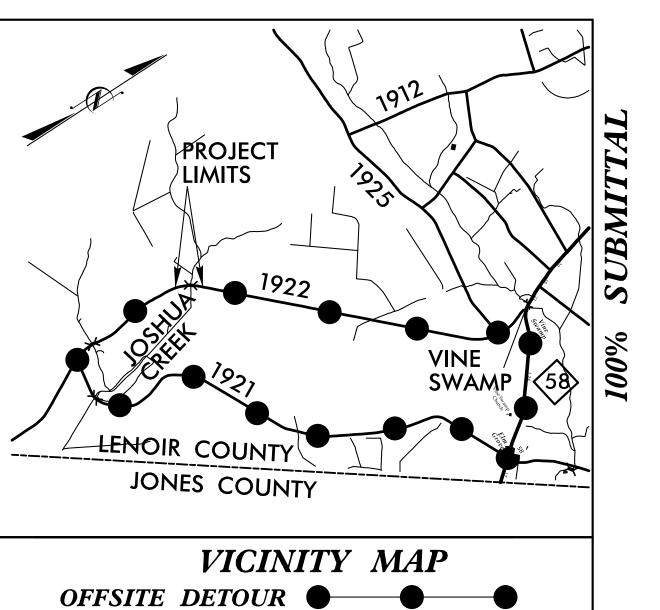
80 BP

See Sheet 1A For Index of Sheets

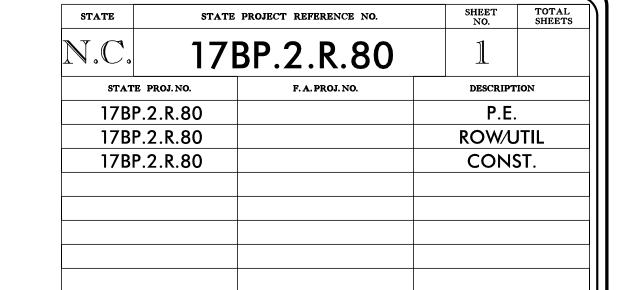


STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

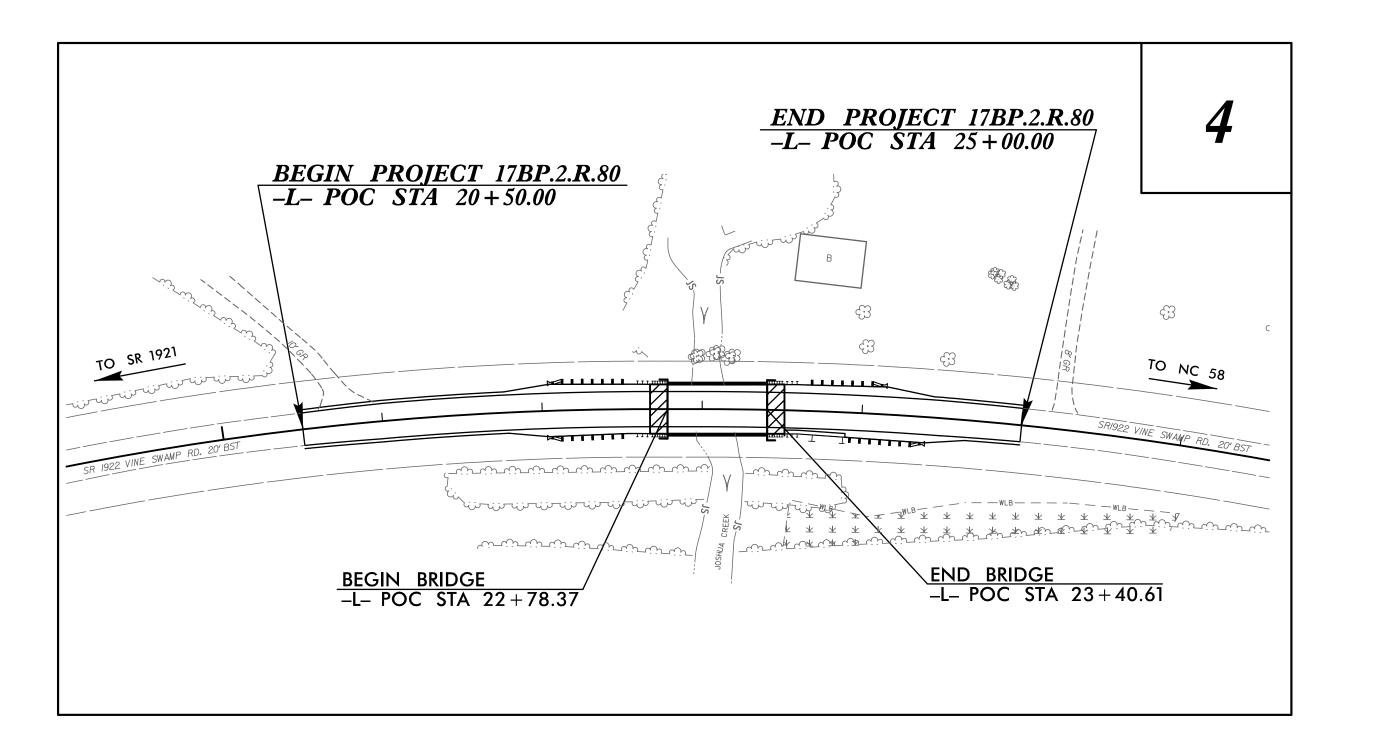
LENOIR COUNTY

LOCATION: REPLACE BRIDGE NO. 143 OVER JOSHUA CREEK ON SR 1922 (VINE SWAMP ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURE







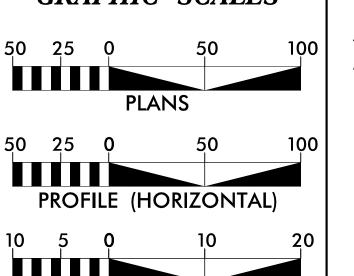
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

90

4

8

GRAPHIC SCALES



PROFILE (VERTICAL)

DESIGN DATA

ADT 2012 = 480ADT 2032 = 960

K = 10 %

V = 60 MPH* TTST = 2% DUAL 4% FUNC CLASS =

MINOR COLLECTOR

SUBREGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY PROJECT 17BP.2.R.80 LENGTH OF STRUCTURE PROJECT 17BP.2.R.80

TOTAL LENGTH OF PROJECT 17BP.2.R.80

= 0.073 MILES

= 0.012 MILES

= 0.085 MILES

Prepared in the Office of: HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Suite 200 Raleigh, North Carolina 27609 NC License No: C-1554

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: OCTOBER 11, 2017

> LETTING DATE: APRIL 11, 2018

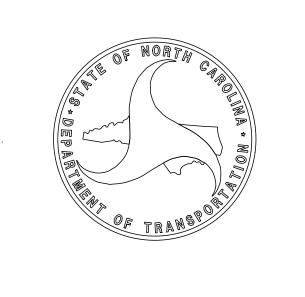
DOUGLAS M. WHEATLEY, PE PROJECT ENGINEER MONICA J. DUVAL PROJECT DESIGN ENGINEER

NCDOT CONTACT

HEATHER C. LANE, PE

15764 James A. Byrd **SIGNATURE:** ROADWAY DESIGN **ENGINEER** 36786 Douglas M. Wheatley SIGNATURE:

HYDRAULICS ENGINEER.



INDEX OF SHEETS

SHEET NUMBER

1 TITLE SHEET

1A INDEX OF SHEETS, GENERAL NOTES & LIST OF STANDARDS

1B SYMBOLOGY SHEET

1C-1 SURVEY CONTROL SHEET

2A-1 TYPICAL SECTION SHEET

3B–1 EARTHWORK, PAVEMENT REMOVAL, GUARDRAIL SUMMARY, SHOULDER BERM GUTTER, ROW SUMMARY, & DRAINAGE SUMMARY SHEET

PLAN & PROFILE SHEET

TMP-1 THRU TMP-2

TRAFFIC CONTROL PLANS

EC-1 THRU EC-4

EROSION CONTROL PLANS

REFORESTATION PLANS

U0-1 THRU U0-2

UTILITIES BY OTHER PLANS

X-1 THRU X-3

CROSS SECTION SHEETS

S-1 THRU S-14

STRUCTURE PLANS

GENERAL NOTES: 2018 SPECIFICATIONS

EFFECTIVE: 01–16–2018

REVISED:

GRADE LINE: GRADING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED OR FUTRUE SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

STRUCTURE SUBSURFACE PLANS ARE AVAILABLE ON THIS PROJECT.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROCHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

POWER – TRI–COUNTY EMC

WATER – DEEP RUN WATER CORPORATION

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS, EXCEPT AS SHOWN ON THE PLANS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

EFF. 01-16-2018

2018 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch — N. C. Department of Transportation — Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE
DIVISION 2 – EARTHWORK

0.02 Method of Clearing – Method II

5.02 Guide for Grading Subgrade — Secondary and Local

Method of Obtaining Superelevation – Two Lane Pavement

DIVISION 3 – PIPE CULVERTS

300.01 Method of Pipe Installation

310.10 Driveway Pipe Construction

DIVISION 4 - MAJOR STRUCTURES

422.02 Bridge Approach Fills – Type II Modified Approach Fill

DIVISION 5 – SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction – High Side of Superelevated Curve – Method I

DIVISION 8 - INCIDENTALS

840.00 Concrete Base Pad for Drainage Structures 840.29 Frames and Narrow Slot Flat Grates

840.29 Frames and Narrow Slot Flat Ğrates
840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates

840.66 Drainage Structure steps

846.01 Concrete Curb, Gutter and Curb & Gutter

862.01 Guardrail Placement 862.02 Guardrail Installation

862.03 Structure Anchor Units 876.01 Rip Rap in Channels

876.02 Guide for Rip Rap at Pipe Outlets

ROADWAY DESIGN
ENGINEER

WAR CARO

SEAL
36786

Dogu Signed by GINEE

272AB/2018^{8124DA}

SHEET NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO.

\Koadway\Proj\1/BP.Z.K.8W_KUY_UEN.dgn 1NTB STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

CSX TRANSPORTATION

⊙ MILEPOST 35

CR

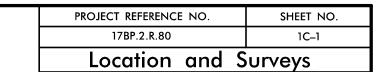
*S.U.E. = Subsurface Utility Engineering

PROJECT REFERENCE NO.	SHE
17BP.2.R.80	,

		CONVENTION	ΔI
BOUNDARIES AND PROPERT	<i>Y:</i>	Note: Not to	
State Line —		RAILRUADS:	
County Line		Standard Gauge	CSX TRANSPORT
Township Line		RR Signal Milepost	MILEPOST 3.
City Line		Switch —	SWITCH
Reservation Line		RR Abandoned	
Property Line		RR Dismantled	
Existing Iron Pin	<u>.</u>		
Computed Property Corner	×	RIGHT OF WAY & PROJECT CO	ONTROL:
Property Monument		Secondary Horiz and Vert Control Point ——	
Parcel/Sequence Number		Primary Horiz Control Point	
Existing Fence Line		Primary Horiz and Vert Control Point	
Proposed Woven Wire Fence		Exist Permanent Easment Pin and Cap	\Diamond
Proposed Chain Link Fence		New Permanent Easement Pin and Cap ——	
Proposed Barbed Wire Fence		Vertical Benchmark	
Existing Wetland Boundary		Existing Right of Way Marker	\triangle
Proposed Wetland Boundary		Existing Right of Way Line	
Existing Endangered Animal Boundary —		New Right of Way Line	$\frac{R}{W}$
Existing Endangered Plant Boundary		New Right of Way Line with Pin and Cap—	$\frac{R}{W}$
	———— HPB ———————————————————————————————	New Right of Way Line with	•
Known Contamination Area: Soil		Concrete or Granite R/W Marker	
Potential Contamination Area: Soil		New Control of Access Line with Concrete C/A Marker	
Known Contamination Area: Water		Existing Control of Access ——————————————————————————————————	(C)
Potential Contamination Area: Water			
Contaminated Site: Known or Potential —			Δ
BUILDINGS AND OTHER CUI	LTURE:		_
Gas Pump Vent or U/G Tank Cap	O	'	———E—
Sign —	<u>©</u>		TDE -
Well —		New Permanent Drainage Easement ——	PDE -
Small Mine	——	New Permanent Drainage / Utility Easement	——DUE-
Foundation —		New Permanent Utility Easement New Target Utility Easement	
Area Outline		New Temporary Utility Easement	
Cemetery		New Aerial Utility Easement —————	AUE-
Building —	<u> </u>	ROADS AND RELATED FEATUR	FS.
School —		Existing Edge of Pavement	
Church —		Existing Curb	
Dam —		Proposed Slope Stakes Cut	
HYDROLOGY:		Proposed Slope Stakes Fill —————	
Stream or Body of Water —————		Proposed Curb Ramp	
Hydro, Pool or Reservoir —	— [Existing Metal Guardrail	
Jurisdictional Stream		Proposed Guardrail	
Buffer Zone 1		Existing Cable Guiderail	
Buffer Zone 2	BZ 2	Proposed Cable Guiderail	
Flow Arrow		Equality Symbol	•
Disappearing Stream ————————————————————————————————————	<u> </u>	Pavement Removal	
Spring —		VEGETATION:	
Wetland ————————————————————————————————————	<u> </u>	Single Tree	- - 쓼
Proposed Lateral, Tail, Head Ditch ———	₹ FLOW	Single Tree Single Shrub	
False Sump —	$ \Leftrightarrow$	Single Sinob	رئ

Hedge ————	
Woods Line	(;)(;)(;)(;)-
Orchard —	සි සි සි සි
Vineyard ————————————————————————————————————	Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert ————	CONC
Bridge Wing Wall, Head Wall and End Wall –) CONC WW (
MINOR:	
Head and End Wall	
Pipe Culvert —————	
Footbridge	
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole ————	(\$)
Storm Sewer —	s
UTILITIES:	
POWER:	
Existing Power Pole ————	•
Proposed Power Pole ———	4
Existing Joint Use Pole ————	
Proposed Joint Use Pole	-6-
Power Manhole ————	P
Power Line Tower ————	
Power Transformer ———————————————————————————————————	\square
U/G Power Cable Hand Hole	
H_Frame Pole	•—•
U/G Power Line LOS B (S.U.E.*)	P
U/G Power Line LOS C (S.U.E.*)	
U/G Power Line LOS D (S.U.E.*)	
TELEPHONE:	
Existing Telephone Pole	-
Proposed Telephone Pole	-0-
Telephone Manhole	
Telephone Pedestal —————	
Telephone Cell Tower —	<u></u>
U/G Telephone Cable Hand Hole —	HH
U/G Telephone Cable LOS B (S.U.E.*)	
U/G Telephone Cable LOS C (S.U.E.*)	
U/G Telephone Cable LOS D (S.U.E.*)	
U/G Telephone Conduit LOS B (S.U.E.*)	
U/G Telephone Conduit LOS C (S.U.E.*)	
U/G Telephone Conduit LOS D (S.U.E.*)	
U/G Fiber Optics Cable LOS B (S.U.E.*)	
U/G Fiber Optics Cable LOS C (S.U.E.*)	

WATER:	
Water Manhole	- W
Water Meter —	
Water Weler Water Valve	
Water Hydrant	
U/G Water Line LOS B (S.U.E*)	
U/G Water Line LOS B (S.U.E*)	
U/G Water Line LOS C (S.U.E*)	
Above Ground Water Line	
Above Ground Water Line	
TV: TV Pedestal	
TV Tower	
	_
U/G TV Cable Hand Hole	
U/G TV Cable LOS B (S.U.E.*)	
U/G TV Cable LOS C (S.U.E.*)	
U/G TV Cable LOS D (S.U.E.*)	
U/G Fiber Optic Cable LOS B (S.U.E.*)	
U/G Fiber Optic Cable LOS C (S.U.E.*)	
U/G Fiber Optic Cable LOS D (S.U.E.*)	TV F0
GAS:	
Gas Valve	- \Q
Gas Meter —	- ♦
U/G Gas Line LOS B (S.U.E.*)	
U/G Gas Line LOS C (S.U.E.*)	
U/G Gas Line LOS D (S.U.E.*)	G
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	-
Sanitary Sewer Cleanout	
U/G Sanitary Sewer Line —	
Above Ground Sanitary Sewer —	
SS Forced Main Line LOS B (S.U.E.*) ———	
SS Forced Main Line LOS C (S.U.E.*)	
SS Forced Main Line LOS D (S.U.E.*)	
MISCELLANEOUS:	
Utility Pole ————————————————————————————————————	- •
Utility Pole with Base ————————————————————————————————————	
Utility Located Object ————————————————————————————————————	- <u></u>
Utility Traffic Signal Box ———————————————————————————————————	- 5
Utility Unknown U/G Line LOS B (S.U.E.*)	?UTL
U/G Tank; Water, Gas, Oil ———————————————————————————————————	-
Underground Storage Tank, Approx. Loc. —	UST
A/G Tank; Water, Gas, Oil ———————————————————————————————————	-
Geoenvironmental Boring	-
U/G Test Hole LOS A (S.U.E.*)	- ©
Abandoned According to Utility Records —	
End of Information —	



PROJECT

SURVEY CONTROL SHEET

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

BASELINE

BL							
	POINT	DESC.	NORTH	EAST	ELEVATION	L STATION	OFFSET
1		BL - 1	503272.6510	2423276.4270	82.07	17+38.34	13.28 L
2		BL-2	503821.8860	2423542.0100	74.88	23+46.31	17.10 L
3		NCDOT GPS 53Ø14	504180.8260	2423852.0870	72.91	28+18.54	16.40 L

BENCHMARK END STATE PROJECT 17BP.2.R.80 -L- STA 25 + 00.00ELEVATION = 72.94 LOCALIZED PROJECT COORDINATES BM-1N 503853 E 2423442 N: 503935.350 L STATION 23+18.00 117 LEFT E: 2423647.987 R/R SPIKE SET IN 24" HARDWOOD BEGIN STATE PROJECT 17BP.2.R.80 -L- STA 20+50.00 LOCALIZED PROJECT COORDINATES N: 503557.932 **BL**-2 E: 2423404.443 NCDOT GPS

ROW MARKER PERMANENT EASEMENT-E

LALIGN	STATION	OFFSET	NORTH	EAST
L	21+00.00	-30.00	503616.4721	2423400.8628
L	21+00.00	-40.00	503621.1707	2423392.0354
L	21+50.00	-40.00	503665.9234	2423416.6268
L	23+40.00	-65.00	503843.6499	2423498.8553
L	23+75.00	40.00	503812.5883	24236Ø5.1557
L	23+75.00	30.00	503818.3722	2423596.9981
L	23+95.00	30.00	503834.3831	24236Ø8.4718
L	23+95.00	40.00	503828.5173	2423616.5707
L	24+00.00	-65.00	503894.2867	2423534.5666
L	24+50.00	-55.00	503929.5558	2423573.4174
L	25+00.00	-40.00	503960.4940	2423616.8772
L	25+00.00	-30.00	503954.2081	2423624.6546

DESIGN ALIGNMENT

		L	
TYP	E STATION	NORTH	EAST
POT	10+00.00	502539.4070	2423187.2810
PC	13+28.88	502867.2812	2423213.0047
PT	22+68.17	503747.0939	2423512.9045
PC	22+99.55	503773.3599	2423530.0658
PT	26+80.79	504070.5877	2423767.87Ø5
POT	40+43.81	505048.3250	2424717.5410

DATUM DESCRIPTION

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "BL-3"

WITH NAD 83/NA 2011 STATE PLANE GRID COORDINATES OF NORTHING: 504180.826(ft) EASTING: 2423852.087(ft) ELEVATION: 72.91(ft)
THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT

(GROUND TO GRID) IS: 0.999877160

THE N.C. LAMBERT GRID BEARING AND
LOCALIZED HORIZONTAL GROUND DISTANCE FROM
"BL-3" TO -L- STATION 20+50.00 IS

S 35°42′10.8″ W 767.06′

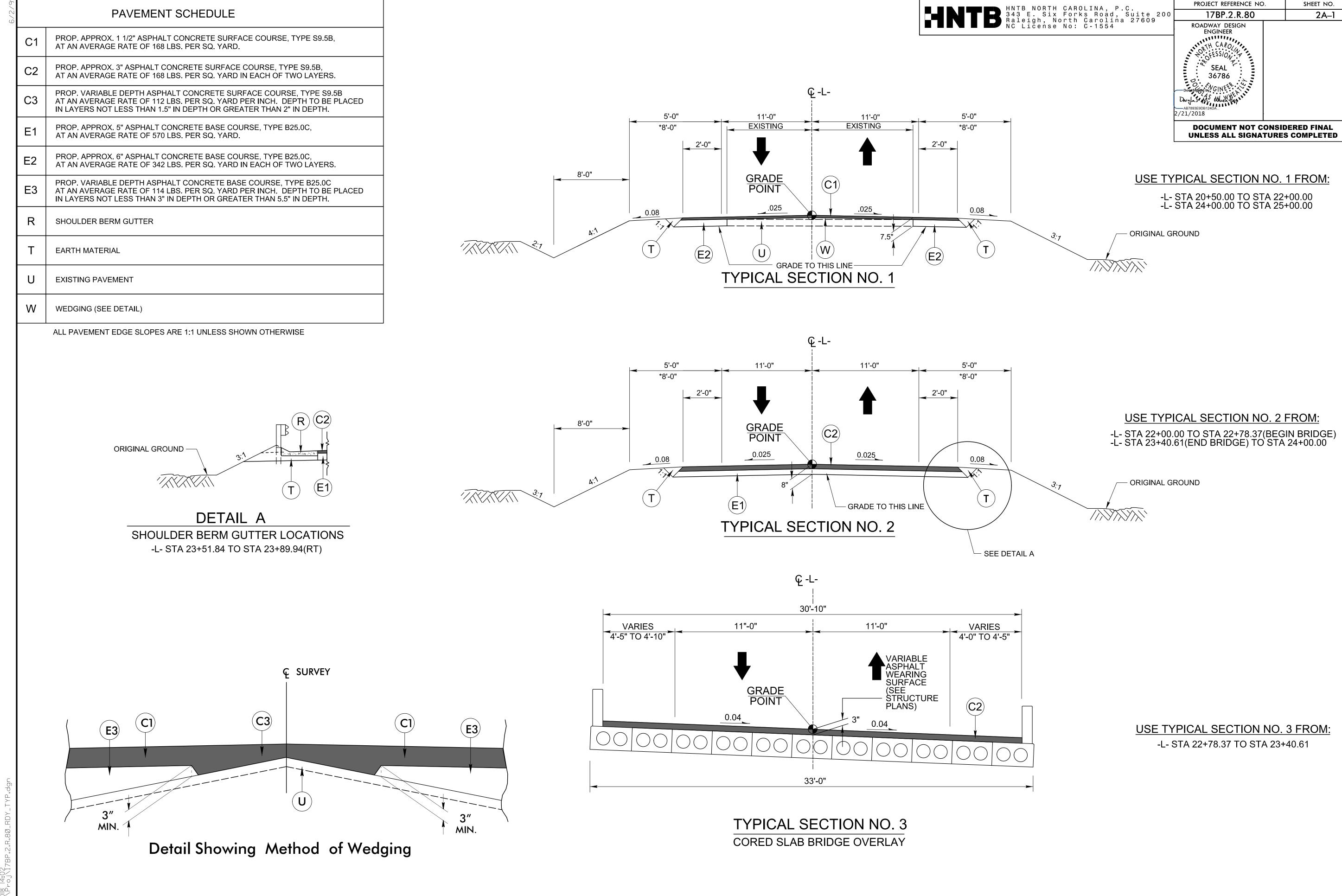
ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES

VERTICAL DATUM USED IS NAVD 88

NOTES:

I. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

2. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.



20-FEB-2018 14:02 | Roadwail\Pro_i\17RP.2.R.8M_RI

NOTES: * SHOULDER WIDTH INCREASED 3' WITH THE USE OF GUARDRAIL

SUMMARY OF EARTHWORK

STATION	STATION	UNCL. EXCAV.	EMBANK. +25%	BORROW	WASTE
-L- STA 20+50.00	-L- STA 22+78.37(BRIDGE)	49	248	199	
–L– STA 23+40.61(BRIDGE)	_L_ STA 25+00.00	262	251		11
TOTALS:		311	499	199	11
WASTE IN LII	EU OF BORROW			-11	-11
PROJEC	CT TOTALS:	311	499	188	
5% TO REPLACE TOP	SOIL ON BORROW PIT			9	
GRANI	D TOTALS:	311	499	197	
SAY:		325		225	

Note: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading."

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.
G = GATING IMPACT ATTENUATOR TYPE 350

PAVEMENT REMOVAL SUMMARY

SURVEY LINE	STATION	STATION	LOCATION LT/RT/CL	YD ²
-L-	22+00.00	22 + 84.49	CL	220
-L-	23+30.27	24+00.00	CL	191
			TOTAL:	411
			SAY:	420

ROW AREA DATA SUMMARY

PARCEL NO.	PROPERTY OWNERS NAMES	PROP. R/W	PERM. UTILTIY EASE.	PERM. DRAIN. EASE.	PERM. DRAINAGE UTILITY EASE.	CONST. EASE.
	CARAFNIA ZIMAMERAMANI			21/5 / 4 6 5		240.24.6.5
1	CARMENA ZIMMERMAN			3165.64 S.F.		248.24 S.F.
2	WILLIAM H. SCOTT, JR. ET UX					391.30 S.F.
3	WALTER L. MOORE			4937.56 S.F.		
4	JERRY R. TAYLOR			196.80 S.F		663.65 S.F.

SHOULDER BERM GUTTER SUMMARY

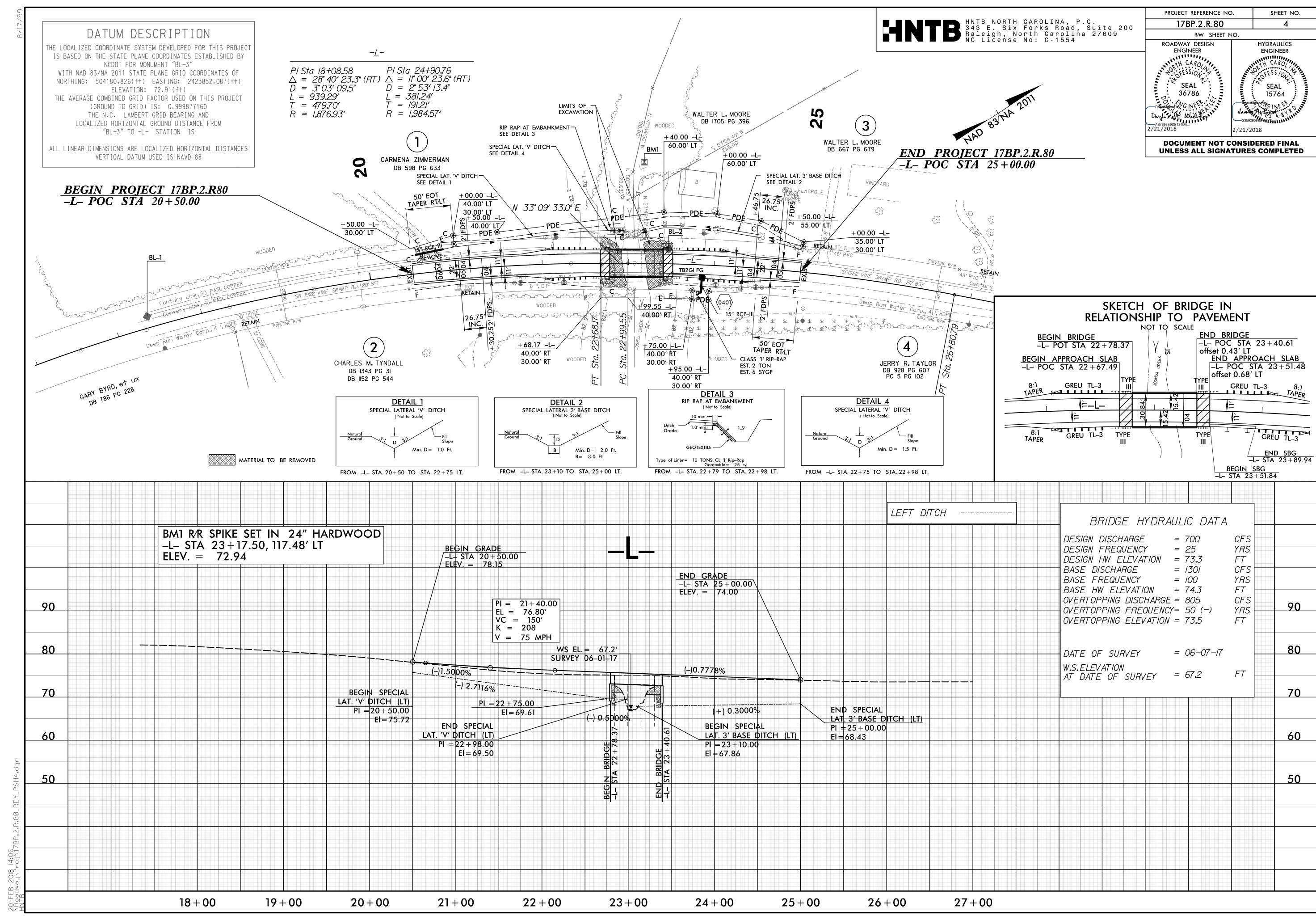
SURVEY LINE	STATION	STATION	LENGTH (FT)
-L-	23 + 51.84(RT)	23 + 89.94(RT)	38.10′
		TOTAL:	38.10′
		SAY:	40′

GUARDRAIL SUMMARY

= N	NON-GATING IMPACT A	ATTENUATOR TYPE 350		1		İ				1	1		1		1				
/EY	BEG. STA.	END CTA	LOCATION		LENGTH		WARRA	NT POINT	"N" DIST.	TOTAL	FLARE	LENGTH		W			ANCHORS		IMPACT ATTENUATOR SINGLE REMOVE AND TL-3 FACED EXISTING STOCKPILE REMARKS
E		END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	SHOUL. WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	TYPE III	GREU TL–3			ATTENUATOR TL-3 SINGLE FACED GUARDRAIL EA G NG EA G NG REMOVE EXISTING STOCKPILE EXISTING GUARDRAIL EXISTING GUARDRAIL
	22+03.37	22 + 78.37(BRIDGE)	RT	75′			22 + 78.37(BRIDGE)		4.42'	8′	50′		1′		1	1			
	22 + 03.37	22 + 78.37(BRIDGE)	LT	75′				22 + 78.37(BRIDGE)	4.42′	8′		50′		1′	1	1			
	23 + 40.94(BRIDGE)	24+40.94	RT	100′				23 + 40.94(BRIDGE)	4.00′	8′		81.25′		1.62′	1	1			
	23 + 40.30(BRIDGE)	24+15.30	LT	75′			23 + 40.30(BRIDGE)		4.84′	8′	50′		1′		1	1			
			SUBTOTAL:	325′											4	4			
		ANCH	OR DEDUCTIONS:																
			REU, TL-3: 4@50'	-200′															
		1	YPE III: 4@18.75'	-75′															
			TOTAL:	50′															
			SAY:	62.50′											4	4			
			ADDITIONAL POST	5															

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

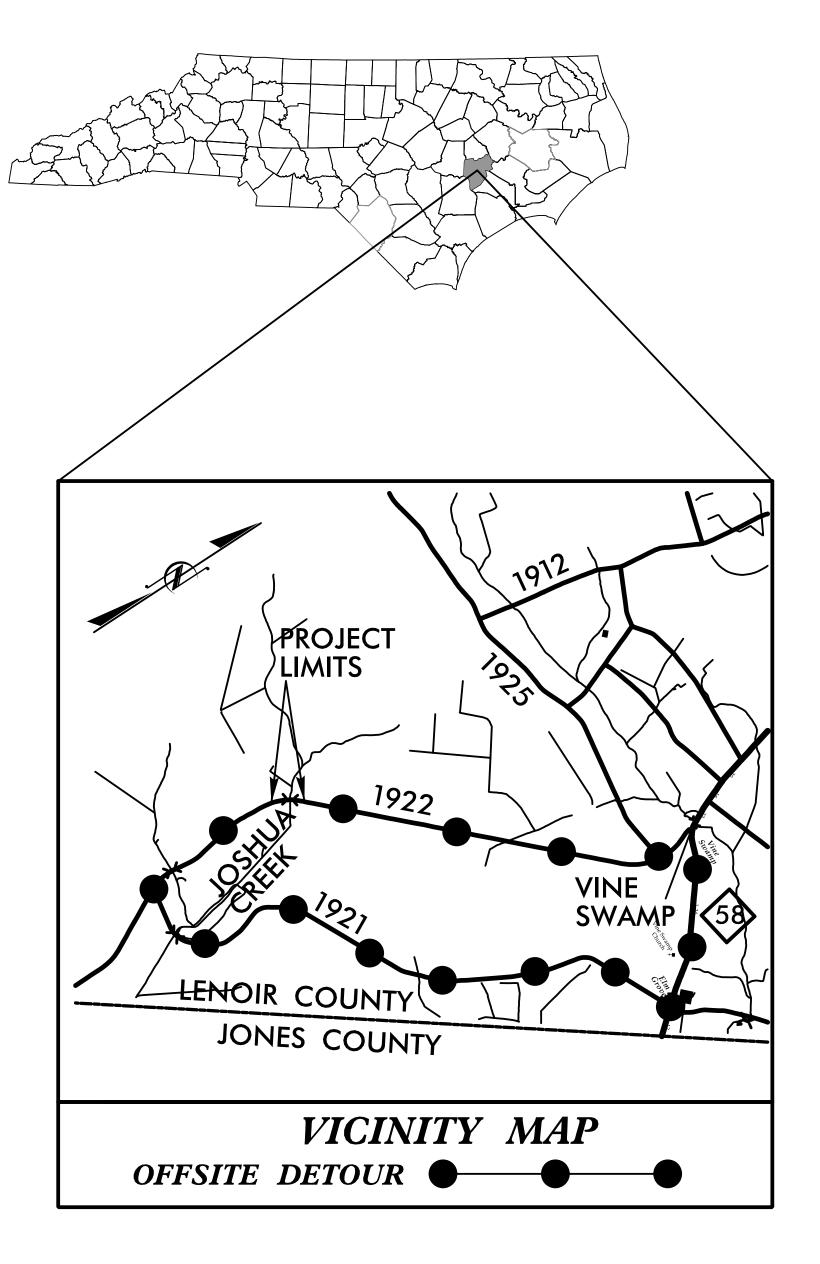
STATION	ON (LT,RT, OR CL) STRUCTURE NO.	VATION	ELEVATION	ELEVATION	CRITICAL		CAAP			BITUMINOUS COATED C.S. PIPE TYPE B (UNLESS NOTED OTHERWISE)		CLASS III R.C. PIPE OR C.S. PIPE, TYPE IR OR HDPE PIPE, TYPE S OR D			STD. 838 STD. 838 OR STD. 838 (UNLES NOTEL		QUANTITIES FOR DRAINAGE STRUCTURES * TOTAL L.F. FOR PA		FRAME, GRATES AND HOOD STANDARD 840.03	STD. 840.15 ID. 840.16	0.1 0.1 0.1 0.1	<u>~</u>	TE STD. 840.	1 7 1 0 1	940.32 'B' STD. 840.35 ND TWO GRATES STD. 840.29	4O. & SIZE	PLUG, C.Y. STD. 840.71 O O O O O O O O O O O O O O O O O O O	.I. NARROW DROP INLET DROP INLET				
SIZE	LOCATIC	TOP ELEY	NVERT E	INVERT	SLOPE 15	" 15" 18	3" 24" 30"	36" 42	2" 48" 1	2" 15" 18	24"	30"	36"	42"	48"	12" 15" 18" 24" 30	36" 42" 48	, BIPE	PIPE	CU. YDS.	RU 5.0′	B S S		OR 3	STD. 8.	STD. 8	VITH GR	AME WIT	OR 8	BOWS N	₩.H	JUNCTION BOX MANHOLE
THICKNESS OR GAUGE	FROM TO		_				.064	ة ا	601.	.064	9	.079	.079	.109	.109			SIDE DRAIN	SIDE DRAIN	SIDE DRAIN R.C.P.	EACH (0' TH ' THRU 10.0'	3' AND ABOVI	TYPE OF GRATE	.I. STD. 840.14	G.D.I. TYPE "A"	.D.I. TYPE "D"	.D.I. FRAME W	5.D.I. (N.S.) FRA	B. STD. 840.31 3 GRATED D.I B.D.I. (N.S.) FR	ORR. STEEL EL	ONC. & BRICI	D.I. TRAFFIC BEARING DROP INL J.B. TRAFFIC BEARING JUNCTION
																		15″	18,	244	PER 5.0′	10.0 C.B.	E F G		0 0	٥	0 0	0 0	8.U 8T 1.B	Ŭ Ŭ	0 🖥	REMARKS
-L- 23 + 85.71	RT 0401	74.20																			1								1 1			
	0401 OUT		70.24	70.05												16																
-L- 20 + 71.01	LT															32															41′	REMOVE 15" RCP



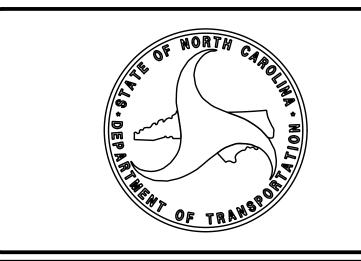
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

TRANSPORTATION MANAGEMENT PLAN

LENOIR COUNTY



LOCATION: REPLACE BRIDGE NO. 143 OVER JOSHUA CREEK ON SR 1922 (VINE SWAMP ROAD)



SHEET NO.

TITLE

TMP - 1

TMP-2

TITLE SHEET, VICINITY MAP, INDEX OF SHEETS AND LIST OF APPLICABLE ROADWAY STANDARDS

GENERAL NOTES AND DETOUR

ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C. DATED JAN 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	<u>TITLE</u>
1101.03	TEMPORARY ROAD CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS - LINE TYPES & OFFSETS
1205.02	PAVEMENT MARKINGS - 2 LANE & MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	PAVEMENT MARKER SPACING
1251.01	RAISED PAVEMENT MARKERS - PERMANENT AND TEMPORARY
1261.01	GUARDRAIL AND BARRIER DELINEATOR SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATOR TYPE
1262.01	GUARDRAIL END DELINEATION

L. D. STOUCHKO, PE J. A. PHILLIPS

TRAFFIC CONTROL PROJECT ENGINEER

APPROVED Lori D. Stouchko, P.E.

DATE:

A11E0A9B32E84 \$\frac{1}{2}\frac{5}{2}\frac{7}{2}\text{018}

S. J. HAMILTON, PE, CPM NCDOT CONTACT

TRAFFIC CONTROL DESIGN ENGINEER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Ste 200 Raleigh, North Carolina 27609 NC License No: C-1554

SEAL

SHEET NO.

TMP-1

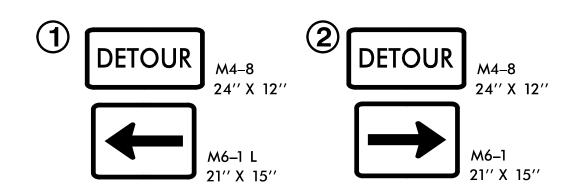
PROJ. REFERENCE NO.	SHEET NO.
17BP.2.R.80	TMP-2

GENERAL NOTES

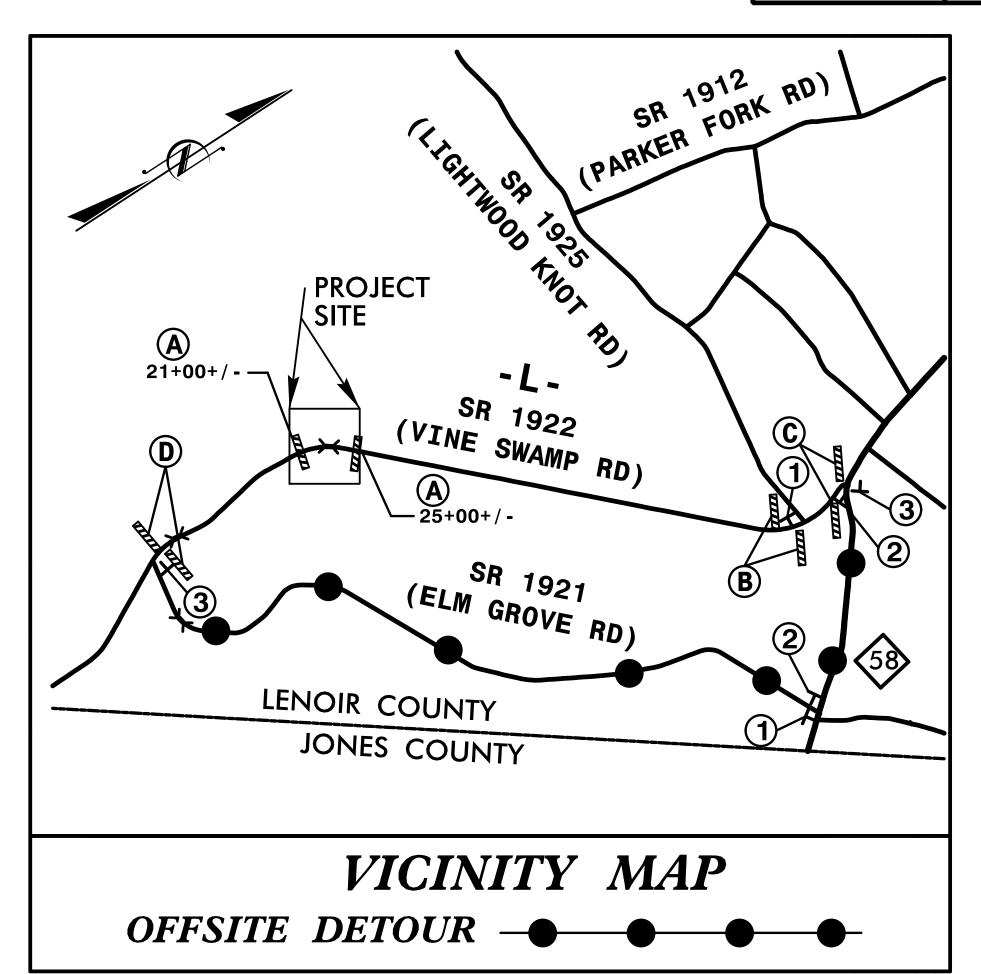
IMPLEMENT TRAFFIC CONTROL IN ACCORDANCE WITH THE ROADWAY STANDARD DRAWINGS LISTED ON TMP-1

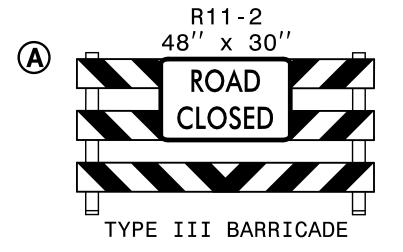
CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN THE DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATIONS MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

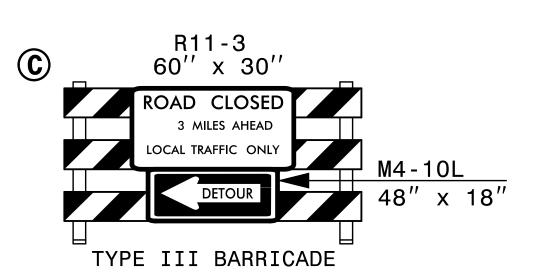
STATE FORCES WILL INSTALL AND MAINTAIN THE PROJECT DETOUR AND TYPE III BARRICADES AT THE PROJECT LIMITS. STATE FORCES WILL INSTALL MARKINGS AND MARKERS ON THE FINISHED PROJECT. CONTACT JEFF DUNNING AT 252-830-3493 TWO WEEKS PRIOR TO CLOSING THE ROAD FOR DETOUR INSTALLATION.

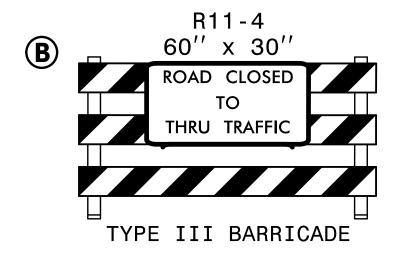


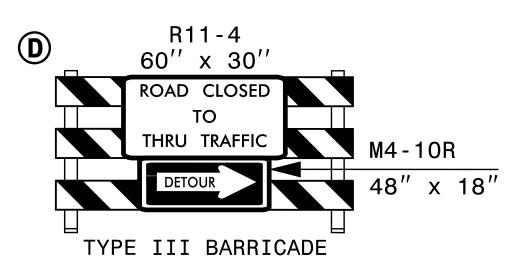








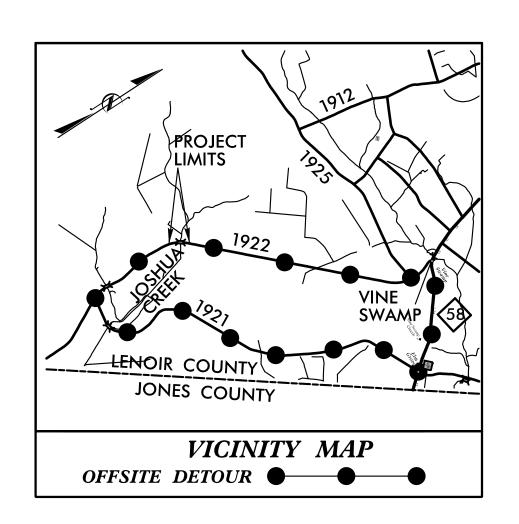






TRANSPORTATION MANAGEMENT PLAN

GENERAL NOTES, AND DETOUR



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

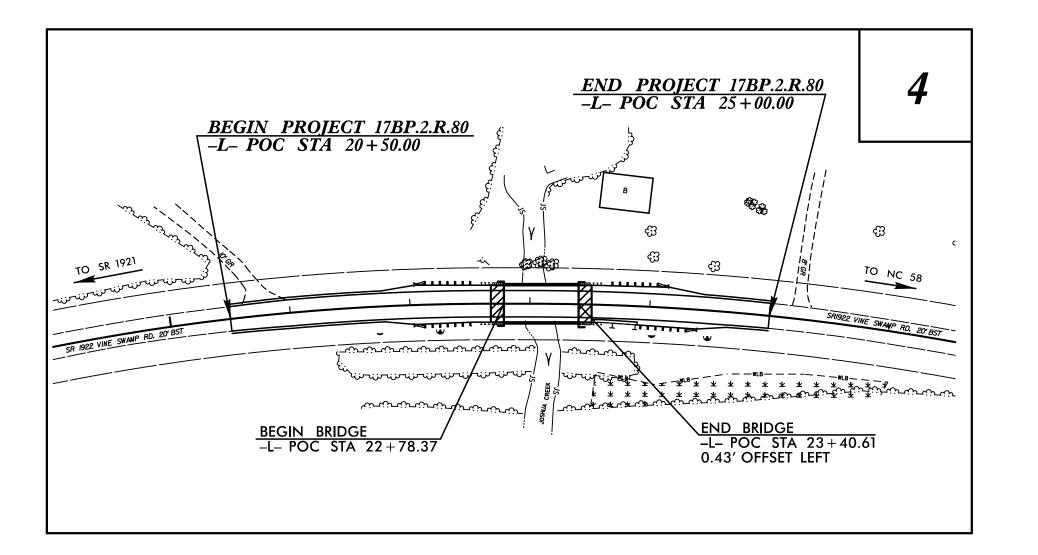
PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

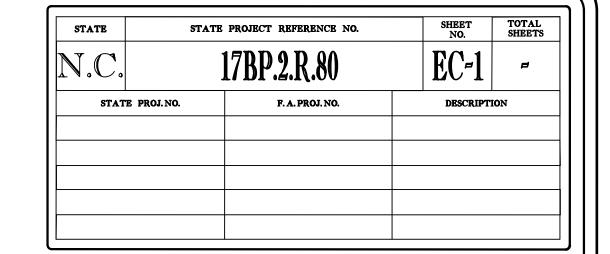
LENOIR COUNTY

LOCATION: REPLACE BRIDGE NO. 143 OVER JOSHUA CREEK ON SR 1922 (VINE SWAMP ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURE







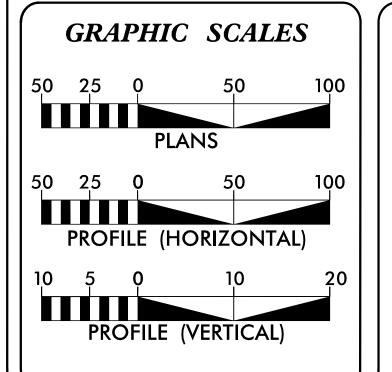
EROSION AND SEDIMENT CONTROL MEASURES Temporary Silt Ditch Temporary Silt Fence. Special Sediment Control Fence Temporary Berms and Slope Drains Silt Basin Type B. Temporary Rock Silt Check Type-A. Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM) Temporary Rock Silt Check Type-B. Wattle / Coir Fiber Wattle. Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)... 1634.01 Temporary Rock Sediment Dam Type-A. Temporary Rock Sediment Dam Type-B...

Rock Pipe Inlet Sediment Trap Type-A. Rock Pipe Inlet Sediment Trap Type-B. Stilling Basin 1630.06 Special Stilling Basin. Rock Inlet Sediment Trap: Туре А. 1632.01 1632.02 Туре В. 1632.03 Туре С. Skimmer Basin Tiered Skimmer Basin Infiltration Basin

> THIS PROJECT HAS BEEN DESIGNED TO SENSITIVE WATERSHED STANDARDS.

ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT

> Refer To E. C. Special Provisions for Special Considerations.



ROADSIDE ENVIRONMENTAL UNIT **DIVISION OF HIGHWAYS** STATE OF NORTH CAROLINA

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 1, 2016 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

Prepared in the Office of: HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No: C-1554

2018 STANDARD SPECIFICATIONS

NATALIE CHAN, PE **EROSION CONTROL** LEVEL III CERTIFICATION #3444 Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings" - Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drains 1630.01 Riser Basin 1630.02 Silt Basin Type B

1630.03 Temporary Silt Ditch 1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin 1631.01 Matting Installation

1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle

1645.01 Temporary Stream Crossing

COIR FIBER WATTLE WITH POLYACRYLAMIDE (PAM) DETAIL

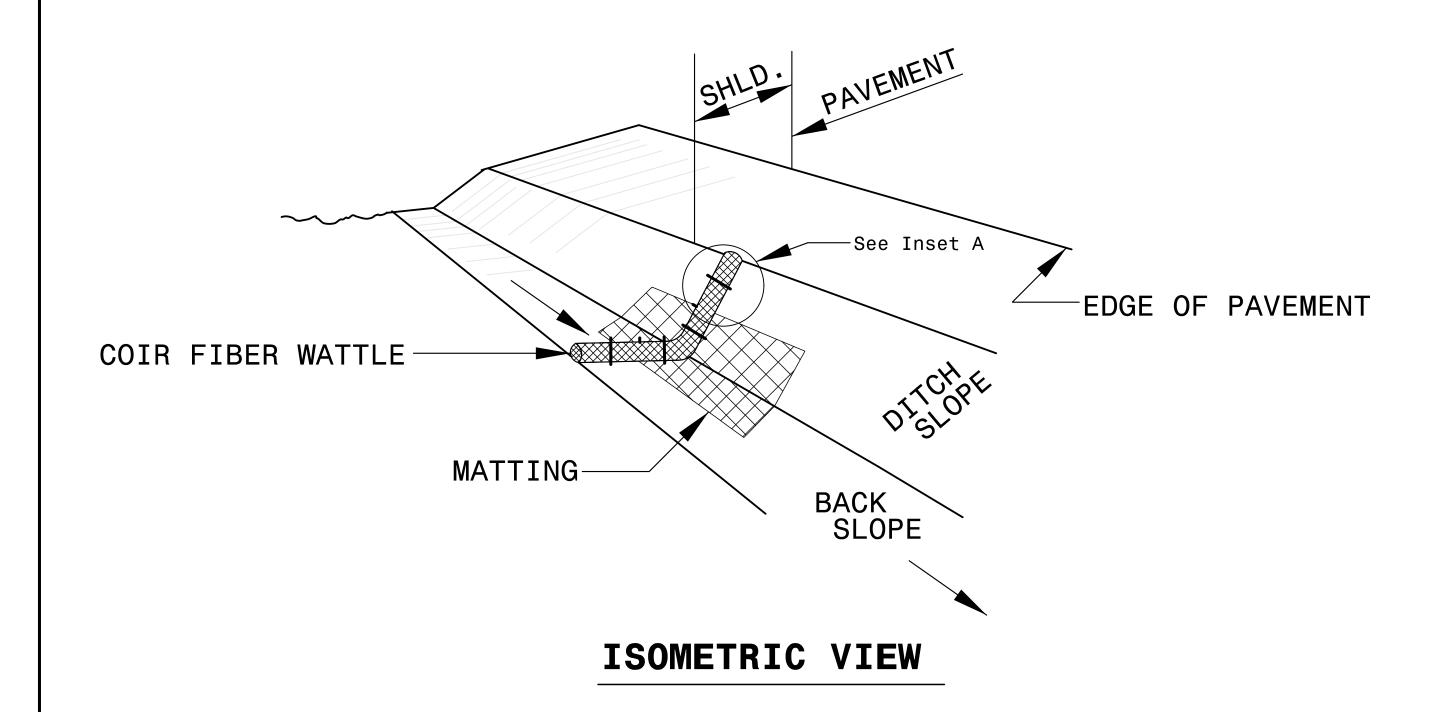
PROJECT REFERENCE NO.

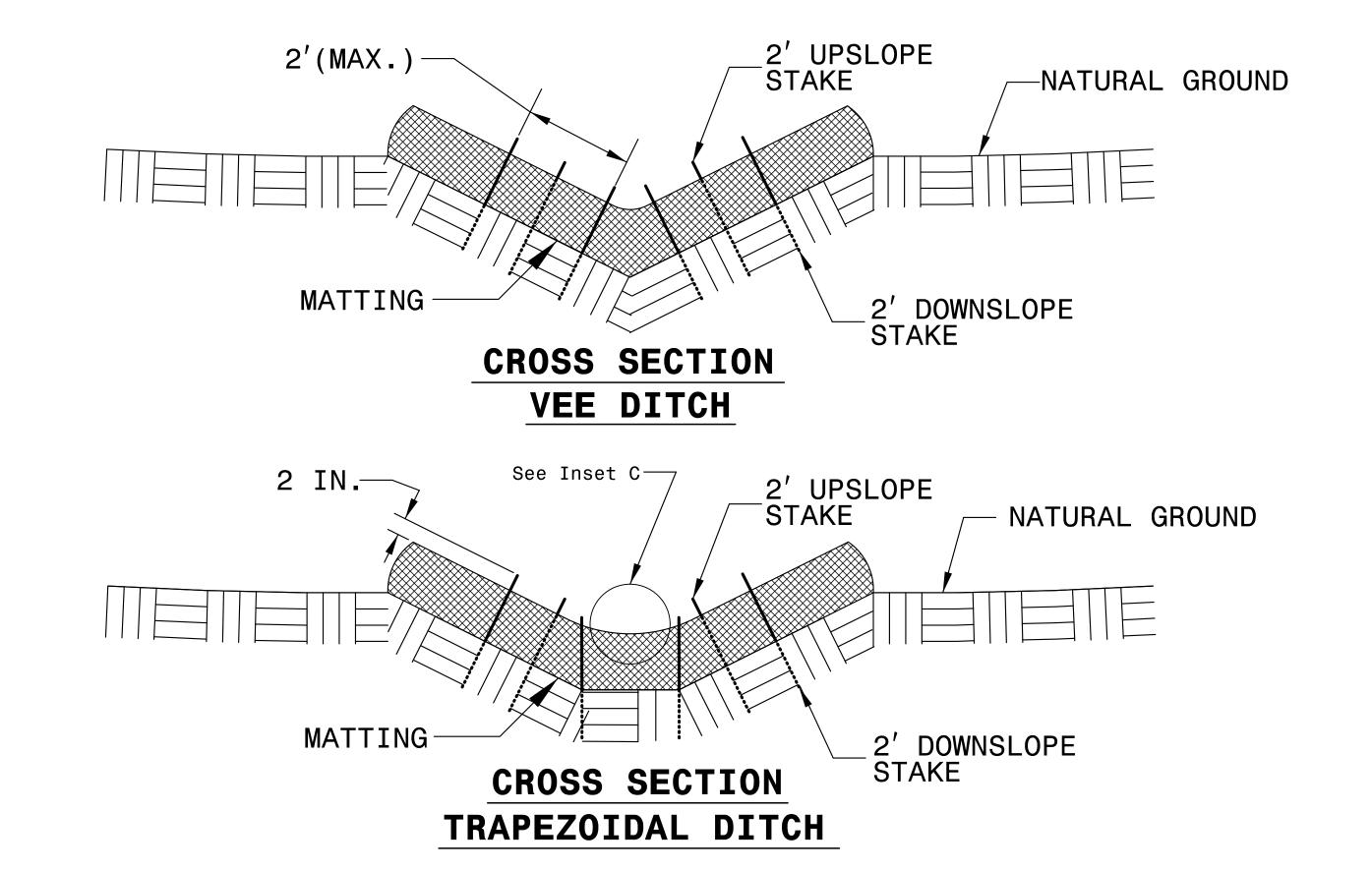
ITBP.2.R.80

RW SHEET NO.

ROADWAY DESIGN
ENGINEER

HYDRAULICS
ENGINEER





NOTES:

FLOW

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

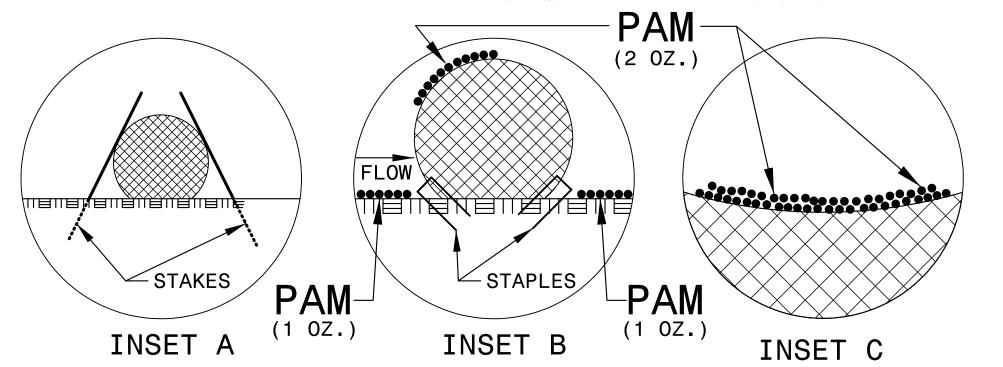
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

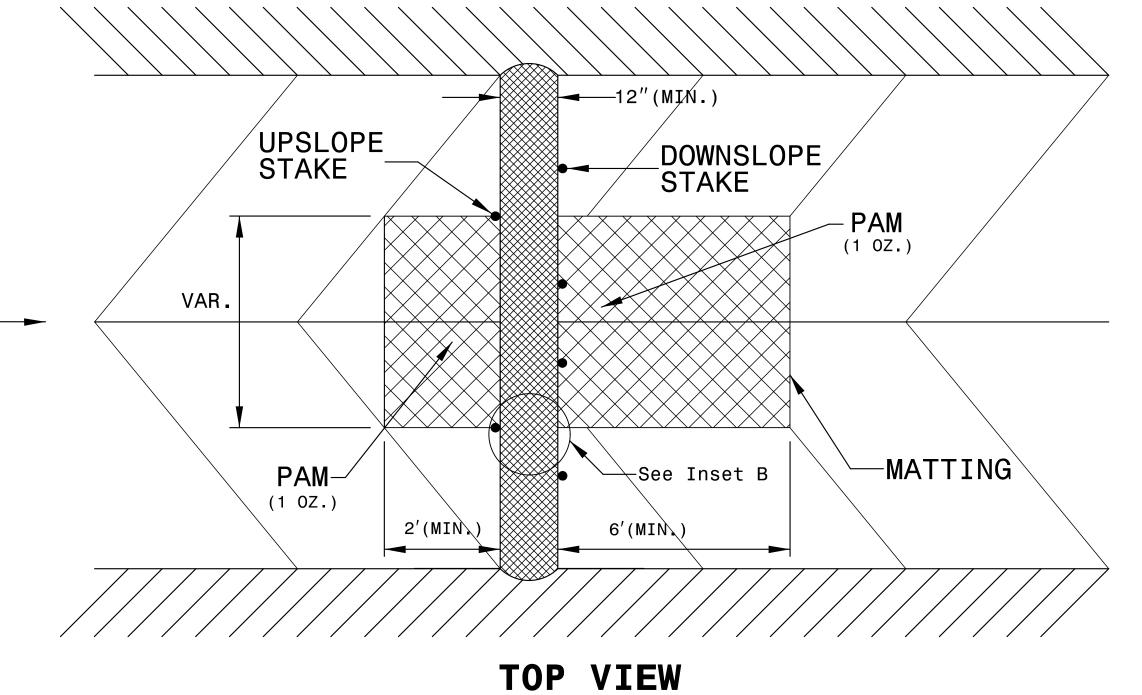
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH WATTLE.

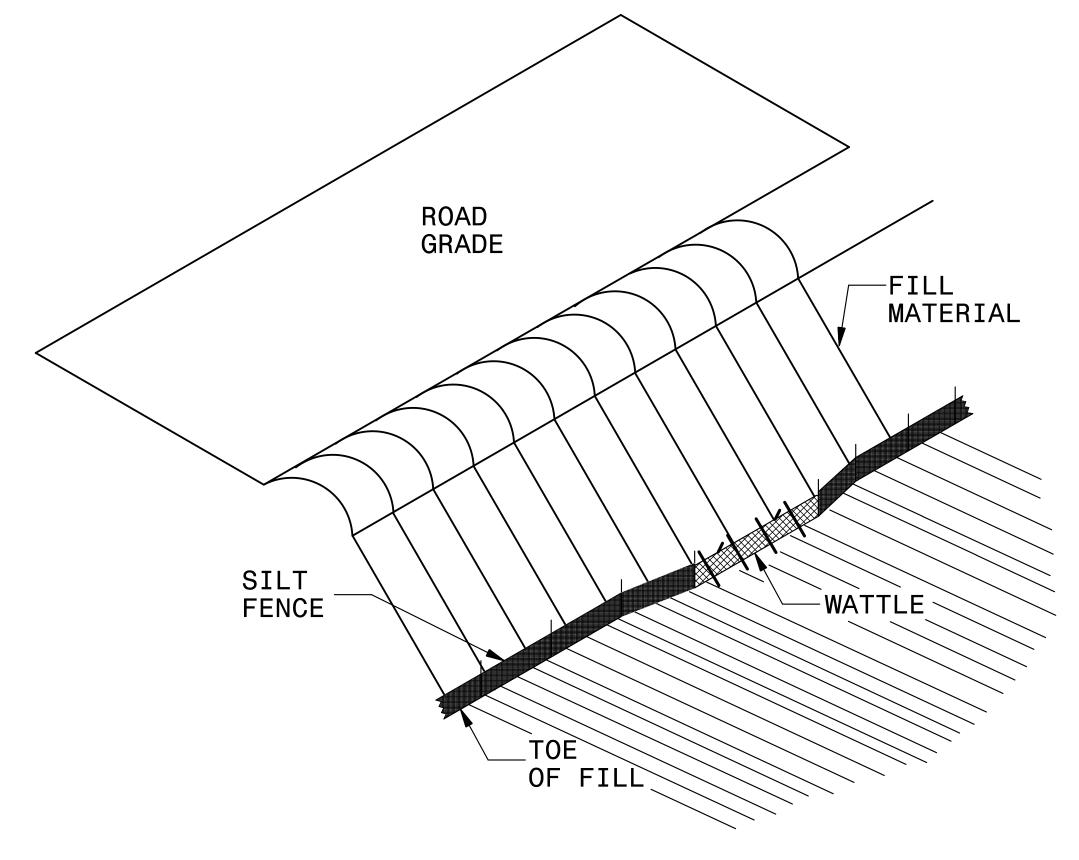
INITIALLY APPLY 2 OUNCES OF ANIONIC OR NEUTRALLY CHARGED PAM OVER WATTLE WHERE WATER WILL FLOW AND 1 OUNCE OF PAM ON MATTING ON EACH SIDE OF WATTLE. REAPPLY PAM AFTER EVERY RAINFALL EVENT THAT IS EQUAL TO OR EXCEEDS 0.50 IN.



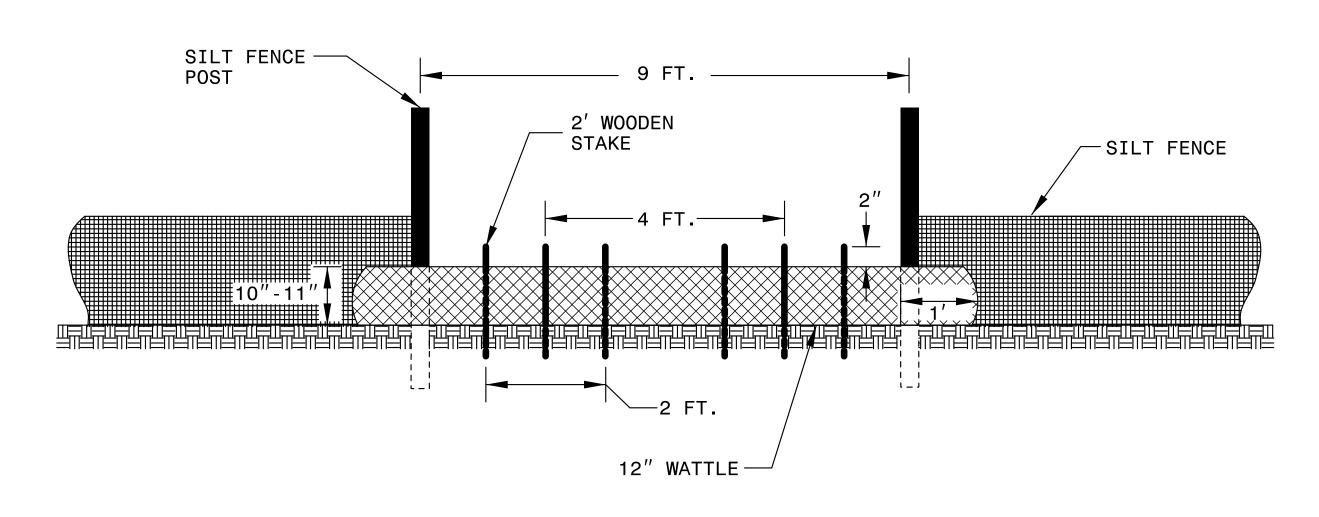


SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO).	SHEET NO.						
17BP.2.R.80		EC-2A						
R/W SHEET N	NO.							
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER						



ISOMETRIC VIEW



VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

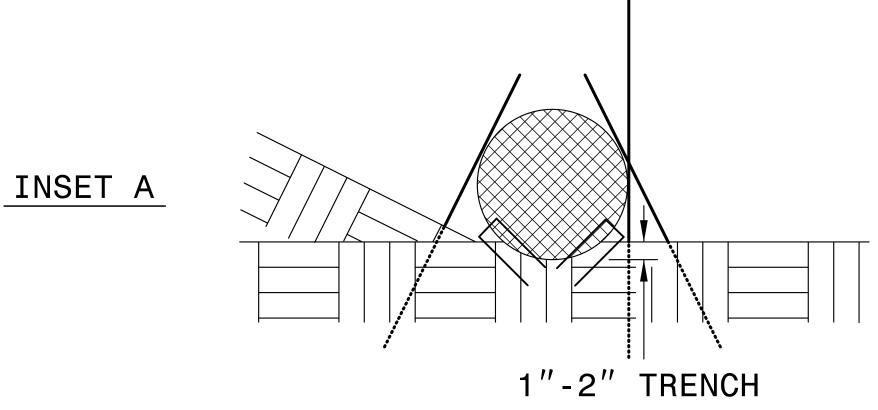
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

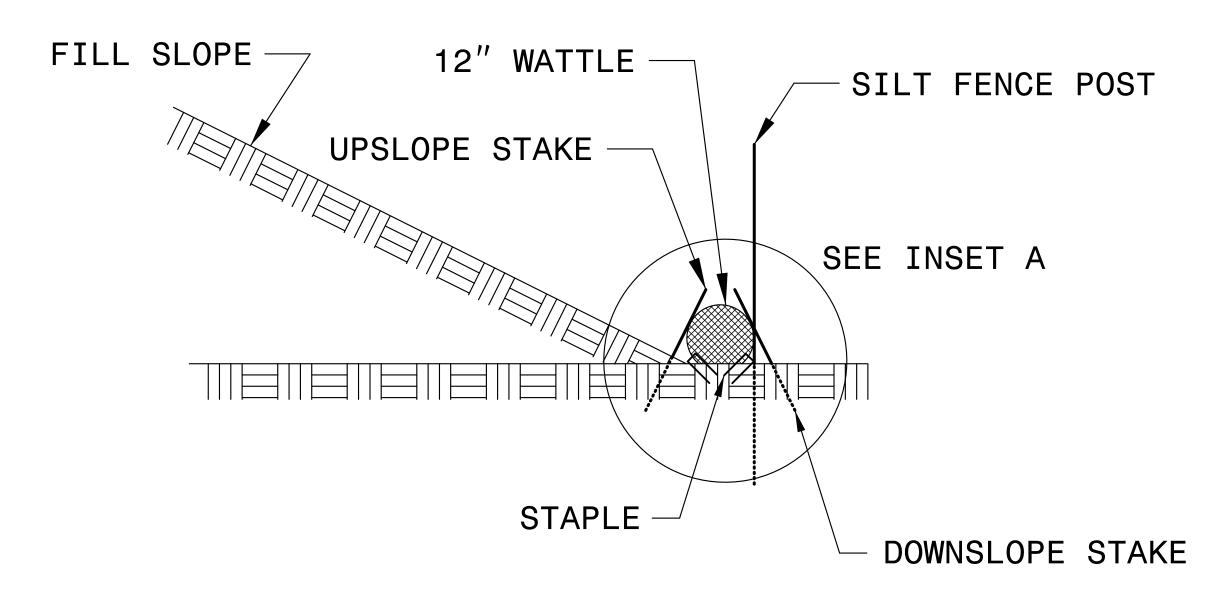
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

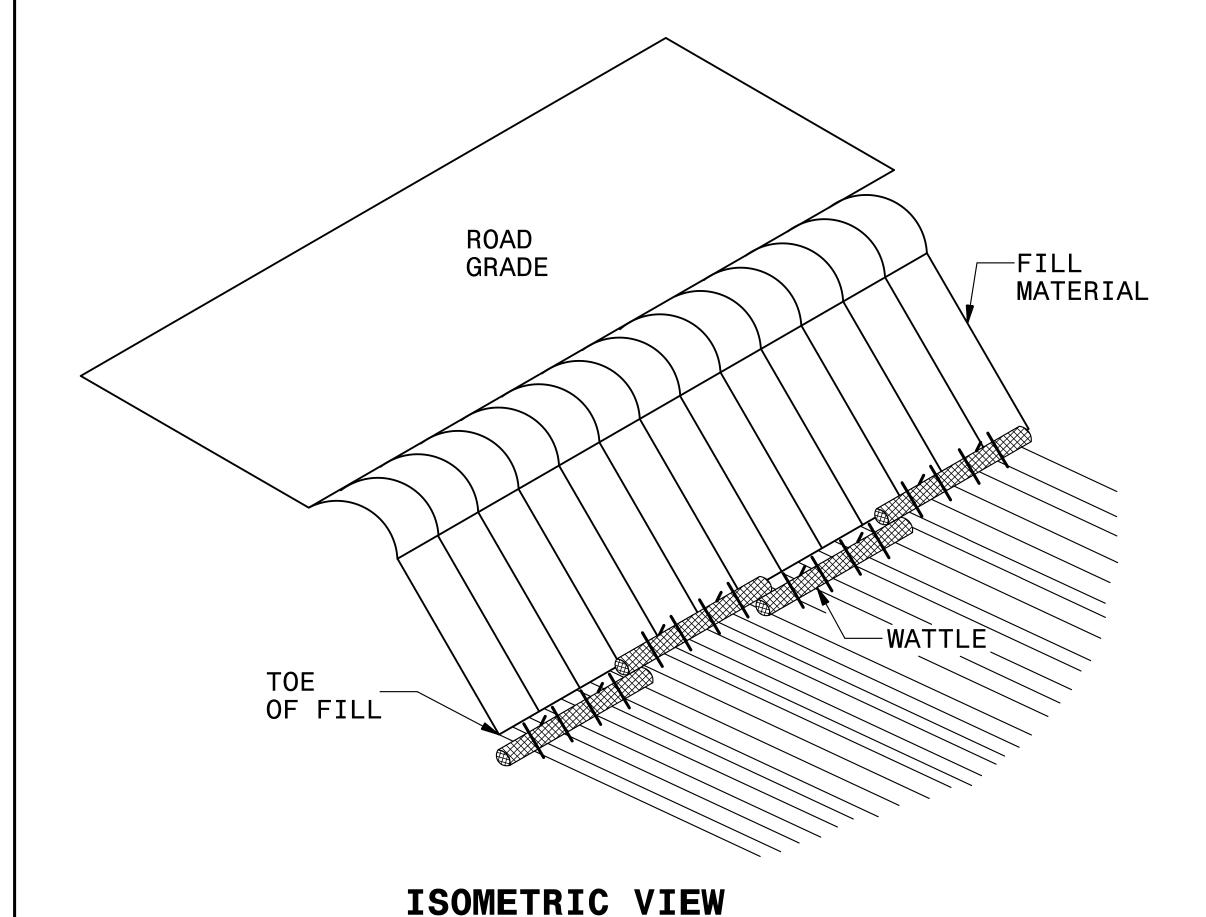


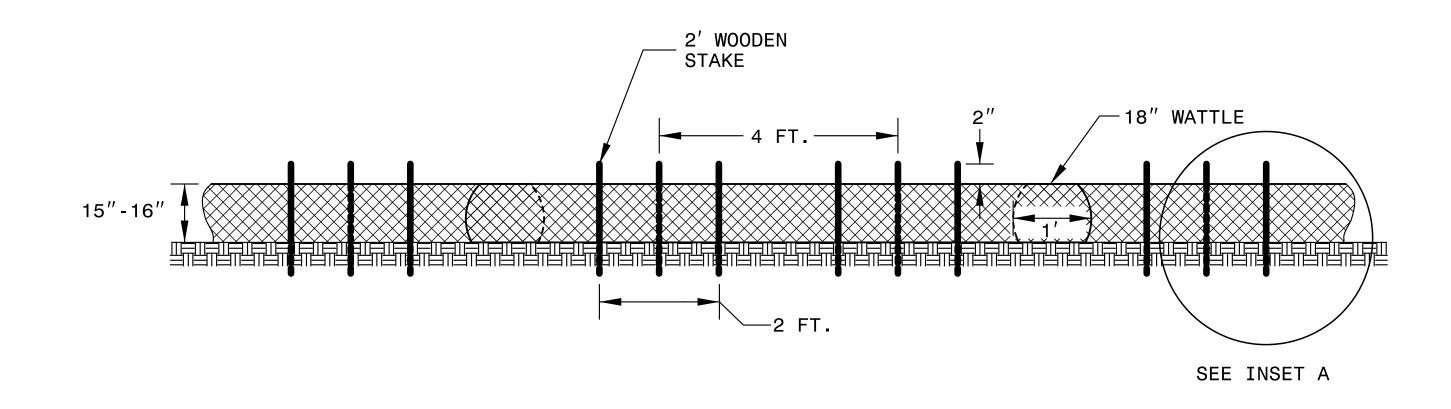


SIDE VIEW

COIR FIBER WATTLE BARRIER DETAIL

PROJECT REFERENCE NO).	SHEET NO.						
17BP.2.R.80		EC-2B						
R/W SHEET N	NO.							
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER						





FRONT VIEW

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

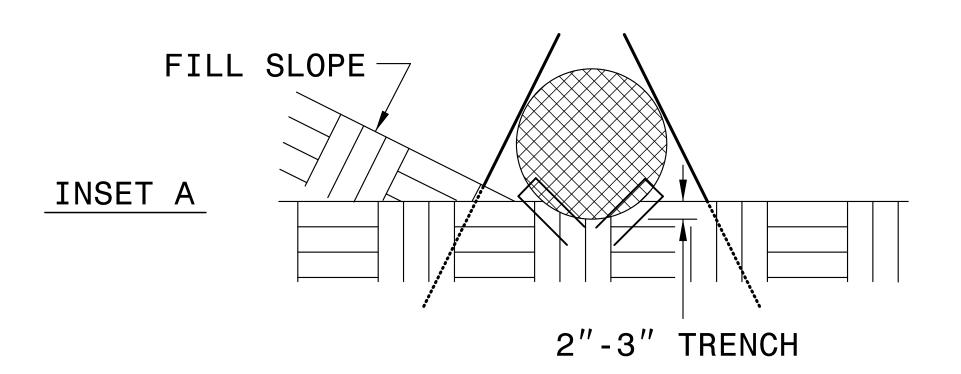
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

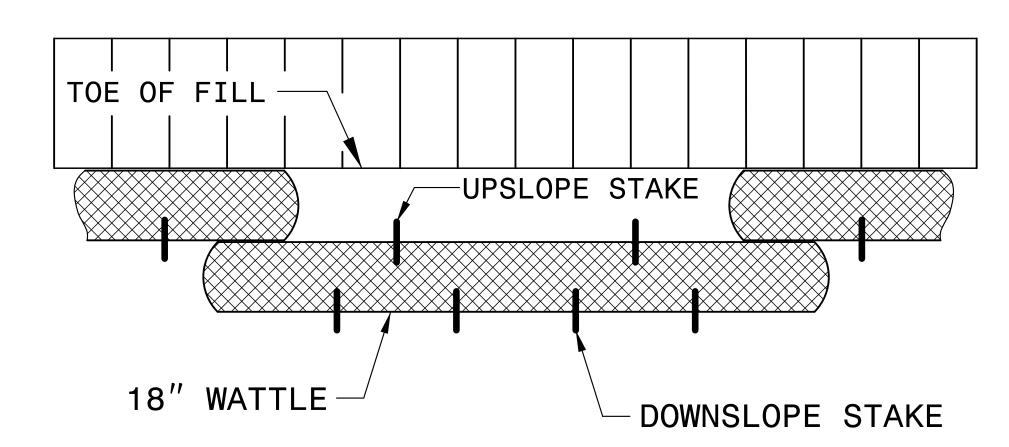
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.



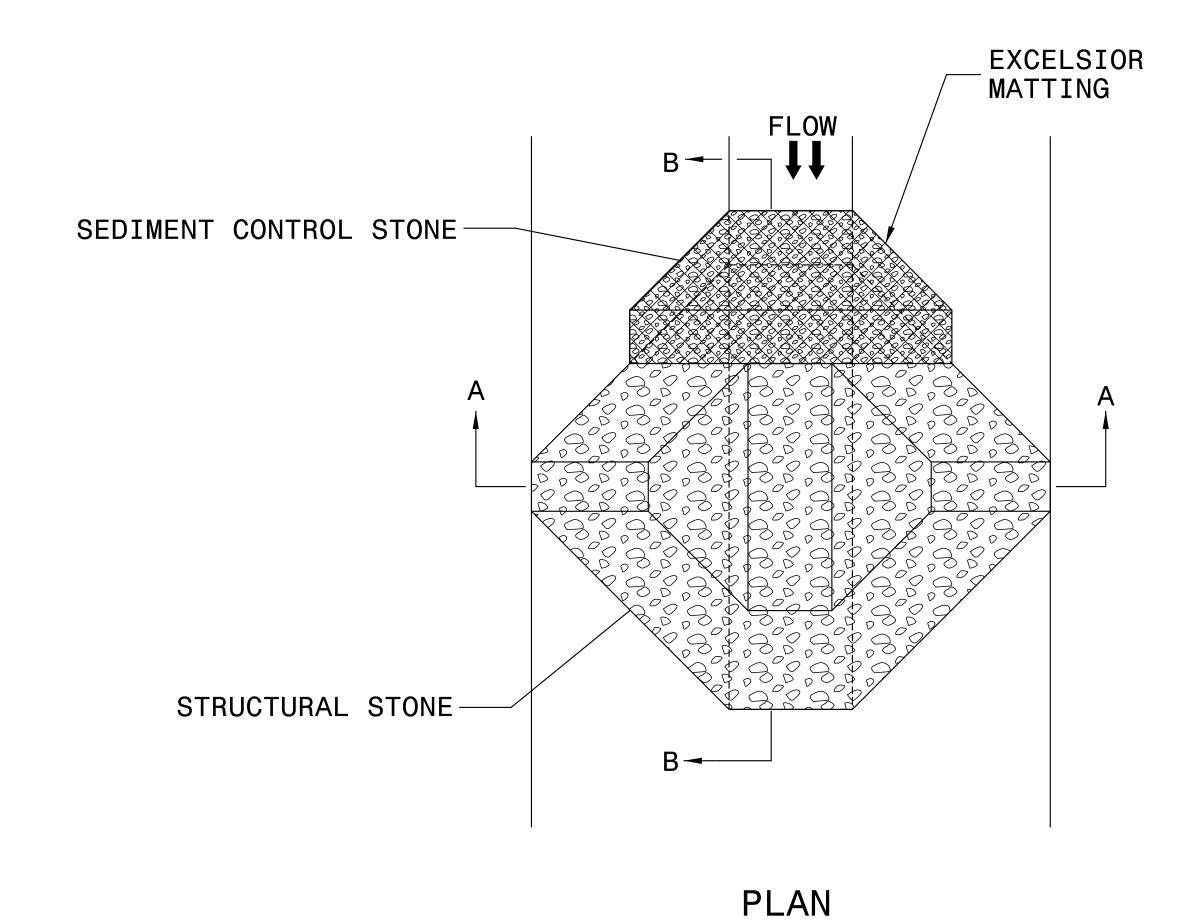


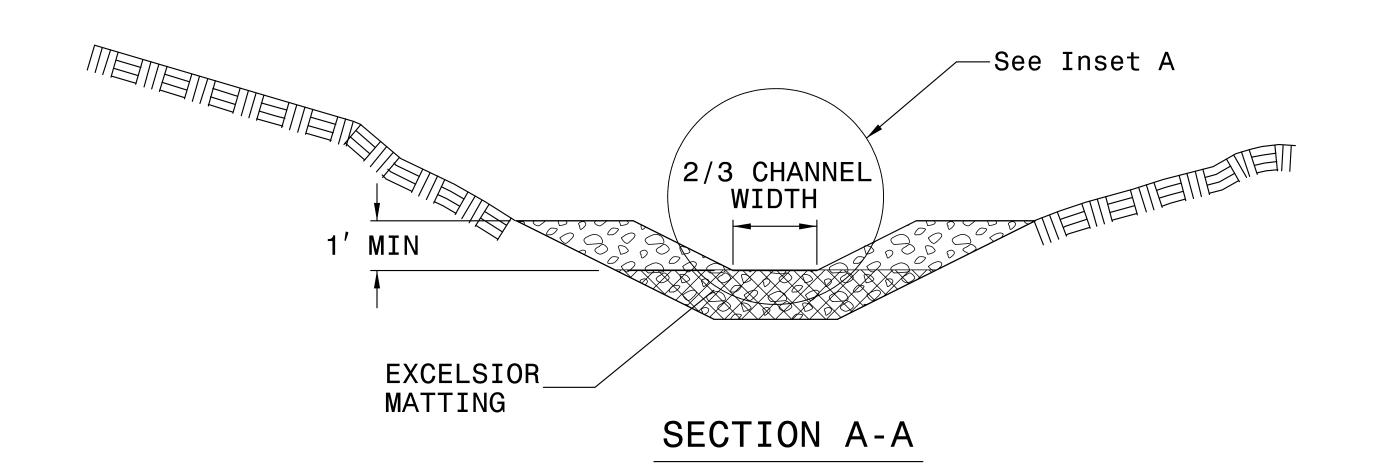
TOP VIEW

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

PROJECT REFERENCE NO	O. SHEET NO.	
17BP.2.R.80	EC-2C	
R/W SHEET N	NO.	
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER	

NOT TO SCALE





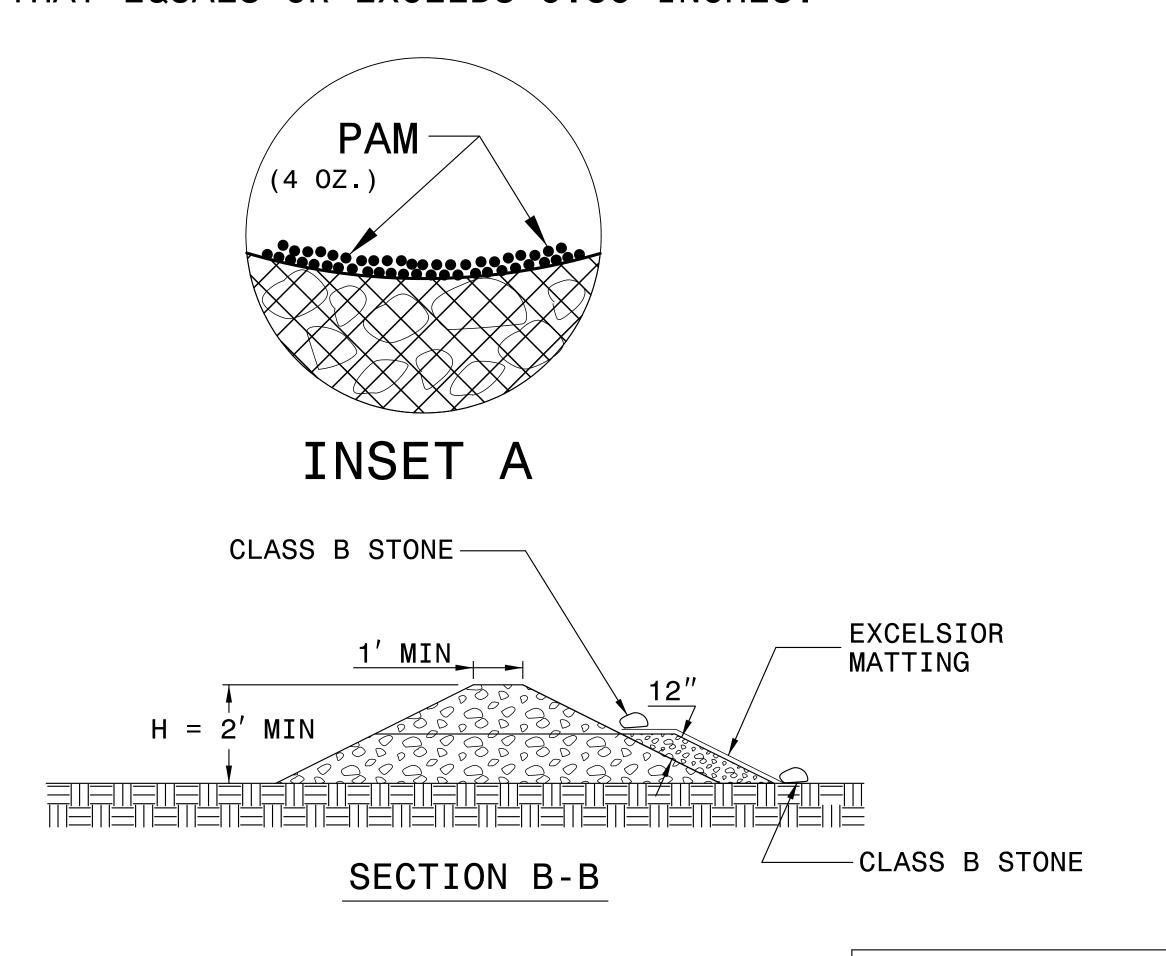
NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

PROJECT REFERENCE NO).	SHEET NO.					
17BP.2.R.80	EC-3						
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER					

SOIL STABILIZATION SUMMARY SHEET

MATTING FOR EROSION CONTROL

MATTING FOR EROSION CONTROL

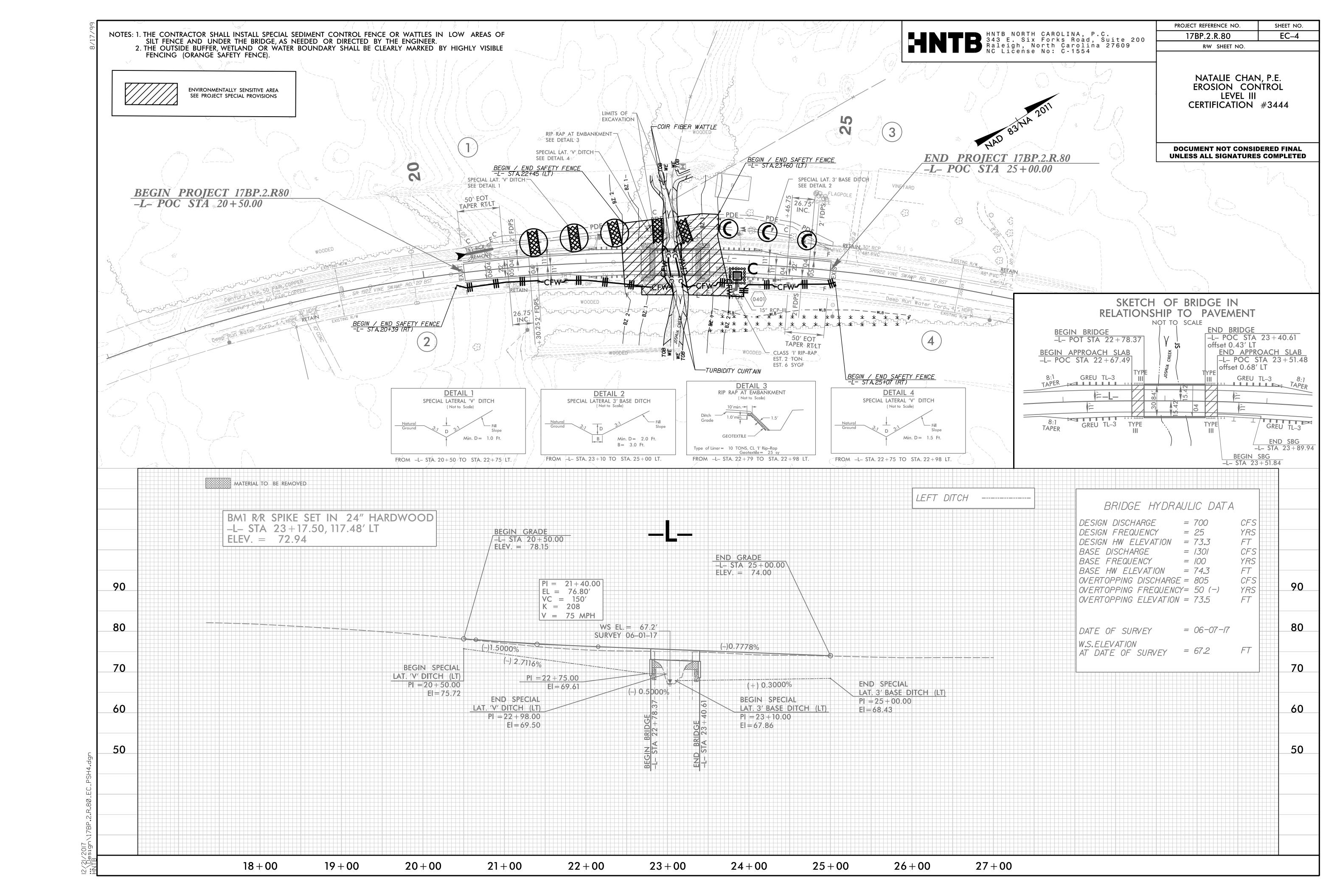
CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)	CONST SHEET NO.	LINE	FROM STATION	TO STATION SIDE	ESTIMATE (SY)			
4	L	20+50	22+98	レイ	175								
			SUE	STOTAL	175								
MISCELLANEOUS	5 MATTING TO BE INST	ALLED AS DIRE	CTED BY THE	ENGINEER	1270								
				TOTAL	1445								
				SAY	1500								

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

PROJECT REFERENCE NO).	SHEET NO.
17BP.2.R.80		EC-3A
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.



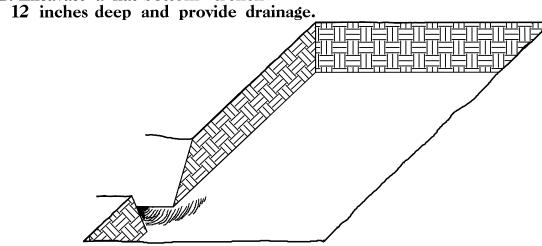
PLANTING DETAILS

SEEDLING / LINER BAREROOT PLANTING DETAIL

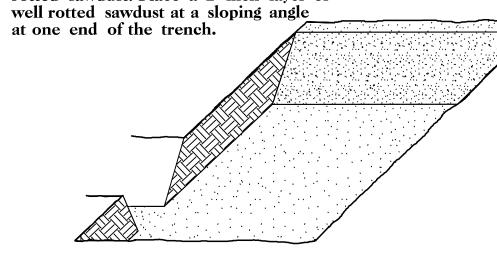
HEALING IN

1. Locate a healing—in site in a shady, well protected area.

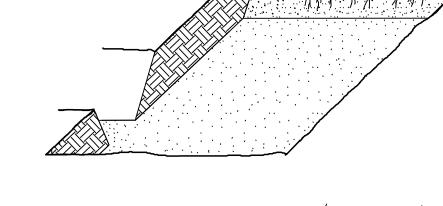
2. Excavate a flat bottom trench



3. Jackfill the trench with 2 inches well rotted sawdust. Place a 2 inch layer of well rotted sawdust at a sloping angle



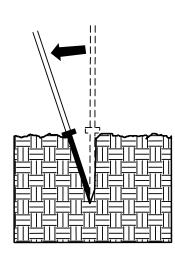
4. Place a single layer of plants against the sloping end so that the root collar is at ground level.



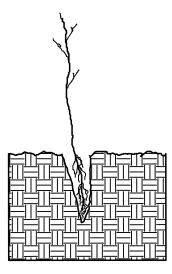
5. Place a 2 inch layer of well rotted sawdust over the roots maintaining a sloping angle.

6. Repeat layers of plants and sawdust as necessary and water thoroughly.

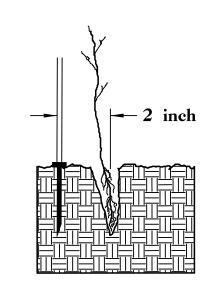
DI33LE PLANTING METHOD USING THE K3C PLANTING 3AR



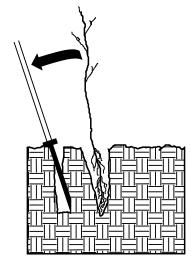
1. Insert planting bar as shown and pull handle toward planter.



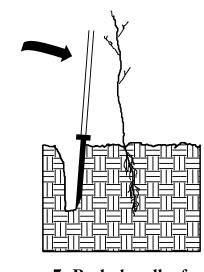
2. Remove planting bar and place seedling at correct depth.



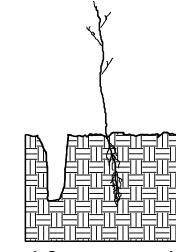
3. Insert planting bar
2 inches toward planter



4. Pull handle of bar toward planter, firming soil at bottom.



5. Push handle forward firming soil at top.



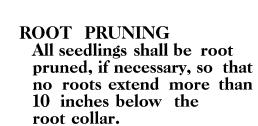
6. Leave compaction hole open. Water thoroughly.

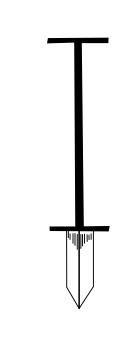
PLANTING NOTES:

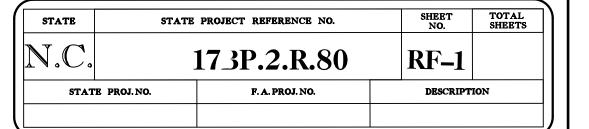
PLANTING 3AG
During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.



K3C PLANTING 3AR
Planting bar shall have a
blade with a triangular
cross section, and shall
be 12 inches long,
4 inches wide and
1 inch thick at center.







REFORESTATION

TREE REFORESTATION SHALL 3E PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION

MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

25% LIRIODENDRON TULIPIFERA TULIP POPLAR 12 in – 18 in 3R
25% PLATANUS OCCIDENTALIS AMERICAN SYCAMORE 12 in – 18 in 3R
25% FRAXINUS PENNSYLVANICA GREEN ASH 12 in – 18 in 3R
25% 3ETULA NIGRA RIVER 3IRCH 12 in – 18 in 3R

REFORESTATION DETAIL SHEET

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

PROJECT LIMITS LENOIR COUNTY JONES COUNTY VICINITY MAP OFFSITE DETOUR

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

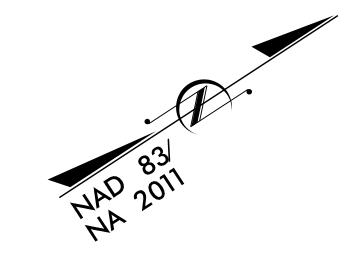
UTILITIES BY OTHERS PLANS LENOIR COUNTY

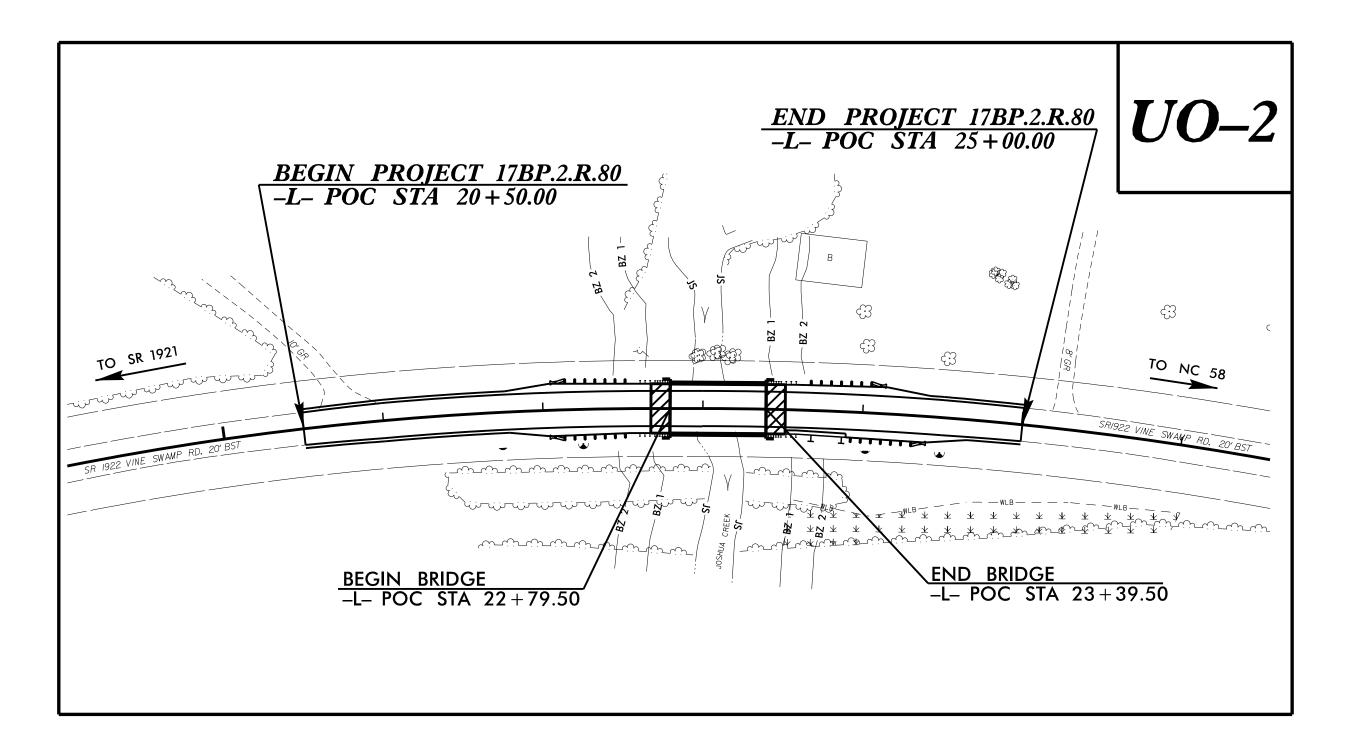
LOCATION: REPLACE BRIDGE NO.143 OVER JOSHUA CREEK ON SR 1922 (VINE SWAMP RD)

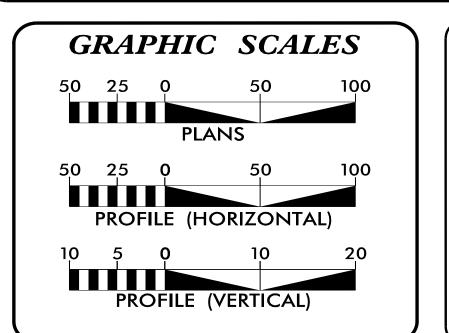
TYPE OF WORK: RELOCATE TELEPHONE



NOTE: ALL UTILITY WORK SHOWN ON THIS SHEET IS DONE BY OTHERS. NO PAYMENT WILL BE MADE TO THE CONTRACTOR FOR UTILITY WORK SHOWN ON THIS SHEET.







INDEX OF SHEETS

DESCRIPTION: SHEET NO.: TITLE SHEET *UO-1* **UO**–2 UBO PLAN SHEET

UTILITY OWNERS WITH CONFLICTS

(A) PHONE – CENTURYLINK

PREPARED IN THE OFFICE OF:

M A Engineering NC License: Consultants, Inc. F-0160

598 East Chatham Street Suite 137 Cary, NC 27511 Phone: 919.297.0220 Fax: 919.297.0221

WEBB WHITE UTILITY PROJECT MANAGER DWAYNE SMITH PROJECT UTILITY COORDINATOR



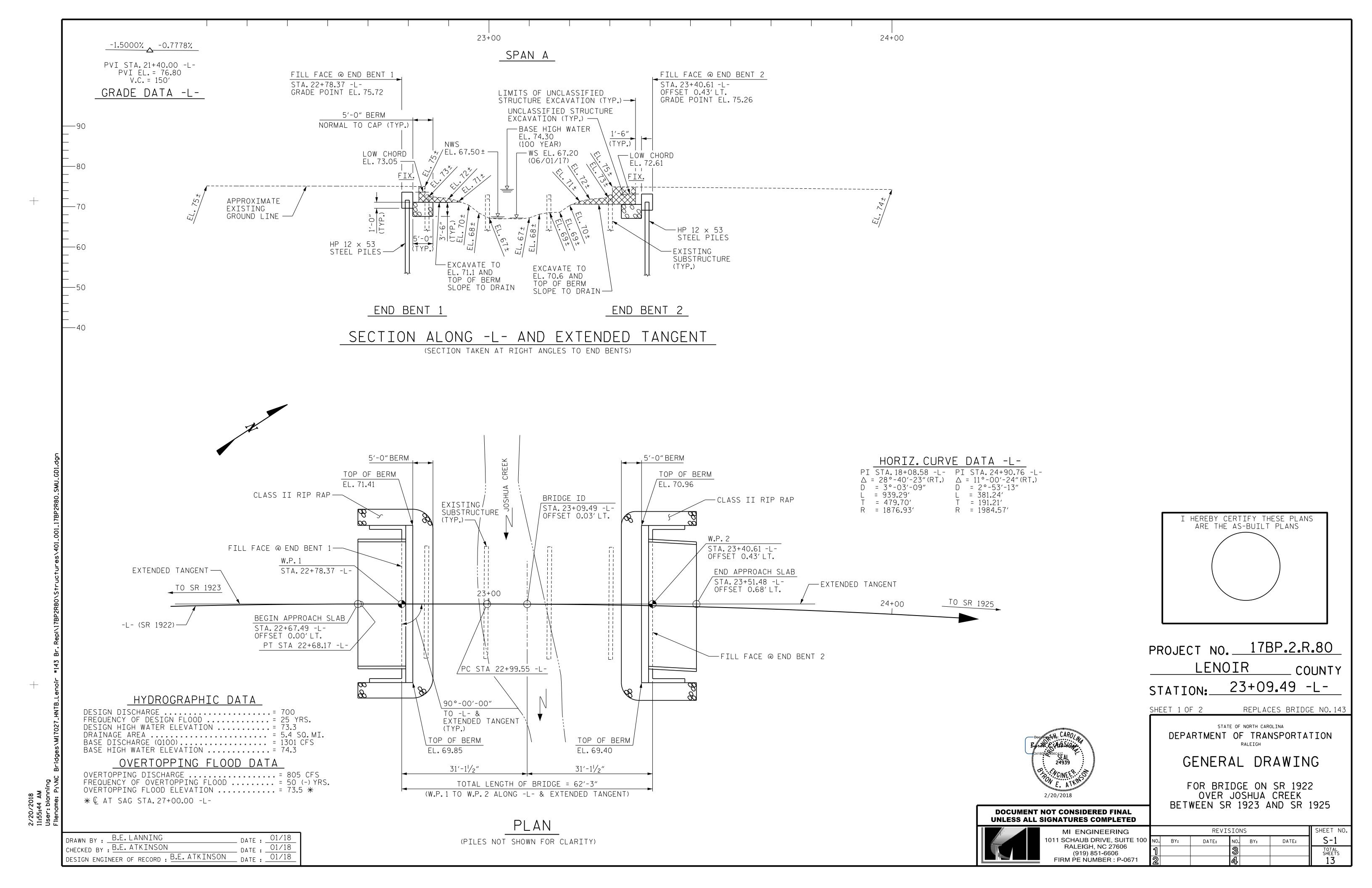
DIVISION OF HIGHWAYS DIVISION 2 PROJECT DEV UNIT DIV ADDRESS 1037 W.H. SMITH BLVD PO BOX 1587 GREENVILLE, NC 27835

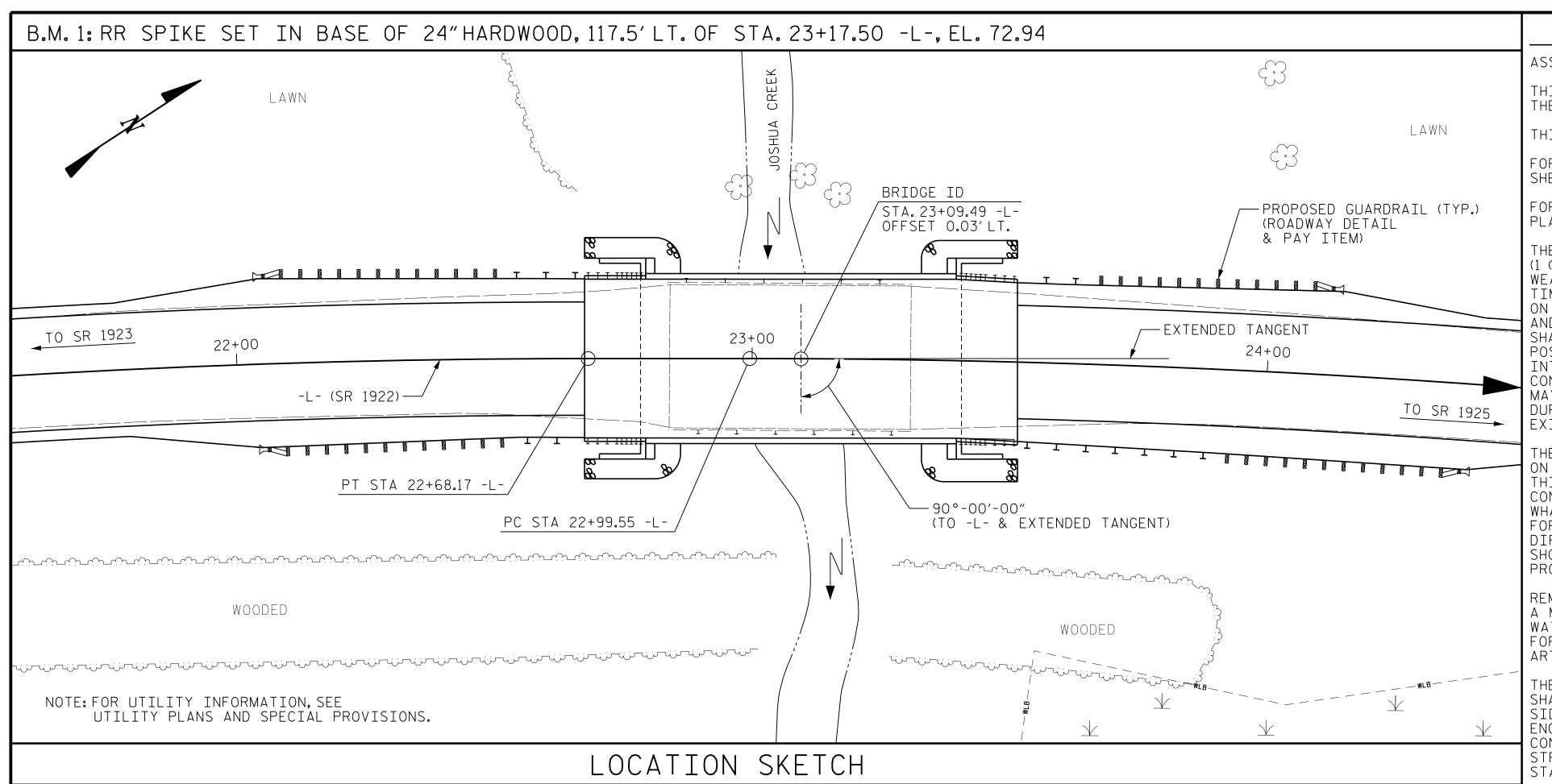
HEATHER LANE, P.E.

PROJECT DEVELOPMENT UNIT DIVISION BRIDGE PROGRAM MANAGER

SHEET NO. PROJECT REFERENCE NO. 17BP.2.R.80 UTILITIES BY OTHERS NOTE: ALL PROPOSED UTILITY WORK SHOWN ON THIS SHEET WILL BE DONE BY OTHERS. NO PAYMENT WILL BE MADE TO THE CONTRACTOR FOR PROPOSED UTILITY WORK SHOWN ON THIS SHEET. 25 WALTER L. MOORE

3 DB 1705 PG 396 RIP RAP AT EMBANKMENT — SEE DETAIL 3 WALTER L. MOORE DB 667 PG 679 SPECIAL LAT. 'V' DITCH-SEE DETAIL 4 CARMENA ZIMMERMAN DB 598 PG 633 SPECIAL LAT. 3' BASE DITCH _____SEE DETAIL 2 / / SPECIAL LAT. 'V' DITCH — SEE DETAIL 1 \mathfrak{S} WOODED WOODED WOODED JERRY R. TAYLOR DB 928 PG 607 PC 5 PG 102 CHARLES M. TYNDALL DB 1343 PG 31 DB 1152 PG 544 GARY BYRD. et UX DB 786 PG 228 ₹ Jacksonville Jones-Onslow EMC





NOTES

ASSUMED LIVE LOAD = HL 93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL

THE EXISTING STRUCTURE CONSISTING OF THREE SPANS (1 @ 15'-6",1 @ 15'-3" AND 1 AT 15'-9"), WITH ASPHALT WEARING SURFACE ON REINFORCED CONCRETE DECK WITH TIMBER JOISTS AND A CLEAR ROADWAY WIDTH OF 28'-0" ON TIMBER CAP WITH TIMBER PILE END BENTS AND BENTS AND LOCATED AT THE SITE OF THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

ASPHALT WEARING SURFACE IS INCLUDED IN THE ROADWAY QUANTITY. SEE ROADWAY QUANTITIES.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18- EVALUATING SCOUR AT BRIDGES".

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 75 TONS PER PILE.

DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 125 TONS PER PILE.

TESTING PILES WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

	TOTAL BILL OF MATERIAL																
	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HP STEE	12 X 53 L PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL		GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES CON	'X 1'-9" TRESSED ICRETE D SLABS
	LUMP SUM	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EACH	NO.	LIN.FT.	EACH	LIN.FT.	TON	SQ. YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE						LUMP SUM						120.25			LUMP SUM	11	660.00
END BENT 1				LUMP SUM	21.6		2636	7	7	400	7		52	38			
END BENT 2				LUMP SUM	21.6		2636	7	7	400	7		52	38			
																	_
TOTAL	LUMP SUM	LUMP SUM	1	LUMP SUM	43.2	LUMP SUM	5272	14	14	800	14	120.25	104	76	LUMP SUM	11	660.00

PROJECT NO. 17BP.2.R.80

LENOIR COUNTY

STATION: 23+09.49 -L-

SHEET 2 OF 2

DEPARTMENT OF TRANSPORTATION

RALEIGH

GENERAL DRAWING

FOR BRIDGE ON SR 1922 OVER JOSHUA CREEK BETWEEN SR 1923 AND SR 1925

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



MI ENGINEERING
011 SCHAUB DRIVE, SUITE 100
RALEIGH, NC 27606
(919) 851-6606
FIRM PE NUMBER: P-0671

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-2

1 3 TOTAL SHEETS
2 4 13

DRAWN BY: B.E. LANNING

CHECKED BY: B.E. ATKINSON

DATE: 01/18

DESIGN ENGINEER OF RECORD: B.E. ATKINSON

DATE: 01/18

					STRENGTH I LIMIT STATE											SERVICE III LIMIT STATE								
										MOMENT					SHEAR						MOMENT]
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.18		1.75	0.242	1.18	60′	I	29.5	0.600	1.80	60′	I	5.9	0.80	0.242	1.37	60′	I	29.5	
DESIGN		HL-93(0pr)	N/A		1.52		1.35	0.242	1.52	60′	I	29.5	0.600	2.38	60′	I	5.9	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.49	53.64	1.75	0.242	1.49	60′	I	29.5	0.600	2.23	60′	I	5.9	0.80	0.242	1.73	60′	I	29.5	
IVATINO		HS-20(0pr)	36.000		1.93	69.48	1.35	0.242	1.93	60′	I	29.5	0.600	2.94	60′	I	5.9	N/A						
		SNSH	13.500		3.73	50.36	1.4	0.242	4.01	60′	I	29.5	0.600	6.77	60′	I	5.9	0.80	0.242	3.73	60′	I	29.5	
		SNGARBS2	20.000		2.86	57.20	1.4	0.242	3.07	60′	I	29.5	0.600	4.82	60′	I	5.9	0.80	0.242	2.86	60′	I	29.5	
		SNAGRIS2	22.000		2.74	60.28	1.4	0.242	2.94	60′	I	29.5	0.600	4.49	60′	I	5.9	0.80	0.242	2.74	60′	I	29.5	
		SNCOTTS3	27.250		1.86	50.69	1.4	0.242	2.00	60′	I	29.5	0.600	3.29	60′	I	5.9	0.80	0.242	1.86	60′	I	29.5	
	NS	SNAGGRS4	34.925		1.58	55.18	1.4	0.242	1.70	60′	I	29.5	0.600	2.75	60′	I	5.9	0.80	0.242	1.58	60′	I	29.5	
		SNS5A	35.550		1.55	55.10	1.4	0.242	1.66	60′	I	29.5	0.600	2.80	60′	I	5.9	0.80	0.242	1.55	60′	I	29.5	
		SNS6A	39.950		1.43	57.13	1.4	0.242	1.54	60′	I	29.5	0.600	2.55	60′	I	5.9	0.80	0.242	1.43	60′	I	29.5	
LEGAL		SNS7B	42.000		1.36	57.12	1.4	0.242	1.46	60′	I	29.5	0.600	2.53	60′	I	5.9	0.80	0.242	1.36	60′	I	29.5	
LOAD RATING		TNAGRIT3	33.000		1.75	57.75	1.4	0.242	1.88	60′	I	29.5	0.600	3.05	60′	I	5.9	0.80	0.242	1.75	60′	I	29.5	
NATING		TNT4A	33.075		1.76	58.21	1.4	0.242	1.89	60′	I	29.5	0.600	2.97	60′	I	5.9	0.80	0.242	1.76	60′	I	29.5	
		TNT6A	41.600		1.45	60.32	1.4	0.242	1.56	60′	I	29.5	0.600	2.77	60′	I	5.9	0.80	0.242	1.45	60′	I	29.5	
	S	TNT7A	42.000		1.46	61.32	1.4	0.242	1.57	60′	I	29.5	0.600	2.63	60′	I	5.9	0.80	0.242	1.46	60′	I	29.5	
		TNT7B	42.000		1.53	64.26	1.4	0.242	1.64	60′	I	29.5	0.600	2.45	60′	I	5.9	0.80	0.242	1.53	60′	I	29.5	
		TNAGRIT4	43.000		1.44	61.92	1.4	0.242	1.55	60′	I	29.5	0.600	2.37	60′	I	5.9	0.80	0.242	1.44	60′	I	29.5	
		TNAGT5A	45.000		1.36	61.20	1.4	0.242	1.46	60′	I	29.5	0.600	2.38	60′	I	5.9	0.80	0.242	1.36	60′	I	29.5	
		TNAGT5B	45.000	3	1.33	59.85	1.4	0.242	1.43	60′	I	29.5	0.600	2.24	60′	I	5.9	0.80	0.242	1.33	60′	I	29.5	

LOAD FACTORS:

	DESIGN LOAD RATING	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
		STRENGTH I	1.25	1.50
	FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2.

3.

4.

(#) CONTROLLING LOAD RATING

 $\langle 1 \rangle$ DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.2.R.80

LENOIR COUNTY

STATION: 23+09.49 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

LRFR SUMMARY FOR 60' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

S-3

TOTAL SHEETS 13

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



MI ENGINEERING

1011 SCHAUB DRIVE, SUITE 100
RALEIGH, NC 27606
(919) 851-6606
FIRM PE NUMBER: P-0671

2

 NG
 REVISIONS

 SUITE 100
 NO.
 BY:
 DATE:
 NO.
 BY:

 3
 3
 3
 3
 3

123

<u>LRFR SUMMARY</u>

FOR SPAN 'A'

11:55:48 AM User: blanning Filename: P:\NC Bri

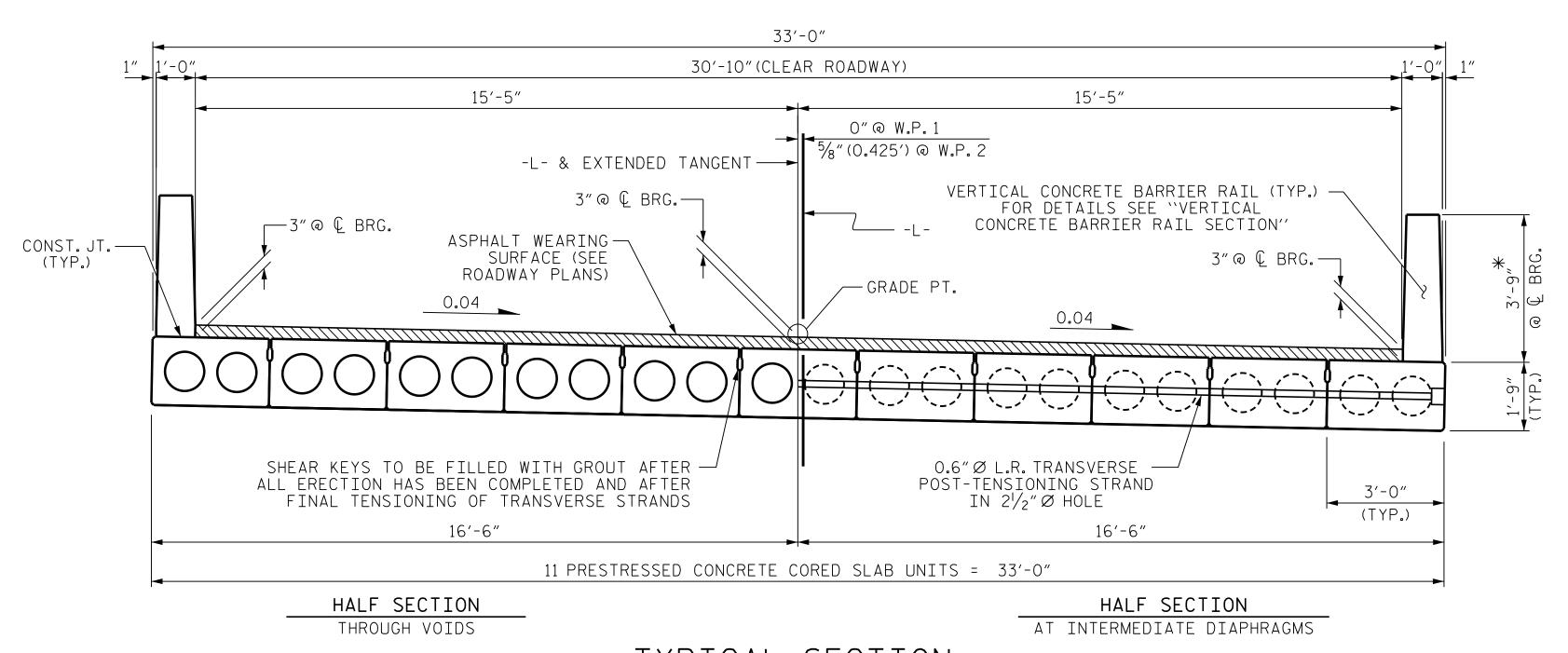
DRAWN BY: J.I. BREWER

CHECKED BY: B.E. ATKINSON

DATE: 01/18

DESIGN ENGINEER OF RECORD: B.E. ATKINSON

DATE: 01/18



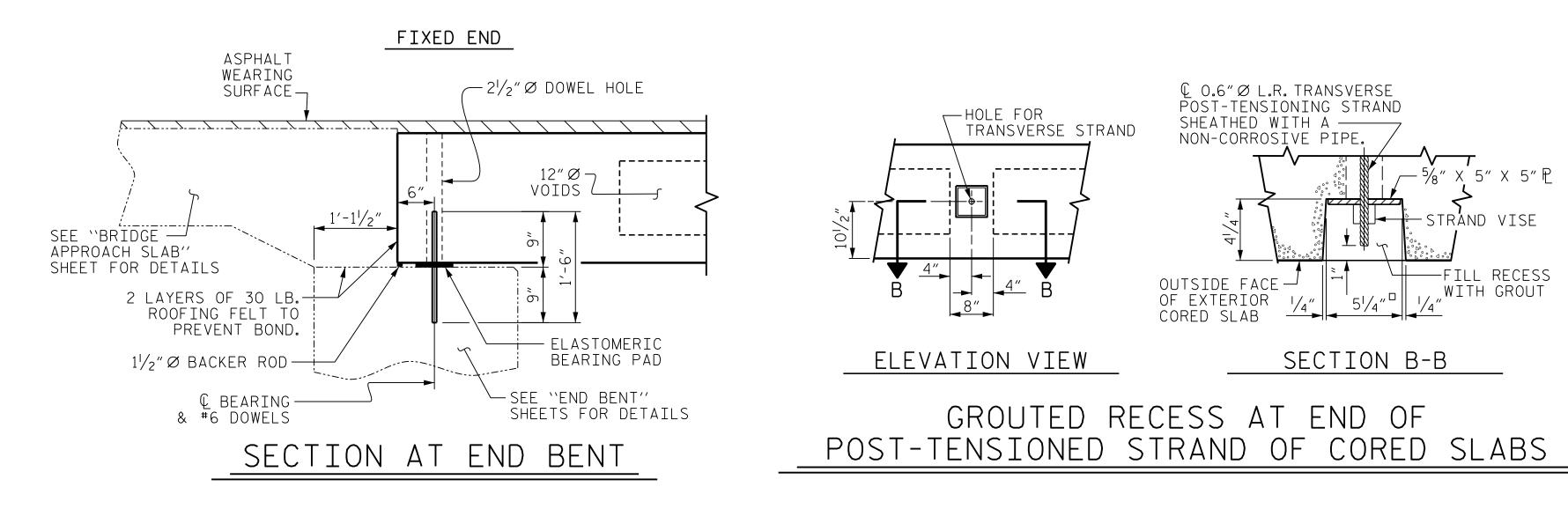
TYPICAL SECTION

*-THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

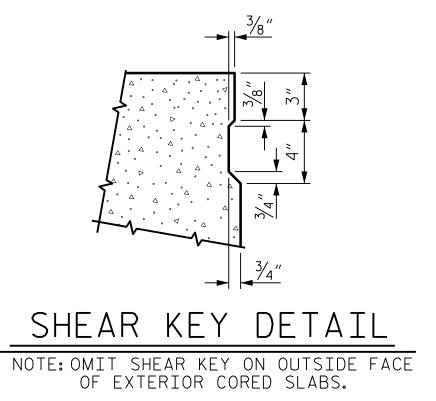
PERMITTED THREADED INSERT CAST IN OUTSIDE FACE OF EXTERIOR UNIT AND RECESSED 3/8". SIZE TO BE DETERMINED BY CONTRACTOR.

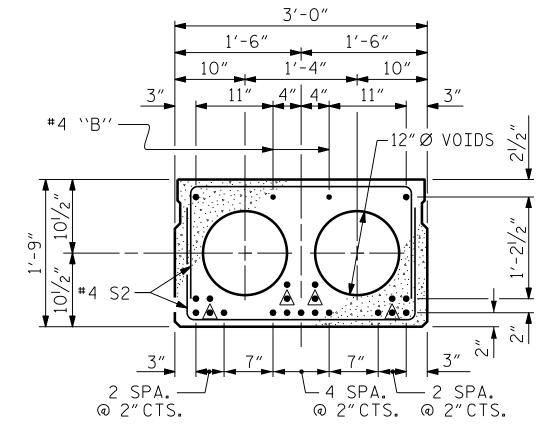
THREADED INSERT DETAIL

01/18



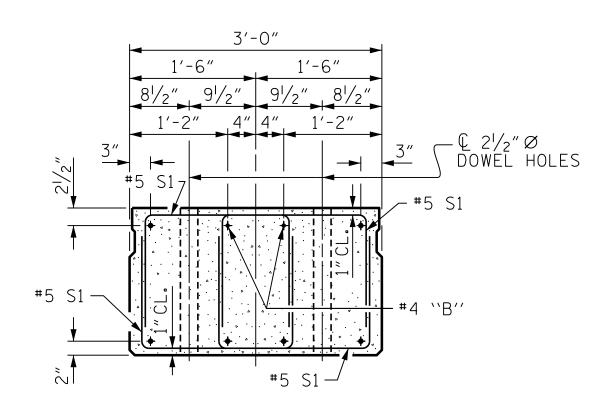






INTERIOR SLAB SECTION (60'UNIT) (21 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT



END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.) INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

3'-0" 1'-4" 10" $3\frac{3}{8}$ " CL. 12″Ø VOIDS →

EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

PROJECT NO. 17BP.2.R.80 LENOIR COUNTY

23+09.49 -L-STATION:

SHEET 1 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

SHEET NO

S-4

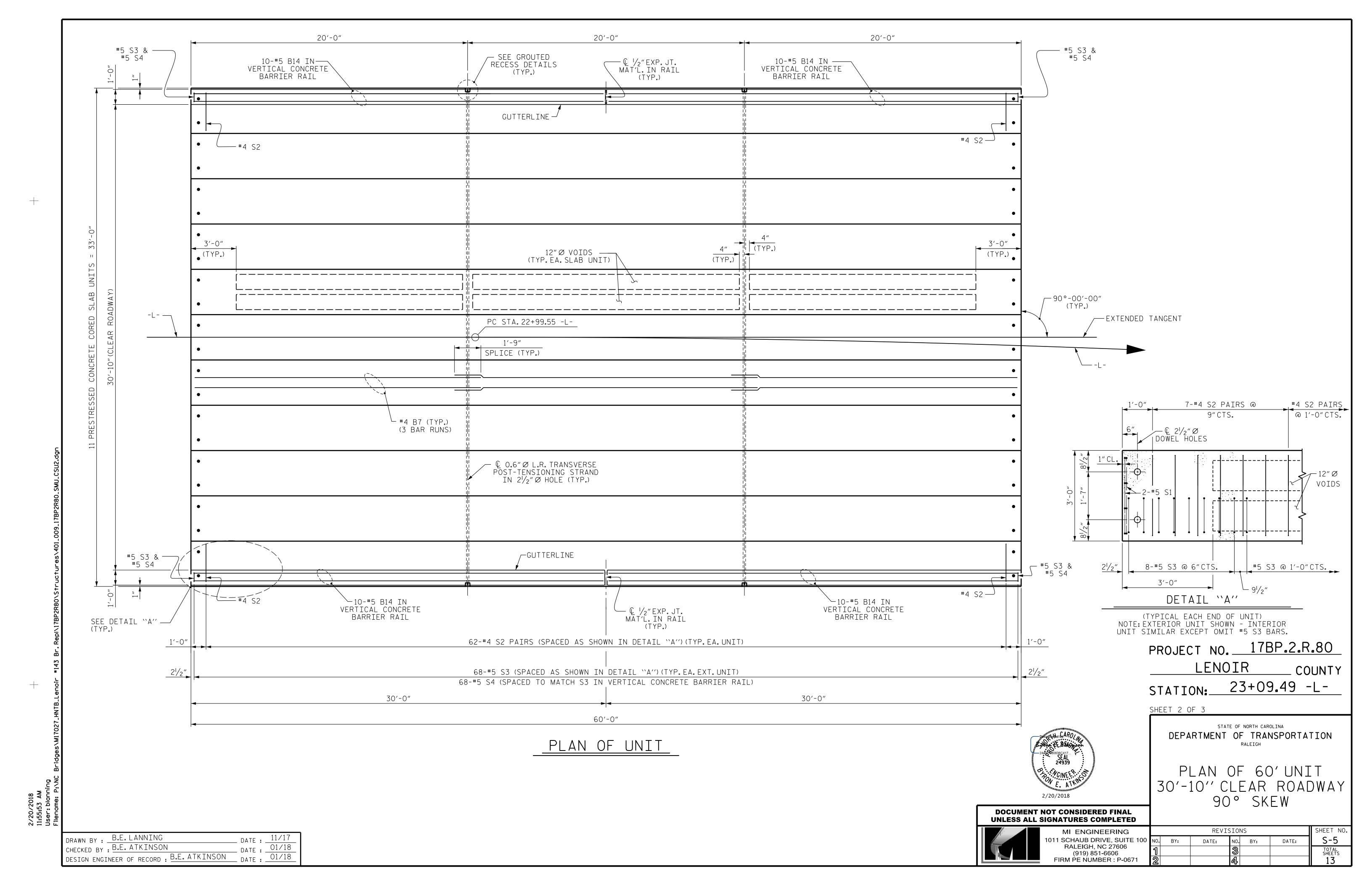
TOTAL SHEETS

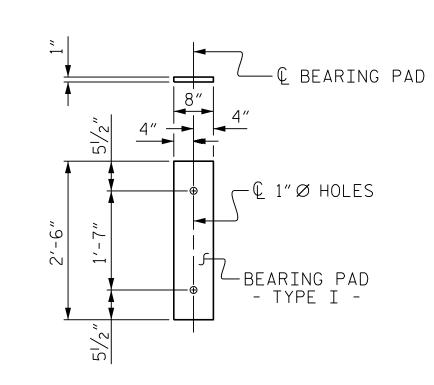
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

MI ENGINEERING

REVISIONS BY: DATE: NO. BY: DATE:

DRAWN BY : B.E. LANNING CHECKED BY : B.E. ATKINSON __ DATE : _ DATE: 01/18 DESIGN ENGINEER OF RECORD : B.E. ATKINSON





FIXED END (TYPE I - 22 REQ'D.)

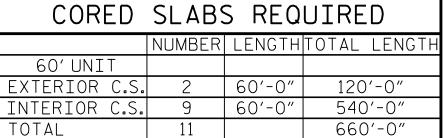
ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
60'UNITS	1"/16"	3'-7"/16"

1'-0"

10"



DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
60'CORED SLAB UNIT	0.6"Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 7⁄8″ 🕴
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD **	%6″ ♦
FINAL CAMBER	15⁄ ₁₆ ″ ∤

(SQUARE INCHES) ULTIMATE STRENGTH

(LBS.PER STRAND

APPLIED PRESTRESS

(LBS.PER STRAND)

GRADE 270 STRANDS

0.6" Ø L.R.

0.217

58,600

43,950

+ +	INCLUDES	FUTURE	WEARING	SURFACE	

6"
2 3'-4"
73/4"
<u>S5</u>
TTO OUT.
Į

BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL							
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT	
	60' UNIT						
 ₩B14	40	40	#5	STR	29'-7"	1234	
* S4	136	136	#5	2	7′-2″	1017	
* EPOXY COATED REINFORCING STEEL LBS. 22						2251	
CLASS AA CONCRETE CU.YDS. 1						15.4	
TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT. 120						120.25	

BILL OF MATERIAL FOR ONE 60'CORED SLAB UNIT							
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
В7	6	#4	STR	21'-2"	85	21'-2"	85
S1	8	#5	3	4'-3"	35	4'-3"	35
S2	124	#4	3	5′-4″	442	5′-4″	442
* S3	68	#5	1	5′-7″	396		
REINFORCING STEEL LBS.			Š.	562		562	
* EPOXY COATED							
REINFORCING STEEL LBS. 396							
6500	P.S.I. CO	NCRETE	CU. YDS) <u>.</u>	8.5		8.5
0.6"Ø	L.R. STR	ANDS	No) .	21		21

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ " \alpha DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

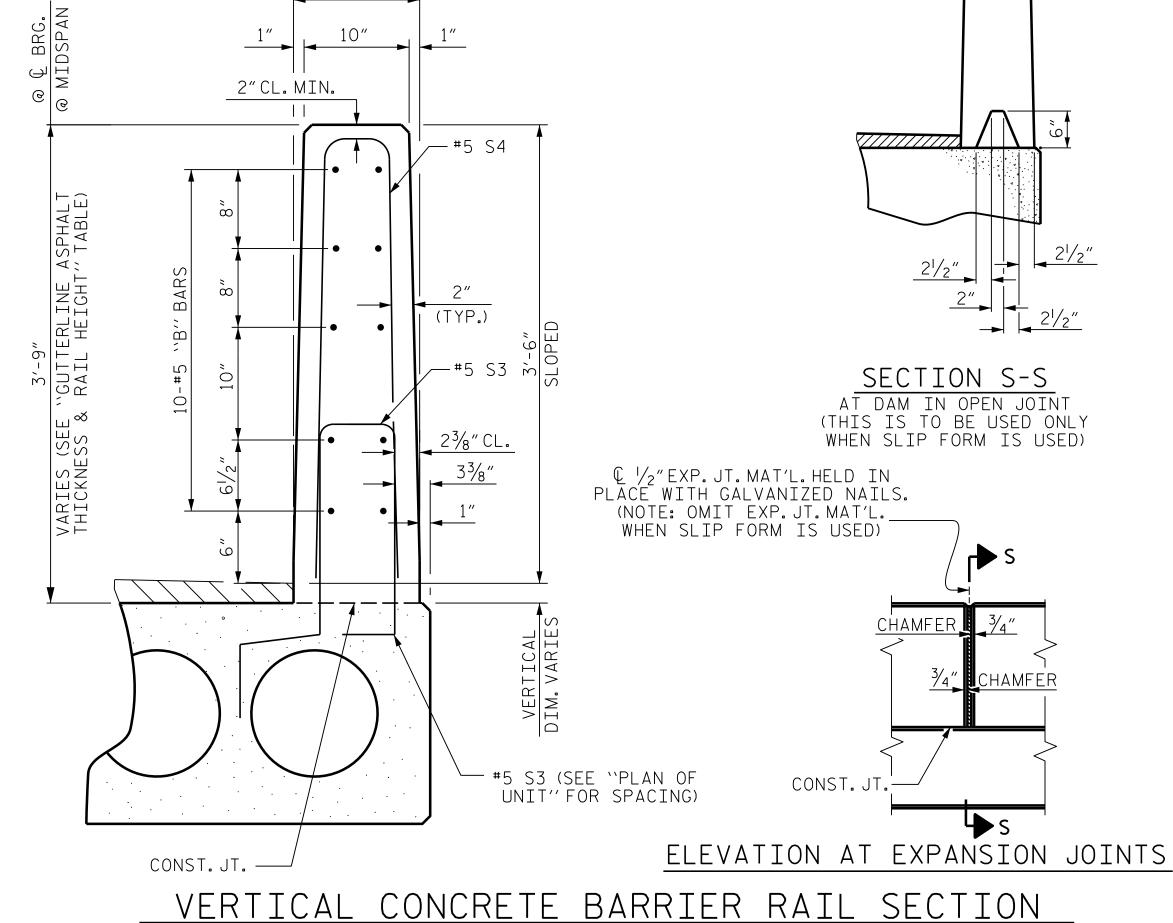
FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

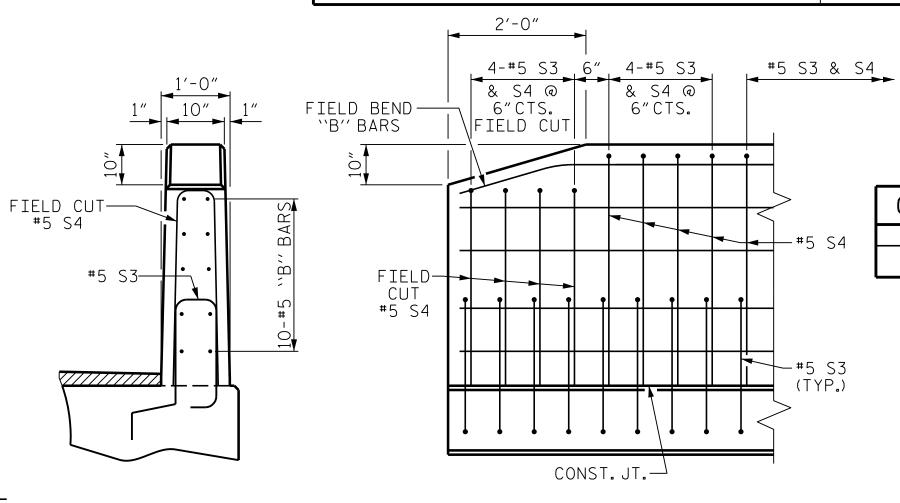
THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

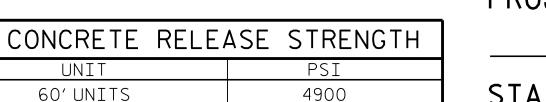




END VIEW

SIDE VIEW

END OF RAIL DETAILS



PROJECT NO. <u>17BP.2.R.80</u> LENOIR COUNTY

23+09.49 -L-STATION:

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0'' X 1'-9'' PRESTRESSÉD CONCRETE CORED SLAB UNIT 90° SKEW

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



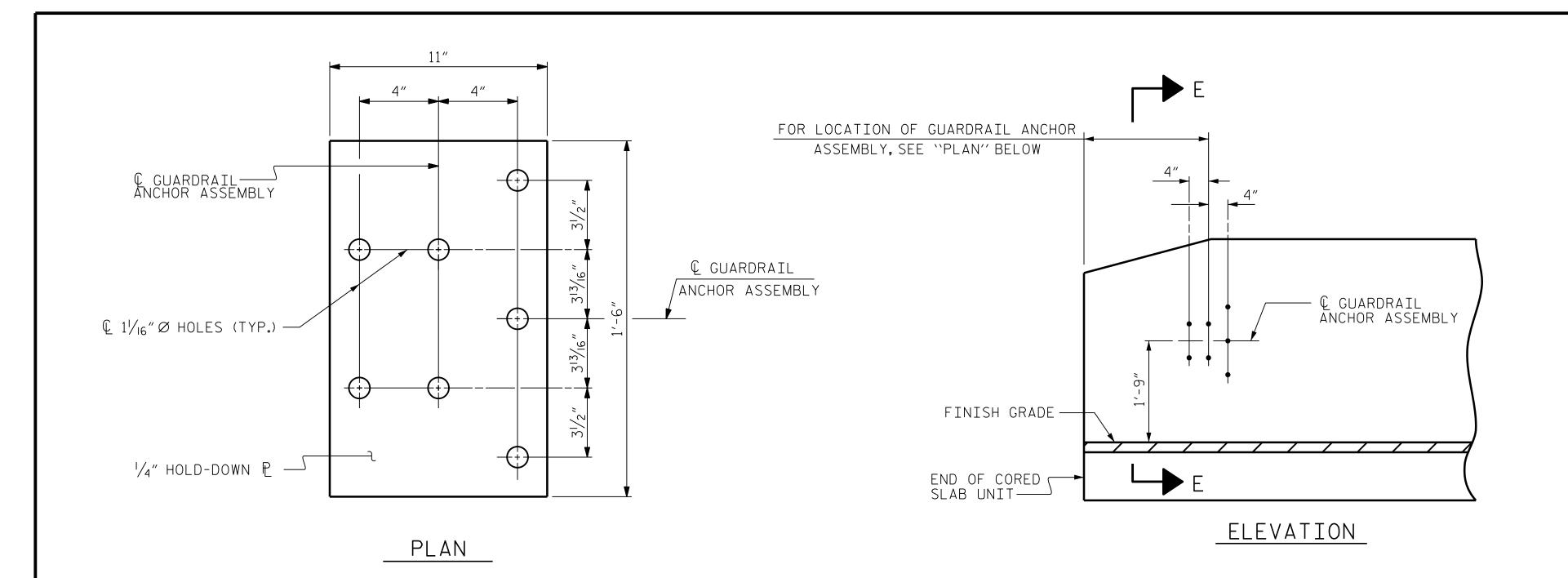
UNIT

60'UNITS

MI ENGINEERING 1 SCHAUB DRIVE, SUITE 100 No. RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

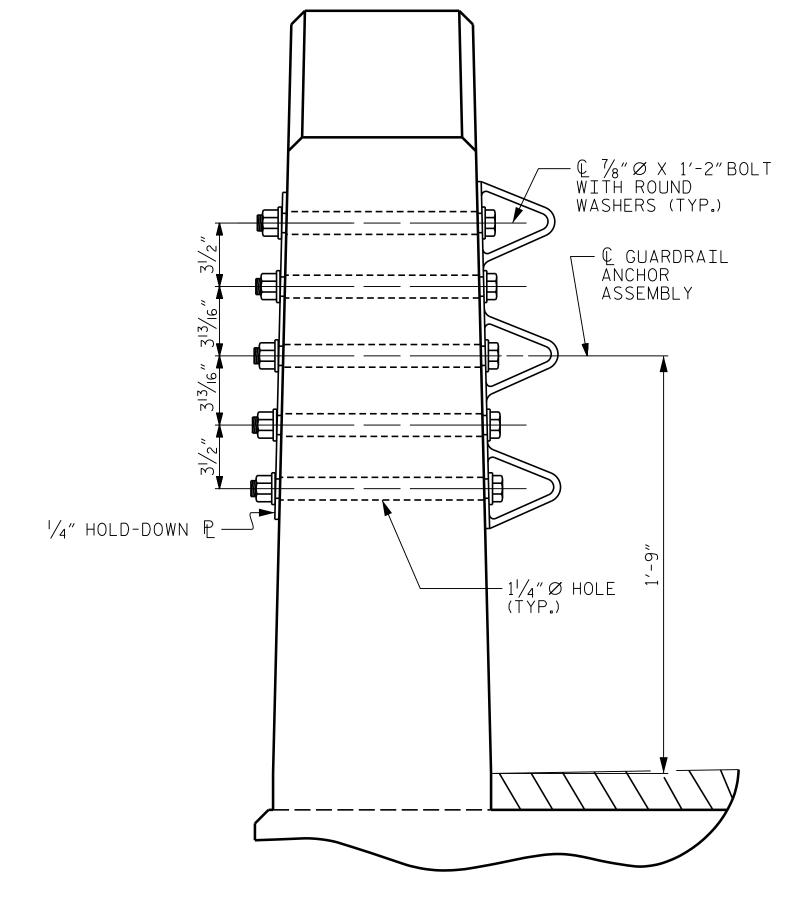
SHEET NO REVISIONS S-6 NO. BY: TOTAL SHEETS

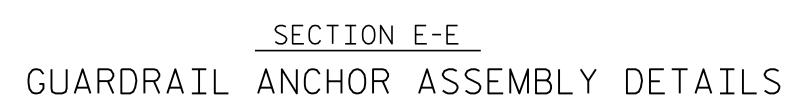
B.E. LANNING CHECKED BY : B.E. ATKINSON __ DATE : 01/18 DATE: 01/18 DESIGN ENGINEER OF RECORD : B.E. ATKINSON

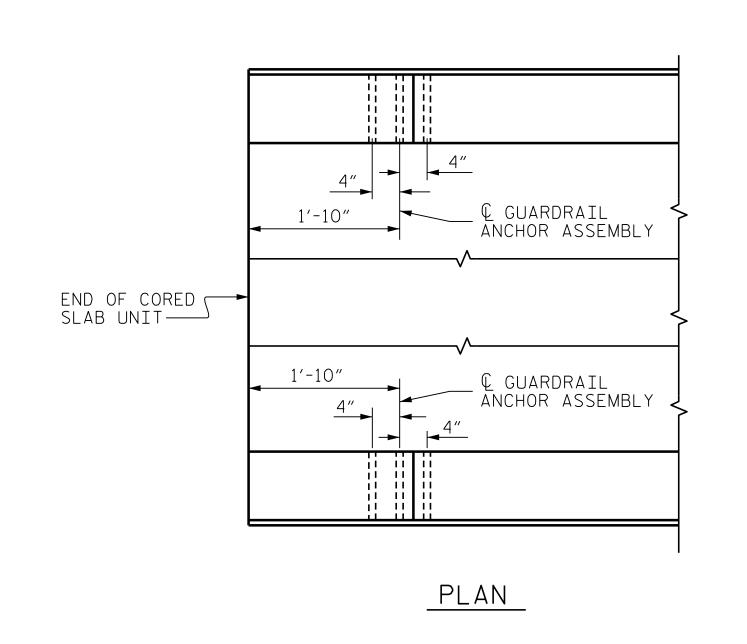


NOTES

- THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD DOWN PLATE AND 7 1/8" Ø BOLTS WITH NUTS AND WASHERS.
- THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.
- BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE \(\frac{1}{8}'' \) \(\text{\sigma} \) GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.
- AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.
- THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.
- THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.
- THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

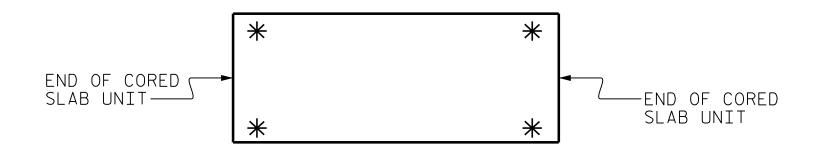






LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.2.R.80

LENOIR COUNTY

STATION: 23+09.49 -L-



DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE

DETAILS

FOR VERTICAL CONCRETE

BARRIER RAIL

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



MI ENGINEERING
011 SCHAUB DRIVE, SUITE 100
RALEIGH, NC 27606
(919) 851-6606
FIRM PE NUMBER: P-0671

REVISIONS

NO. BY: DATE: NO. BY: DATE: S-7

1 3 TOTAL SHEETS
2 4 13

T 1) STD. NO. GRA3

11:55:57 AM User: blanning Filename: P:\NC Bridges`

ASSEMBLED BY: B.E. LANNING DATE: 11/17
CHECKED BY: B.E. ATKINSON DATE: 01/18
DESIGN ENGINEER
OF RECORD: B.E. ATKINSON DATE: 01/18

DRAWN BY: MAA 5/10

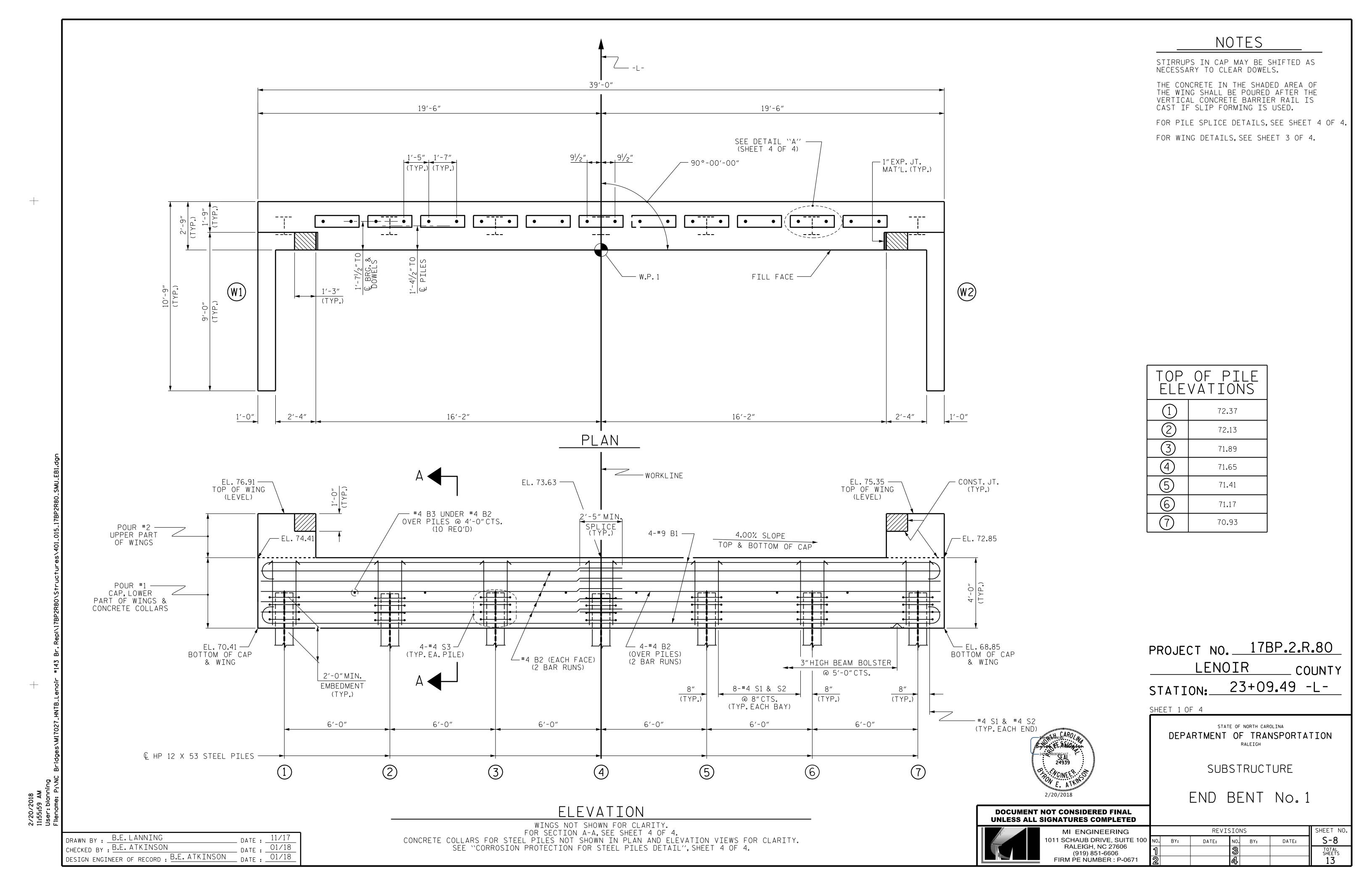
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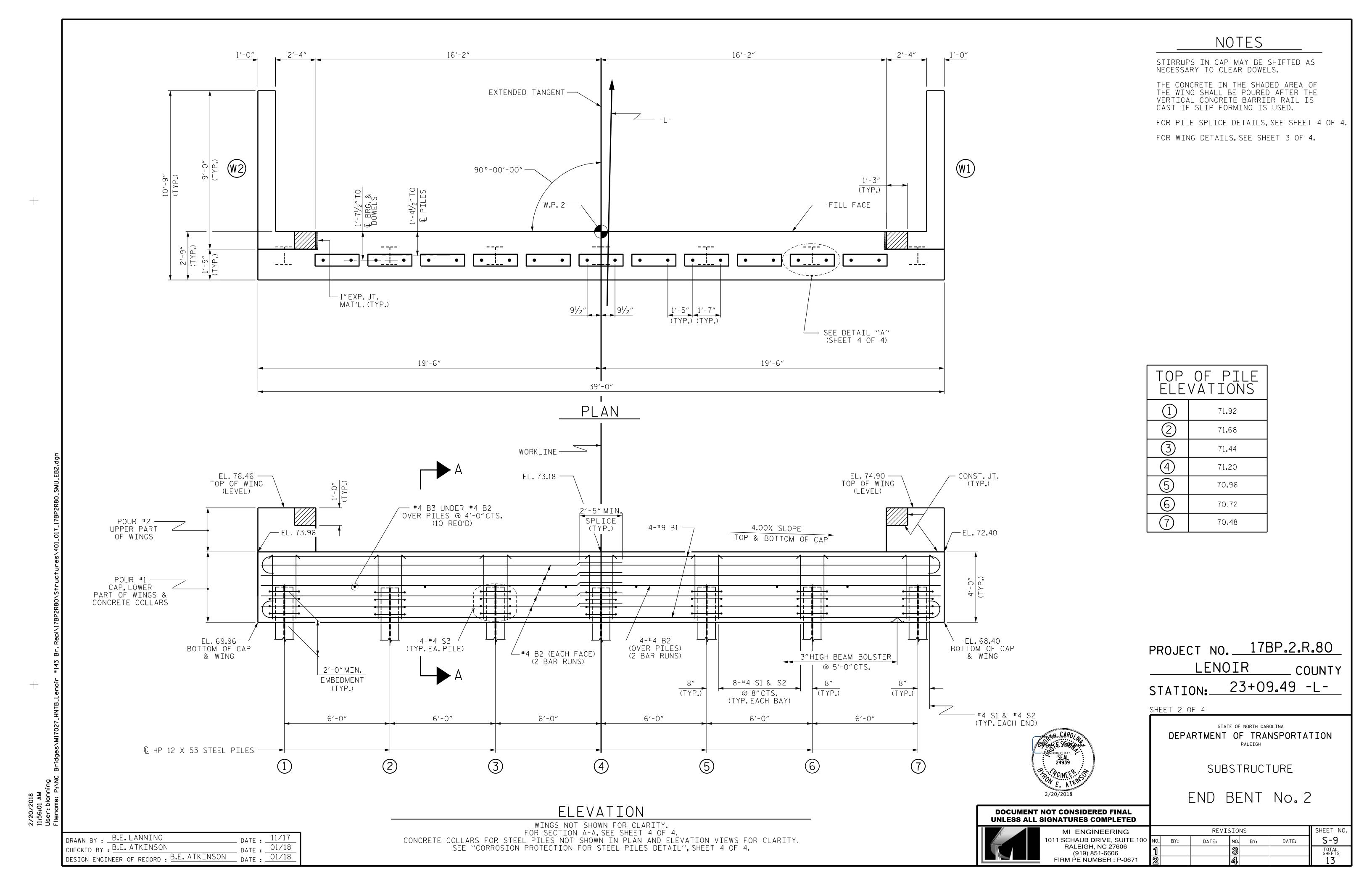
MAA/GM

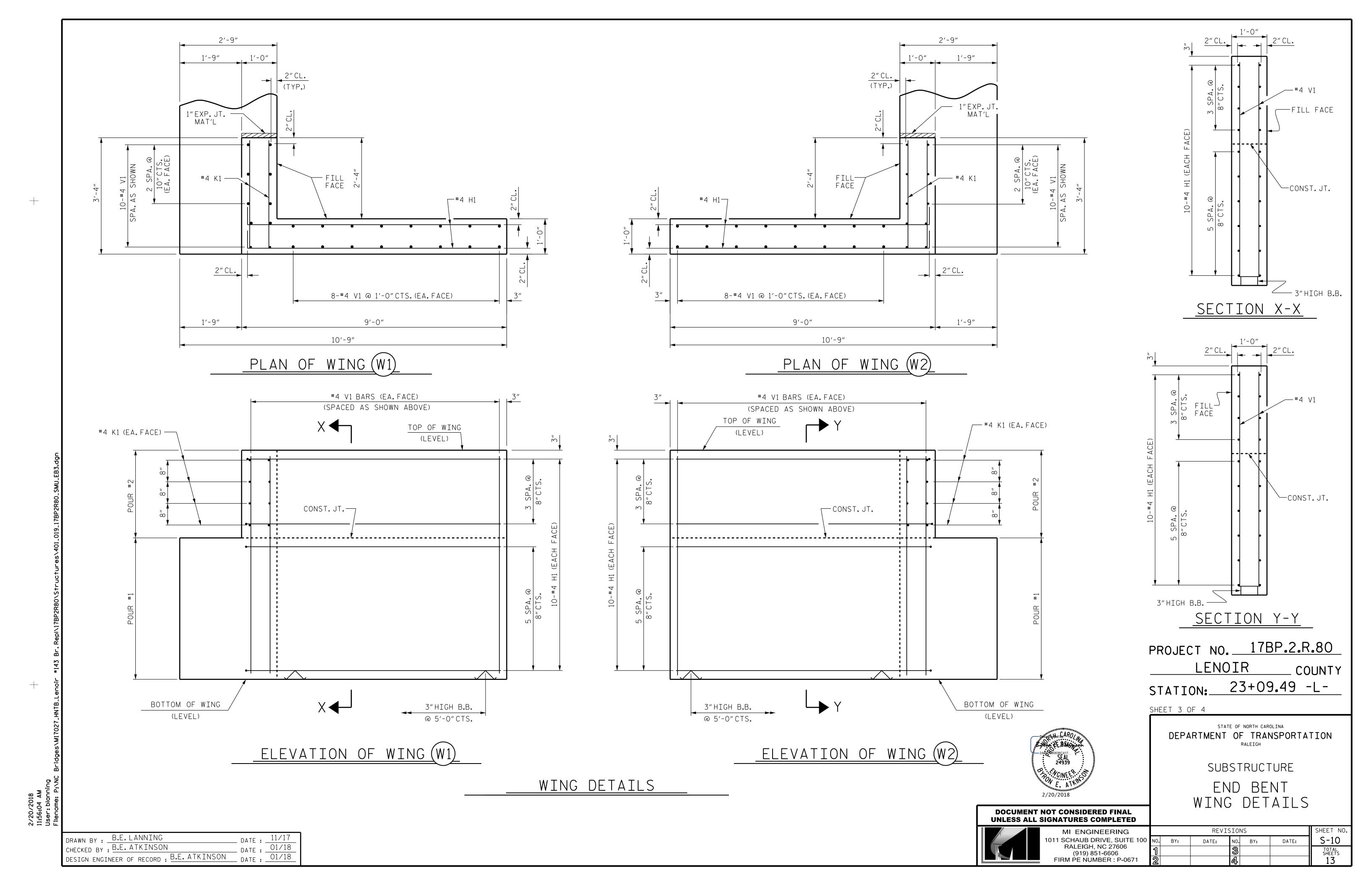
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MAA/TMC

MAA/THC





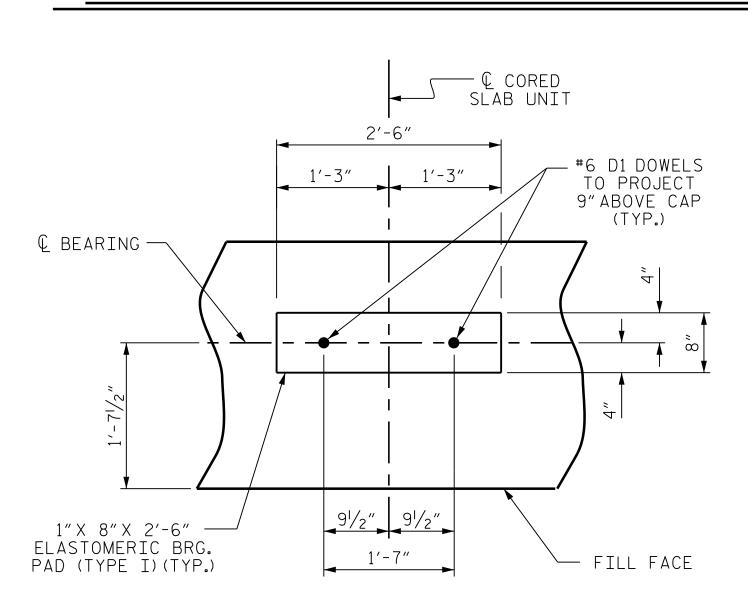


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

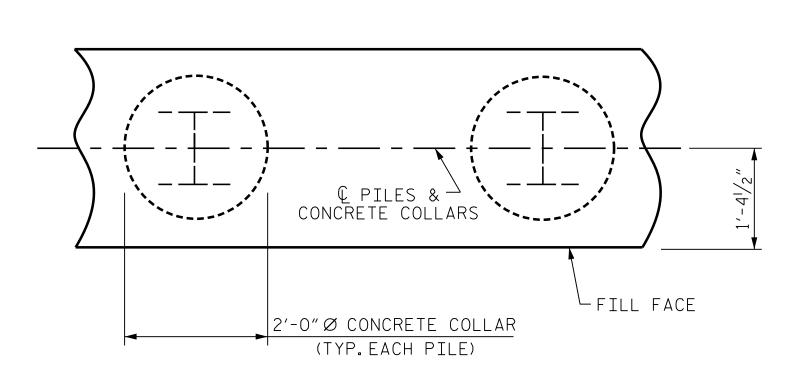
NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

DETAIL "A"

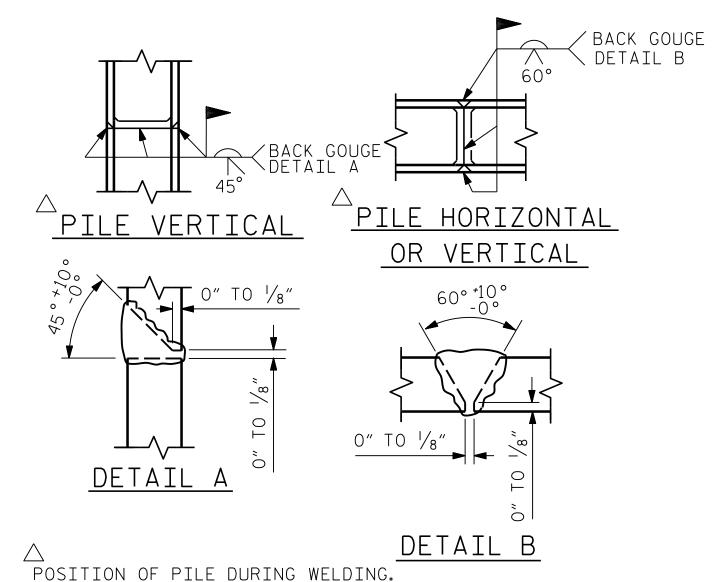


PROTECTION FOR STEEL PILES DETAIL

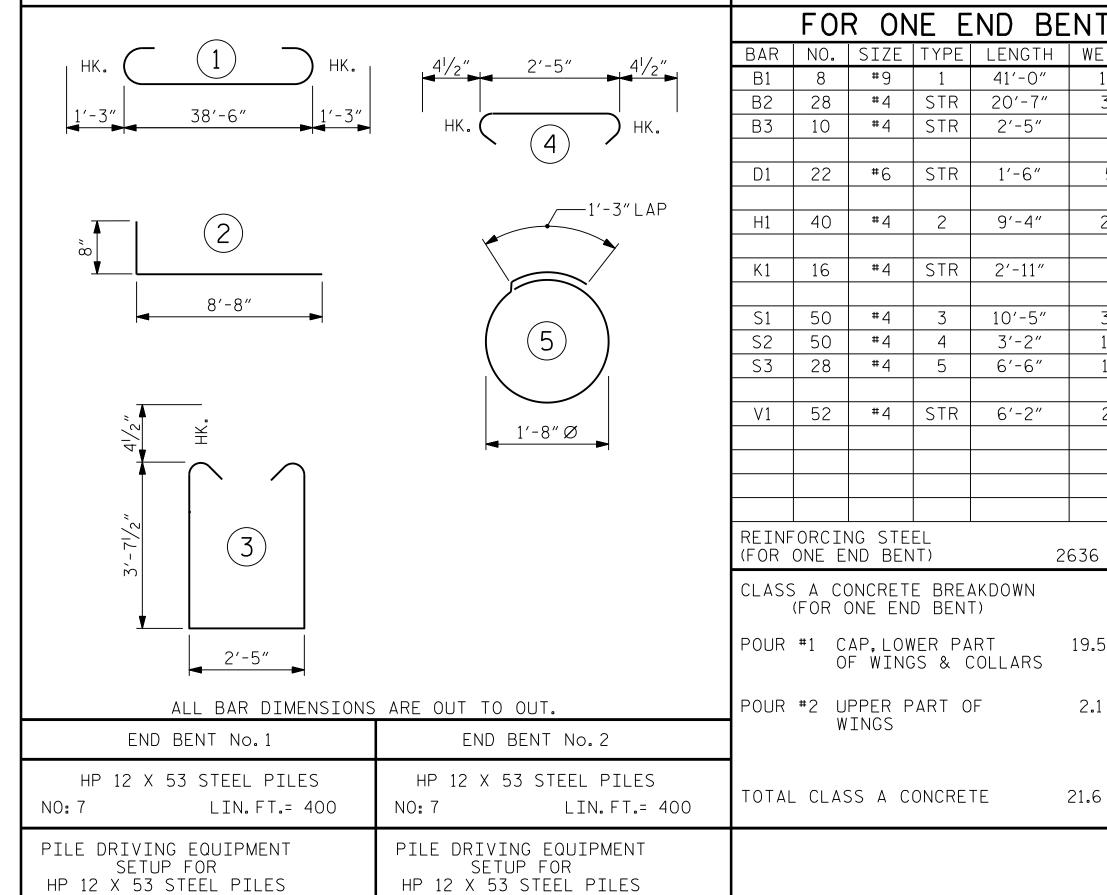
(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

DRAWN BY: B.E. LANNING CHECKED BY : B.E. ATKINSON __ DATE : 01/18 _ DATE : 01/18 DESIGN ENGINEER OF RECORD : B.E. ATKINSON

PLAN



PILE SPLICE DETAILS



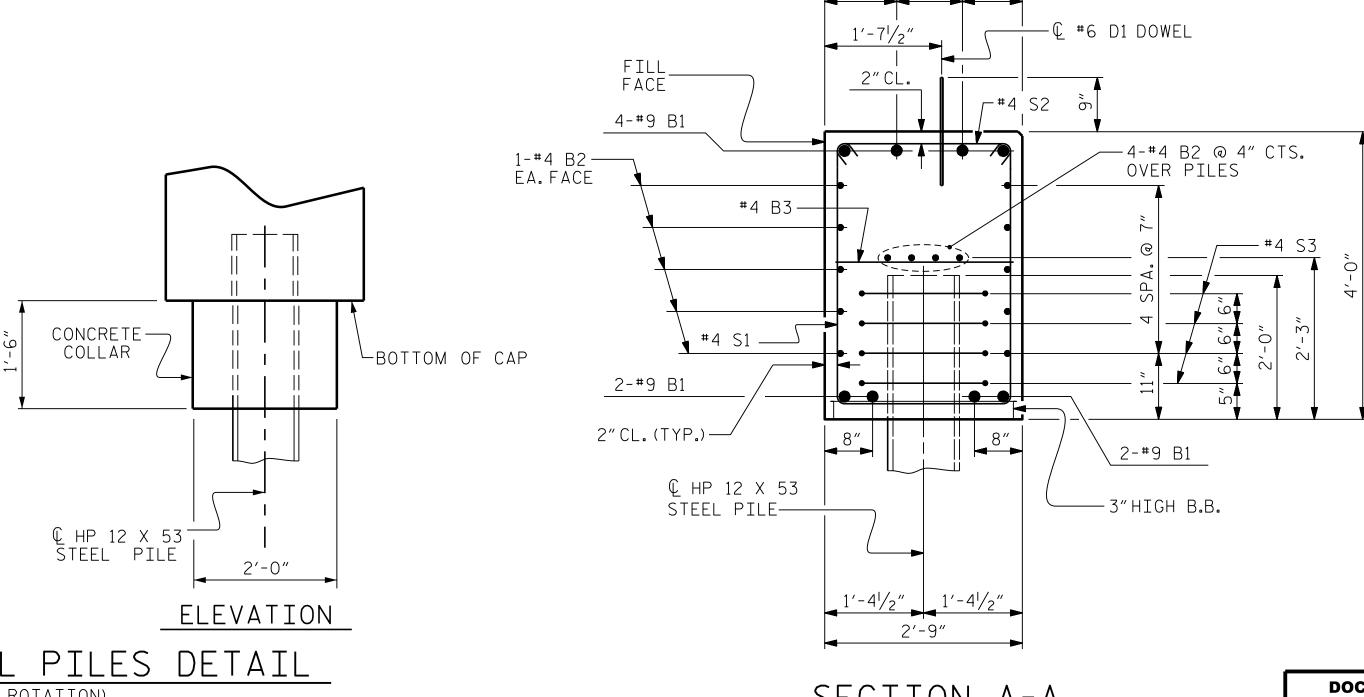
PILE REDRIVES

BAR TYPES

NO: 7

NO: 7

PILE REDRIVES



PROJECT NO. 17BP.2.R.80 LENOIR

23+09.49 -L-STATION:

SHEET 4 OF 4

NO: 7

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED MI ENGINEERING

11 SCHAUB DRIVE, SUITE 100 NO. RALEIGH, NC 27606 (919) 851-6606 FIRM PE NUMBER : P-0671

SHEET NO REVISIONS S-11 NO. BY: BY: DATE: DATE: TOTAL SHEETS 13

SECTION A-A

(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

9′-4″ #4 | STR | 2'-11" 10′-5″ 3′-2″ 6′-6″ #4 | STR | 6′-2″

31 348 106 122 214

BILL OF MATERIAL

LENGTH | WEIGH

1115

385

16

50

249

2.1 C.Y.

21.6 C.Y

COUNTY

41′-0″

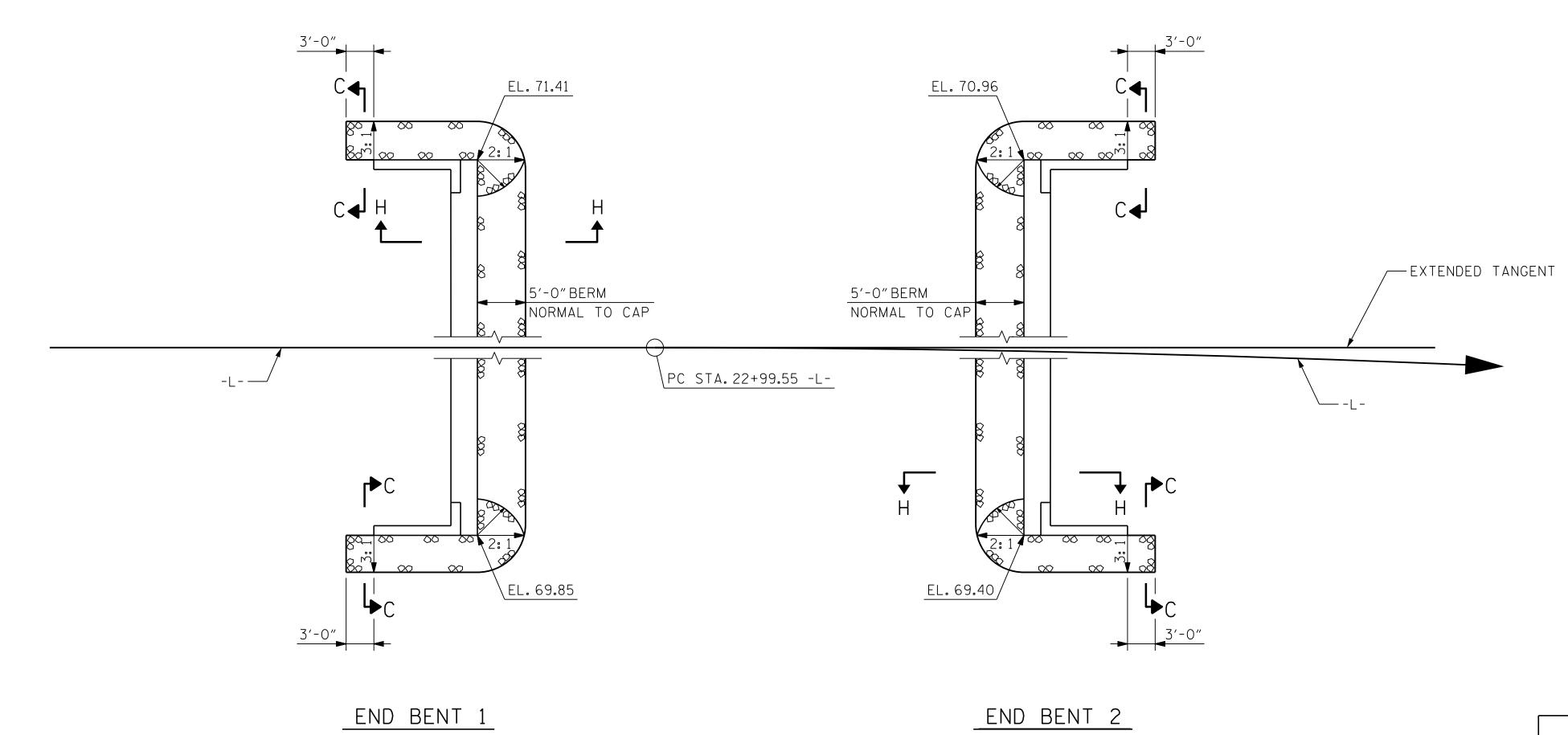
20′-7″

2′-5″

2636 LBS

19.5 C.Y. OF WINGS & COLLARS

TOTAL CLASS A CONCRETE



PLAN

ESTIMATED QUANTITIES					
BRIDGE @ STA. 23+09.49 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
END BENT 1	52	38			
END BENT 2	52	38			

SHOULDER LINE

SHOULDER LINE

5'-0" BERM

NORMAL

TO CAP

GEOTEXTILE

SHOULDER LINE

EL. 71.41 (LT. @ END BENT 1)

EL. 69.85 (RT. @ END BENT 1)

EL. 70.96 (LT. @ END BENT 2)

EL. 70.96 (LT. @ END BENT 2)

EL. 69.40 (RT. @ END BENT 2)

SECTION H-H

SECTION C-C

PROJECT NO. 17BP.2.R.80

LENOIR COUNTY

STATION: 23+09.49 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

RIP RAP DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



REVISIONS

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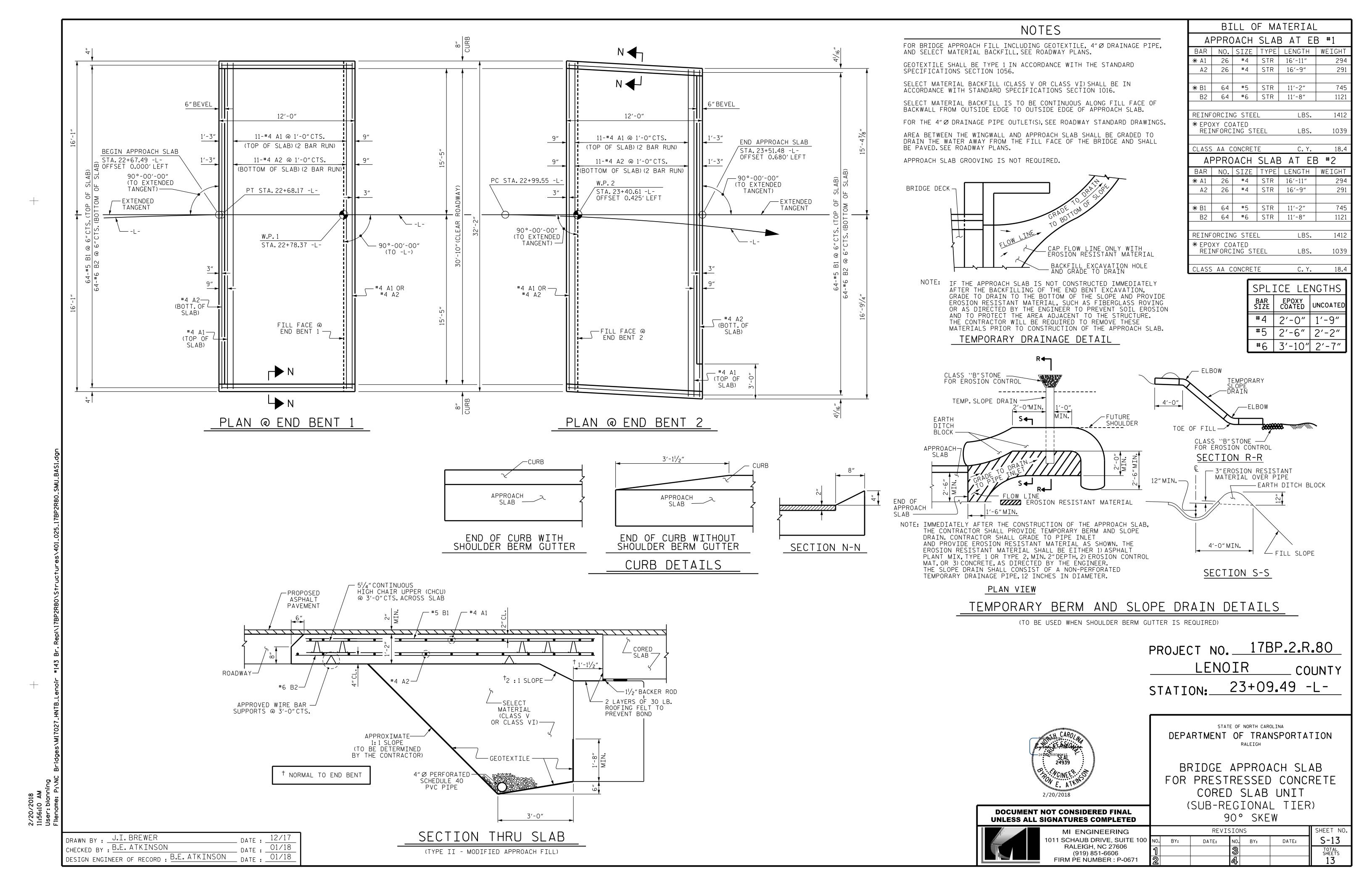
1 3 TOTAL SHEETS
2 4 13

DRAWN BY: B.E. LANNING
CHECKED BY: B.E. ATKINSON
DESIGN ENGINEER OF RECORD: B.E. ATKINSON
DATE: 01/18

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| |7027_HNTB_Lenoir #14

:/20/2010 1:56:08 AM Jser: blanning Tlename: P:\NC Bridge



DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O.(CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT,

ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ " Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.