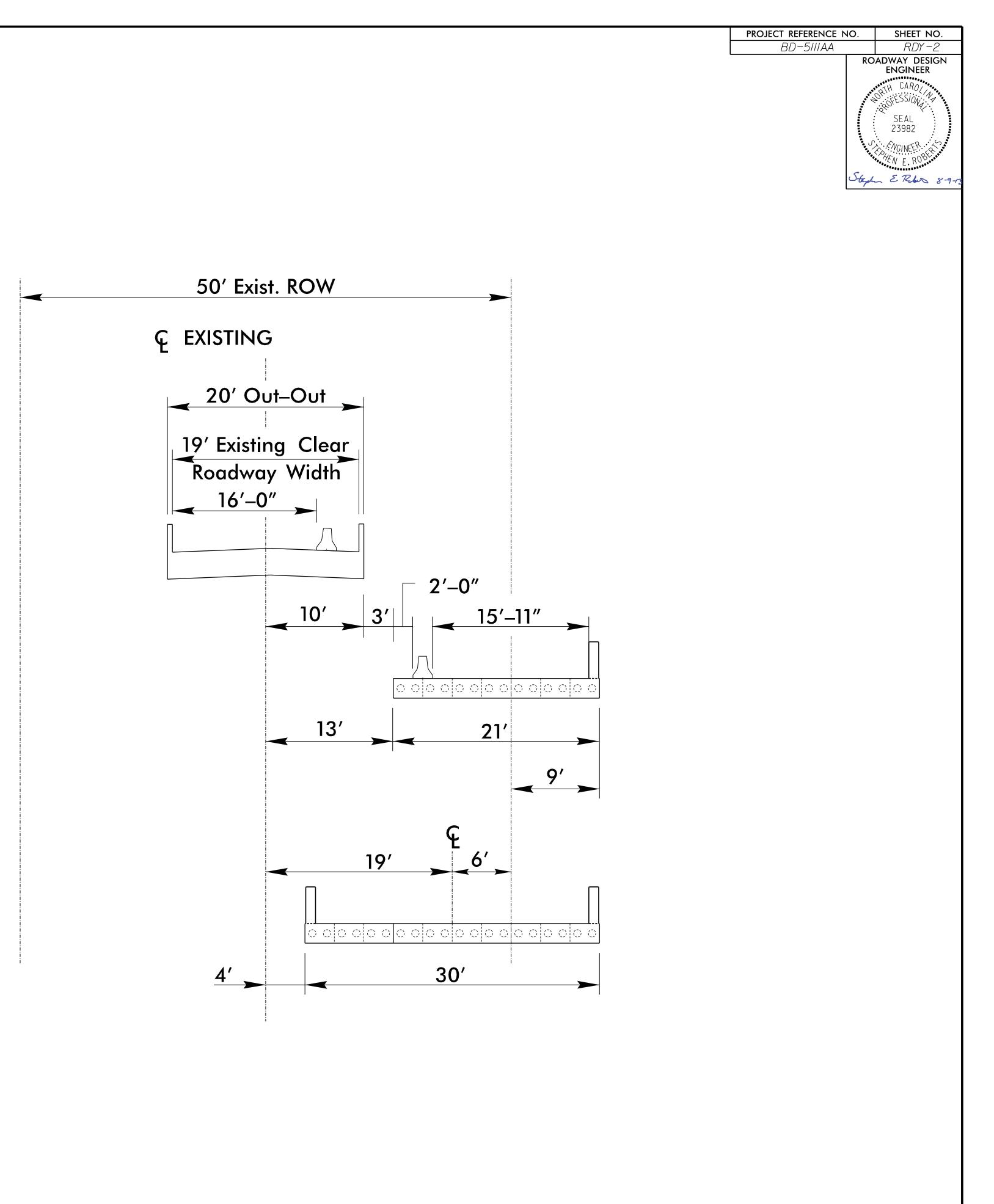


# PHASED CONSTRUCTION

Phase 1

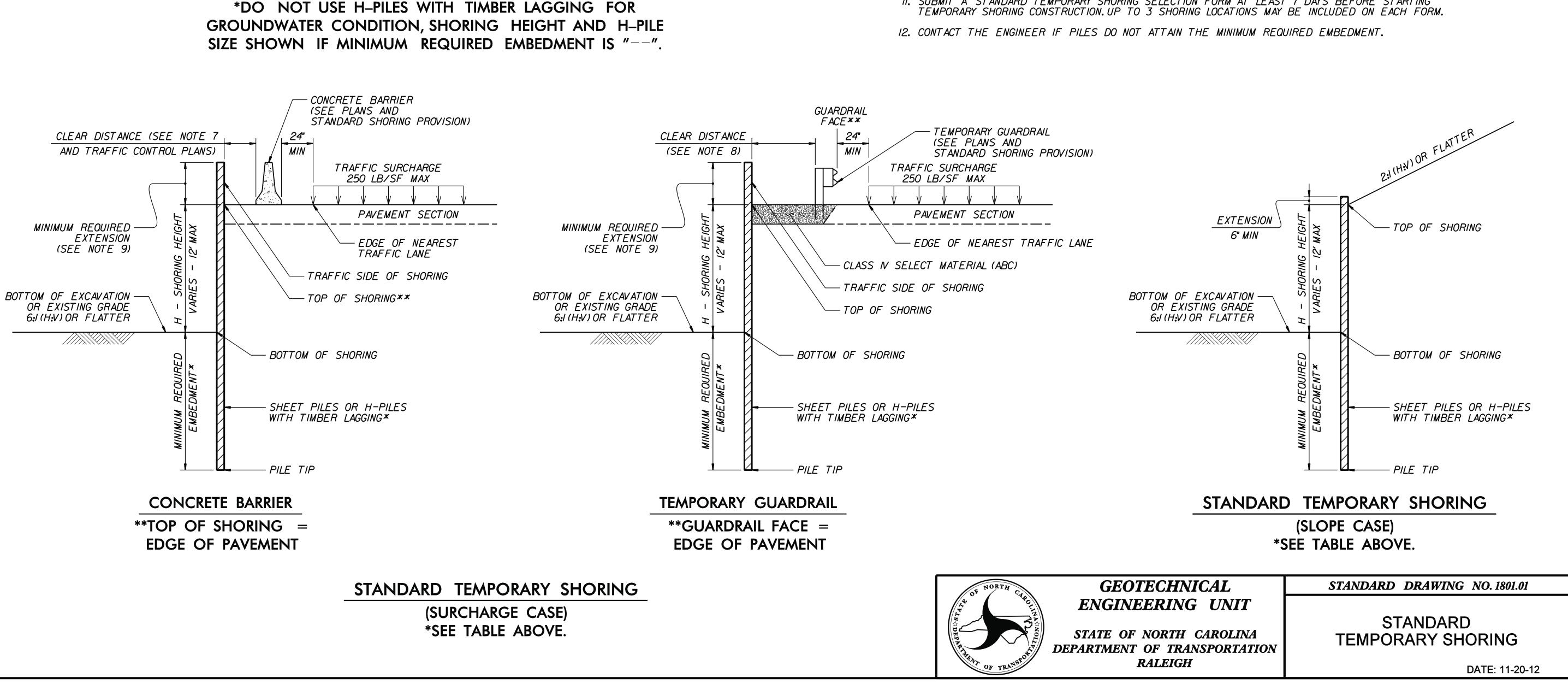
Phase 2

8/2/2013 8/2/2013 - 1 - 1 - 1 - 1 - 1 - 1



		SLOPE	OR SURCHARGE CASE	E WITH NO	TRAFFIC IM	SURCHARGE CASE WITH TRAFFIC IMPACT						
		SHL	EET PILES	H-PILES I	NITH TIMBE	R LAGGING	SHL	EET PILES	H-PILES I	NITH TIMBE	R LAGGING	
GROUNDWATER CONDITION	H SHORING HEIGHT	MINIMUM REQUIRED EMBEDMENT	MINIMUM REQUIRED SECTION MODULUS (IN <sup>3</sup> /FT)	(5	EQUIRED EN (FT) SEE NOTE I	0)	MINIMUM REQUIRED EMBEDMENT	MINIMUM REQUIRED SECTION MODULUS (IN <sup>3</sup> /FT)				
(SEE NOTE 6)	(FT)	(FT)	(IN <sup>3</sup> /FT)	HP IOx42	HP 12x53	HP 14x73	(FT)	(IN3/FT)	HP IOx42	HP 12x53	HP 14x7	
20	< 6	II <b>.</b> 5	4.5	<i>II.</i> 5	<i>II.</i> 5	II <b>.</b> 5	16.0	12.0	13.0	13.0	13.0	
P P P NN N N N N N	7	13.0	7.0	13.0	13.0	13.0	17.0	14.5	14.5	14.5	14.5	
SH( SH(	8	15.0	10.0		15.0	15.0	18.0	17.0		15.5	15.5	
	9	17.0	14.0		17.0	17.0	19.0	20.0		17.0	17.0	
NU NI NI NI NI	10	<i>18</i> <b>.</b> 5	19.5			18.5	20.0	23.5			18.5	
GROUNDWATER ELEVATION BEWTEEN BOTTOM OF SHORING AND PILE TIP	//	20.5	26.0				21.0	28.0			20.0	
BG	12	22.5	33.0				22.0	33.0			21.5	
	< 6	7.5	3.0	8.0	8.0	8.0	11.0	10.0	9.5	9.5	9.5	
LOW	7	8.5	4.5	9.5	9.5	9.5	12.0	12.0	10.5	10.5	10.5	
ATE BEI 'IP	8	10.0	6.5	10.5	10.5	10.5	12.5	14.0	II <b>.</b> 5	11.5	11.5	
E 1 E 1	9	11.0	9.5		12.0	12.0	/3.5	16.5		12.5	12.5	
GROUNDWATER ELEVATION BELOW PILE TIP	10	12.5	13.0			13.5	14.0	19.5		13.5	13.5	
ELE 6f	//	13.5	17.0			14.5	15.0	22.5			14.5	
~	12	15.0	21.5			16.0	16.0	25.5			15.5	

### MINIMUM REQUIRED EMBEDMENT AND SECTION MODULUS

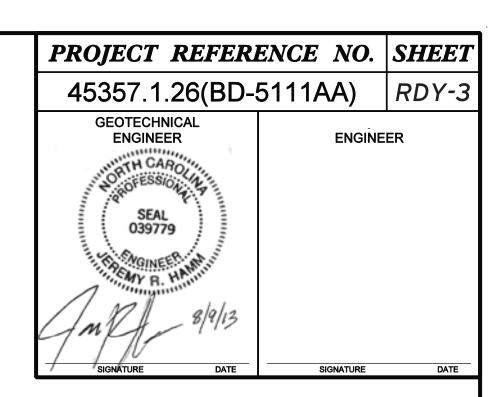


#### NOTES:

- I. AT THE CONTRACTOR'S OPTION, USE STANDARD TEMPORARY SHORING AS NOTED IN THE PLANS.
- 2. FOR STANDARD TEMPORARY SHORING, SEE STANDARD SHORING PROVISION.
- 3. STANDARD TEMPORARY SHORING IS BASED ON THE FOLLOWING IN-SITU ASSUMED SOIL PARAMETERS: UNIT WEIGHT,  $\gamma = 120 LB/CF$ FRICTION ANGLE,  $\phi = 30$  DEGREES COHESION.c = O LB/SF
- 4. DO NOT USE STANDARD TEMPORARY SHORING IF ASSUMED SOIL PARAMETERS ARE NOT APPLICABLE.
- 5. DO NOT USE STANDARD TEMPORARY SHORING WHEN VERY LOOSE OR SOFT SOIL OR MUCK IS WITHIN THE EMBEDMENT DEPTH.
- "SURCHARGE CASE WITH TRAFFIC IMPACT".
- CASE WITH TRAFFIC IMPACT".
- FOR "SURCHARGE CASE WITH TRAFFIC IMPACT".
- DRILLED-IN H-PILES.

6. USE GROUNDWATER ELEVATION NOTED IN THE PLANS. IF NO GROUNDWATER ELEVATION IS SHOWN IN THE PLANS, USE "GROUNDWATER ELEVATION BETWEEN BOTTOM OF SHORING AND PILE TIP" FOR GROUNDWATER CONDITION. DO NOT USE STANDARD TEMPORARY SHORING IF GROUNDWATER IS ABOVE BOTTOM OF SHORING. 7. AT THE CONTRACTOR'S OPTION OR IF AVAILABLE CLEAR DISTANCE IS LESS THAN THE MINIMUM REQUIRED FOR CONCRETE BARRIER, SET BARRIER NEXT TO AND UP AGAINST TRAFFIC SIDE OF PILES AND USE 8. AT THE CONTRACTOR'S OPTION OR IF AVAILABLE CLEAR DISTANCE IS LESS THAN 4' FOR TEMPORARY GUARDRAIL.ATTACH GUARDRAIL TO TRAFFIC SIDE OF PILES AS SHOWN IN THE PLANS AND USE "SURCHARGE" 9. MINIMUM REQUIRED EXTENSION IS 6" FOR "SLOPE OR SURCHARGE CASE WITH NO TRAFFIC IMPACT" AND 32" IO. MINIMUM REQUIRED EMBEDMENT FOR H-PILES WITH TIMBER LAGGING IS BASED ON DRIVEN H-PILES AT MAXIMUM 6' SPACING. AT THE CONTRACTOR'S OPTION. EMBEDMENT DEPTHS MAY BE REDUCED BY 25% FOR

II. SUBMIT A "STANDARD TEMPORARY SHORING SELECTION FORM" AT LEAST 7 DAYS BEFORE STARTING



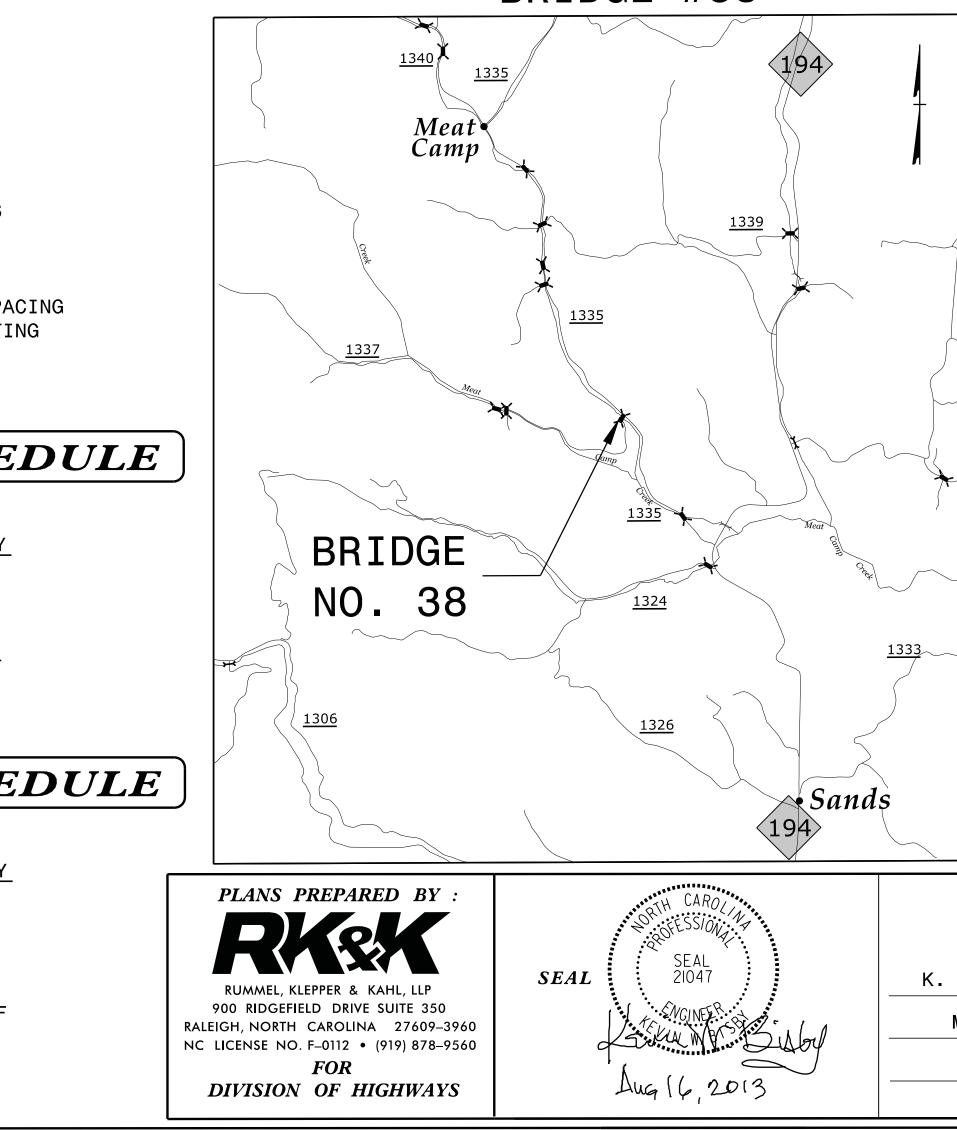
BD-5111A	PLAN FOR TRAFFIC CONTROL, MARI WATAUGA	KING &
ASHE	Bits   Description     101.01   WORK ZONE WARNING SIGNS     1101.02   TEMPORARY MOAD     1101.03   TEMPORARY MOAD     1101.04   TEMPORARY MOAD     1101.03   TEMPORARY MOAD     1101.04   TEMPORARY MOAD     1101.05   TEMPORARY MOAD     1101.01   WORK ZONE WARNING SIGNS     1101.02   TEMPORARY LANE CLOSURES     1101.03   TEMPORARY MOAD     1101.04   TEMPORARY WORK ZONE SIGNS     1100.01   STATIONARY WORK ZONE SIGNS     1101.02   PORTABLE WORK ZONE SIGNS     1101.03   TEMPORARY WORK ZONE SIGNS     1101.04   TEMPORARY WORK ZONE SIGNS     1100.01   PORTABLE WORK ZONE SIGNS     1100.01   PORTABLE WORK ZONE SIGNS     1100.01   PORTABLE WORK ZONE SIGNS     1100.01   FUMMO     1100.01   PORTABLE WORK ZONE SIGNS     1100.01   PORTABLE WORK ZONE SIGNS <t< th=""><th>Image: state of the state</th></t<>	Image: state of the state
PROJECT:	TEMP. PAVEMENT   MARKING   SCHEDULE     SYMBOL   DESCRIPTION   QUANTITY   PAY ITEM   TOTAL     PAVEMENT MARKING LINES   PAINT (4")   PAINT (4")     PA   WHITE EDGELINE 2X   1192 LF   TOTAL 1192 LF     2X = TWO APPLICATIONS   ESCRIPTION   QUANTITY   PAY ITEM   TOTAL     SYMBOL   DESCRIPTION   QUANTITY   PAY ITEM   TOTAL     SYMBOL   DESCRIPTION   QUANTITY   PAY ITEM   TOTAL	BRIDGE NO. 38

# E OF NORTH CAROLINA VISION OF HIGHWAYS

# AN FOR PROPOSED ROL, MARKING & DELINEATION

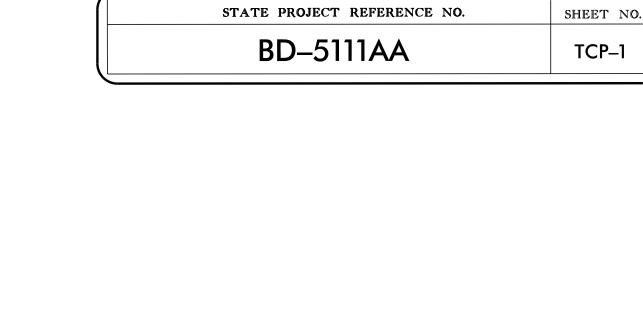


> VICINITY MAP BRIDGE #38









LEGEND

EXIST. PVMT.

PROPOSED PVMT.

- NORTH ARROW

WORK AREA

DIRECTION OF TRAFFIC FLOW

<u>GENERAL</u>



1402	
<u>1403</u>	
$\langle \langle \langle \rangle$	
$\left\{ \right\}$	
h K	

TRAFFIC CONTROL DEVICES
BARRICADE (TYPE III) DRUM TEMPORARY CRASH CUSHION
PORTABLE CONCRETE BARRIER
FLAGGER
TRUCK MOUNTED ATTENUATOR (TMA
TEMPORARY SIGNING
DORTABLE SIGN
- STATIONARY SIGN

K. W. BISBY, PE TRAFFIC CONTROL ENGINEER M. A. COLE TRAFFIC CONTROL PROJECT DESIGNER A. TUTT TRAFFIC CONTROL PROJECT DESIGNER

#### GENERAL NOTES / LOCAL NOTES CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER. LANE AND SHOULDER CLOSURE REQUIREMENTS A. REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED OR AS DIRECTED BY THE ENGINEER. B. WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE. CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED C. WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING ON THE SHOULDER ADJACENT TO AN UNDIV FACILITY AND WITHIN 5 FT OF AN OPEN TRAVEL LANE. CLOSE THE NEAREST OPEN TRAVEL LA USING ROADWAY STANDARD DRAWING NO. 1101.02 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL. D. WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN A LANE OF TRAVEL OF AN UNDIVID OR DIVIDED FACILITY, CLOSE THE LANE ACCORDING TO THE TRAFFIC CONTROL PLANS, ROADW STANDARD DRAWINGS, OR AS DIRECTED BY THE ENGINEER. CONDUCT THE WORK SO THAT ALL PERSONNEL AND/OR EQUIPMENT REMAIN WITHIN THE CLOSED TRAVEL LANE. E. DO NOT WORK SIMULTANEOUSLY WITHIN 15 FT ON BOTH SIDES OF AN OPEN TRAVELWAY, RAMP, OR LOOP WITHIN THE SAME LOCATION UNLESS PROTECTED WITH GUARDRAIL OR BARRIER. PAVEMENT EDGE DROP OFF REQUIREMENTS F. BACKFILL AT A 6:1 SLOPE UP TO THE EDGE AND ELEVATION OF EXISTING PAVEMENT IN AREA ADJACENT TO AN OPENED TRAVEL LANE THAT HAS AN EDGE OF PAVEMENT DROP-OFF AS FOLLOW BACKFILL DROP-OFFS THAT EXCEED 2 INCHES ON ROADWAYS WITH POSTED SPEED LIMITS OF 45 MPH OR GREATER. BACKFILL DROP-OFFS THAT EXCEED 3 INCHES ON ROADWAYS WITH POSTED SPEED LIMITS LESS THAN 45 MPH. BACKFILL WITH SUITABLE COMPACTED MATERIAL, AS APPROVED BY THE ENGINEER, AT NO EXPENSE TO THE DEPARTMENT. G. DO NOT EXCEED A DIFFERENCE OF 2 INCHES IN ELEVATION BETWEEN OPEN LANES OF TRAFFIC FOR NOMINAL LIFTS OF 1.5 INCHES. INSTALL ADVANCE WARNING "UNEVEN LANES" SIGNS (W8-11) (500') IN ADVANCE AND A MINIMUM OF EVERY HALF MILE THROUGHOUT THE UNEVEN AREA. SIGNING H. INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION. TRAFFIC BARRIER I. INSTALL TEMPORARY BARRIER ACCORDING TO THE TRANSPORTATION MANAGEMENT PLANS A MAXIMUM OF TWO (2) WEEKS PRIOR TO BEGINNING WORK IN ANY LOCATION. ONCE TEMPORARY BARRIER IS INSTALLED AT ANY LOCATION PROCEED IN A CONTINUOUS MANNER TO COMPLETE THE PROPOSED WORK IN THAT LOCATION UNLESS OTHERWISE STATED IN THE TRANSPORTATION MANAGEMENT PLANS OR AS DIRECTED BY THE ENGINEER DO NOT PLACE BARRIER DIRECTLY ON ANY SURFACE OTHER THAN ASPHALT OR CONCRETE. ONCE TEMPORARY BARRIER IS INSTALLED AT ANY LOCATION AND NO WORK IS PERFORMED

BEHIND THE TEMPORARY BARRIER FOR A PERIOD LONGER THAN TWO (2) MONTHS, REMOVE/RESET TEMPORARY BARRIER AT NO COST TO THE DEPARTMENT UNLESS OTHERWISE STATED IN THE TRANSPORTATION MANAGEMENT PLANS, TEMPORARY BARRIER IS PROTECTING A HAZARD, OR AS DIRECTED BY THE ENGINEER.

INSTALL TEMPORARY BARRIER WITH THE TRAFFIC FLOW BEGINNING WITH THE UPSTREAM SIDE OF TRAFFIC. REMOVE TEMPORARY BARRIER AGAINST THE TRAFFIC FLOW BEGINNING WITH THE DOWNSTREAM SIDE OF TRAFFIC.

INSTALL AND SPACE DRUMS NO GREATER THAN TWICE THE POSTED SPEED LIMIT (MPH) TO CLOSE OR KEEP THE SECTION OF THE ROADWAY CLOSED UNTIL THE TEMPORARY BARRIER IS REMOVED.

J. PROTECT THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER AT ALL TIMES DURING THE INSTALLATION AND REMOVAL OF THE BARRIER BY EITHER A TRUCK MOUNTED ATTENUATOR (MAXIMUM 72 HOURS) OR A TEMPORARY CRASH CUSHION.

PROTECT THE APPROACH END OF MOVEABLE/PORTABLE CONCRETE BARRIER FROM ONCOMING TRAFFIC AT ALL TIMES BY A TEMPORARY CRASH CUSHION UNLESS THE APPROACH END OF MOVEABLE/PORTABLE CONCRETE BARRIER IS OFFSET FROM ONCOMING TRAFFIC AS FOLLOWS OR AS SHOWN IN THE PLANS: (SEE ALSO 1101.05)

OSTED	SPEED	LIMIT	
40 0	R LES		
45 -	50		
55			
60 M	PH OR H	HIGHER	

MINIMU	M	OFFSET
15	F1	-
20	F٦	-
25	F٦	-
30	F٦	

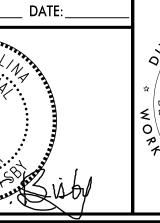
TRAFFIC CONTROL DEVICES

	K. SPACE CHANNELIZING DEVICES IN WORK AREAS NO GREATER IN FEET THAN TWICE THE POSTED SPEED LIMIT (MPH), EXCEPT 10 FT ON-CENTER IN RADII, AND 3 FT OFF THE EDGE OF AN OPEN TRAVELWAY WHEN LANE CLOSURES ARE NOT IN EFFECT. WHEN SKINNY DRUMS ARE ALLOWED REFER TO SECTION 1180 OF STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES OR AS SHOWN IN THE PLANS.	PHASE I STEP 1:
	L. PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.	ERECT ADVANCED WORK ZONE SIGNS ON SR 1337 AND SR 1335 IN ACCORDANCE WITH RDWY STD 1101.01
[ND	PAVEMENT MARKINGS AND MARKERS	SHEET 3.
THE	M. INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE AS SHOWN IN THE PAVEMENT MARKING PLAN.	STEP 2:
, ESS LED.	N. INSTALL TEMPORARY PAVEMENT MARKINGS AND TEMPORARY PAVEMENT MARKERS ON INTERI LAYERS OF PAVEMENT AS FOLLOWS:	PLACE TEMPORARY PAVEMENT MARKING FROM -L- STA 10+00 TO -L- STA 12+50 AND PLACE PORTABLE
IVIDED LANE /	ROAD TYPEMARKINGMARKERASPHALTPAINTRAISEDCONCRETECOLD APPLIED PLASTIC (TYPE IV)RAISED	CONCRETE BARRIER (ANCHORED) FROM -L- STA. 11+00 TO -L- STA. 12+00 WITH TEMPORARY CRASH CUSHIONS, (SEE TCP-3).
	O. INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE.	STEP 3:
/IDED ADWAY _L	P. PLACE ONE APPLICATION OF PAINT FOR TEMPORARY TRAFFIC PATTERNS. PLACE A SECON APPLICATION OF PAINT SIX (6) MONTHS AFTER THE INITIAL APPLICATION AND EVERY SIX MONTHS AS DIRECTED BY THE ENGINEER.	D USING RDWY STD 1101.02 SHEET 1 IN A CONTINUOUS OPERATION DIRECT TRAFFIC ON SR 1337 TO ITS TEMPORARY ONE LANE TWO WAY TRAFFIC PATTERN. CONSTRUCT PROPOSED -L- SR 1337 STRUCTURE AND APPROACHES FROM -L- 10+71 TO -L- STA. 12+20, UP TO BUT NOT INCLUDING THE FINAL LAYER OF SURFACE COURSE PAVEMENT, (SEE TCP-3, ROADWAY AND STRUCTURE PLANS).
ΛP,	Q. TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.	OF SURFACE COURSE PAVEMENT, (SEE TOF-S, ROADWAY AND STRUCTURE PLANS).
	R. REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS BY THE END OF EACH DAY'S OPERATION.	PHASE II
REAS LOWS :		STEP 1:
ESS		USING RDWY STD 1101.02 SHEET 1 IN A CONTINUOUS OPERATION REMOVE AND RESET PORTABLE CONCRETE BARRIER (ANCHORED) FROM -L- STA. 11+00 TO -L- STA. 12+00 WITH TEMPORARY CRASH CUSHIONS. DIRECT TRAFFIC ON SR 1337 TO ITS TEMPORARY ONE LANE TWO WAY TRAFFIC PATTERN ONTO THE NEWLY CONSTRUCTED STRUCTURE STAGE I, (SEE TCP-4, ROADWAY AND STRUCTURE PLANS).
		STEP 2:
6″		USING RDWY STD 1101.02 SHEET 1, COMPLETE CONSTRUCTION OF PROPOSED -L- SR 1337 STRUCTURE AND APPROACHES FROM -L- 10+71 TO -L- STA. 12+20, UP TO BUT NOT INCLUDING THE FINAL LAYER OF SURFACE COURSE, (SEE TMP-4).
		PHASE III
		STEP 1:
		USING RDWY STD 1101.02 SHEET 1 IN A CONTINUOUS OPERATION (ALTERNATING LANE CLOSURES), PLACE THE FINAL LAYER OF SURFACE COURSE, FINAL PAVEMENT MARKINGS AND MARKERS FROM -L- STA. 10+71 TO -L- STA. 12+21,(SEE NCDOT ROADWAY STANDARD DRAWINGS).
		STEP 2:
		REMOVE ANY REMAINING TRAFFIC CONTROL DEVICES FROM THE PROJECT LIMITS AND OPEN SR 1337 TO ITS PROPOSED TRAFFIC PATTERN.
		APPROVED: DATE: OF H/O
		SIOF NORTH CAL

KESSION, SEAL 21047

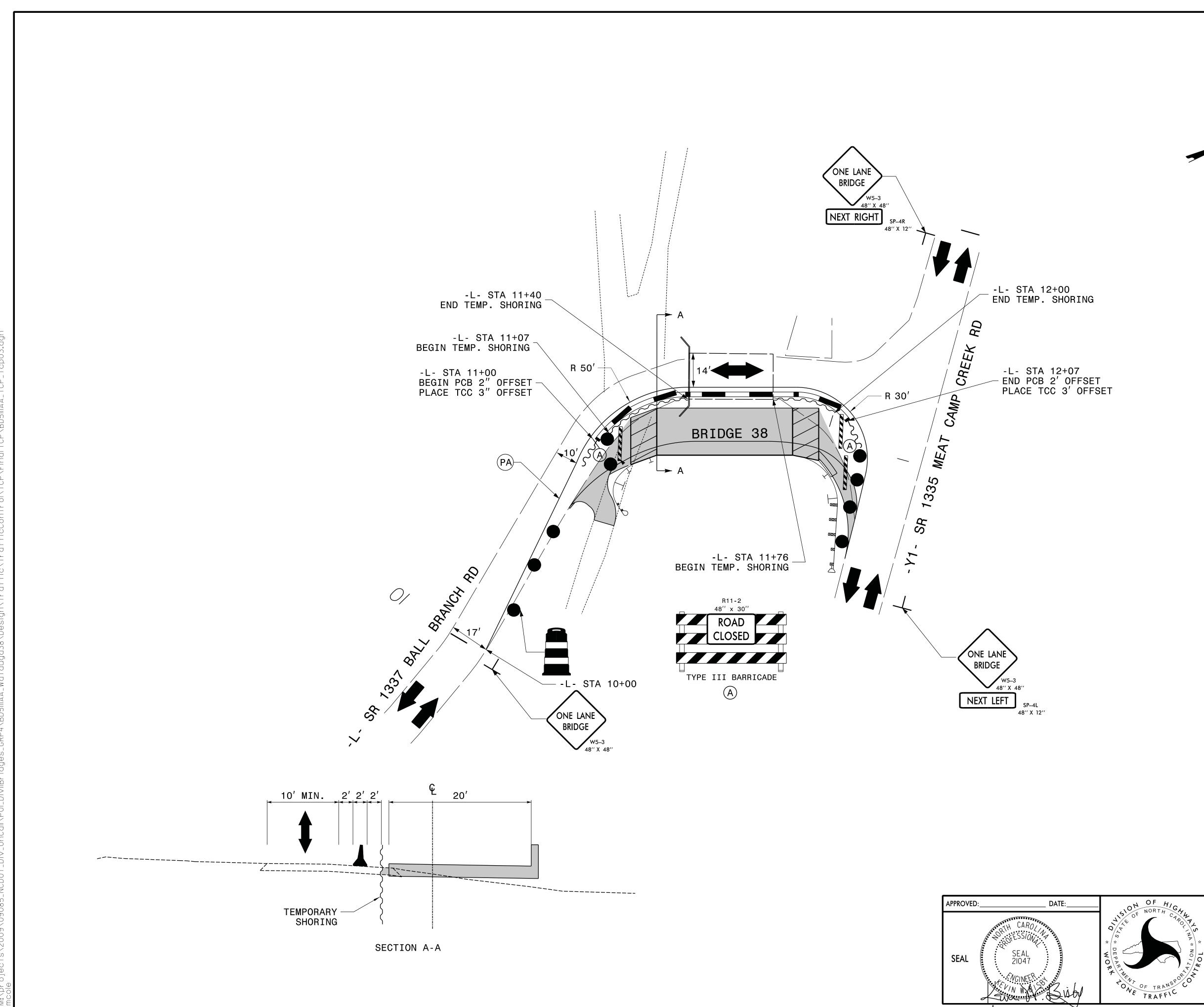
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TRAFFIC CONTROL PHA	SING	
TRAFFIC CONTROL THA	31140	





## TRAFFIC CONTROL GENERAL NOTES AND **PROJECT PHASING**



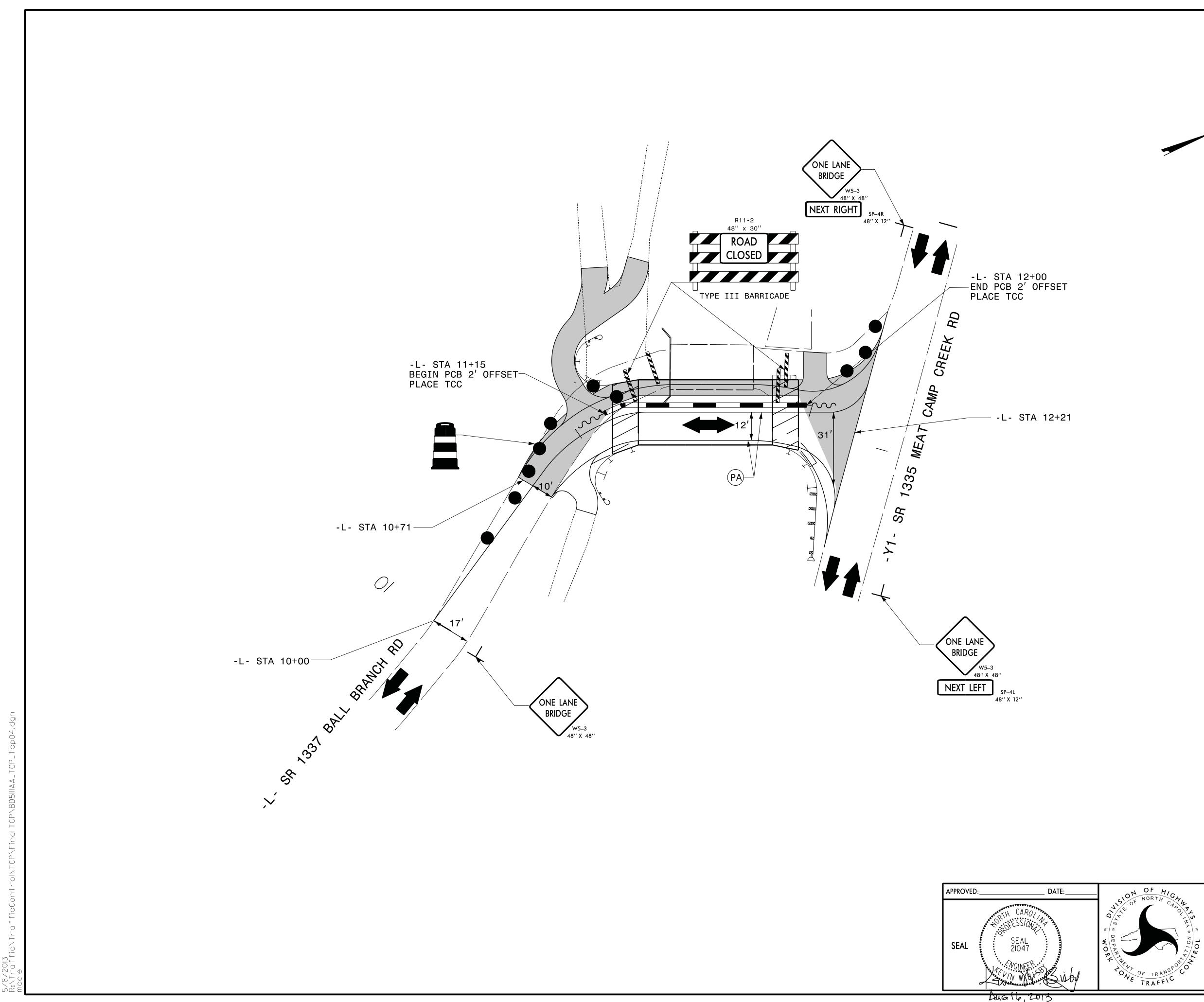
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	PROJ. REFERENCE NO.	SHEET NO.
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12+00		
P. SHORING		
TA 12+07		
TA 12+07 CB 2' OFFSET TCC 3' OFFSET		
TCC 3' OFFSET		
L 12''		
$DATE: \qquad O^{N} \xrightarrow{OF} \frac{H}{G}$		
DATE: OF OF HIGH		

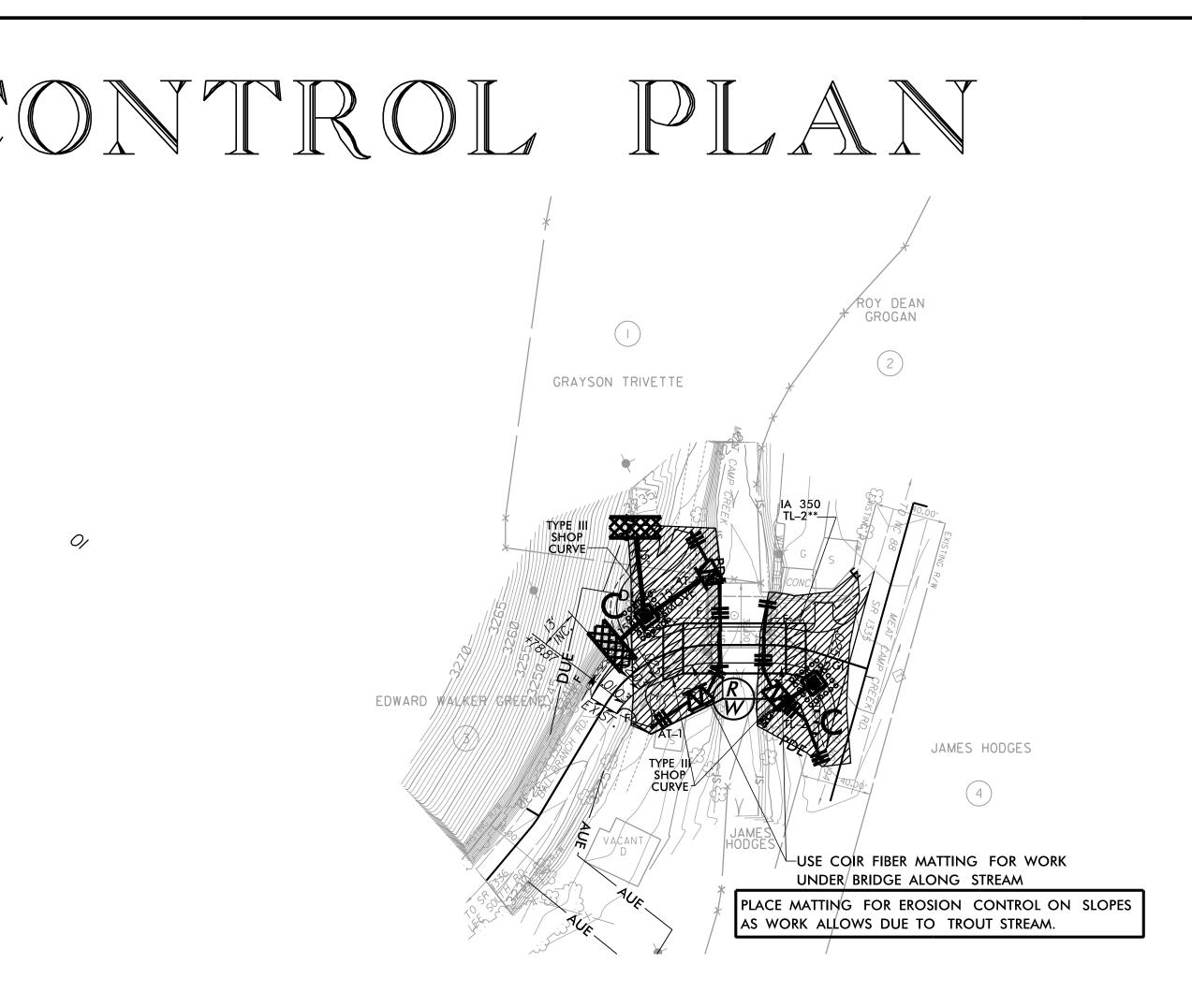
PHASE I DETAIL



PROJ. REFERENCE NO. BD-5111AA	sheet no. TCP-4
BD-5111AA	109-4

PHASE II DETAIL

	ROS		
	ENVIRONMENTALLY SEN PLEASE SEE NO	NSITIVE AREA	
SEN	ENVIRONMENTALLY SITIVE AREA(S) EXIST ON THIS PROJECT efer To E. C. Special Provisions for Special Considerations.		
NCG-010 ISSUED	HESE EROSION AND SEDIMENT CO WITH THE REGULATIONS SET 000 GENERAL CONSTRUCTION PERT BY THE NORTH CAROLINA DEPAR NATURAL RESOURCES DIVISION ANY DEVIATION FROM O REQUIRE PRIOR APPROVAL	T FORTH BY THE MIT EFFECTIVE AUGUST S RTMENT OF ENVIRONMENT OF WATER QUALITY.	
	ADDITIONAL EROSION CO NEED TO BE INSTALLED / ENGINEER.		



<u>Std.</u> #	Description Symbol
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
16 <b>22.0</b> 1	Temporary Berms and Slope Drains
1630.02	Silt Basin Type B·····
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion
1630.06	Special Stilling Basin
1632.03	Rock Inlet Sediment Trap Type C
1633.01	Temporary Rock Silt Check Type-A
	Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)
1633.02	Temporary Rock Silt Check Type=B
	Wattle
	Wattle with Polyacrylamide (PAM)
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A

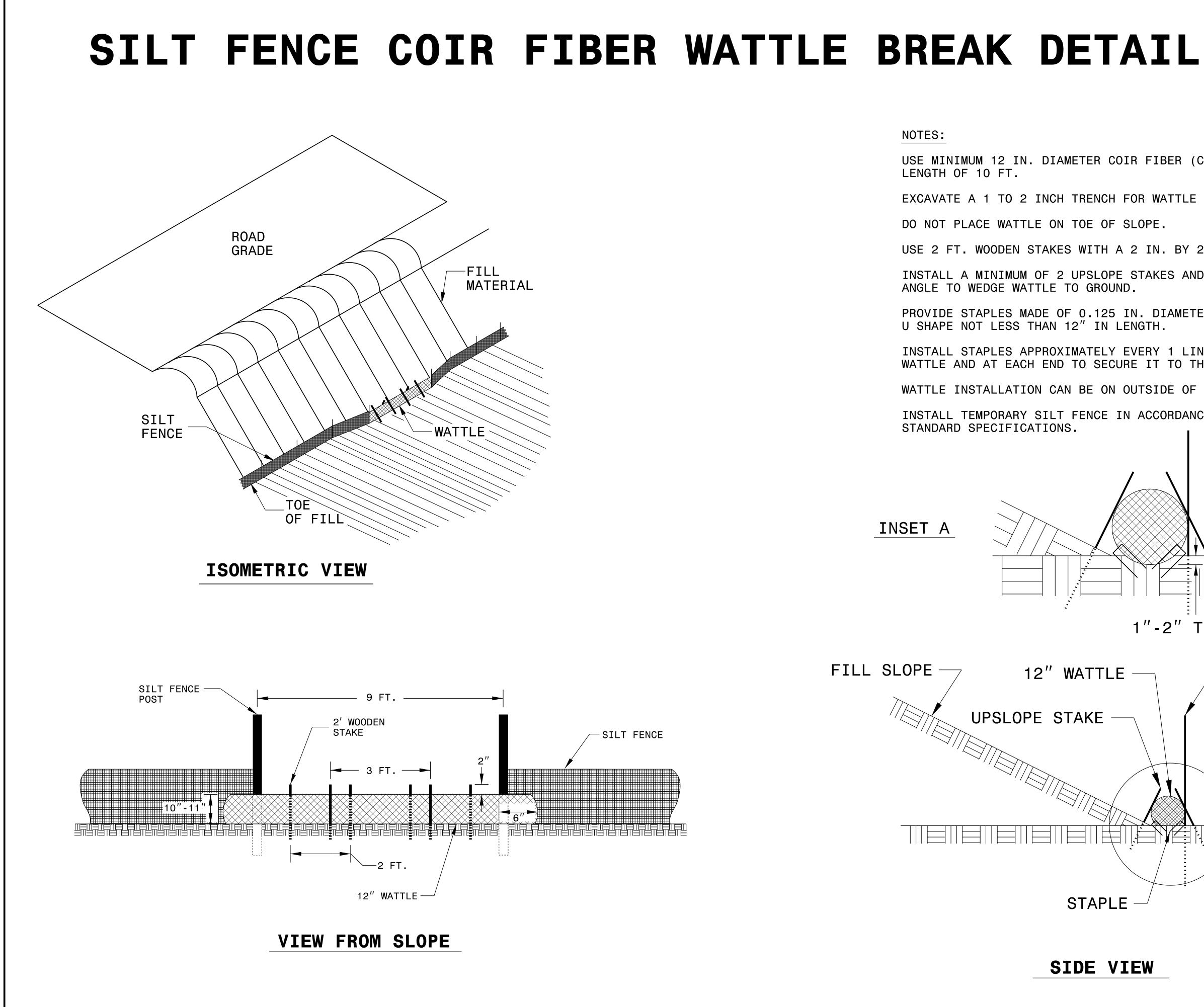
	PROJECT REFERENCE NO.	SHEET NO.
	BD-5IIIAA	EC-I
-01	R/W SHEET NC	).
NAD 83/NSRS 2007	ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
2012	STANDARD SPEC	CIFICATIONS

NOTES: 1. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

#### 2012 STANDARD DRAWINGS

$\begin{array}{c} 1605.01\\ 1606.01\\ 1607.01\\ 1622.01\\ 1630.01\\ 1630.02\\ 1630.03\\ 1630.04 \end{array}$	Railroad Erosion Control Detail Temporary Silt Fence Special Sediment Control Fence Gravel Construction Entrance Temporary Berms and Slope Drains Riser Basin Silt Basin Type B Temporary Silt Ditch Stilling Basin Temporary Diversion Special Stilling Basin Matting Installation	1632.02 1632.03 1633.01 1633.02 1634.01	Rock Pipe Inlet Sediment Trap Type B Coir Fiber Baffle
--	--	---	---

AUDREY BURNETTE LEVEL IIIA NAME	
3081 LEVEL IIIA CERTIFICATION NO.	PLANS PREPARED BY :
TO BE EXCAVATED	RUMMEL, KLEPPER & KAHL, LLP 900 RIDGEFIELD DRIVE SUITE 350
	RALEIGH, NORTH CAROLINA 27609–3960



8/2/2013 R:\Hydrau Fkeys

NOTES:

LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

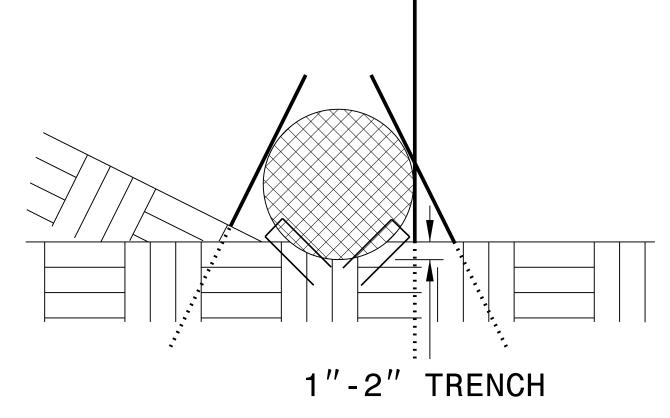
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

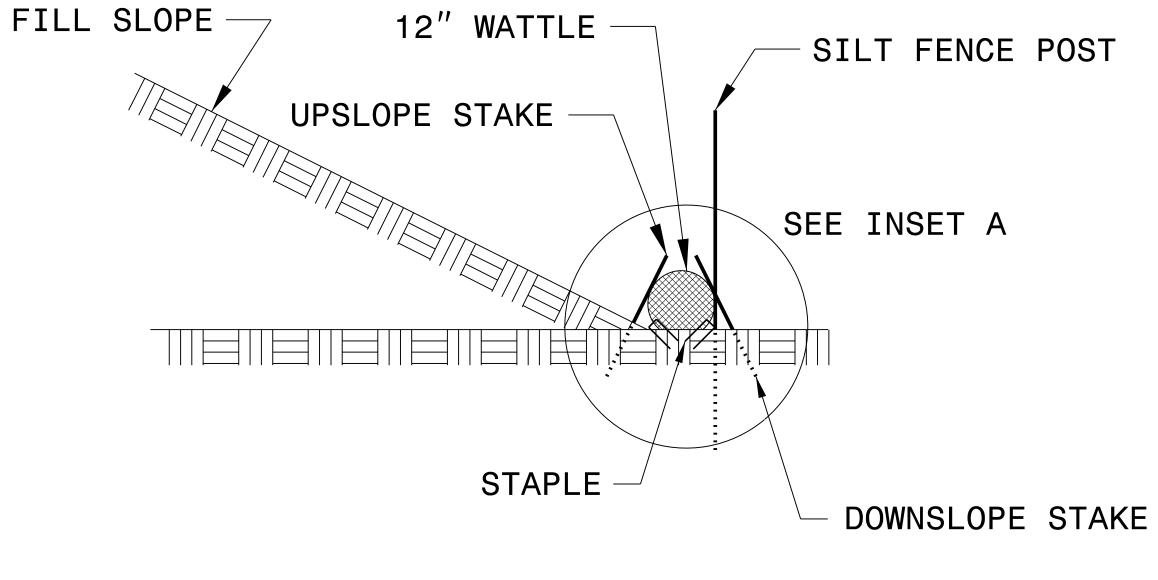
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

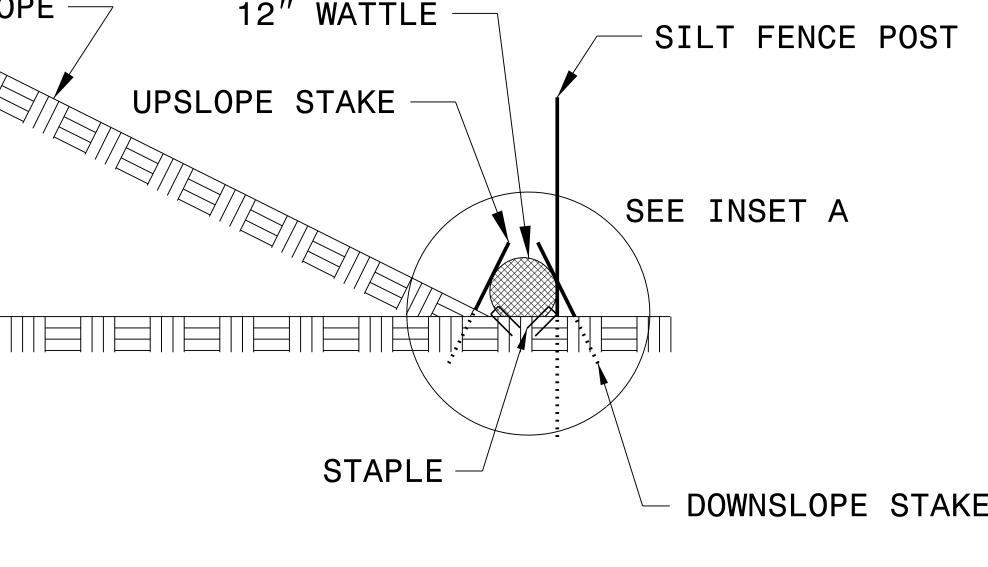
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED. INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

INSET A







PROJECT REFERENCE NO.	SHEET NO.
BD-5IIIAA	EC-2

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND

SIDE VIEW

# SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND S

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 OR FLATTER

ALL OTHER AREAS WITH SLOPES FLATTER

## DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

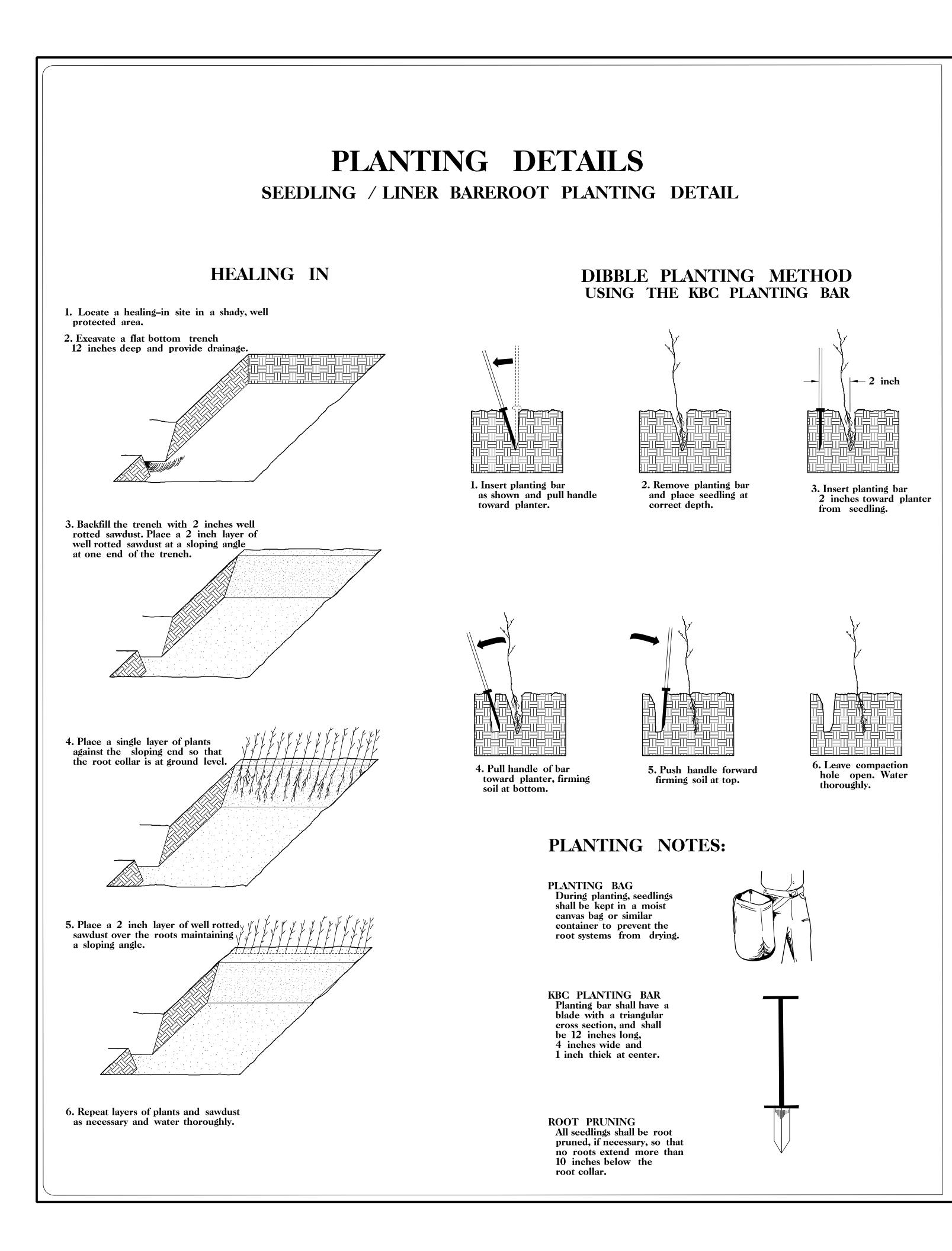
# SOIL STABILIZATION TIMEFRAMES

	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
SLOPES	7 DAYS	NONE
	7 DAYS	NONE
	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ER THAN 4:I	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

PROJECT REFERENCE NO. BD-5///AA	sheet no. EC-3 Iydraulics
BD-5IIIAA	
	ENGINEER

# F

## S.



## REFORESTATION

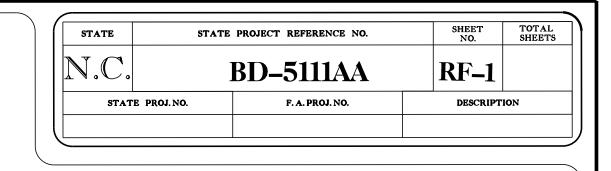
□ TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

#### Rŀ -----

EFORESTATION		
MIXTURE, TYPE, SIZE, AND FURNISH SHALL	CONFORM TO THE FOLL	OWING:
25% LIRIODENDRON TULIPIFERA	TULIP POPLAR	12 in – 18 in BR
25% PLATANUS OCCIDENTALIS	SYCAMORE	12 in – 18 in BR
25% FRAXINUS PENNSYLVANICA	GREEN ASH	12 in – 18 in BR
25% BETULA NIGRA	<b>RIVER BIRCH</b>	12 in – 18 in BR

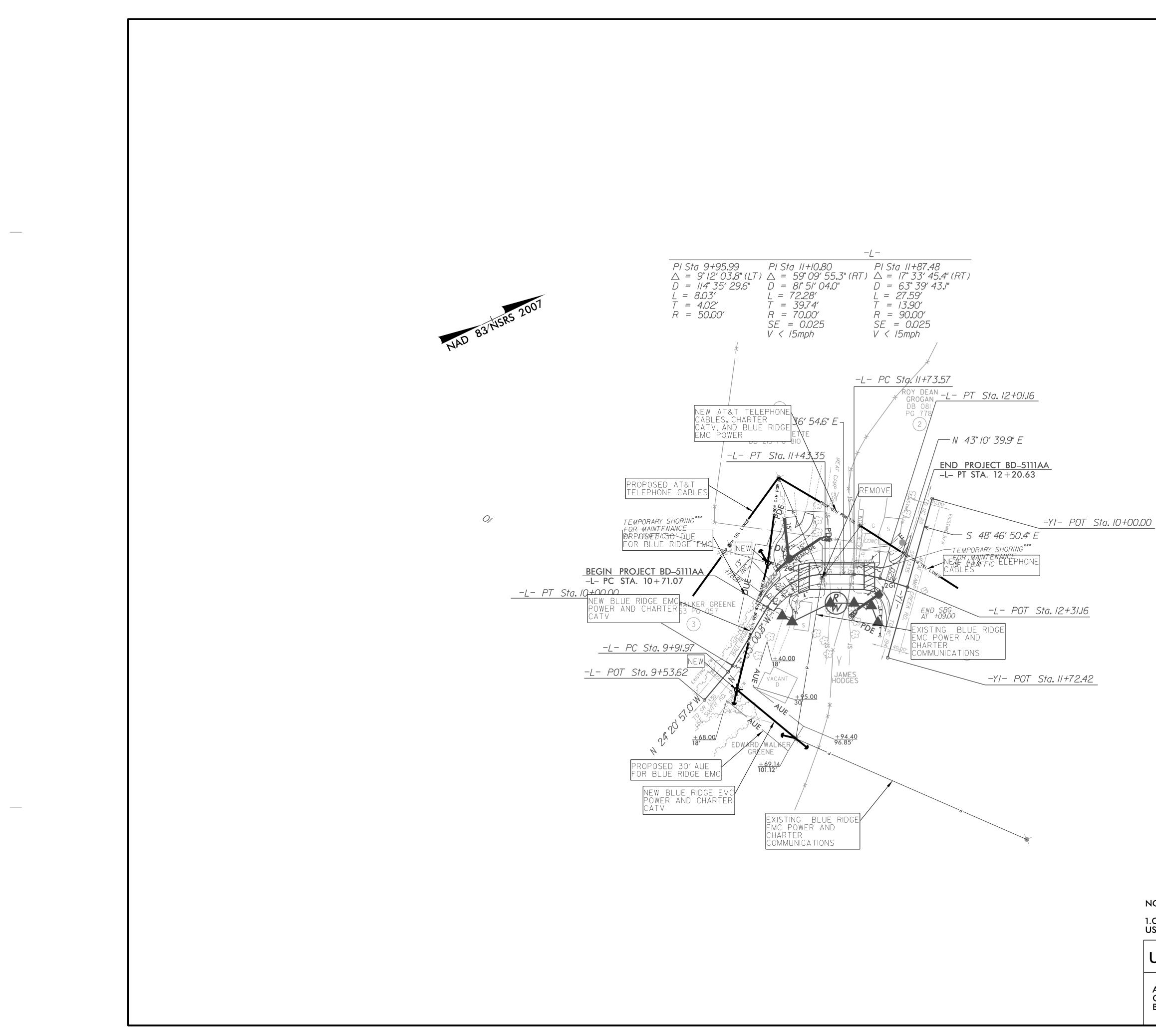
25%	BETULA	NIGRA	R	J





## **REFORESTATION DETAIL SHEET**

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT



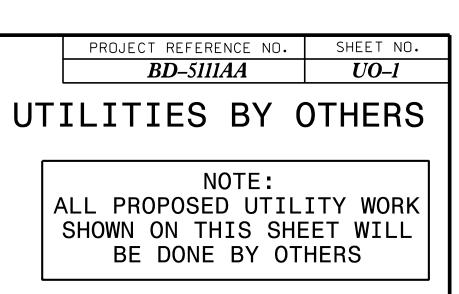
NOTES: 1.CHARTER COMMUNICATION IS IN JOINT USE WITH BLUE RIDGE EMC.

AT&T – TELEPHONE AND FIBER OPTIC CHARTER COMMUNICATIONS – CATV BLUE RIDGE EMC – POWER

UTILITY OWNERS ON PROJECT

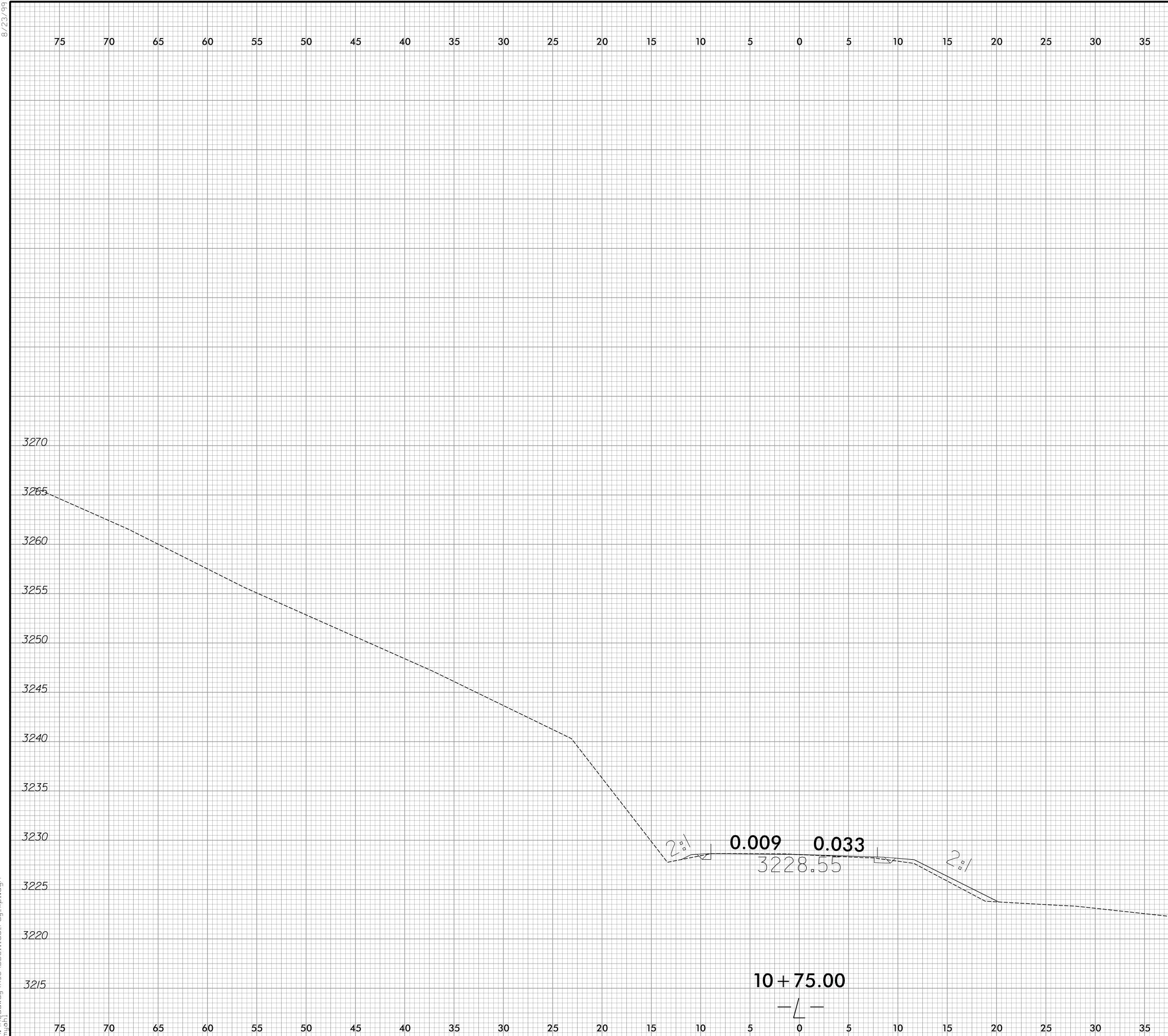
RUMMEL, KLEPPER & KAHL, LLP 900 RIDGEFIELD DRIVE SUITE 350 RALEIGH, NORTH CAROLINA 27609–3960 NC LICENSE NO. F-0112 • (919) 878-9560

PLANS PREPARED BY :





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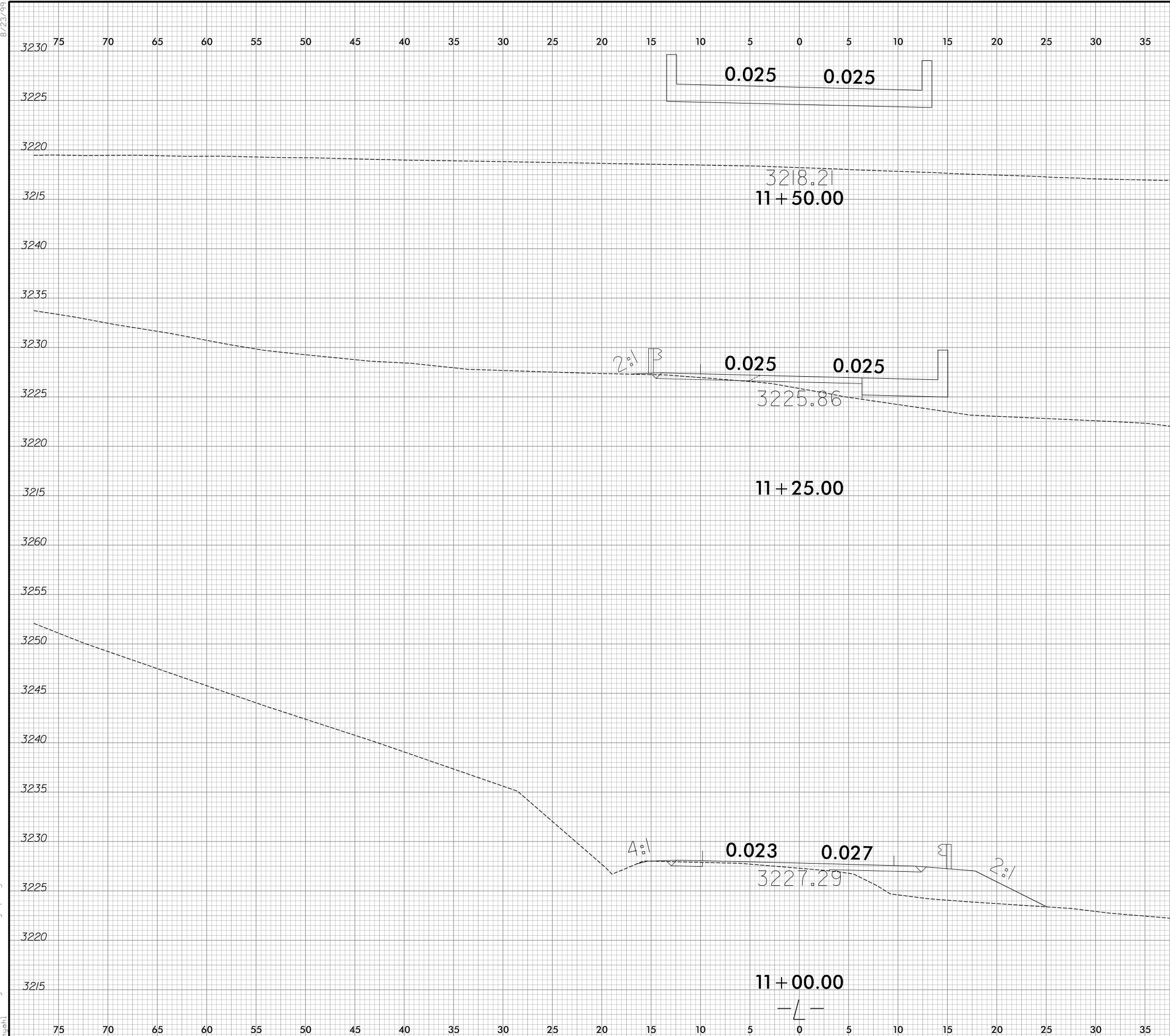


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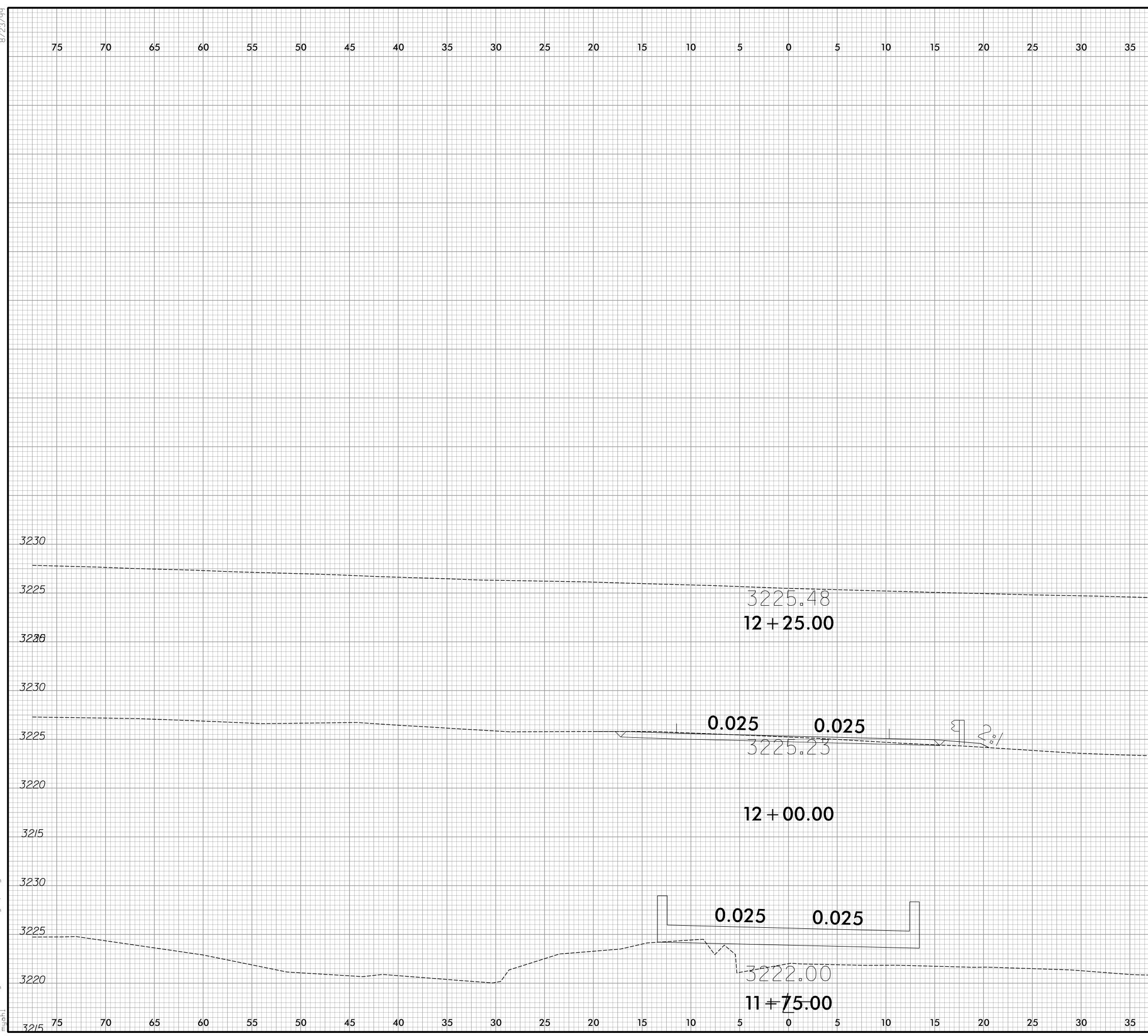
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