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INDEX of SHEETS, GENERAL NOTES, and LIST of STANDARDS

INDEX OF SHEETS

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1 – A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS
1 – B	CONVENTIONAL SYMBOLS
2A-1	PAVEMENT SCHEDULE AND TYPICAL SECTIONS
2D-1	DRAINAGE DETAILS
3B-1	GUARDRAIL, EARTHWORK, & PAVEMENT REMOVAL SUMMARIES
3D-1	DRAINAGE SUMMARY SHEET
3P-1	RIGHT OF WAY AREA DATA SHEET
4	PLAN/PROFILE SHEET
5	PLAN SHEET
RW-01 THRU RW-05	SURVEY CONTROL, EXISTING CENTERLINES, RIGHT OF WAY, EASEMENTS, & PROPERTY TIES
TMP-1 THRU SD-1	TRANSPORTATION MANAGEMENT & SIGNING PLANS
PM-1 THRU PM-2	PAVEMENT MARKING PLANS
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UO-1 THRU UO-2	UTILITIES BY OTHER PLANS
X-1 THRU X-7	CROSS-SECTIONS
S-1 THRU S-21	STRUCTURE PLANS

\Roadway\Proj\010291_rdy_1A.dgn

2018 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch -N. C. Department of Transportation - Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans: STD.NO. TITLE DIVISION 2 – EARTHWORK 200.03 Method of Clearing - Method III 225.02 Guide for Grading Subgrade - Secondary and Local CLEARING: 225.04 Method of Obtaining Superelevation - Two Lane Pavement DIVISION 3 - PIPE CULVERTS 300.01 Method of Pipe Installation SUPERELEVATION: DIVISION 4 - MAJOR STRUCTURES 422.01 Bridge Approach Fills - Type I Standard Approach Fill DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I SHOULDER CONSTRUCTION: DIVISION 6 – ASPHALT BASES AND PAVEMENTS 654.01 Pavement Repairs SIDE ROADS: DIVISION 8 - INCIDENTALS 806.01 Concrete Right-of-Way Marker 840.00 Concrete Base Pad for Drainage Structures 840.14 Concrete Drop Inlet - 12" thru 30" Pipe 840.15 Brick Drop Inlet - 12" thru 30" Pipe 840.29 Frames and Narrow Slot Flat Grates DRIVEWAYS: 840.35 Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates 840.45 Precast Drainage Structure 840.46 Traffic Bearing Precast Drainage Structure GUARDRAIL: 846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter 862.01 Guardrail Placement 862.02 Guardrail Installation 862.03 Structure Anchor Units 876.01 Rip Rap in Channels END BENTS: 876.02 Guide for Rip Rap at Pipe Outlets UTILITIES:

UTILITIES: UT

. . .

RIGHT-OF-WAY MARKERS: All RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY CONTRACT.



<u>General notes</u>

GRADING AND SURFACING OR RESURFACING AND WIDENING: THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.

VATION: ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

CONSTRUCTION: ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

DRIVEWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. 848.03 AT LOCATIONS SHOWN ON PLANS OR AS DIRECTED BY THE ENGINEER.

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITY OWNERS ON THIS PROJECT ARE Duke energy (power) & at&t (telecommunications).

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.



BOUNDARIES AND PROPERTY:

State Line	
County Line	
Township Line	
City Line	
Reservation Line	· ·
Property Line	
Existing Iron Pin	Cip
Computed Property Corner	×
Property Monument	
Parcel/Sequence Number	— (123)
Existing Fence Line	
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	— — — — wlb— — — —
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	— EAB — EAB —
Existing Endangered Plant Boundary	— ЕРВ ———
Existing Historic Property Boundary	— нрв
Known Contamination Area: Soil	— - 🔆 — s — 🔆 -
Potential Contamination Area: Soil	<u>x</u> ? - s - <u>x</u> ? -
Known Contamination Area: Water	ŷ€ w ĵ€ -
Potential Contamination Area: Water	𝔅?𝔅 ₩ 𝔅?𝔅 -
Contaminated Site: Known or Potential	
BUILDINGS AND OTHER CULT	URE:
Gas Pump Vent or U/G Tank Cap	- O
Gas Pump Vent or U/G Tank Cap	- O - O
Gas Pump Vent or U/G Tank Cap Sign Well	— O - O s - W
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine	- O - O s - W W - X
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building	
Gas Pump Vent or U/G Tank Cap	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School	
Gas Pump Vent or U/G Tank Cap	
Gas Pump Vent or U/G Tank Cap	
Gas Pump Vent or U/G Tank Cap	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Foundation Area Outline Cemetery Building School Church Dam <i>HYDROLOGY:</i> Stream or Body of Water Hydro, Pool or Reservoir	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam <i>HYDROLOGY:</i> Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam <i>HYDROLOGY:</i> Stream or Body of Water Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1	$ = \qquad \bigcirc \\ S \\$
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School School Church Dam HYDROLOGY: Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2	$ = \qquad \bigcirc \qquad$
Gas Pump Vent or U/G Tank Cap	$ = \qquad \bigcirc \qquad$
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Gas Pump Vent or U/G Tank Cap	$ = \bigcirc \\ \bigcirc$

RAILROADS:

Standard RR Signal Switch — RR Abanda RR Dismar

RIGHT OF WAY & PROJECT CONTROL:

Secondary Primary H Primary H Exist Perm New Peri Vertical Be Existing R Existing R New Rigł New Righ New Righ Concre New Con Concre Existing C New Con Existing E New Terr New Tem New Perr New Perr New Perr New Terr New Aer

ROADS AND RELATED FEATURES:

Existing Ed Existing C Proposed Proposed Proposed Existing A Proposed Existing C Proposed Equality Sy Pavement VEGETA Single Tre Single Shr

STATE OF NORTH CAROLINA CONVENTIONAL PLA Note: Not to Scale *S.U.

Gauge	CSX TRANSPORTATION
Milepost	💮 MILE POST 35
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loned	++++V
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Ioriz Control Point	
loriz and Vert Control Point	
nanent Easment Pin and Cap ———	$\langle \cdot \rangle$
manent Easement Pin and Cap ——	\diamond
enchmark	
Right of Way Marker	\bigtriangleup
Right of Way Line	
ht of Way Line	
ht of Way Line with Pin and Cap —	
ht of Way Line with ete or Granite R⁄W Marker	
ntrol of Access Line with ete C/A Marker	
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ntrol of Access	(A)
asement Line	E
nporary Construction Easement –	E
nporary Drainage Easement	TDE
manent Drainage Easement	PDE
manent Drainage / Utility Easement	DUE
manent Utility Easement	PUE
nporary Utility Easement	TUE
ial Utility Easement	AUE

dge of Pavement	
Curb	
Slope Stakes Cut	<u>C</u>
Slope Stakes Fill	F
Curb Ramp ————	CR
Aetal Guardrail ————	<u>T T T</u>
Guardrail ———	<u> </u>
Cable Guiderail	
Cable Guiderail	
Symbol	\bullet
Removal	
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DIVISION OF HIGHWA	VS	PROJECT REFERENCE 17BP.12.R.	се no. sheet 88 /-Е
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	JLS	\W/ATED.	
E. = Subsurface Utility Engineering		Water Manholo	- W
edge			
/oods Line		Water Meter	
erchard	ස් සි සි සි	Water Valve	۔ ب
neyard	Vineyard		- uç
EXISTING STRUCTURES:		U/G water line LOS B (S.U.E [*])	w
AJOR:		U/G Water Line LOS C (S.U.E*)	
ridge, Tunnel or Box Culvert	CONC	U/G water Line LOS D (S.U.E*)	A/G Water
ridge Wing Wall, Head Wall and End Wall-	-) CONC WW (Above Ground Water Line	
NOR:	<i>,</i> , ,	TV:	
lead and End Wall	CONC HW	TV Pedestal	
ipe Culvert		TV Tower	
ootbridge	≻≺	U/G TV Cable Hand Hole	H _H
Prainage Box: Catch Basin, DI or IB	Св	U/G TV Cable LOS B (S.U.E.*)	— — — — TV— — —
aved Ditch Gutter		U/G TV Cable LOS C (S.U.E.*)	- <u> </u>
torm Sewer Manhole —	S	U/G TV Cable LOS D (S.U.E.*)	Tv
torm Sewer	s	U/G Fiber Optic Cable LOS B (S.U.E.*) ——	— — — TV FO— — —
		U/G Fiber Optic Cable LOS C (S.U.E.*) ——	TV FO
TILITIES:		U/G Fiber Optic Cable LOS D (S.U.E.*) ——	TV F0
OWER:		GAS:	
xisting Power Pole	\bullet	Gas Valve	- ♦
roposed Power Pole	6	Gas Meter	· 🖒
kisting Joint Use Pole	- -	U/G Gas Line LOS B (S.U.E.*)	
roposed Joint Use Pole	-0-	U/G Gas Line LOS C (SUE*)	G
ower Manhole	(\mathbb{P})	U/G Gas Line LOS D (SILE*)	
ower Line Tower ————	\boxtimes	Above Ground Gas Line	A/G Gas
ower Transformer		Above Ground Ods Line	
G Power Cable Hand Hole		SANITARY SEWER:	
–Frame Pole –	••	Sanitary Sewer Manhole	
G Power Line LOS B (S.U.E.*)	— — — P— — — —	Sanitary Sewer Cleanout	· (†)
/G Power Line LOS C (S.U.E.*)	P	U/G Sanitary Sewer Line	
/G Power Line LOS D (S.U.E.*)	P	Above Ground Sanitary Sewer	A/G Sanitary Sewer
		SS Forced Main Line LOS B (S.U.E.*) ———	— — — — FSS — — —
		SS Forced Main Line LOS C (S.U.E.*) ———	
xisting Telephone Pole	-•-	SS Forced Main Line LOS D (S.U.E.*)———	FSS
roposed Telephone Pole	-0-		
elephone Manhole	\bigcirc		
elephone Pedestal	T		
elephone Cell Tower —————	,• ,	Utility Pole with Base	
/G Telephone Cable Hand Hole ———	Η _Η	Utility Located Object	• •
/G Telephone Cable LOS B (S.U.E.*)	t	Utility Trattic Signal Box	S
/G Telephone Cable LOS C (S.U.E.*)	T	Utility Unknown U/G Line LOS B (S.U.E.*)	?UTL
/G Telephone Cable LOS D (S.U.E.*)	T	U/G Tank; Water, Gas, Oil	-
G Telephone Conduit LOS B (S.U.E.*) ——	— — — TC— — — –	Underground Storage Tank, Approx. Loc. ——	UST
/G Telephone Conduit LOS C (S.U.E.*)——	TC	A/G Tank; Water, Gas, Oil	
G Telephone Conduit LOS D (S.U.E.*)	TC	Geoenvironmental Boring	
VG Fiber Optics Cable LOS B (S.U.E.*) ——	— — — T FO— — ·	U/G Test Hole LOS A (S.U.E.*)	
J/G Fiber Optics Cable LOS C (S.U.E.*)	——————————————————————————————————————	Abandoned According to Utility Records —	AATUR
J/G Fiber Optics Cable LOS D (S.U.E.*)	T F0	End of Information	E.O.I.

Bridge, Tunnel or Box Culvert	CONC
MINOR: Head and End Wall	CONC H
Pipe Culvert	
Footbridge	≻
Drainage Box: Catch Basin, DI or JB ———	
Paved Ditch Gutter	
Storm Sewer Manhole	S
Storm Sower	s_

POWER:	
Existing Power Pole	
Proposed Power Pole	6
Existing Joint Use Pole	
Proposed Joint Use Pole	- \
Power Manhole	P
Power Line Tower	\boxtimes
Power Transformer	\bowtie
U/G Power Cable Hand Hole	
H–Frame Pole	•
U/G Power Line LOS B (S.U.E.*)	— — — P-
U/G Power Line LOS C (S.U.E.*)	——— — P -
U/G Power Line LOS D (S.U.E.*)	———— P –

Existing Telephone Pole	-•-
Proposed Telephone Pole	-0-
Telephone Manhole	\bigcirc
Telephone Pedestal	T
Telephone Cell Tower ————	, Į
U/G Telephone Cable Hand Hole ———	H _H
U/G Telephone Cable LOS B (S.U.E.*)	T _
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U/G Telephone Conduit LOS B (S.U.E.*)	— — — TC—
U/G Telephone Conduit LOS C (S.U.E.*)	——————————————————————————————————————
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U/G Fiber Optics Cable LOS B (S.U.E.*)	— — — — T FO
U/G Fiber Optics Cable LOS C (S.U.E.*)	——— — T FO
U/G Fiber Optics Cable LOS D (S.U.E.*)	T FO



SCI	CHEDULE								
1	PROP. APPROX. 5.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 627 LBS. PER SQ. YD.								
2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" OR GREATER THAN 5.5" IN DEPTH.								
Г	EARTH MATERIAL.								
J	EXISTING PAVEMENT.								
N	VARIABLE DEPTH ASPHALT PAVEMENT								









DRAINAGE DETAILS



FROM -L- STA. 16+84 TO STA. 18+25 RT FROM -L- STA. 16+91 TO STA. 18+25 LT





SUMMARY OF EARTHWORK

IN CUBIC YARDS

		EXCAV	ATION			
STATION	STATION	TOTAL	UNDERCUT	EMBANK.	BORROW	WASTE
		UNCLASS.				
-L- 14+30.00	-L- 15+65.21 (Begin Bridge)	133		184	51	
	SUBTOTAL #1	133		184	51	
-L- 16+87.79 (End Bridge)	-L- 18+40.00	170		486	316	
	SUBTOTAL #2	170		486	316	
TO'	TAL	303		670	367	
LOSS DUE TO CLEA	ARING & GRUBBING	-15			15	
ESTIMATED SHOU	ULDER MATERIAL			74	74	
	ΤΩΤΑΙ	297		742	455	
		207		/43	455	
EST. 5% TO REPLACE TO	OP SOIL ON BORROW PIT				23	
GRAND	TOTAL	287		743	477	
SA	AY	290			480	

ALN. BEG. STA. E	END STA.					LENGTH		WARRAN	IT POINT	"N" DIST.	TOTAL	FLARE	LENGTH		N	4	NCHOR	S	IMF ATTEN TYP	PACT NUATOR PE 350	REMOVE	
		LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPR. END	TRAIL. END	FROM E.O.L.	WIDTH	APPRO ACH END	TRAILI NG END	APPR. END	TRAIL. END	TYPE-III	GREU 350 TL-3	AT-1	G	NG	GR			
																					<u> </u>	
-L-	15+12.76	15+68.99	LT	43.97	31.03		15+68.99		3.92	6.92					1		1				<u> </u>	
-L-	15+07.90	15+61.53	RT	39.53	22.97			15+61.53	3.92	6.92					1		1				<u> </u>	
-L-	16+91.49	17+72.74	LT	81.25				16+91.49	3.92	6.92	50		1		1	1						
-L-	16+84.03	17+65.28	RT	81.25			16+84.03		3.92	6.92		50		1	1	1						
SUBTOTAL:		246.00	54.00										4	2	2							
	ANCI	HOR UNIT DE	EDUCTIONS:																			
	Type-III @	0 18.75' Each		-75.00																		
	TL-3 @	0,50' Each		-100.00																		
	AT-1 @	6.25' Each			-12.50																	
	LESS GU	IARDRAIL DE	EDUCTIONS:	71.00	41.50																	
		PROJ		71 00	41 50																	
			SAY.	75.0	50.0										4	2	2				L	
				75.0	50.0										-	L	L				—	
″N″ = TOTAL FLARE	"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT. FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL																					

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SAFETY FENCE SUMMARY

IN LINEAR FEET

LINE	STATION	STATION	SIDE	LENGTH (LF)
-L-	13+85.000	14+55.000	RT	76.35
	J	1	TOTAL SAY	76.35 80

LINE	STATION	STATION	LOCATION	LENGTH OR AREA	WIDTH	SQUARE YARDS
-L-	15+00	15+78	CL	1,272.29		141.37
-L-	16+74	18+00	CL	2,143.23		238.14
			1		TOTAL	379.50
					SAY	380

GUARDRAIL SUMMARY

PROJECT REFERENCE NO.	SHEET NO.
17BP.12.R.88	3B-1



IN SQUARE YARDS

REMARKS
Rigid Object
Rigid Object
Rigid Object
Rigid Object



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ECKED BY: JMW			DATE:	<u>o /1/ /20</u>														$\mathbb{V} \parallel \mathbb{S}$	DIC	$\mathbb{I} \setminus \mathbb{N}$	C	∥ [¶				I <i>I</i>	J												L	/	1 BP.12.	Π.ԾԾ		ו-עכ
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	L) NC																						OTHER	WISE)		l	_IN.					40.16	840.2 840.2 840.2	840.2 GRA1	ES SI GRAT	T GR			840.5			& SIZ	Y. STI	3TD. 8
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SHEET 4																															1													
-L- 17+13	LT	401		1144.77																					1											1		1						
-L- 17+13	LT	401	402		1139.8	1134.3	0.3	28																																	2 7	2@15"		
-L- 17+05	RT	403		1144.80																					1											1		1						
-L- 17+05	RT	403	404		1139.8	1134.9	0.3	24																																	2 7	2@15"		
-L- 14+75	LT	405			1143.2	1132.7												96																							3 10			
-L- 15+00	LT																																											41
Project Total								52										96							2											2		2			7 24			41



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COMPUTED BY: ESP DATE: 5/15/18 CHECKED BY: DATE: DATE:	DIVISIO STATE O	ON OF HIGHWAYS F NORTH CAROLINA		PROJECT REFERENCE NO. SHEET NO. 17BP.12.R.88 3P-1
	RIGHT OF WA	IY AREA DATA SHEET		
PARCEL NO.	PROPERTY OWNERS NAME	TOTAL AREA AREA TAKEN AREA AREA TOTAL AREA AREA TAKEN REMAINING REMAINING	CONSTR. EASEMENT	
	DONOCAN & SANDRA FULTON	RIGHT LEFT 1.860 AC 0.037 AC	0.067 AC	
2	PATRICIA L. BELK	0.046 AC	0.002 AC	
3	LINDA C. & GARRELL S. PENNELL	0.147 AC	0.012 AC	
Q3P-1.dgn				
11_rdy_psh				
ay Nroj N01029				P: (919) 878-9560 8601 Six Forks Road, Forum 1,Suite 700 Raleigh, North Carolina 27615-3960 NC License No. F-0112
Arit Actor default Arit				Engineers Construction Managers Planners Scientists www.rkk.com Responsive People Creative Solutions





BL				
	POINT	DESC.	NORTH	EAST
LS1 LS2 LS3		B4980 - 1 B4980 - 2 BL - 3	799811.7780 800036.8730 800273.4846	1315899.525 1316453.309 1317096.436
		BM1 ELEVA N 799858 E CHISELED SQUARE	TION = 1166.21 1315938 IN ROCK OUTCROP	* * * * * * * * * *
		BM2 ELEVA N 799697 E RR SPIKE IN BAS	TION = 1134.31 1316489 5E 38/" SYCAMORE	* * * * * * * * * *
		BM3 ELEVA N 800207 E RR SPIKE FLUSH	TION = 1150.70 1317319 IN PAVEMENT SR 1	348
		BM4 ELEVA N 800106 E NCSHC 24JS13	TION = 1149.24 1316563	* * * * * * * * * *

BL				
POINT	N	E	BEARING	DIST
POT	799811.778	1315899.525		
LINE			N 67°52′47.5" E	597.78
POT	800036.873	1316453.309		
LINE			N 69°48′03.7" E	685.27
POT	800273.485	1317096.436		

EL									
POINT	N	E	BEARING	DIST	DELTA	D	L	Т	R
PC	799841.266	1315943.186							
CURVE			N 68°55′26.Ø"E	96.28	02°07′18.7"(LT)	Ø2°12′13.3"	96.29	48.15	2600.00
PCC	799875.890	1316033.027							
CURVE			N 67°30′51.6"E	311.52	00°41′50.0"(LT)	00°13′25.7"	311 . 52	155.76	25600.00
PCC	799995.031	1316320.862							
CURVE			N 67°30′33.8" E	568.62	00°41′14.4"(RT)	00°07′15.2"	568.63	284.32	47400.00
PCC	800212.547	1316846.238							
CURVE			N 71°57′25.7" E	107.35	Ø8°12′29.2"(RT)	Ø7°38′22.Ø"	107.44	53.81	750.00
PCC	800245.797	1316948.311							
CURVE			S 87°16′41.2" E	303.91	33°19′17 . 1"(RT)	10°48′37.9"	308.23	158.61	530.00
PT	800231.366	1317251.874							

SURVEY CONTROL SHEET

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

NO I ES:

I. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.

2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

PROJECT REFERENCE NO.	SHEET NO.
17BP.12.R.88	RW-02
Location and S	ourveys

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PROPOSED ALIGNMENT CONTROL SHEET

STATION	NORTH	EAST
10+00.00	799876.1137	1316032.9355
13+11.52	799995.0309	1316320.8623
18+8Ø.14	800212.5475	1316846.2376
19+87.61	800245.8897	1316948.2947
22+00.00	800254.8237	1317159.1271

NOTES:

I. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.

2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES.IF FURTHER INFORMATINO REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

PROJECT REFERENCE NO.	SHEET NO.
I7BP.12.88	RW-03
Location and	Surveys

RIGHT (OF WA	Y AND	PERN	ANENT EA	9
				I, KYLE A. WOLFF, a Profession to the best of my knowled performed under my respon Location & Surveys guidelin I further certify that the outlined in the tables show and are accurate represen on the corresponding highw easement points shown her survey control provided by by others; and these monun staking which may be subje determination). Witness my original signatur	
				ProfessionalLand Surveyor	
F ALIGN STA L 14+ L 14+ L 14+ L 14+ L 18+ L 18+ L 18+ L 18+	ROW MARKER TION 0 80.00 0 80.00 0 85.00 0 15.00 0 40.00 0	CONCRETE DFFSET 45.00 29.29 -30.72 -46.00 -46.00 45.00 -30.22 29.78	OR GRANI NORTH 800017.9034 800032.4195 800089.7795 800103.8948 800230.1303 800149.8765 800225.1177 800169.6804	TE-E EAST 1316493.7443 1316487.7343 1316469.3975 1316463.5536 1316768.4546 1316812.5044 1316797.5879 1316820.5401	
ALIGN STA L 13+ L 13+ L 13+ L 17+ L 19+ L 19+ L 19+ L 19+ L 19+ L 21+ L 21+ L 21+ L 21+	ROW MARKEI TION 0 79.00 0 47.00 0 47.00 0 63.00 0 63.00 0 63.00 0 63.00 0 63.00 0 63.00 0 63.00 0 63.00 0 60.00 0 60.00 0 60.00 0 60.00 0 90.00 0	PERMANEN DFFSET -58.00 -30.36 -62.00 -74.00 -74.00 -74.00 -74.00 -59.00 -59.00 -54.00 -30.05	T EASEMEN NORTH 800074.4337 800048.8933 800218.9012 800229.9885 800229.9885 800225.0217 800225.0217 800291.8707 800304.2905 800304.2905 800308.0523 800308.0523 800308.0523	T - E EAST 1316361.0253 1316371.5997 1316699.5060 1316694.9156 1316709.6987 1316714.2891 1316889.5021 1316901.8130 1316905.3863 1317123.8991 1317152.1806 1317153.2999	

SEMENT CONTRO

nalLand Surveyor in the state of North lge and belief that the following work ite nsible charge meet NCDOT Survey Standard nes and procedures.

right of way and permanent easement po n hereon (localized coordinates, station/o ntations of the right of way and perman vay plans. Lalso certify that the right of ein have been field monumented under my others; that the depicted property data ments denote the right of way and easer ect to change due to right of way revisions (See deeds for final

e, registration number and sealthis 29th day of January, 2020.

L-4870 PLS #

NOTES:

2. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.

	PROJECT REFERENCE NO.	SHEET NO.
	17BP.12.R.88	RW03E-I
UL SHEEI	Location and S	urveys
Carolina hereby certify em(s)(R/W Staking) rds as directed in the NCDOT	PROJECT SUR	VEYOR
points shown herein and offset)have been checked hent easement points depicted	DA180 448574901E	
of way and permanent y supervision from existing a shown berein were surveyed	DOCUMENT NOT CONS UNLESS ALL SIGNATURES	DERED FINAL S COMPLETED
ement boundaries at the time of		

I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL

IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

I, Kyle A. WOIff, a ProfessionalLand Surveyor in the state of North Carolina hereby certify to the best of my knowledge and belief that the following work item(s)(R/W Staking) performed under my responsible charge meet NCDOT Survey Standards as directed in the NCDOT Location & Surveys guidelines and procedures.

Ifurther certify that the right of way and permanent easement points shown herein and outlined in the tables shown hereon (localized coordinates, station/offset) have been checked and are accurate representations of the right of way and permanent easement points depicted on the corresponding highway plans. Lalso certify that the right of way and permanent easement points shown herein have been field monumented under my supervision from existing survey controlprovided by others; that the depicted property data shown herein were surveyed by others; and these monuments denote the right of way and easement boundaries at the time of staking which may be subject to change due to right of way revisions (See deeds for final determination).

ProfessionalLand Surveyor

SHEET NO.

- TMP 1
- TMP-1A
- TMP-1B
- TMP-2
- TMP-3
- SD 1

INDEA OF SHEEIS	INDEX	OF	SHEETS
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TITLE

TITLE SHEET AND INDEX OF SHEETS LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS AND LEGEND TRANSPORTATION OPERATIONS PLAN: (MANAGEMENT STRATEGIES AND GENERAL NOTES) TRAFFIC CONTROL PHASING OFFSITE DETOUR

SIGN DESIGN

SHEET NO.

TMP-1

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	DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED
BY :	APPROVED: C. Byton Holden DATE: 5C5A055EAF0545F 1/30/2021
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ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.

ΩĽ

TITLE

1101.01	WORK ZONE ADVANCE WARNING SIGNS
1101.02	TEMPORARY LANE CLOSURES
1101.03	TEMPORARY ROAD CLOSURES
1101.04	TEMPORARY SHOULDER CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1130.01	DRUM
1145.01	BARRICADES
1150.01	FLAGGING DEVICES
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTAL
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES
1262.01	GUARDRAIL END DELINEATION

LEGEND

<u>GENERAL</u>

NORTH ARROW

LLATION SPACING AND MOUNTING

APPRO	VED: C. Byron Holden	
	5C5A055EAF0545F	
DATE:	1/30/2021	
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PROJ. REFERENCE NO. SHEET NO. TMP-1A 17BP.12.R.88

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TRAFFIC CONTROL DEVICES

BARRICADE (TYPE III)

TEMPORARY SIGNING

STATIONARY SIGN

TRAFFIC MANAGEMENT STRATEGIES: OFF-SITE DETOURS / USE OF ALTERNATIVE ROUTES GENERAL NOTES / LOCAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING B) TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

AND

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED C) TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

AND

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

- D) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.
- E) THE CONTRACTOR SHALL REPLACE ANY EXISTING SIGNS DAMAGED BY CONSTRUCTION OPERATIONS. THE SIGNS SHALL BE FURNISHED AND INSTALLED BY THE CONTRACTOR ACCORDING TO NCDOT'S SPECIFICATIONS. THE CONTRACTOR SHALL REMOVE ANY BRIDGE SIGNS NO LONGER APPLICABLE AFTER THE BRIDGE REPLACEMENT BOTH WITHIN AND OUTSIDE THE CONSTRUCTION LIMITS.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

G) INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE AS SHOWN IN THE PAVEMENT MARKING PLAN.

LANE AND SHOULDER CLOSURE REQUIREMENTS

I) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.

- J)
- K)

APPROVE	DocuSigned by: C. Byron Holdin	
	5C5A055EAF0545F	
DATE:	1/30/2021	
	SEAL	

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H) REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED OR AS DIRECTED BY THE ENGINEER.

WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING ON THE SHOULDER ADJACENT TO AN UNDIVIDED FACILITY AND WITHIN 5 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN TRAVEL LANE USING ROADWAY STANDARD DRAWING NO. 1101.02 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL.

WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN A LANE OF TRAVEL OF AN UNDIVIDED OR DIVIDED FACILITY, CLOSE THE LANE ACCORDING TO THE TRAFFIC CONTROL PLANS, ROADWAY STANDARD DRAWINGS, OR AS DIRECTED BY THE ENGINEER. CONDUCT THE WORK SO THAT ALL PERSONNEL AND/OR EQUIPMENT REMAIN WITHIN THE CLOSED TRAVEL LANE.

L) DO NOT WORK SIMULTANEOUSLY WITHIN 15 FT ON BOTH SIDES OF AN OPEN TRAVELWAY, RAMP, OR LOOP WITHIN THE SAME LOCATION UNLESS PROTECTED WITH GUARDRAIL OR BARRIER.

TRAFFIC CONTROL PHASING

- STEP 1) USING AN APPROVED METHOD PER THE DISCRETION OF THE ENGINEER, INSTALL SIGNING FOR THE SR 1348 (OLD NC 90) OFFSITE DETOUR, KEEPING SIGNS COVERED. REFER TO ROADWAY STANDARD DRAWING NUMBER 1101.03 SHEET 1, TMP-3 AND SD-1 FOR SIGN LOCATIONS AND DESIGN.
- STEP 2) UNCOVER OFF-SITE DETOUR SIGNS AND CLOSE SR 1348 (OLD NC 90) TO THRU TRAFFIC USING ROADWAY STANDARD DRAWING NUMBER 1101.03 SHEET 2.
- STEP 1) DEMOLISH AND REMOVE EXISTING BRIDGE NO. 291 OVER DUCK CREEK.
- STEP 2) CONSTRUCT PROPOSED STRUCTURE AND APPROACH ROADWAY WIDENING AND PAVING (SEE ROADWAY PLANS AND STRUCTURE PLANS). REPLACE ANY EXISTING SIGNS REMOVED DUE TO CONSTRUCTION OPERATIONS.
- STEP 3) PLACE THE FINAL PAVEMENT MARKINGS AND MARKERS IN ACCORDANCE WITH THE FINAL PAVEMENT MARKING PLANS AND OPEN SR 1348 (OLD NC 90) TO TRAFFIC. REMOVE ANY REMAINING TRAFFIC CONTROL DEVICES AND DETOUR SIGNS FROM THE PROJECT.

	Docusigned by:
APPRO	VED: Vigton 17 datin
	5C5A055EAF0545F
DATE:	1/30/2021
	SEAL

PROJ. REFERENCE NO.	SHEET NO.	
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P: (919) 878-9560 8601 Six Forks Road, Forum 1,Suite 700 Raleigh, North Carolina 27615-3960 NC License No. F-0112		
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SIGN NUMBER: SD-1	BA
TYPE: STATIONARY	CO
QUANTITY: SEE PLANS	SYMB
SIGN WIDTH: 2'-6"	
HEIGHT: 1'-0"	
OTAL AREA: 2.5 Sq.Ft.	
BORDER TYPE: INSET	
RECESS: 0.38"	
WIDTH: 0.63"	
RADII: 1.5 "	
NO. Z BARS:	MAT'L
LENGTH:	
USE NOTES:	1,2

 Legend and border shall be direct applied black non-reflective sheeting.
 Background shall be NC GRADE B fluoresent orange retroreflective sheeting.

LETTER POSITIONS

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APPROVED: C. Byton Holden 505A055EAF0545F
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EROSION AND SEDIMENT CONTROL MEASURES Sid Description Symbol 1650.05 Temporary Silt Ditch Temporary Silt Pance Temporary Silt Pance 1605.01 Temporary Diversion Temporary Silt Pance Temporary Silt Pance 1606.01 Special Scidmett Control Pance Temporary Silt Pance Temporary Silt Pance 1606.01 Special Scidmett Control Pance Temporary Reck Silt Check TyperA Temporary Reck Silt Check TyperA 1632.02 Temporary Reck Silt Check TyperA With Matting and Polyacrylamide (PAM) Temporary Reck Silt Check TyperA 1635.02 Temporary Reck Silt Check TyperA Temporary Reck Silt Check TyperA Temporary Reck Sciencet Dam TyperA 1635.02 Temporary Reck Sciencet Dam TyperA Temporary Reck Sciencet Dam TyperA Temporary Reck Sciencet Dam TyperA 1635.02 Temporary Reck Sciencet Trap TyperA Temporary Reck Sciencet Trap TyperA Temporary Reck Sciencet Trap TyperA 1635.02 Reck Inlet Sciencet Trap TyperA Temporary Reck Sciencet Trap TyperA Temporary Reck Sciencet Trap TyperA 1632.02 Type B BE BE Temporary Reck Sciencet Trap TyperA 1632.03 Type C CO Stimmer Basin Temporary Rec		
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Std. Description Stable 1630.03 Temporary Silt Ditch me 1630.03 Temporary Silt Pince me 1605.01 Temporary Silt Pence me 1606.01 Special Sediment Cantrol Pence me 1605.02 Temporary Silt Pence me 1630.02 Silt Basin Type B me 1635.01 Temporary Rock Silt Check Type A me 1635.02 Temporary Rock Silt Check Type A me 1655.02 Temporary Rock Silt Check Type B me 1634.01 Temporary Rock Silt Check Type B me 1634.02 Temporary Rock Sediment Dam Type B me 1635.02 Rock Pipe Inlet Sediment Trap Type B me 1635.02 Rock Pipe Inlet Sediment Trap Type B me 1635.03 Type A A me 1632.04 Stilling Basin me me 1632.05 Type A A me 1632.05 Type A A me 1632.05 Type A A me 1632.05 Type C C me	EROSIO	N AND SEDIMENT CONTROL MEASURES
1630.03 Temporary Silt Ditch. 1630.05 Temporary Diversion 1605.01 Temporary Bit Fence 1605.01 Special Soliment Control Fence 1630.02 Silt Basin Type B. 1635.02 Temporary Rock Silt Check Type-A. 1635.02 Temporary Rock Sediment Dam Type-A. 1634.01 Temporary Rock Sediment Dam Type-B. 1635.02 Reck Pipe Lalet Sediment Trap Type-B. 1635.03 Reck Pipe Lalet Sediment Trap Type-B. 1635.04 Special Stilling Basin 1632.05 Type A. 1632.01 Type A. 1632.02 Type B. 1632.03 Type C. 1632.04 Type C. 1632.05 Type C. 1632.06 Type C. 1632.07 Type C. 1632.08 Type C. 1632.09 Type C. 1632.00 Type C.	<u>Std.</u> #	Description Symbol
1605.01 Temporary Silt Fence	1630.03	Temporary Silt Ditch
1606.01 Special Sediment Control Fence 1622.01 Tomporary Borns and Slope Drains 1630.02 Silt Basin Type B. 1635.01 Temporary Rock Silt Check TyperA. with Matting and Polyacrylamide (PAM). 1635.02 Temporary Rock Silt Check TyperB. Wattle/Coir Fiber Wattle. Wattle/Coir Fiber Wattle. 1654.02 Tomporary Rock Sediment Dam Type*B 1655.02 Rock Pipe Inlet Sediment Trap: 1652.03 Type A. 1652.04 Stilling Basin 1652.05 Type C. Skimmer Basin FOR CLEARING AND G	1605.01	Temporary Silt Fence
1622.01 Temporary Berms and Slope Drains	1606.01	Special Sediment Control Fence
1633.01 Temporary Rock Silt Check Type-A With Matting and Polyacrylamide (PAM) 1633.02 Temporary Rock Silt Check Type-A With Matting and Polyacrylamide (PAM) 1633.02 Temporary Rock Silt Check Type-B With Polyacrylamide (PAM) 1634.01 Temporary Rock Sediment Dam Type-A Wattle/Coir Fiber Wattle 1634.02 Temporary Rock Sediment Dam Type-A Wattle/Coir Fiber Wattle 1635.02 Rock Pipe Inlet Sediment Dam Type-A O 1635.03 Rock Pipe Inlet Sediment Trap Type-A O 1630.04 Stilling Basin O 1632.02 Type A A 1632.03 Type C C 1632.03 Type C C Skimmer Basin Infiltration Basin Infiltration Basin Infiltration Basin Infiltration Basin Infiltration Basin Infiltration Basin Infiltration Basin Infiltration Basin	1622.01 1630.02	Silt Basin Type B
Temporary Reck Silt Check Type-A with Matting and Palyacrylamide (PAM) 1633.02 Temporary Rock Silt Check Type-B	1633.01	Temporary Rock Silt Check Type-A
1633.02 Temporary Rock Silt Check Type-B		Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)
Wattle/Coir Fiber Wattle with Polyacrylamide (PAM) 1634.01 Temporary Rock Sediment Dam Type-A. 1635.02 Rock Pipe Inlet Sediment Trap Type-A. 1635.02 Rock Pipe Inlet Sediment Trap Type-B. 1630.06 Special Stilling Basin. Rock Inlet Sediment Trap: 1632.01 Type A. 1632.02 Type B. 1632.03 Type C. Skimmer Basin. Tiered Skimmer Basin. Infiltration Basin. THIS PROJECT CONTAINS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION.	1633.02	Temporary Rock Silt Check Type=B
1634.01 Temporary Rock Sediment Dam Type*A. 1634.02 Temporary Rock Sediment Dam Type*B. 1635.01 Rock Pipe Inlet Sediment Trap Type*A. 1635.02 Rock Pipe Inlet Sediment Trap Type*B. 1630.04 Stilling Basin 1630.05 Special Stilling Basin 1630.06 Special Stilling Basin Rock Inlet Sediment Trap: A 1632.02 Type A. 1632.03 Type Q. Skimmer Basin A Infiltration Basin B Infiltration Basin C Infiltration C Infiltration Basin C Infiltration C Infiltration <td< th=""><th></th><th>Wattle // Coir Fiber Wattle with Polyacrylamide (PAM)</th></td<>		Wattle // Coir Fiber Wattle with Polyacrylamide (PAM)
1635.01 Rock Pipe Inlet Sediment Trap Type A 1635.02 Rock Pipe Inlet Sediment Trap Type A 1630.04 Stilling Basin 1630.05 Special Stilling Basin Rock Inlet Sediment Trap: Rock Inlet Sediment Trap: 1632.01 Type A 1632.02 Type B 1632.03 Type C 1632.03 Type C Skimmer Basin Image: Construct Contains Infiltration Basin Image: Construct Contains FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHIP	1634.01 1634 09	Temporary Rock Sediment Dam Type-A
1635.02 Rock Pipe Inlet Sediment Trap Type-B 1630.04 Stilling Basin 1630.06 Special Stilling Basin Rock Inlet Sediment Trap: 1632.01 Type A 1632.02 Type B 1632.03 Type C Skimmer Basin Image: Comparison of the second	1635.01	Rock Pipe Inlet Sediment Trap Type A
1630.06 Special Stilling Basin 1630.06 Special Stilling Basin Rock Inlet Sediment Trap: 1632.01 Type A 1632.02 Type B 1632.03 Type C Skimmer Basin Image: Construction Basin Tiered Skimmer Basin Image: Construction Basin This PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED	1635.02 1630 04	Rock Pipe Inlet Sediment Trap Type=B
Rock Inlet Sediment Trap: 1632.01 Type A. A 1632.02 Type B. B 1632.03 Type C. C Skimmer Basin Tiered Skimmer Basin Infiltration Basin THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED		Special Stilling Basin
1632.01 Type A A 1632.02 Type B B 1632.03 Type C C Skimmer Basin Image: Comparison of the second sec		Rock Inlet Sediment Trap:
1632.02 Type B. B 1632.03 Type C. C Skimmer Basin Image: Comparison of the second secon	■ 1632.01	
1632.03 Type C	1632.02	Type B
Skimmer Basin. Tiered Skimmer Basin. Infiltration Basin. THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATEPSHED	1632.03	
Tiered Skimmer Basin. Infiltration Basin THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED		Skimmer Basin
Infiltration Basin THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATEPSHED		Tiered Skimmer Basin
THIS PROJECT CONTAINS EROSION CONTROL PLANS FOR CLEARING AND GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED		Infiltration Basin
TO TAYLORSVILLE TO TAYLORSVILLE THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATEPSHED		THIS PROJECT CONTAINS
TO TAYLORSVILLE GRUBBING PHASE OF CONSTRUCTION. THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATEPSHED		EROSION CONTROL PLANS FOR CLEARING AND
THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATEPSHED	TO TAYLO	GRUBBING PHASE OF
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BEEN DESIGNED TO SENSITIVE WATERSHED		THIS PROJECT HAS
		BEEN DESIGNED TO SENSITIVE WATERSHED
STANDARDS.		STANDARDS.
SENSITIVE AREA(S) EXIST	le contraction de la contracti	S EINVIKUINMEINIALLY SENSITIVE AREA(S) EXIST
ON THIS PROJECT Draw Transmission	See See	ON THIS PROJECT
<i>Kefer To E. C. Special Provisions</i> for Special Considerations.		for Special Considerations.

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans.

1604.01	Railroad Erosion Control Detail	1632.01	Rock Inlet Sediment Trap Type A
1605.01	Temporary Silt Fence	1632.02	Rock Inlet Sediment Trap Type B
1606.01	Special Sediment Control Fence	1632.03	Rock Inlet Sediment Trap Type C
1607.01	Gravel Construction Entrance	1633.01	Temporary Rock Silt Check Type A
1622.01	Temporary Berms and Slope Drains	1633.02	Temporary Rock Silt Check Type B
1630.01	Riser Basin	1634.01	Temporary Rock Sediment Dam Type A
1630.02	Silt Basin Type B	1634.02	Temporary Rock Sediment Dam Type B
1630.03	Temporary Silt Ditch	1635.01	Rock Pipe Inlet Sediment Trap Type A
1630.04	Stilling Basin	1635.02	Rock Pipe Inlet Sediment Trap Type B
1630.05	Temporary Diversion	1640.01	Coir Fiber Baffle
1630.06	Special Stilling Basin	1645.01	Temporary Stream Crossing
1631.01	Matting Installation		* *

PROJECT REFERENCE NC	SHEET NO.	
17BP.12.R.88		EC-2
R/W SHEET N	10.	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.

PROJECT REFERENCE NO	PROJECT REFERENCE NO.				
17BP.12.R.88		EC-2A			
R/W SHEET N	10.				
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER			

SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 OR FLATTER

ALL OTHER AREAS WITH SLOPES FLATTER

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

	STABILIZATION TIME	7/
SLOPES	7 DAYS	NONE
	7 DAYS	NONE
	7 DAYS	IF SLOPE Not ste
	14 DAYS	7 DAYS I Length.
R THAN 4:1	14 DAYS	NONE, EX

PROJECT REFERENCE NC).	SHEET NO.
17BP.12.R.88		EC-3
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

IMEFRAME EXCEPTIONS

ES ARE IO'OR LESS IN LENGTH AND ARE EEPER THAN 2:1,14 DAYS ARE ALLOWED. FOR SLOPES GREATER THAN 50' IN

CEPT FOR PERIMETERS AND HQW ZONES.

UTILITY OWNERS ON THIS SHEET

DUKE ENERGY – POWER (DIST) AT&T COMMUNICATIONS – TELECOMMUNICATIONS/FIBER OPTIC

P: (919) 878-9560 900 Ridgefield Drive Suite 350 | Raleigh, North Carolina 27609-3960 NC License No. F-0112

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								1120
								1125
35	40	45	50	55	60	65	70	75

DESIGN DISCHARGE	2,500 C.F.S.
FREQUENCY OF DESIGN FLOOD	25 YR.
DESIGN HIGH WATER ELEVATION	1,139.0
DRAINAGE AREA	12.3 SQ.MI.
BASE DISCHARGE (Q100)	3,400 C.F.S.
BASE HIGH WATER ELEVATION	1,140.3

OVERTOPPING	DISCHARGE		C.F.S.
FREQUENCY OF	OVERTOPPING	FL00D500	YR.+
OVERTOPPING	FLOOD ELEVAT	ION1	,143.4
OVERTOPPING	STATION	18+94	1.50-L-

GENERAL NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET S-21.

FOR SUBMITTAL OR WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20'EACH SIDE OF CENTERLINE AT END BENT NO.1 AND 15'EACH SIDE OF CENTERLINE AT END BENT NO.2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 1 SPAN AT 32.125', 1 SPAN AT 32.5' AND 1 SPAN AT 32.125' WITH A 8" REINFORCED CONCRETE DECK AND A 7" AWS ON 3 LINES OF RC DECK GIRDERS; CLEAR ROADWAY OF 17'; SUBSTRUCTURE CONSISTING OF RC ABUTMENTS FULL HEIGHT AND RC ROUND NOSE POST AND WEB AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. SEE SPECIAL PROVISIONS FOR REMOVAL OF EXISTING STRUCTURE AT STA. 16+26.50 -L-.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE.SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON THE DIFFERENCES BETWEEN THE EXISTING SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER.CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH ``HEC 18- EVALUATING SCOUR AT BRIDGES.''

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, PRESTRESSED CONCRETE END BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.

EXISTING FOUNDATIONS ARE FROM THE BEST INFORMATION AVAILABLE.

EXISTING INTERIOR BENTS SHALL BE CUT OFF AT GROUND LEVEL AND EXISTING RIP RAP TO REMAIN IN PLACE. ADDITIONAL CLASS II RIP RAP MAY BE ADDED TO EXISTING RIP RAP AS DIRECTED BY THE ENGINEER.

EXISTING CONCRETE END BENTS SHALL BE REMOVED BELOW THE PROPOSED 2'CLASS II RIP RAP AS DIRECTED BY THE ENGINEER.

FOR FIBER OPTIC CONDUIT SYSTEM AT STATION 16+26.50 -L-, SEE SPECIAL PROVISIONS.

DRAWN BY :	T.ROBERSON	DATE :	SEP	2018
CHECKED BY	T.L.COGGINS	DATE :	SEP	2018
DESIGN ENGI	NEER OF RECORD : T.L. COGGINS	DATE :	SEP	2018

FOUNDATION NOTES:

FOR PILES SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 66 TONS PER PILE.

DRIVE PILES AT END BENT NO.1 TO REQUIRED DRIVING RESISTANCE OF 110 TONS PER PILE.

PILES AT END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 66 TONS PER PILE.

DRIVE PILES AT END BENT NO.2 TO REQUIRED DRIVING RESISTANCEOF 110 TONS PER PILE.

STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT NO.1. FOR STEEL PILE POINTS, SEE SECTION 450 OF STANDARD SPECIFICATIONS.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

DRILLED PIERS AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 352 TONS PER PIER.CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 5 TSF.

INSTALL DRILLED PIERS AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN 1103 FT WITH A PENETRATION OF AT LEAST 12 FT INTO WEATHERED ROCK OR ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION 1112.8 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

DRILLED PIERS AT BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 354 TONS PER PIER.CHECK FIELD CONDITIONS FOR THE REQUIRED TIP RESISTANCE OF 5 TSF.

INSTALL DRILLED PIERS AT BENT NO.2 TO A TIP ELEVATION NO HIGHER THAN 1094 FT (LT) AND 1097 FT (RT) WITH A PENETRATION OF AT LEAST 12 FT INTO WEATHERED ROCK OR ROCK AS DEFINED BY ARTICLE 411-1 OF THE STANDARD SPECIFICATIONS.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.2 IS ELEVATION 1112.8 FT.SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

SPT TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SPT TESTING.FOR SPT TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

SID INSPECTIONS MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR SID INSPECTIONS. FOR SID INSPECTIONS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

CSL TUBES ARE REQUIRED AND CSL TESTING MAY BE REQUIRED FOR DRILLED PIERS. THE ENGINEER WILL DETERMINE THE NEED FOR CSL TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

PERMANENT STEEL CASINGS MAY BE REQUIRED FOR DRILLED PIERS AT BENT NO.1. IF REQUIRED, DO NOT EXTEND PERMANENT CASING BELOW ELEVATION 1115 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

PERMANENT STEEL CASINGS MAY BE REQUIRED FOR DRILLED PIERS AT BENT NO.2. IF REQUIRED, DO NOT EXTEND PERMANENT CASING BELOW ELEVATION 1115 FT WITHOUT PRIOR APPROVAL FROM THE ENGINEER.

		PROJEC ALEXAI STATIC	CT NO. NDER ON: 16+	17BI -26.5	<u>12.</u> R CO 0 -L-	2 <u>.88</u> UNTY
		SHEET 2 (DF 3			
Suite 700 NC License No. F-0112	BRIDGE NO. 010291	DEPA G FOR E ON O BETW AND SF	STATE RTMENT SENERA BRIDGE LD NC VEEN SF R 1302	OF NORTH CAR OF TRAN RALEIGH OVER HWY 9 1301 (DOVEI	NSPORTA RAWIN DUCK O (SR (BLAIF R CHUR	TION G CREEK 1348) RD) CH RD)
	1/26/2021		REVIS		DATE	SHEET NO.
INT NOT CONSID	ERED FINAL COMPLETED	1 2		3 3 4	UATE:	TOTAL SHEETS 21

				TOTAL	BILL O	F MAT	ERIAL					
	REMOVAL OF EXISTING STRUCTURE AT STA.16+26.50 -L-	ASBESTOS ASSESSMENT	3'-O″DIA DRILLED PIERS IN SOIL	3'-O″DIA DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-O″DIA DRILLED PIER	PDA TESTING	SID INSPECTIONS	SPT TESTING	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE (BRIDGE)	BRIDGE APPROACH SLABS AT STA.16+26.50 -L-
	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	EA.	EA.	EA.	EA.	LUMP SUM	CU.YDS.	LUMP SUM
SUPERSTRUCTURE												LUMP SUM
END BENT NO.1											▲ (20.5)	
BENT NO.1			49.5	36	54						16.8	
BENT NO.2			78.5	36	60						15.6	
END BENT NO.2											▲ (20.5)	
TOTAL	LUMP SUM	LUMP SUM	128.0	72	114	1	2	2	2	LUMP SUM	(73.4)	LUMP SUM
	•		•	•		•			•	•	· · · · · · ·	

	TOTAL BILL OF MATERIAL												
	REINFORCING STEEL (BRIDGE)	SPIRAL COLUMN REINFORCING STEEL (BRIDGE)	PILE DRIVING EQUIPMENT SET- UP FOR HP 12 X 53 STEEL PILES	HP 1 Steei	2 X 53 L PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O") THICK	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	FIBER OPTIC CONDUIT SYSTEM AT STA.16+26.50 -L-	CONC	3'-0"X 1'-9" PRESTRESSED RETE CORE SLABS
	LBS.	LBS.	EACH	NO.	LIN.FT.	EACH	LIN.FT.	TONS	SQ.YDS.	LUMP SUM	LIN.FT.	NO.	LIN.FT.
SUPERSTRUCTURE							240.50				236.50	30	1200.00
END BENT NO.1	▲ (2,522)		5	5	75	5		170	189				
BENT NO.1	10,944	1,926											
BENT NO.2	10,970	2,293											
END BENT NO.2	▲ (2,522)		5	5	140			160	178				
TOTAL	(26,958)	4,219	10	10	215	5	240.50	330	367	LUMP SUM	236.50	30	1200.00

02. tboyd 3/11/20

\sim					
. \ \	DRAWN BY :	DATE :		SEP	201
	CHECKED BY : T.L.COGGINS	DATE :	:	SEP	201
$\frac{3}{2}$	DESIGN ENGINEER OF RECORD : T.L.COGGINS	DATE :	:	SEP	201

		LOAD AN	ND RE	SIST	ANCE	E FAC	CTOR	RAT	ING	(LRF	D) S	UMMA	RY F	ORF	PRES	TRES	SSED	CON	CRETE	EGI	RDEF	RS		
										STRE	ENGTH	I LIN	IIT ST	ATE				SE	RVICE	III	LIMI	r sta	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.06		1.75	0.275	1.38	35′	EL	16.982	0.623	1.2	35′	EL	1.698	0.80	0.275	1.06	35′	EL	16.982	
DESTGN	Γ	HL-93(0pr)	N/A		1.549		1.35	0.275	1.79	35′	EL	16.982	0.623	1.55	35′	EL	1.698	NZA						
LOAD	Γ	HS-20(Inv)	36.000	2	1.377	49.573	1.75	0.275	1.82	35′	EL	13.586	0.623	1.38	35′	EL	1.698	0.80	0.275	1.41	35′	EL	16.982	
RATING	Γ	HS-20(0pr)	36.000		1.785	64.262	1.35	0.275	2.36	35′	EL	13.586	0.623	1.79	35′	EL	1.698	NZA						
		SNSH	13.500		2.424	32.72	1.4	0.275	3.95	35′	EL	16.982	0.623	3.55	35′	EL	1.698	0.80	0.275	2.42	35′	EL	16.982	
		SNGARBS2	20.000		2.082	41.635	1.4	0.275	3.34	35′	EL	13.586	0.623	2.68	35'	EL	1.698	0.80	0.275	2.08	35′	EL	13.586	
	Γ	SNAGRIS2	22.000		2.076	45.668	1.4	0.275	3.31	35′	EL	13.586	0.623	2.56	35′	EL	1.698	0.80	0.275	2.08	35′	EL	13.586	
	F	SNCOTTS3	27.250		1.213	33.066	1.4	0.275	1.98	35′	EL	16.982	0.623	1.79	35′	EL	1.698	0.80	0.275	1.21	35′	EL	16.982	
	S<	SNAGGRS4	34.925		1.123	39.207	1.4	0.275	1.83	35′	EL	16.982	0.623	1.6	35′	EL	1.698	0.80	0.275	1.12	35′	EL	16.982	
	F	SNS5A	35.550		1.09	38.739	1.4	0.275	1.77	35′	EL	16.982	0.623	1.69	35′	EL	1.698	0.80	0.275	1.09	35′	EL	16.982	
		SNS6A	39.950		1.052	42.014	1.4	0.275	1.71	35′	EL	16.982	0.623	1.58	35′	EL	1.698	0.80	0.275	1.05	35′	EL	16.982	
ΙΕΩΔΙ		SNS7B	42.000	3	1.004	42.153	1.4	0.275	1.63	35′	EL	16.982	0.623	1.62	35′	EL	1.698	0.80	0.275	1.00	35′	EL	16.982	
LOAD		TNAGRIT3	33.000		1.299	42.872	1.4	0.275	2.11	35′	EL	16.982	0.623	1.85	35′	EL	1.698	0.80	0.275	1.30	35′	EL	16.982	
RATING		TNT4A	33.075		1.298	42.933	1.4	0.275	2.11	35′	EL	16.982	0.623	1.75	35′	EL	1.698	0.80	0.275	1.30	35′	EL	16.982	
	F	TNT6A	41.600		1.137	47.314	1.4	0.275	1.85	35′	EL	16.982	0.623	1.71	35′	EL	1.698	0.80	0.275	1.14	35′	EL	16.982	
	L S	TNT7A	42.000		1.175	49.358	1.4	0.275	1.92	35′	EL	16.982	0.623	1.59	35′	EL	1.698	0.80	0.275	1.18	35′	EL	16.982	
		TNT7B	42.000		1.156	48.536	1.4	0.275	1.88	35′	EL	16.982	0.623	1.54	35'	EL	1.698	0.80	0.275	1.16	35′	EL	16.982	
	F	TNAGRIT4	43.000		1.17	50.308	1.4	0.275	1.89	35′	EL	13.586	0.623	1.48	35′	EL	1.698	0.80	0.275	1.17	35′	EL	16.982	
	F	TNAGT5A	45.000		1.079	48.572	1.4	0.275	1.76	35′	EL	16.982	0.623	1.56	35'	EL	1.698	0.80	0.275	1.08	35′	EL	16.982	
	F	TNAGT5B	45.000		1.041	46.853	1.4	0.275	1.69	35′	EL	16.982	0.623	1.4	35′	EL	1.698	0.80	0.275	1.04	35′	EL	16.982	

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		F	С

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, ∕ ,	DRAWN BY :M.J.ZIEHL	DATE :	<u>MAY 2018</u>
J0I 12	CHECKED BY : T.L.COGGINS	. DATE :	<u>JUN 2018</u>
ЪÇ	DESIGN ENGINEER OF RECORD : T.L.COGGINS	DATE :	JUN 2018

FR SUMMARY

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	γ_{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

- 1. 2.

(#) CONTROLLING LOAD RATIN
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
<pre>3 LEGAL LOAD RATING **</pre>
* * SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER

								STRENGTH I LIMIT STATE										SERVICE III LIMIT STATE					
				MOMENT SHEAR							MOMENT												
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING Load Rating	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)
		HL-93(Inv)	N⁄A	1	1.205		1.75	0.271	1.59	50′	EL	24.482	0.616	1.2	50′	EL	4.896	0.80	0.271	1.46	50′	EL	24.482
DESIGN		HL-93(0pr)	N⁄A		1.562		1.35	0.271	2.06	50′	EL	24.482	0.616	1.56	50′	EL	4.896	N⁄A					
LOAD	HS-20(Inv)	36.000	2	1.434	51.614	1.75	0.271	1.97	50′	EL	24.482	0.616	1.43	50′	EL	4.896	0.80	0.271	1.81	50′	EL	24.482	
RATING	RATING	HS-20(0pr)	36.000		1.859	66.906	1.35	0.271	2.56	50′	EL	24.482	0.616	1.86	50′	EL	4.896	N⁄A					
		SNSH	13.500	-	3.678	49.655	1.4	0.271	5.02	50′	EL	24.482	0.616	4	50′	EL	4.896	0.80	0.271	3.68	50′	EL	24.482
		SNGARBS2	20.000	-	2.905	58.101	1.4	0.271	3.97	50′	EL	24.482	0.616	2.93	50′	EL	4.896	0.80	0.271	2.91	50′	EL	24.482
		SNAGRIS2	22.000	-	2.748	60.456	1.4	0.271	3.83	50′	EL	19.586	0.616	2.75	50′	EL	4.896	0.80	0.271	2.81	50′	EL	24.482
		SNCOTTS3	27.250		1.835	49.998	1.4	0.271	2.5	50′	EL	24.482	0.616	2.01	50′	EL	4.896	0.80	0.271	1.83	50′	EL	24.482
	S S	SNAGGRS4	34.925		1.595	55.714	1.4	0.271	2.18	50′	EL	24.482	0.616	1.72	50′	EL	4.896	0.80	0.271	1.60	50′	EL	24.482
		SNS5A	35.550		1.556	55.303	1.4	0.271	2.12	50′	EL	24.482	0.616	1.77	50′	EL	4.896	0.80	0.271	1.56	50′	EL	24.482
		SNS6A	39.950		1.455	58.112	1.4	0.271	1.99	50′	EL	24.482	0.616	1.64	50′	EL	4.896	0.80	0.271	1.45	50′	EL	24.482
IFGAL		SNS7B	42.000		1.386	58.224	1.4	0.271	1.89	50′	EL	24.482	0.616	1.65	50′	EL	4.896	0.80	0.271	1.39	50′	EL	24.482
LOAD		TNAGRIT3	33.000		1.782	58.809	1.4	0.271	2.43	50′	EL	24.482	0.616	1.94	50′	EL	4.896	0.80	0.271	1.78	50′	EL	24.482
RAIING		TNT4A	33.075		1.798	59.458	1.4	0.271	2.45	50′	EL	24.482	0.616	1.86	50′	EL	4.896	0.80	0.271	1.80	50′	EL	24.482
		TNT6A	41.600		1.497	62.293	1.4	0.271	2.04	50′	EL	24.482	0.616	1.8	50′	EL	4.896	0.80	0.271	1.50	50′	EL	24.482
	ST	TNT7A	42.000		1.52	63.842	1.4	0.271	2.08	50′	EL	24.482	0.616	1.67	50′	EL	4.896	0.80	0.271	1.52	50′	EL	24.482
		TNT7B	42.000		1.585	66.559	1.4	0.271	2.16	50′	EL	24.482	0.616	1.59	50′	EL	4.896	0.80	0.271	1.58	50′	EL	24.482
		TNAGRIT4	43.000		1.504	64.667	1.4	0.271	2.05	50′	EL	24.482	0.616	1.53	50′	EL	4.896	0.80	0.271	1.50	50′	EL	24.482
		TNAGT5A	45.000		1.405	63.217	1.4	0.271	1.92	50′	EL	24.482	0.616	1.56	50′	EL	4.896	0.80	0.271	1.40	50′	EL	24.482
		TNAGT5B	45.000	3	1.376	61.936	1.4	0.271	1.88	50′	EL	24.482	0.616	1.45	50′	EL	4.896	0.80	0.271	1.38	50′	EL	24.482

bgonfa 3/12/202(

N L			
/	DRAWN BY : _ M.J.ZIEHL	DATE :	JUN 2018
Z Z	CHECKED BY : T.L.COGGINS	. DATE :	JUN 2018
2/	DESIGN ENGINEER OF RECORD : T.L. COGGINS	DATE :	JUN 2018

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LRFR SUMMARY

FOR SPAN 'B'

LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	γ_{DW}
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

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MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS: 2. (#) CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) \int

2 DESIGN LUAD KATING (HS-20)												
(3) LEGAL LOAD RATING $**$												
* * SEE CHART FOR VEHICLE TYPE												
GIRDER LOCATION												
I - INTERIOR GIRDER												
EL - EXTERIOR LEFT GIRDER												
ER - EXTERIOR RIGHT GIRDER												

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR. SPACED AT 4'-O" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4"X 8". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE CORED SLAB UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

MBER × 1'-9″ Ø L.R. RAND 1/2″ ♣		PROJECT NO. 1/BP.12.R.88 ALEXANDER COUNTY STATION: 16+26.50 -L- SHEET 4 OF 4									
8″ ↓ 1/8″ ↓ -9560 rks Road, Forum 1 Suite 700 th Carolina 27615 NC License No. F-0112	BRIDGE NO. 010291	DEPA PRES	RTMENT ST 3'-0 STRES ORED 105	e of north car OF TRAI RALEIGH ANDARI '' X 1' SED (SLAB 5° SK	NSPORTA D 2000 CONCR UNIT EW	TION					
Construction Managers Planners Scientists n	1/26/2021		REVIS	SIONS	0.175	SHEET NO.					
Decople Creative Solutions DOCUMENT NOT CONSID UNLESS ALL SIGNATURES	™. Br: 1 ຄ	DATE:	NU. BY:	DATE:	TOTAL SHEETS						
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COUNTY

STANDARD GUARDRAIL ANCHORAC DETAILS FOR VERTICAL CONCRE BARRIER RAIL	GE ETE
agers Planners Scientists 1/26/2021 REVISIONS SH	HEET NO.
NO. BY: DATE: NO. BY: DATE:	S-10
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NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

		PR AL ST	OJEC <u>EXAI</u> ATI(CT NO. NDER ON: 16	+2	17BF 26.5	P.12.R CO 0 -L-	2 <u>.88</u> UNTY
	BRIDGE NO.010291		DEPA	STA RTMENT	te of Of	NORTH CARG F TRAN RALEIGH	NSPORTA	TION
	CAROLUNE Denosigner by: SESSION: TWINGOUY COMPILAS 17BCB235E910 14045			sue END	ss⁻ Bl	truc ⁻ ENT	iure No. 1	1
Suite 700 NC License No. F-0112 gers Planners Scientists	1/26/2021			REVI	SION	١S		SHEET NO.
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Responsive People | Creative S

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2 XI X	2 ¹ / ₁₆ "		FOF	<u> </u>		ND RF	 `NT			
HK.		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
1′-3″	$\tilde{m}/1/2$	B1	8	#9	1	39'-4"	1070			
		B2 B3	28	#4 #⊿	STR	<u> 19'-9" </u>	369			
	H1 8′-5″		10	T	311	2 3	10			
	H2 8'-7"	D1	20	#6	STR	1'-6"	45			
	_~°	H1	10	#4 #1	2	9'-1"	61			
		H3	10	#4	3	9'-6"	63			
H4		H4	10	#4	3	9'-4"	62			
, ►	^{"2} /L	К1	16	#4	STR	3'-1"	33			
	, m m m m m m m m m m m m m m m m m m m	S1	48	#4	4	10'-5"	334			
		S2	48	#4	5	3'-2"	102			
AP		S3	20	#4	6	6'-6"	87			
	2'-5"	V1	53	#4	STR	6'-2"	218			
		REINFORCING STEEL (FOR ONE END BENT) 2522 LBS.								
		CLASS	A CC	NCRET	e brea	KDOWN				
		(FOR ONE END BENT) POUR #1 CAP, LOWER PART 18.4 C.Y.								
			#0 11							
MENSIC	NS ARE OUT TO OUT.	POUR	#2 U W	PPER P INGS	ARI U	F	2.I U.Y.			
	END BENT NO.2									
	HP 12 X 53 STEEL PILES									
.= 75	NO:5 LIN.FT.= 140	TOTAL	. CLAS	SS A C	ONCRE	ΓE	20.5 C.Y.			
NO: 5	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES NO: 5									
NO: 5		,								
	A REPL CORF BY: 1 CK'D	ACED (RECT B IKB 03 BY: TL	ENTIR OM AN /09/2 .C 03/	E SHEE ID SEC 021 (09/20	T 4 OF TION 4 21	F 4 TO S⊦ A-A.	IOW			
	PRO <u>ALE</u> STA	JEC XAN	t no <u>DER</u> n:_10	0. <u>1</u> 6+2	.7BF	<u>-12.R</u> CO L-	<u>.88</u> UNTY -			
	SHEE	T 4 OF	4							
	BRIDGE NO. 010291	DEPAR		IT OF	ORTH CARO TRAN LEIGH	ISPORTA TURE	TION			

Suite 700 NC License No. F-0112	Docusig SE AL Docusig SE AL 14045 Timothy L CARDINS PROFILE TOMOTHY L CONTINUES	END BENT DETAILS AND BOM									
	3/11/2021			REVIS	SIO	NS		SHEET NO.			
lutions		N0.	BY:	DATE:	NO.	BY:	DATE:	S-14			
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ALL SIGNATURES	2			4			21				

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			BT		OF MA	TERTAI					
			F	OR		BENT	-				
	t	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT				
		B1 B2	10	#11 #5	1 STR	37'-2"	1,975 214				
		D1	40	#6	STR	1'-6"	90				
<u>1′-7″</u> <u>10′-9″</u>		M1	30	#11	STR	38'-4"	6,110				
11/2 EXTRA TURNIS		S1	58	#5	2	9'-0"	544				
INTO CAP		U1	6	#4	3	5'-8"	23				
		U2	5	#11	3	5'-6" 12'-4"	1.96.6				
(9) [3" PI]			FORCIN	NG ST	eel	10	944 LBS				
	ŀ		3	<u> </u>		<u>/57'-/"</u>	1 / 31				
		SP-2	3	**	6	247'-3"	495				
4 SPACERS		SPIRA (FOR (L COL DNE BE	UMN F Ent)	REINFOR	CING STEEL 1,	- 926 LBS.				
		* TH SH WI	E SP-1 All BI RE OR	. SPIF E W31 #5 P	CAL REIN OR D-31 LAIN OF	NFORCING S COLD DRA DEFORMED	STEEL WN) BAR				
<u>2'-2"Ø</u>		** TH SH WI	all Bi Re Or	2 SPI E W20 #4 P	OR D-2 LAIN OF	O COLD DR O COLD DR DEFORME	AWN D BAR				
			CLAS	SA((F(CONCRETE DR ONE E	E BREAKDO' BENT)	WN				
	-	POUR POUR	#2 (C(#3 (C/	DLUMN AP)	S)		4.7 C.Y. 12.1 C.Y.				
		TOTAL CLASS A CONCRETE 16.8 C.Y.									
				DRI (FOF	LLED PI R ONE BI	ERS: ENT)					
	DRILLED PIER CONCRETE POUR #1 (DRILLED PIERS) 22.4 C.Y.										
#6 D1 DOWELS		3'-O"Ø DRILLED PIER NOT IN SOIL 36 i tn.ft.									
		3′-0″¢	Ø DRIL	LED F	PIER IN	SOIL 49.5	LIN.FT.				
້ ດີ	PERMANENT STEEL CASING FOR										
2"CL. TYP.) o		CSL T	UBES			360	LIN.FT.				
× 0 - , ℃											
َ ص											
/¥¥ 3″HIGH B.B.											
	PR			10. _	<u>178</u>	P.12.R	.88				
	<u>аг</u> ст	<u>Δ</u> ΤΤ		16+	26.5	CO 0 -L-	UNIY -				
	SHF	FT 2	OF 2								
		-		STATE	OF NORTH CAR		T T O M				
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Doceletradoy				SUBS	STRUC	TURE					
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1 Suite 700											
hagers Planners Scientists 1/26/2021				REVISI	ONS		SHEET NO.				
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ENT NOT CONSIDERED FINAL ALL SIGNATURES COMPLETED	12				» }		SHEETS 21				

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	BILL OF MATERIAL									
				FO	R BE	ENT				
		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT			
HK. (4		B1	10	#11	1	37'-2"	1,975			
		82	Ь	#5	SIK	541-2"	214			
<u>1'-1"</u> 50'-7" M1	-	D1	40	#6	STR	1'-6″	90			
49′-7″ M2	-	<u>М</u> 1	10	#11	STR	52'-2"	2 772			
47′-7″ M3	-	M2	10	#11	STR	51'-2"	2,718			
$1^{1}/_{2}$ EXTRA TURNS	,	M3	10	#11	STR	49'-2"	2,612			
INTO CAP		S1	58	#5	2	9′-0″	544			
) I CH		U1 112	6	#4 #4	3 3	5′-8″ 5′-6″	23			
							<u> </u>			
		_			- 					
		REIN (FOR	⊢ORCII BENT)	NG STE	.EL	10.	970 LBS.			
P-4										
4 SPACERS		SP-1	1	*	5	633'-1"	660			
		SP-2	1	* *	ວ 5	<u>585'-1"</u>	610			
· • •		SP-4	3	**	6	189'-1"	379			
		SPIR/ (FOR	AL COL BENT)	UMN RI	EINFORG	CING STEEL 2.	_ 293 LBS_			
Z'-2″∅ T		* TF ST	IE SP-1 EEL SI	I,SP-2, HALL B	, AND SF E_W31 C	P-3 SPIRAL	_ REINF. _D_DRAWN			
		W]	LRE OR	#5 PL	AIN OR) BAR			
		₩ TF SF	IL SP-4	4 SPIR E_W2O	AL REI	NFURCING O COLD DR	SIEEL AWN			
		W	lre or	#4 PL	AIN OR	DEFORME) BAR			
			CLAS	SAC	ONCRETE For RFM	E BREAKDON NT)	WN			
		POUR	#2 (C() LUMNS		· · · ·	3.5 C.Y.			
		POUR	#3 (CA	AP)			12.1 C.Y.			
		ΤΟΤΑΙ	_ CLAS	S A C	ONCRETE		15.6 C.Y.			
					IFN PT	FRS.	•			
#6 D1 DOWELS				(F	OR BEN	T)				
			ED PI	ER CON TAI	NCRETE		29 8 C Y			
				PIER #	*1 = 10.3	3 C.Y.				
o l		DR]	illed	FIER #	*3 = 9.6					
2" (1		3'-0"	Ø DRII	_LED P	IER NO	T IN SOIL				
TYP.) or						36.0	LIN.FT.			
		3'-0"	Ø DRII	_LED P	IER IN	SOIL				
້ອ						(Q°Q)				
<u>+</u>		PERM/ 3'-0"	ANENT Ø DRTI	STEEL	CASINC TFR	G FOR	I TN FT			
ູ້			- UNT			л <i>т</i> с				
		UJL I	JULJ			Ø I F	∟ ⊥ No o			
/3″HIGH B.B.										
			от ·		17R	P 1 2 D	22			
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NOTES : FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.

ESTIMATED QUANTITIES RIP RAP CLASS II (2'-0" THICK) GEOTEXTILE FOR DRAINAGE SQUARE YARDS TONS 170 189

160

178

PROJECT NO. 178P.12.R.88

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ALEXANDER

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APPROACH-SLAB 2'-6" MIN. END OF APPROACH SLAB ——— NOTE: IMMEDIATEL THE CONTRA DRAIN. CON AND PROVID EROSION RE PLANT MIX, MAT, OR 3) (THE SLOPE TEMPORARY

SPLICE LENGTHS							
BAR SIZE	EPOXY COATED	UNCOATED					
#4	2'-0"	1'-9″					
#5	2'-6"	2'-2"					
#6	3'-10"	2'-7"					

NOTES BILL OF MATER				TERIA	L		
BRIDGE APPROACH FILL INCLUDING GEOTEXTLE 4" CORATNAC	F PTPF	A	PR	DACH	SLAE	<u>Β ΑΤ Ε</u>	B #1
SELECT MATERIAL BACKFILL, SEE ROADWAY PLANS.		BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
EXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD	ŀ	* A1 A2	13 13	#4 #4	STR STR	29'-10"	259
CT MATERIAL BACKFILL (CLASS V OR CLASS VT) SHALL BE TN	Ē		-				
DRDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.	┠	* B1 R2	58 58	#5 #6	STR STR	<u>11'-1"</u> 11'-7"	670 1009
CT MATERIAL BACKFILL IS TO BE CONTINUOUS ALONG FILL FA	ACE OF	52	50			<u> </u>	1 1003
THE 4"Ø DRAINAGE PIPE OUTLETS. SEE ROADWAY STANDARD DR.	AWINGS.		ORCIN	IG STEE	Ľ	LBS.	1268
A BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADE	ED TO	⊼ ∟ΡΟ REI	NFORC	ING ST	EEL	LBS.	929
IN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND AVED. SEE ROADWAY PLANS.	SHALL _	<u> </u>			. Ը		16.7
ROACH SLAB GROOVING IS NOT REQUIRED.	F				SI AF	R ΔT F	B #2
7.4	F	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	F	* A1	13	#4	STR	29'-10"	259
	-	A2	13	#4	SIR	29'-10"	259
GRADETON	_	* B1	58	#5	STR	11'-1"	670
	F	B2	58	#6	STR	11'-7"	1009
FLOW LINE	-	REINF	ORCIN	IG STEE	Ľ	LBS.	1268
CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL		₩ EPO RET	XY CO NFORC	ATED ING ST	EEL	LBS-	929
BACKFILL EXCAVATION HOLE	E	+	0				
ANU GRADE IU URAIN	L	CLASS	ΑΑ Ο	CONCRET	E	С.Ү.	16.7
RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING IRECTED BY THE ENGINEER TO PREVENT SOIL EROSION PROTECT THE AREA ADJACENT TO THE STRUCTURE. RACTOR WILL BE REQUIRED TO REMOVE THESE S PRIOR TO CONSTRUCTION OF THE APPROACH SLAB. PORARY DRAINAGE DETAIL Rem							
ASS ``B" STONE R EROSION CONTROL TEMP. SLOPE DRAIN 2'-O'MIN. 1'-O''		BOW	EMPOF LOPE RAIN	RARY			
S MIN. FUTURE SHOULDER		X	Ľ				
	TOE OF FIL	_L		8000	003 W		
	CLAS FOR	S ``B″ EROSI	STONE	DNTROL			
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TO DREET VIE 4	Û I	– 3″ ER Mate	OSION	N RESIS	STANT PTPF		
S ← B ← 12″ MI	N	1007 (12		— EART	H DITC	H BLOCK	
FLOW LINE		_/	7 /	`	12		
<u>1'-6" MIN.</u>		//		$\overline{)}$	I		
TELY AFTER THE CONSTRUCTION OF THE APPROACH SLAB,					\		
CONTRACTOR SHALL PROVIDE TEMPORARY BERM AND SLOPE					\bigwedge		
RESISTANT MATERIAL AS SHOWN. THE RESISTANT MATERIAL SHALL BE EITHER 1) ASPHALT	4	1′-0″ M	IN.		FILLS	SLOPE	
3) CONCRETE, AS DIRECTED BY THE ENGINEER.							
RY DRAIN SHALL CUNSIST OF A NON-PERFORATED RY DRAINAGE PIPE, 12 INCHES IN DIAMETER.	SI	ECTI	EON	S-S			
PLAN VIEW							
TEMPORARY RERM AND SLOPE		JD	FΤ	ΔΤΙ «	5		
TO DE LICED WHEN CHOILDED DEDUC OUTTED				<u>¬ ⊥ ∟ ,</u>	<u></u>	-	
UID BE USED WHEN SHOULDER BERM GUITER	IS KEQUIKE	יט					
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UNLESS ALL SIGNATURES COMPLETED	2			4 4			SHEETS 21

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS. CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ "with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

DRAWN BY :	DATE :	SEP 2018
CHECKED BY : T.L.COGGINS	DATE :	SEP 2018
DESIGN ENGINEER OF RECORD : T.L.COGGINS	DATE :	SEP 2018

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 1/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 1/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY. AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{6}$ in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:

PROJECT NO	17BP.12.R.88
ALEXANDER	COUNTY
STATION: 16+	26.50 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD

NOTES

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