

	STATE	STA	TE PROJECT REFERENCE NO.		SHEET NO.	TOTAL SHEETS
	$\mathbb{N}.\mathbb{C}.$	В	P2.R018.1		11	
	STAT	E PROJ. NO.	F. A. PROJ. NO.		DESCRIPT	TION
	BP2	.R018.1		-	PE	
	BP2.	R018.2			W, UTIL	
		<u>. NU 10.3</u>			NOIKU	
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SHEETS						
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WORK QUANTITY						
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NAD 83/2011						
<u>END PRO</u> /-L- STA 1	<u>JJECT</u> 7+86.00					
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			TO SR 1779	) RD		
			2/4/11/10/11/2			
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EEER 1022 NOTESS / OFFESS / OFFES			DOCUMENT UNLESS ALL	NOT CC	NSIDEREE JRES COM	) FINAL PLETED
EER 1022 NOT HESS / OFFESS / O			DOCUMENT UNLESS ALL	NOT CC	NSIDEREE JRES COM	D FINAL PLETED
EER 1022 NO22 NO22 NO10 FESS/04 SEAL 049338 NG INE P. AAOLANNA			DOCUMENT UNLESS ALL	- NOT CC SIGNATU	NSIDEREE JRES COM	D FINAL PLETED
EER 1022 NOT H CARO/ SEAL 049338 NG INEER			DOCUMENT UNLESS ALL		NSIDEREE JRES COM	D FINAL PLETED
EER 2022 COLOR FESSION SEAL 049338 VGINEER CGINEER 2022 COLOR FESSION P. AADUMUM P. AADUMUM P. AADUMUM CGINEER 2022 COLOR FESSION P. AADUMUM			DOCUMENT UNLESS ALL		NSIDEREE JRES COM	O FINAL PLETED
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INDEX OF SHE	ETS	ENERAL NOTI
SHEET NUMBER	SHEET	
1	TITLE SHEET	GRADE LINE:
1A	INDEX OF SHEETS, GENERAL NOTES, AND STANDARD DRAWINGS	
1B	CONVENTIONAL SYMBOLS	SURFAC
2A-1	PAVEMENT SCHEDULE AND TYPICAL SECTIONS	ENGINE
2C-1 THRU 2C- 3	SPECIAL DETAILS	CLEARING:
3B-1	ROADWAY SUMMARIES	
3D-1	DRAINAGE SUMMARIES	
3G-1	GEOTECHNICAL SUMMARIES	
4	PLAN SHEET	STD. NO
4A	RIGHT OF WAY AND EASEMENT MONUMENT PLACEMENT	SECTION
5	PROFILE SHEET	SHOULDER C
RW02C-1 THRU RW04	RIGHT OF WAY SHEETS	ASPHALT SUPERE
TMP-1 THRU TMP-2	TRAFFIC MANAGEMENT PLANS	
PMP-1	PAVEMENT MARKING PLANS	SUBSUR
EC-1 THRU EC-5	EROSION CONTROL PLANS	LOCATIC
RF-1	REFORESTATION PLANS	GUARDRAIL:
UO-1 THRU UO- 2	UTILITIES BY OTHERS PLANS	
X-1 THRU X-5	CROSS-SECTIONS	WITH TH
S-1 THRU S-21	STRUCTURE PLANS	TEMPORARY

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC WILL BE PAID FOR AS "EXTRA WORK" IN ACCORDANCE WITH SECTION 104-7.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

**RIGHT-OF-WAY MARKERS:** 

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY CONTRACT.

ES:

2018 SPECIFICATIONS EFFECTIVE: 01-16-2018 **REVISED**:

#### ID SURFACING:

RADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED CING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE FED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE EER IN ORDER TO SECURE A PROPER TIE-IN.

NG ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY D II.

#### ATION:

RVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. ELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL NS.

#### CONSTRUCTION:

LT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF ELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

E DRAINS:

RFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.02 AT ONS DIRECTED BY THE ENGINEER.

ARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING RUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT HE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

**/ SHORING:** 

REV.

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch -N. C. Department of Transportation - Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

TITLE STD.NO. **DIVISION 2 - EARTHWORK** 200.02 Method of Clearing - Method II 225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement **DIVISION 3 - PIPE CULVERTS** 300.01 Method of Pipe Installation **DIVISION 4 - MAJOR STRUCTURES** 422.02 Bridge Approach Fills - Type II Modiefied Approach Fill **DIVISION 5 - SUBGRADE, BASES AND SHOULDERS** 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I **DIVISION 8 - INCIDENTALS** 815.02 Subsurface Drain 840.00 Concrete Base Pad for Drainage Structures 840.25 Anchorage for Frames - Brick or Concrete or Precast 840.35 Traffic Bearing Grated Drop Inlet - for Cast Iron Double Frame and Grates 840.66 Drainage Structure Steps 846.01 Concrete Curb, Gutter and Curb & Gutter 846.02 Drop Inlet Installation in Expressway Gutter 862.01 Guardrail Placement 862.02 Guardrail Installation 862.03 Structure Anchor Units 876.02 Guide for Rip Rap at Pipe Outlets

ADDITIONAL NOTES

AND JOINTS FOR SEALING THE SAW CUT JOINTS AT THE BEGIN/END PROJECT LIMITS.

EFF. 01-16-2018

#### 2018 ROADWAY ENGLISH STANDARD DRAWINGS





State Line

# STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS CONVENTIONAL PLAN SHEET SYMBOLS

\_\_\_\_\_

## **BOUNDARIES AND PROPERTY:**

Note: Not to Scale

County Line ————————————————————————————————————	
Township Line	
City Line ————————————————————————————————————	
Reservation Line	· ·
Property Line	
Existing Iron Pin (EIP)	€IP
Computed Property Corner —	×
Existing Concrete Monument (ECM) ———	ECN
Parcel/Sequence Number	(123)
Existing Fence Line	_×××-
Proposed Woven Wire Fence	<b>.</b>
Proposed Chain Link Fence	
Proposed Barbod Wire Eence	
Evisting Wetland Boundary	
Existing wetland boundary	wlb
Proposed Wetland Boundary	
Existing Endangered Animal Boundary ———-	ЕАВ ————
Existing Endangered Plant Boundary	ЕРВ ————
Existing Historic Property Boundary	HPB
Known Contamination Area: Soil —————	
Potential Contamination Area: Soil ————	
Known Contamination Area: Water	
Potential Contamination Area: Water	
Contaminated Site: Known or Potential ——	XX XX
BUILDINGS AND OTHER CULTU	RE:
Gas Pump Vent or U/G Tank Cap	0
Gas Pump Vent or U/G Tank Cap Sign	O O
Gas Pump Vent or U/G Tank Cap Sign Well	O S S
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine	O ♀ ♀ ☆
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation	○ § ₽ ★
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline	○ ⓒ ☞ ☆
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery	○ ♀ ₩ ★ ↓
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Foundation Area Outline Cemetery Building School Church Dam	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Foundation Area Outline Cemetery Building School Church Dam Foundation Stream or Body of Water	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Foundation Area Outline Cemetery Building School Church Dam HYDROLOGY: Stream or Body of Water	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam <i>HYDROLOGY:</i> Stream or Body of Water Hydro, Pool or Reservoir	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Foundation Area Outline Cemetery Building School School Church Dam HYDROLOGY: Stream or Body of Water Stream or Body of Water Hydro, Pool or Reservoir Suffer Zene 1	
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam Dam HYDROLOGY: Stream or Body of Water Stream or Body of Water Hydro, Pool or Reservoir Jurisdictional Stream Buffer Zone 1 Buffer Zone 2	○ ♀ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓
Gas Pump Vent or U/G Tank Cap	O S S C C C C C C C C C C C C C C C C C
Gas Pump Vent or U/G Tank Cap	O S S C C C C C C C C C C C C C C C C C
Gas Pump Vent or U/G Tank Cap	O S S C T S C T S C S C S C S C S C S C S
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Gas Pump Vent or U/G Tank Cap	○ ♀ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓ ↓
Gas Pump Vent or U/G Tank Cap Sign Well Small Mine Foundation Area Outline Cemetery Building School Church Dam Dam <i>HYDROLOGY:</i> Stream or Body of Water <i>HyDROLOGY:</i> Stream or Body of Water Jurisdictional Stream Buffer Zone 1 Buffer Zone 2 Flow Arrow Disappearing Stream Spring Wetland Proposed Lateral, Tail, Head Ditch Ealso Sump	$\bigcirc$
Gas Pump Vent or U/G Tank Cap	$\bigcirc$ $\bigcirc$ $\bigcirc$ $\bigcirc$ $\bigcirc$ $\bigcirc$ $\bigcirc$ $\bigcirc$ $\bigcirc$ $\square$ $\bigcirc$ $\bigcirc$ $\square$

### **RAILROADS:**

Standard Gauge ——	
RR Signal Milepost —	
Switch	
RR Abandoned ——	
RR Dismantled	

## RIGHT OF WAY & PROJECT CONTROL:

Primary Horiz Control Po Primary Horiz and Vert Secondary Horiz and Ve Vertical Benchmark —— Existing Right of Way Ma Proposed Right of Way N (Rebar and Cap) Proposed Right of Way N (Concrete) Existing Permanent Easer Proposed Permanent Eas (Rebar and Cap) Existing C/A Monument Proposed C/A Monumen Proposed C/A Monumen Existing Right of Way Line Proposed Right of Way Li Existing Control of Access Proposed Control of Acce Proposed ROW and CA Existing Easement Line – Proposed Temporary Cor Proposed Temporary Dra Proposed Permanent Drai Proposed Permanent Drai Proposed Permanent Utili Proposed Temporary Util Proposed Aerial Utility Ec

Existing Edge of Pavemer
Existing Curb ———
Proposed Slope Stakes C
Proposed Slope Stakes F
Proposed Curb Ramp —
Existing Metal Guardrail -
Proposed Guardrail ——
Existing Cable Guiderail
Proposed Cable Guidera
Equality Symbol ——
Pavement Removal ——
VEGETATION:
Single Tree ———
Single Shrub ———

Hedge

	CSX TRANSPORTATION
	⊙ MILE POST 35
	SWITCH
·	

pint	$\bigcirc$
Control Point	
ert Control Point ——	
	Ň
onument	$\bigtriangleup$
Nonument	
Aonument ———	
ment Monument ——	$\diamond$
ement Monument ——	$\bigotimes$
	$\land$
nt (Rebar and Cap) —	
nt (Concrete) ———	
e	
ine	
s Line ———	( <u>¯</u> ¯)
ss line	
notion Factoriant	E
nstruction casement—	E
ainage Easement ——	TDE
iinage Easement ——	PDE
iinage⁄Utility Easement	DUE
ity Easement	PUE
lity Easement	TUE
asement	AUE

## **ROADS AND RELATED FEATURES:**



Woods Line	
Orchard	හි හි හි හි
Vineyard	Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall-	- ) CONC WW (
MINOR:	
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge ———	≻≺
Drainage Box: Catch Basin, DI or JB ———	СВ
Paved Ditch Gutter	
Storm Sewer Manhole ————	\$
Storm Sewer	s
UTILITIES:	
* SUE – Subsurface Utility Engineering	
LOS - Level of Service - A, B, C or D	(Accuracy)
Existing Power Pole	4
Proposed Power Pole	4
Existing Joint Liso Polo	
Proposod Joint Liso Polo	Å
Proposed Joini Ose Pole	
	E M
Power Line Tower	
Power Transformer	
U/G Power Cable Hand Hole	CH
H-Frame Pole	••
U/G Power Line Test Hole (SUE – LOS A)* –	<b>X</b>
U/G Power Line (SUE – LOS B) <sup>2</sup> —	P
U/G Power Line (SUE – LOS C)*	P
U/G Power Line (SUE – LOS D)*	
TELEPHONE:	
Proposed Telephone Pole	• •
Tolombone Manhole	
Telephone Mannole	
Telephone Fedesial	Li I
LIC Telephone Cell Tower	<ul> <li>✓</li> <li>●</li> </ul>
U/G Telephone Test Hole (SUE - LOS A)*	
U/G Telephone Cable (SUE – LOS B)* —	
U/G Telephone Cable (SUE – LOS C)* ——	T
U/G Telephone Cable (SUE – LOS D)* —	T
U/G Telephone Conduit (SUF – LOS R)* —	— — — — TC — — — –
U/G Telephone Conduit (SUF – LOS C)* —	TC
U/G Telephone Conduit (SUF – LOS D)* —	тс
U/G Fiber Ontics Cable (SLIF - LOS R)*	— — — —T FO— — — ·
U/G Fiber Ontice Cable (SUF = 105 C)*	T FO
U/G Fiber Ontice Cable (SUF = 105 C)	T F0
$C_{0}$ $C_{0$	

\*S.U.E. = Subsurface Utility Engineerin

BP2.R018.1

1B

4RD1

U.E. = Subsurface Utility Engineering	
WATER:	
Water Manhole ———	W
Water Meter ————	0
Water Valve ———	$\otimes$
Water Hydrant —	¢
U/G Water Line Test Hole (SUE – LOS A)*—	۲
U/G Water Line (SUE – LOS B)*	<b>w</b>
U/G Water Line (SUE – LOS C)*	w
U/G Water Line (SUE – LOS D)*	
Above Ground Water Line ————	A/G Water
TV:	
TV Pedestal	
TV Tower —	$\otimes$
U/G TV Cable Hand Hole	HH
U/G TV Test Hole (SUE – LOS A)*	۲
U/G TV Cable (SUE – LOS B)*	— — — Tv— — — -
U/G TV Cable (SUE – LOS C)*	
U/G TV Cable (SUE – LOS D)*	Tv
U/G Fiber Optic Cable (SUE – LOS B)*	- — — — TV FO— — —
U/G Fiber Optic Cable (SUE – LOS C)*	—TV FO
U/G Fiber Optic Cable (SUE – LOS D)*	TV F0
GAS:	•
Gas Valve ———	$\diamond$
Gas Meter	$\Diamond$
U/G Gas Line Test Hole (SUE – LOS A)*	
U/G Gas Line (SUE – LOS B)*	c
U/G Gas Line (SUE – LOS C)*	
U/G Gas Line (SUE – LOS D)*	C
Above Ground Gas Line	A/G GOS
SANITARY SEWER:	
Sanitary Sewer Manhole	•
Sanitary Sewer Cleanout	(  i )
U/G Sanitary Sewer Line	SS
Above Ground Sanitary Sewer	A/G Sanitary Sewer
SS Force Main Line Test Hole (SUE – LOS A)*	
SS Force Main Line (SUE – LOS B)* $$	— — — — FSS — — — —
SS Force Main Line (SUE – LOS C)* $-$	—— — — FSS — — —
SS Force Main Line (SUE – LOS D)*	FSS
MISCELLANEOUS:	
Utility Pole	•
Utility Pole with Base	Ē
Utility Located Object	$\odot$
Utility Trattic Signal Box	S
Utility Unknown U/G Line (SUE – LOS B)* – -	
U/G Tank; Water, Gas, Oil	
Underground Storage Tank, Approx. Loc. ——	UST
A/G Tank; Water, Gas, Oil	
Geoenvironmental Boring	
Abandoned According to Utility Kecords ——	AATUR
	E.U.I.

	PAVEMENT SCHEDULE
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF THE TWO LAYERS.
C2	PROP. VARIABLE DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110.0 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 1" OR TO EXCEED 1.5" IN DEPTH.
E1	PROP. APPROX. 4.0" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS PER SQ. YD.
R1	SHOULDER BERM GUTTER
т	EARTH MATERIAL.
U	EXISTING PAVEMENT.
V	INCIDENTAL MILLING

NOTE: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE



TO BE USED IN CONJUCTION WITH TYPICAL SECTION NO. 1 -L- STA. 14+29.00 (RT) TO STA. 14+45.00 (BEGIN APPROACH SLAB) -L- STA. 15+75.00 (END APPROACH SLAB) TO STA. 17+34.00 (RT)















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NOTE:

#### PAVEMENT REMOVAL SUMMARY IN SQUARE YARDS

SURVEY LINE	Station	Station	LOCATION LT/RT/CL	ASPHALT REMOVAL	ASPHALT BREAKUP	CONCRETE REMOVAL	
-L-	11+00.00	14+62.77		806.16			
-L-	15+54.24	17+86.00		515.02			
		TOTAL:		1321.18			
		SAY:		1330			

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT. FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL

SURVEY	BEG. STA.	END STA.	LOCATION		LENGTH		WARRAN	T POINT	"N" DIST.	TOTAL SHOUL	FLARE L	ENGTH	w	1				ANCHORS		ADDITIONAL ATTEN	PACT IUATOR E 350	SINGLE FACED	REMOVE EXISTING	REMOVE & STOCKPILE	REMARKS
LINE					SHOP	DOUBLE	APPROACH	TRAILING	FROM	WIDTH	APPROACH	TRAILING	APPROACH	TRAILING	XI	GREU	J		VI	POSTS		CONCRETE	GUARDRAII	EXISTING	
				STRAIGHT	CURVED	FACED	END	END	E.O.L.		END	END	END	END	MOD XI	TL-3	M-35	50 TYPE III CAT-1	MOD BIC	G	NG	BARRIER		GUARDRAIL	
-L-	13+81.00	14+56.00	LT	75			14+56.00	13+81.00	4	7	50		1			1		1							
-L-	13+81.00	14+56.00	RT	75			13+81.00	14+56.00	4	7		50		1		1		1							
-L-	15+63.38	16+38.38	LT	75			16+38.38	15+63.38	4	7	50		1			1		1							
-L-	15+63.38	17+82.13	RT	218.75			15+63.38	16+38.38	4	7		50		1		1		1							
			SUBTOTAL:	443.75												4		4		5					
		Less GREU TL-3	@ 50' Each	200																					
		Less Type III @	18.75' Each	75																					
			PROJECT TOTALS:	168.75												4		4		5					
																					1 1				

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

### SUMMARY OF EARTHWORK IN CUBIC YARDS

ation	Station	Uncl. Excav.	Embank. +%	Borrow	Waste
1+00.00	-L- 14+56.00	<mark>1</mark> 39	445	356	
5+63.38	-L- 17+86.00	183	330	147	
PROJECT	TOTALS:	322	775	503	
ce Topsoil o	on Borrow Pit (5%)			25	
GRAND	TOTALS:	322	775	528	
SA	Y:	330		530	

APPROXIMATE QUANTITIES ONLY, UNCLASSIFIED EXCAVATION, BORROW EXCAVATION, FINE GRADING CLEARING AND GRUBBING AND REMOVAL OF EXISTING PAVEMENT WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR "GRADING".

ALL EARTHWORK QUANTITIES WERE DERIVED FROM ORD QUANTITIES BY NAMED BOUNDARY REPORTS AS DESCRIBED IN THE ORD QUICKSTART TRAINING.

## SHOULDER BERM GUTTER SUMMARY

IN LINEAR FEET

ETE (UP	LINE	Station
	-L- RT	14+29.00
	-L- RT	15+75.00

# **GUARDRAIL SUMMARY**



Station	LENGTH
14+45.00	16
17+34.00	159
TOTAL:	175
SAY:	180

G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE

350

\_\_\_\_\_

-

	D BY: <u>JRM</u> DATE: <u>09/2021</u>	C
	BY: <u>EPA</u> DATE: <u>09/2021</u>	C
_	BRIE. 0//2021	Ľ

Note: Inve	rt Elevat	ions indic	ated are fo	r Bid Purp	oses on	nly and s	shall not l	be used fo	r proje	ct con	structio	on stak	eout.																									
									-	LL	<b>S</b> T	<b>O</b> F	<b>P</b>	PE	ES, EN	DW	AL	LS,	<i>E</i> 7	ГС.	(F0	R P	PE	ES	48 INC	CHE	<mark>S &amp;</mark>		ND	ER	2)	 	· · ·				1	
STATION	STRUCTURE NO.	TOP ELEVATION	INVERT ELEVATION SLOPE CRITICAL		(1	DRA RCP, CSP, CA	AINAGE PIPE AAP, HDPE, or I	PVC)				C.S. PIPI	E			R.C. PIPE CLASS III				R.C. PIPI CLASS IV	E V	PACTOR DESIGN	PACTOR DESIGN	RACTOR DESIGN	ENDWALLS STD. 838.01 838.11 OR STD. 838.80 (UNLESS NOTED OTHERWISE)	QUANTITIES FOR DRAINAGE STRUCTURES	TOTAL L.F. FOR PAY QUANTITY SHALL BE COL. 'A' + (1.3 X COL.'B')	F GI AN ST/ 8	RAME, RATES, D HOOD ANDARD 140.03	CONCRETE TRANSITIONAL SECTION	GRATES STD. 840.29		. & SIZE	Y. STD. 840.71	STD. 840.72		C.B. N.D.I. D.I. G.D.I. G.D.I.(N.S.) J.B. M.H.	BBREVIATIONS CATCH BASIN NARROW DROP INLE DROP INLET GRATED DROP INLET (NARROW SLOT) JUNCTION BOX MANHOLE TRAFFIC BEARING
SIZE				12" 15'	18"	24" 30"	36" 42'	" 48" d	CSP AAP	а 12"	15" 18"	24" 30	)" 36"	42" 48"	12" 15" 18" 24	." 30" 36'	42" 48	" 12"	15" 18"	24" 30"	36" 42"	SV) 81		RTS, CONT	CU. YARDS	¥ ()	FT. B	STD. 840.			RAME W/ 2		BOWS NO	E PLUG, C.	"B" C.Y.	Ë	T.B.D.I. T.B.J.B.	DROP INLET TRAFFIC BEARING JUNCTION BOX
THICKNESS OR GAUGE	FROM	2						DO NOT USE F	DO NOT USE ( DO NOT USE C	DO NOT USE H .064	.064 .064	.064	670.	.109 .109								R.C. PIPE (CLAS		kc Pipe Culver Side drain Pipe	SIDE DRAIN PIPE R.C.P. C.S.P.	EACH (0' THRU THRU 10.0'	AND ABOVE	STD. 840.01 OR	(PE OF RATE	P INLET CH BASIN	D.I. STD 840.35 I. (N.S. FLAT) FF		AINAGE PIPE EL	C. & BRICK PIPE	C. COLLARS CL	REMOVAL LIN.		
																						**		***" F 15" S	18.	PER 5.0'1	10.0'	B. B. B.	FG	CAT	G.D.I		R	CON	CON	PIPE		REMARKS
16+28 -L- 15 F	RT 401	26.7																								1					1 1							
15 1	RT 401 4	)2 2	4.0 23.8		56				x x x	х																												
16+81 -L- 15 F	RT 402	26.8																								1					1 1							
15	RT 402 4	)3 2	3.8 23.6	4	10				X X X	×			_				_																					
17+21 -L- 15 F	RT 403	27.0																								1					1 1							
	RT 403 4	)4 2	3.6 23.5		24		<u> </u>						_												_			$\left\{ \begin{array}{c} \end{array} \right\}$						_				
SHEET TOTALS				1	20																					3					3 3							

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS



COMPUTED BY: Tyler Bottoms DATE: 8/26/21 . CHECKED BY: <u>Jinyoung Park</u> DATE: <u>10/12/21</u>

### SUMMARY OF SUBSURFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
	CONTIN	IGENCY		SD	700
				TOTAL LF:	700

\*UD = Underdrain \*BD = Blind Drain

\*SD = Subsurface Drain

## SUMMARY OF GEOTEXTILE FOR PAVEMENT STABILIZATION



\*Total tons of "Class IV Subgrade Stabilization" is only the estimated quantity for pavement stabilization and may only represent a portion of the subgrade stabilization quantity shown in the Item Sheets of the Proposal.

#### SUMMARY OF ROCK PLATING

LINE	Beginning Slope (H:V)	Approx. Station	Ending Slope (H:V)	Approx. Station	Location LT/RT	Rock Plating Detail No. 1/2/3/4	Riprap Class* 1/2/B	Rock Plating SY
-L-	2.75:1	13+85 ±	2:1	14+30 ±	RT	1	2	50
-L-	2:1	15+70 ±	2.75:1	16+78 ±	LT	1	2	130
-L-	2.75:1	15+81 ±	2.75:1	16+12 ±	RT	1	2	20
							TOTAL SY:	200

\*Use Class 1, 2 or B riprap if riprap class is not shown for rock plating location.

#### SUMMARY OF PRE-SPLITTING OF ROCK

LINE	Beginning Rock Cut Slope (H:V)	Approx. Station	Ending Rock Cut Slope (H:V)	Approx. Station	Location LT/RT	Pre-splitting of Rock SY
					TOTAL SY:	0

## SUMMARY OF SURCHARGES AND SURCHARGE WAITING PERIODS

LINE	Station	Station	Surcharge Height FT	MONTHS

## (12-17-19) STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

Station	Geotextile for Pavement Stabilization SY	Class IV Subgrade Stabilization TONS
,		
L SY/TONS:	0	0*

## SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION

LINE	Station	Station	Aggregate Type* ASU(1/2)/ AST	Aggregate Thickness INCHES [8" for ASU(2)]	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Soil Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
(	CONTINGENC	Ý							
			TOTAL	CY/TONS/SY:	0	0**	0**	0	0

\*ASU(1/2) = Aggregate Subgrade (Type 1 or 2)

\*AST = Aggregate Stabilization

\*\*Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Soil Stabilization" are only the estimated quantities for ASU(1/2)/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.

#### SUMMARY OF REINFORCED SOIL SLOPES AND SLOPE EROSION CONTROL

LINE	Beginning Slope/ RSS (H:V)	Approx. Station	Ending Slope/ RSS (H:V)	Approx. Station	Location LT/RT	Reinforced Soil Slope (RSS) SY	Geocells SY	Coir Fiber Mat SY	Matting for Erosion Control SY
					TOTAL SY:	0	0	0*	0**

\*Total square yards of "Coir Fiber Mat" is only the estimated quantity for slopes steeper than 2:1 (H:V) and may only represent a portion of the coir fiber mat quantity shown in the Item Sheets of the Proposal. \*\*Total square yards of "Matting for Erosion Control" is only the estimated quantity for RSS and may only represent a portion of the matting quantity shown in the Item Sheets of the Proposal.

#### SUMMARY OF HORIZONTAL DRAINS

#### Elevation Horizontal PVC Pipe Inclination Horizontal Above or Location Schedule Drain Approximate LINE Angle Drain Below LT/RT Station W/O Pipe 40/80 or DEGREES FT Grade NO PIPE FT (+/-) FT CONTINGENCY TOTAL FT: 0 0

## SUMMARY OF EMBANKMENT WAITING PERIODS

LINE	Station	Station	MONTHS

## SUMMARY OF SETTLEMENT GAUGES

Cauga	LINE	Offset				
No.	and Station	Distance FT	Direction LT/RT			
	TOTAL GAUGES (EACH):					

## SUMMARY OF BRIDGE WAITING PERIODS

Bridge De

PROJECT NO.	SHEET NO.
SF-730015	3G-1

escription	End Bent/ Bent No.	MONTHS	

BP2.R018.1 4RD1 3G-1
HIGHWAY DIVISION 2 DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED PREPARED BY
KCA KISINGER CAMPO & ASSOCIATES
NC FIRM LICENSE No: C-1506 301 Fayetteville Street, Suite 1500 Raleigh, NC 27601 (919)882-7839
ISIONS
REV



DocuSign Envelope ID: 19FABA97-A063-499E-A265-9133D0C6B2E8

	PARCEL ROW INDEX TABLE								
			TCE AREA	TCE AREA	DUE AREA	DUE AREA	PDE AREA	PDE AREA	
PARCEL NO.	SHEET NO.	PROPERTY OWNER NAME		(AC)	(SF)	(AC)	(SF)	(AC)	
1	4	GREGORY C. BOYD	1097	0.025	3,776	0.087			
2	4	RACHEL R. PERKINS			4,129	0.095			
3	4	TJA, LLC	3536	0.081			1488	0.034	
4	4	HARDEE FAMILY HOLDINGS, LLC	2433	0.056			270	0.006	







OVERTOPPING ELEVATION

NOTES: 1.) SEE SHEET 4 FOR PLAN VIEW. 2.) FOR STRUCTURES PLANS, SEE SHEETS S-1 THRU S-21

= 27.3 FT





JECT	Prepared in	the Office of:	DOCUMENT UNLESS ALL S
[	2200 Gateway Centre B	PROFESS	
ES	RIGHT OF WAY DATE: 08/03/2021	<i>LETTING DATE:</i> 05/11/2022	Docusigned With E. Docusigned With E. Toynia Gibbs 2E2E55AA0ECB4DA SIGNATURE:

# SURVEY CONTROL SHEET W/ EXISTING CENTERLINE ALIGNMENT PRIOR TO CONSTRUCTION

I, Jimmy E. Liverman Jr., PLS, certify that the Project Control was performed under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

Class of survey: AA Type of GPS field procedure: RTN (VRS) Dates of survey: November 2020 Datum/Epoch: NAD83 (2011) Published/Fixed-control use: N/A Localized around: GPS1 Northing: 652738.670 Easting: 2538951.073 Combined grid factor: 0.9998882468 Geoid model: Geoid12B Units: US Survey Feet

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed in November 2020, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 28th day of march, 2022. -DocuSigned b Jimmy Liverman, Jr. Professional Land Surveyor L-4866

SEAL -4457 Jimmy E.Liverman JR. NCDOT Division 1 Locating Engineer

POT

GPS-2

GPS-I

### NOTES:

1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM. 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS

SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

3. PLAN SHEET PREPARED BY ESP ASSOCIATES, INC.

4. NOT TO SCALE



SEE SHEET RW2C-2 FOR FURTHER ALIGNMENT DETAILS



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPETED

# SURVEY CONTROL SHEET W/ EXISTING CENTERLINE ALIGNMENT PRIOR TO CONSTRUCTION

BL				
POINT	DESC.	NORTH	EAST	ELEVA
1	BL-1	653895.0000	2539093.5720	
2	BL-2	654461.8880	2539281.4650	
GPS1	BP2R006-1	652738.6695	2538951.0730	
GPS2	BP2R006-2	653462.7551	2538994.2631	

BM1 ELEVATION = 20.54 N 653782 E 2538979 STA 13+17.04 OFFSET -65.97 BENCHTIE IN 30" OAK

POINT	NORTHING	EASTING	BEARING	DIST	DELTA	D	L	Т	R
POT	653453.497	2539008.453							
LINE			N02°29'55.0"E	79.84					
PC	653533.263	2539011.933							
CURVE					11°39'38.0" Right	04°30'00.0"	259.12	130.01	1273.24
PT	653789.211	2539049.404							
LINE			N14°09'33.0"E	214.03					
PC	653996.734	2539101.758							
CURVE					09°01'01.0" Right	09°00'00.0"	100.19	50.20	636.62
PT	654091.554	2539133.793							
LINE			N23°10'34.0"E	223.74					
PC	654297.235	2539221.847							
CURVE				_	12°29'16.0" Left	09°00'00.0"	138.75	69.65	636.62
PT	654429.710	2539262.177							
LINE			N10°41'18.0"E	45.25					
POT	654474.176	2539270.570							

## NOTES:

1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.

2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

3.PLAN SHEET PREPARED BY ESP ASSOCIATES, INC.

I, Jimmy E. Liverman Jr., PLS, certify that the Project Control was performed under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

#### Class of survey: **AA**

Type of GPS field procedure: RTN (VRS) Dates of survey: November 2020 Datum/Epoch: NAD83 (2011) Published/Fixed-control use: N/A Localized around: GPS1 Northing: 652738.670 Easting: 2538951.073 Combined grid factor: 0.9998882468 Geoid model: Geoid12B Units: US Survey Feet

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 120,000 (Class AA) and Vertical accuracy to Class A. Field work was performed in November 2020, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 28th day of March, 2022

-DocuSigned by Jimmy Liverman, Jr.

Professional Land Surveyor L-4457

TION \_\_\_\_\_

26.03 27.09 37.95 33.49

#### EL



Jimmy E. Liverman Jr. NCDOT Division 1 Locating Engineer

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



## 2018 STANDARD SPECIFICATIONS

# 00 0 T D II

# PROPOSED ALIGNMENT CONTROL SHEET

TYPE	STATION	NORTH	EAST
POT	10+00.00	653453.4972	2539008.4527
PC	10+79.84	653533.2627	2539011.9334
PT	13+38.96	653789.2108	2539049.4040
PC	15+52.99	653996.7344	2539101.7582
PT	16+53.18	654091.5540	2539133.7932
PC	18+76.92	654297.2348	2539221.8466
PT	20+15.67	654429.7104	2539262.1772
POT	20+60.92	654474.1764	2539270.5697

## NOTES:

1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM. 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

3.PLAN SHEET PREPARED BY ESP ASSOCIATES, INC.

quality of the individual data sources.

This 28th day of March, 2022.

Jimmy Liverman, Jr. C782A59719E7430...

Professional Land Surveyor L-4457



I, Jimmy E. Liverman Jr., PLS, certify that the data compiled came from available surveys/mapping performed by others and provided to me by NCDOT and do not certify to the accuracy or



Jimmy E.Liverman NCDOT Division 1 Locating Engineer

#### DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PERMANENT DRAINAGE EASEMENT MARKER IRON PIN AND CAP' L					
PERMANENT DRAINAGE EASEMENT MARKER IRON FIN AND CAP. L					
STATION	OFFSET	NORTH	EAST		
13+88.00	45.00	653825.7486	2539105.0317		
13+88.00	30.00	653829.4179	2539090.4874		
14+06.00	45.00	653843.2018	2539109.4348		
14+06.00	30.00	653846.8710	2539094.8905		
16+45.00	50.00	654064.9290	2539176.8367		
16+45.00	30.38	654072.4178	2539158.7035		
16+53.18	50.04	654071.8589	2539179.7981		
17+20.00	50.00	654133.3048	2539206.0562		
17+20.00	30.00	654141.1760	2539187.6703		

DRAINAGE UTILITY EASEMENT MARKER IRON PIN AN					
STATION	OFFSET	NORTH	E		
13+14.00	-30.00	653771.7129	25390		
13+14.00	-50.00	653776.2241	25389		
13+38.96	-50.00	653801.4416	25390		
15+52.99	-50.00	654008.9652	25390		
16+53.18	-50.00	654111.2319	25390		
16+99.00	-50.00	654153.3554	25391		
16+99.00	-30.00	654145.4842	25391		

## NOTES:

1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM. 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

3. RIGHT OF WAY MONUMENTATION ESTABLISHED IN JANUARY 2022.

# RIGHT OF WAY CONTROL SHEET

OBSTRUCTED BY ASPHALT MATERIAL PILE. SET MAG NAIL.

NOT SET. SUBMERGED UNDER WATER.

ND CAP: L EAST 014.3080 994.8234 000.9230 053.2772 087.8282 105.8616 124.2475

NOT SET. SUBMERGED UNDER WATER.

I, Toynia Gibbs, certify that the right of way and permanent easement monumentation for this project shown herein was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:10,000 (Class A). Field work was performed on January 26th, 2022 and all coordinates are based on NAD83/2011; That this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This date of

 DocuSigned b Toynia Gibbs

Professional Land Surveyor L-3866

3/28/2022



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PERMANENT DRAINAGE EASEMENT MARKER IRON PIN AND CAP' L					
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STATION	OFFSET	NORTH	EAST		
13+88.00	45.00	653825.7486	2539105.0317		
13+88.00	30.00	653829.4179	2539090.4874		
14+06.00	45.00	653843.2018	2539109.4348		
14+06.00	30.00	653846.8710	2539094.8905		
16+45.00	50.00	654064.9290	2539176.8367		
16+45.00	30.38	654072.4178	2539158.7035		
16+53.18	50.04	654071.8589	2539179.7981		
17+20.00	50.00	654133.3048	2539206.0562		
17+20.00	30.00	654141.1760	2539187.6703		

DRAINAGE UTILITY EASEMENT MARKER IRON PIN AN					
STATION	OFFSET	NORTH	E		
13+14.00	-30.00	653771.7129	25390		
13+14.00	-50.00	653776.2241	25389		
13+38.96	-50.00	653801.4416	25390		
15+52.99	-50.00	654008.9652	25390		
16+53.18	-50.00	654111.2319	25390		
16+99.00	-50.00	654153.3554	25391		
16+99.00	-30.00	654145.4842	25391		

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NOT SET. SUBMERGED UNDER WATER.

I, Toynia Gibbs, certify that the right of way and permanent easement monumentation for this project shown herein was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:10,000 (Class A). Field work was performed on January 26th, 2022 and all coordinates are based on NAD83/2011; That this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This date of

 DocuSigned b Toynia Gibbs

Professional Land Surveyor L-3866

3/28/2022



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED





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	APPROVED	:			
	DATE:			ROLINA	
01 FAYETTEVILLE STREET	i			N. N.	
D1 FAYETTEVILLE STREET JITE 1500 ALEIGH, NC 27601 119) 882-7839		Jacob 29530CE			
D1 FAYETTEVILLE STREET JITE 1500 ALEIGH, NC 27601 919) 882-7839 C FIRM LICENSE: C-1506	SEA	Jacob 29530CE 1L	Dula <sup>3054E</sup> D9SEA 0437 0437		

TITLE

TITLE SHEET, VICINITY MAP, AND INDEX OF SHEETS

SHEET NO.

#### TMP-1

	LEGEND
	GENERAL NORTH ARROW
	TRAFFIC CONTROL DEVICES         BARRICADE (TYPE III)
	TEMPORARY SIGNING STATIONARY SIGN
<b>ROADU</b> HE FOLLOWING RAWINGS" -	<b>VAY STANDARD DRAWINGS</b> G ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C.,
<b>ROADU</b> HE FOLLOWING RAWINGS" - ATED JANUARY EFERENCE HEF	<b>VAY STANDARD DRAWINGS</b> A ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., 2018 ARE APPLICABLE TO THIS PROJECT AND BY REBY ARE CONSIDERED A PART OF THESE PLANS.

## GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

#### TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

#### SIGNING

- B) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFFSITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFFSITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

#### TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

#### LOCAL NOTES

G) PITT COUNTY EMERGENCY SERVICES SHALL BE NOTIFIED OF PROJECT CONSTRUCTION AT LEAST ONE (1) MONTH PRIOR TO BEGINNING OF CONSTRUCTION. CONTACT PITT COUNTY EMERGENCY SERVICES AT 252-902-3950.

PHASE 1

#### SEE CONSTRUCTION MORATORIUM.

STEP 1: PRIOR TO ANY CONSTRUCTION OPERATIONS, PLACE AND COVER OFFSITE DETOUR SIGNS AND DEVICES FOR S GRIMESLAND BRIDGE RD (SR 1565), BEAUFORT ST (SR 1565), BOYDS RD (SR 1780), BLACKJACK GRIMESLAND BRIDGE RD (SR 1777), CHICOD ST (SR 1777), SMITHTOWN RD, AND PITT ST (NC 33) AS SHOWN ON TMP-2. PLACE ADVANCE WARNING SIGNS PER RSD 1101.01 (SHEET 3 OF 3).

STEP 2: USING THE OFFSITE DETOUR, AS SHOWN ON TMP-2, UNCOVER DETOUR SIGNS, CLOSE -L- (S GRIMESLAND RD/SR 1565) TO TRAFFIC AND CONSTRUCT PROPOSED BRIDGE AND ROADWAY UP TO AND INCLUDING THE FINAL LAYER OF SURFACE COURSE PER ROADWAY AND STRUCTURE PLANS.

```
TRAFFIC.
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## PHASING NOTES

STEP 3: UPON COMPLETION OF BRIDGE AND ROADWAY CONSTRUCTION, PLACE FINAL PAVEMENT MARKINGS AND MARKERS PER PAVEMENT MARKING PLANS. REMOVE ALL SIGNS AND DEVICES AND OPEN -L- (S GRIMESLAND RD/SR 1565) TO

> LEGEND, ROADWAY STANDARD DRAWINGS, GENERAL NOTES, AND PHASING NOTES

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ST DEPA	ATE OF RTMENT	NORTH CAROLINA OF TRANSPORTATION		TIP NO.     SH       BP2.R018.1     F       APPROVED:
PAVEN	IENT pi7	MARKING PLA T COUNTY	N	DocuSigned by: Jacob Dude 29530C805444D9 SEAL 0437 10/28/2021
				DOCUMENT NOT CONSIDERED UNLESS ALL SIGNATURES COM
THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" -         PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C.,         DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE         CONSIDERED A PART OF THESE PLANS:         STD. NO.       IITLE         1205.01       PAVEMENT MARKINGS - LINE TYPES AND OFFSETS         1205.02       PAVEMENT MARKINGS - INTERSECTIONS         1205.03       PAVEMENT MARKINGS - SYMBOLS AND WORD MESSAGES         1205.10       RAISED PAVEMENT MARKINGS - SYMBOLS AND WORD MESSAGES         1205.11       RAISED PAVEMENT MARKINGS - BRIDGES         1205.12       PAVEMENT MARKINGS - BRIDGES         1205.11       RAISED PAVEMENT MARKERS - INSTALLATION SPACING         1250.11       RAISED PAVEMENT MARKERS - PERMANENT AND TEMPORARY         1262.01       GUARDRAIL END DELINEATION         QUANTITY         THERMOPLASTIC         TI WHITE EDGELINE (4", 90 MIL)         1372 LF         T13       YELLOW DOUBLE CENTER (4", 90 MIL)       1372 LF		<ul> <li>THE FOLLOWING GENERAL NOTES APPL THE CONSTRUCTION PROJECT, EXCEPT OR DIRECTED BY THE ENGINEER.</li> <li>A) INSTALL PAVEMENT MARKINGS AN AS FOLLOWS:</li> <li>ROAD NAME</li> <li>SR 1565</li> <li>T (S GRIMESLAND BRIDGE RD)</li> <li>B) TIE PROPOSED PAVEMENT MARKIN</li> <li>C) REMOVE/REPLACE ANY CONFLICTION</li> <li>D) STOP BAR LOCATION AT NON-SIG DIRECTED BY THE ENGINEER.</li> <li>E) REMOVE ALL RESIDUE AND SURFA CONCRETE BRIDGE DECKS PRIOR PAVEMENT MARKING MATERIAL.</li> <li>F) UNLESS OTHERWISE SPECIFIED, IN LIEU OF EXTRUDED THERMOPLA AND DIAGONALS. IF HEATED-IN THE EXTRUDED THERMOPLASTIC F</li> <li>G) UNLESS OTHERWISE SPECIFIED, IN LIEU OF COLD APPLIED PLAS AND DIAGONALS ON ASPHALT OR USED, IT SHALL BE PAID FOR U</li> </ul>	Y AT ALL TIMES FOR THE DURATION OF WHEN OTHERWISE NOTED IN THE PLAN, AD PAVEMENT MARKERS ON THE FINAL SURFACE MARKING MARKER HERMOPLASTIC RAISED NG LINES TO EXISTING PAVEMENT MARKING LINES. ING/DAMAGED PAVEMENT MARKINGS AND MARKERS. ING/DAMAGED PAVEMENT MARKINGS AND MARKERS. INALIZED INTERSECTIONS MAY BE ADJUSTED AS ACE LAITANCE BY ACCEPTABLE METHODS ON TO PLACING (insert marking material) HEATED-IN-PLACE THERMOPLASTIC MAY BE USED ASTIC FOR STOP BARS, SYMBOLS, CHARACTERS I-PLACE IS USED, IT SHALL BE PAID FOR USING AY ITEM. HEATED-IN-PLACE THERMOPLASTIC MAY BE USED DIC FOR STOP BARS, SYMBOLS, CHARACTERS CONCRETE ROADWAYS. IF HEATED-IN-PLACE IS ISING THE COLD APPLIED PAY ITEM.	
TIE TO EXIST. MARKINGS -L- STA 11+00.00 -L- TI SR 1565 (S GRIMESLAND BRIDGE RD) (S GRIMESLAND BRIDGE RD) PLAN SUBMITTED TO:	CHICOD CREEK	PLAN PREPARED BY: KISINGER CA	TIE TO EXIST. MARKINGS -L- STA 17+86.00 1565 RIMESLAND BRIDGE RD)	A LICENSE No: C-1506
AYMAN I. ALQUDWAH, P.E. – SIGNING AND DELINEATION REGIONAL ENGINEER	REPARTMENT	JACOB H. DUKE, PE PROJECT ENGINEER	KISINGER CAMPO	" LICENSE NO: C-1506 Fayetteville St., Suite 1500 leigh, NC 27601 (919)882-7839







١	STATE DECLECT DEPENDENCE NO SHEET TOTAL
	STATE     STATE     PROJECT REFERENCE NO.     STATE       NO.     SHEETS       NO.     SHEETS       NO.     SHEETS
	LII 4.1\VIV.1     LIV 1       STATE PROJ. NO.     F. A. PROJ. NO.     DESCRIPTION
	BP2.R018.1 PE
	BP2.R018.2 R/W, UTILITIES BP2 R018.3 CONSTRUCTION
l	
EROSI	ON AND SEDIMENT CONTROL MEASURES
<u>Std. #</u> 1630.03	Description Symbol
1630.05	Temporary Diversion
1605.01	Temporary Silt Fence
1606.01 1622.01	Special Sediment Control Fence Temporary Berms and Slope Drains
1630.02	Silt Basin Type B
1633.01	Temporary Rock Silt Check Type=A
1677.09	Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)
1000.02	Wattle / Coir Fiber Wattle
	with Polyacrylamide (PAM)
1634.01	Temporary Rock Sediment Dam Type=A
1635.01	Rock Pipe Inlet Sediment Trap Type=A
1635.02	Rock Pipe Inlet Sediment Trap Type=B
1630.04	Stilling Basin
1000.00	Rock Inlet Sediment Trap:
1632.01	Туре А А 🛄
1632.02	Туре ВВ
1632.03	Type C
	Skimmer Dasin
	Tiered Skimmer Basin
	Infiltration Basin
END PROJECT	THIS PROJECT CONTAINS
-L- STA 17+86.00	EROSION CONTROL PLANS
	GRUBBING PHASE OF
	CONSTRUCTION.
TO SR 1779	ENVIRONMENTALLY
SMITHTOWN RD	SENSATIVE AREAS
	ON THIS PROJECT
	REFER TO E.C. SPECIAL PROVISIONS FOR SPECIAL CONSDERATIONS.
۱ <sup>۱</sup> <sup>۱</sup> <sup>۱</sup>	
	BEEN DESIGNED TO
	SENSITIVE WATERSHED
	)

#### **Roadway Standard Drawings**

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2012 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans.

1604.01	Railroad Erosion Control Detail	1632.01	Rock Inlet Sediment Trap Type A
1605.01	Temporary Silt Fence	1632.02	Rock Inlet Sediment Trap Type 3
1606.01	Special Sediment Control Fence	1632.03	Rock Inlet Sediment Trap Type C
1607.01	Gravel Construction Entrance	1633.01	Temporary Rock Silt Check Type A
1622.01	<b>Temporary Berms and Slope Drains</b>	1633.02	Temporary Rock Silt Check Type 3
1630.01	Riser Jasin	1634.01	Temporary Rock Sediment Dam Type A
1630.02	Silt Jasin Type J	1634.02	Temporary Rock Sediment Dam Type 3
1630.03	Temporary Silt Ditch	1635.01	Rock Pipe Inlet Sediment Trap Type A
1630.04	Stilling Jasin	1635.02	Rock Pipe Inlet Sediment Trap Type 3
1630.05	Temporary Diversion	1640.01	Coir Fiber Jaffle
1630.06	Special Stilling Jasin	1645.01	Temporary Stream Crossing
1631.01	Matting Installation		



# SILT FENCE COIR FIBER WATTLE BREAK

#### NOTES:

LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED. INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

INSET A

![](_page_25_Picture_13.jpeg)

![](_page_25_Picture_14.jpeg)

![](_page_25_Picture_15.jpeg)

![](_page_25_Picture_16.jpeg)

D	Ε	T	Α	Ι	L

PROJECT REFERENCE NC	<b>)</b> .	SHEET NO.
<u>BP2.R0I8.I</u>		<u>EC-2A</u>
R/W SHEET N	10.	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND

SIDE VIEW

![](_page_26_Figure_0.jpeg)

# **COIR FIBER WATTLE BARRIER DETAIL**

#### NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.

## FILL SLOPE -

INSET A

![](_page_26_Figure_12.jpeg)

![](_page_26_Figure_13.jpeg)

PROJECT REFERENCE NO	<b>)</b> .	SHEET NO.
<u>B2R.R0I8.I</u>		EC-2B
R/W SHEET N	10.	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL

TOP VIEW

![](_page_27_Figure_0.jpeg)

# TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

## NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYP ACCORDANCE WITH ROADWAY STANDARD DRAWIN

USE EXCELSIOR FOR MATTING MATERIAL AN MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.

![](_page_27_Picture_7.jpeg)

![](_page_27_Figure_10.jpeg)

## MATTING FOR EROSION CONTROL

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)	CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
4		13+97	14+03	RT	10						
		14+03	14+30	RT	20						
		15+90	16+80	LT	55						
			CIIC	2 - 0 - 1	Q [						
MIGCELLANEQUE A	ΙΛΤΤΙΝΙΑ ΤΟ ΒΕ ΙΝΙGΤΛ	LIED AG DIRE	501 160 av 146		1260						
MISUFFERNEUUS N	INTERNO TO DE ENSER	VUEV AJ VIRE		TOTAL	1345						
				SAY	1345						

# DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

# SOIL STABILIZATION SUMMARY SHEET

PROJECT REFERENCE NC	). SHEET NO.
BP2.ROI8.I	EC-3A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

## PERMANENT SOIL REINFORCEMENT MATTING

# SOIL

# SITE DESCRIPTION

PERIMETER DIKES, SWALES, DITCHES AND

HIGH QUALITY WATER (HQW) ZONES

SLOPES STEEPER THAN 3:1

SLOPES 3:1 OR FLATTER

ALL OTHER AREAS WITH SLOPES FLATTEF

D STA	IVISION OF HIGHW TE OF NORTH CA	'AYS ROLINA	PROJECT REFERENCE NO. SHEE <u>BR2.R018.J</u> EC-
<b>STAB</b>	ILIZATION	TIME	EFRAMES
	STABILIZATION	TIME	TIMEFRAME EXCEPTIONS
)PES	7 DAYS		NONE
	7 DAYS		NUNE
	7 DAYS 7 DAYS		IF SLOPES ARE IO'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
	7 DAYS 7 DAYS 14 DAYS		IF SLOPES ARE IO'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED. 7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.

![](_page_30_Figure_0.jpeg)

![](_page_31_Figure_0.jpeg)

![](_page_32_Picture_0.jpeg)

# REFORESTATION

□ TREE REFORESTATION SHALL JE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION		
MIXTURE, TYPE, SIZE, AND FURNISH SHALL	CONFORM TO THE FOLLOWING:	
25% LIRIODENDRON TULIPIFERA	TULIP POPLAR	12 in – 18 in <b>3</b> R
25% PLATANUS OCCIDENTALIS	AMERICAN SYCAMORE	12 in – 18 in <b>3</b> R
25% FRAXINUS PENNSYLVANICA	<b>GREEN ASH</b>	12 in – 18 in <b>3</b> R
25% <b>JETULA NIGRA</b>	RIVER <b>JIRCH</b>	12 in – 18 in <b>3</b> R

![](_page_32_Picture_4.jpeg)

		N0.	SHEE
N.C.	BP2.R018.1	<b>RF-1</b>	
STATE PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION

# **REFORESTATION DETAIL SHEET**

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

![](_page_33_Figure_0.jpeg)

![](_page_33_Picture_5.jpeg)

![](_page_34_Figure_0.jpeg)

![](_page_35_Figure_0.jpeg)

![](_page_35_Figure_1.jpeg)

\_\_\_\_\_

![](_page_36_Figure_0.jpeg)

![](_page_37_Figure_0.jpeg)

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![](_page_38_Figure_0.jpeg)

![](_page_39_Figure_0.jpeg)

END PROJECT -L- 17+86.00

![](_page_39_Figure_3.jpeg)

BP2.R018.1

5

![](_page_40_Figure_1.jpeg)

STATE	STATE	PROJECT REFERENCE NO.	SH N	еет 10.	TOTAL SHEETS
N.C.	BP	2.R018.1	-	1	21
STATE P	PROJ. NO.	F. A. PROJ. NO.	DI	ESCRIPT	ION
BP2.F	R018.1	_		P.E.	
BP2.R	018.3	_	C	ONS	T.

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![](_page_41_Figure_1.jpeg)

daguirre

![](_page_42_Figure_1.jpeg)

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FINAL	UNL	ESS
SIGNATU	res	СОМ

## ANGLES

- 1 0° 16' 1"
- 2 90° 0' 0"
- 3 90° 16' 1" TO TANGENT
- (4) 90° 0' 0"
- (5) 90° 16' 1" TO TANGENT
- 6 90° 0' 0"
- (7) 89° 19' 56" TANGENT TO CURVE
- 8 2° 8' 2" TANGENT TO CURVE

	PROJEC	CT NO. <b>PITT</b> DN:	BP2	2.R018 C0 2.69 -L-	.1 UNTY
DUCOSIGNESS CAROUNDER SSION A AGUINT	SHEET 2 C DEPA G F (S. 0 OVEF HWY	STATE STATE RTMENT SENER SOR BRII GRIMES GRIMES CHICO 33 AND REVIS DATE:	OF NORTH CARG OF TRAN RALEIGH AL DF DGE ON LAND B D CREE SR 1780 IONS	DLINA NSPORTA RAWIN I SR 156 RIDGE F EK BETV D (BOYD	TION G 5 RD.) VEEN S RD) SHEET NO. S-2
RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	1		3 4		total sheets 21

						Driven Piles			Predrilling for Piles		''	Drilled-In Piles	
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Resistance per Pile TONS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Lenth per Pile FT	Scour Critical Elevation FT	Min Pile Tip (Tip No Higher Than) Elev FT	Required Driving Resistance (RDR)** per Pile TONS	Total Pile Redrives Quantity EACH	Predrilling Length per Pile Lin FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile Lin FT	Pile E In So per Pi Lin F
End Bent 1, Piles 1-5	85	See Substr. Plans	35			145	<b></b>						
Bent 1, Piles 1-7	120	See Substr. Plans	35	11	-5.0	205	_						
End Bent 2, Piles 1-5	81	See Substr. Plans	35 prodrilling long	th and at the C	 `ontractor's onti	140	with prodrilling	information by	  t no prodrilling long				<u> </u>
		ا( در) // //			GN INFO	PRMATION	רענו זו ע⊂ ול-זו זיזויי כי <sup>≬</sup>						
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile TONS	(Bla Factored Downdrag Load per Pile TONS	PIC amk entries Factored Dead Load* per Pile TONS	LE DESI indicate i Dyr Resi Fa	GN INFO item is not namic stance actor	DRMIATION applicable to st Nominal Downdrag Resistance per Pile TONS	າການແລະໄານເກາອ) Nor Scour Ro per TC	ninal esistance <sup>•</sup> Pile DNS	Scou Resista Facto (Default =	r nce r 1.00)			
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5") End Bent 1, Piles 1-5	Factored Axial Load per Pile TONS	(Bla Factored Downdrag Load per Pile TONS 0.8	PIC ank entries Factored Dead Load* per Pile TONS	LE DESI indicate Dyr Resi Fa	GN INFO item is not namic stance actor	DRMIATION applicable to st Nominal Downdrag Resistance per Pile TONS 0.6	າະນາດປະນາກອ) Nor Scour Re per TC	ninal esistance <sup>•</sup> Pile DNS	Scou Resista Facto (Default =	r nce r 1.00)			
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5") End Bent 1, Piles 1-5 Bent 1, Piles 1-7	Factored Axial Load per Pile TONS 85 120	(Bla Factored Downdrag Load per Pile TONS 0.8	PT amk entries Factored Dead Load* per Pile TONS 0.6	LE DESI indicate Dyr Resi Fa	GN INFO item is not namic stance actor	DRMIATION applicable to st Nominal Downdrag Resistance per Pile TONS 0.6	ruເດເປັນທະອ) Nor Scour Re per TC	ninal esistance Pile DNS	Scou Resista Facto (Default =	r nce r 1.00)			
End Bent/ Bent No, Pile(s) #-# (e.g., "Bent 1, Piles 1-5") End Bent 1, Piles 1-5 Bent 1, Piles 1-7 End Bent 2, Piles 1-5 Factored Dead Load is	Factored Axial Load per Pile TONS 85 120 81 factored weight	(Bla Factored Downdrag Load per Pile TONS 0.8 0.4 0.4 0.4 0.4 0.4	PT ank entries Factored Dead Load* per Pile TONS 0.6 0.6	LE DESI indlicate Dyr Resi Fa	GN INFO item is not namic stance actor	DRMATION applicable to st Nominal Downdrag Resistance per Pile TONS 0.6 0.3	IFUICIUIIIre) Nor Scour R per TC	ninal esistance Pile DNS	Scou Resista Facto (Default =	r nce r 1.00)			

NOTES:

The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Jinyoung Park, PE# 032171) on 11-4-2021.
 Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, i.e., the number of piles with a Required Driving Resistance.
 The Engineer will determine the need for PDA Testing when PDAs may be required.

End Ber Bent N

End Bent 1, F Bent 1, Pile End Bent 2, P

\*EST = Pile order lengths from estimated pile lengths; PDA = Pile order lengths based on PDA testing. For groups of end bents/bents with pile order lengths based on PDA testing, the first end bent/bent no. listed for each group is the representative end bent/bent with the PDA.

#### SUMMARY OF PDA/PILE ORDER LENGTHS

(Blank entries indicate item is not applicable to structure)

Pi	le Driving Analyz	er (PDA)	Pile Order Lei	ngths	
nt/ Io	PDA Testing Required? YES or MAYBE PDA Test Pile Length FT		Total PDA Testing Quantity EACH	End Bent/ Bent No(s)	Pile Order Length Basis* EST or PDA
Piles 1-5	MAYBE	40		End Bent 1, Piles 1-5	EST
es 1-7	YES	40	2	Bent 1, Piles 1-7	EST
Piles 1-5	MAYBE	40		End Bent 2, Piles 1-5	EST

	PRO	DJECT	NO	BP	2.R018	.1 (SF-7	30015)
			PI	TT			COUNTY
	ST	ATION:			15+09	.69 -L-	
OFESSION AT		С	s DEPARTM	TATE O ENT	F NORTH CA OF TRAN RALEIGH	ROLINA ISPORTA	ΓΙΟΝ
SEAL 048223		PILE FOUNDATION					
DocuSigned by: Dicas & Aquirre 2/14/2022 ECF43BB3073D4FB		TABLES					
SIGNATURE DATE			REVI	SIONS	3		SHEET NO. S-3
DOCUMENT NOT CONSIDERED	) NO.	BY:	DATE:	NO.	BY:	DATE:	TOTAL
FINAL UNLESS ALL SIGNATURES COMPLETED	1			3			SHEETS 21

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![](_page_44_Figure_1.jpeg)

## **GENERAL NOTES**

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS. AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA ON SHEET S-1 SHALL BE EXCAVATED FOR A DISTANCE OF APPROXIMATELY 30FT EACH SIDE OF THE CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARDSPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF THREE 30FT SPANS CONSISTING OF A CONCRETE DECK ON PRESTRESSED CONCRETE CHANNEL BEAMS WITH A CLEAR ROADWAY WIDTH OF 29'-2" SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

	TOTAL BILL OF MATERIAL								
TOS 1ENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE (BRIDGE)	BRIDGE APPROACH SLABS	REINFORCING STEEL (BRIDGE)	PILE DRIVING EQUIPMENT SETUP FOR 12"PRESTRESSED CONCRETE PILES	PILE DRIVING EQUIPMENT SETUP FOR 16"PRESTRESSED CONCRETE PILES		
SUM	EA.	LUMP SUM	CU.YDS.	LUMP SUM	LBS.	EA.	EA.		
	*		13.0		1965	5			
	1		9.3		1937		7		
	*		13.0		1965	5			
SUM	2	LUMP SUM	35.3	LUMP SUM	5867	10	7		

.6″ TRESSED CRETE TLES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0") THICK	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0 PRES CO COR	)"X 1-9" Stressed NCRETE ED SLAB
IN.FT.	LIN.FT.	TONS.	SQ.YDS.	LUMP SUM	No.	LIN.FT.
	210.5			LUMP SUM	20	1050
		152.3	170			
245						
		135.7	151			
245	210.5	288.0	321	LUMP SUM	20	1050

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THE SCOUR CRITICAL ELEVATION FOR BENT No. 1 IS ELEVATION 13.5', SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITIES ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND **RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.** 

## HYDRAULIC DATA

DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD DESIGN HIGH WATER ELEVATION DRAINAGE AREA BASE DISCHARGE (Q100) BASE HIGH WATER ELEVATION

1400 CFS 25 YRS. 25.6 17.5 SQ. MI. 2603 CFS 27.1'

## **OVERTOPPING FLOOD DATA**

OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD 100 + YRS. OVERTOPPING FLOOD ELEVATION

2850 CFS 27.3'

@ STA. 16+81.00 -L- LT

	PROJECT NO. BP2.R018.1	
	PITT COUN	ITY
	STATION: 15+09.69 -L-	
Docusigned by: Dictor A WANNAR OL	SHEET 3 OF 3	
SEAL 048223	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATIC RALEIGH	N
2/14/00 A AGUININ	GENERAL DRAWING FOR BRIDGE ON SR 1565 (S. GRIMESLAND BRIDGE RD OVER CHICOD CREEK BETWE HWY 33 AND SR 1780 (BOYDS F	.) EN RD)
<b>ASSOCIATES</b>	REVISIONS SHE	ET NO.
01 FAYETTEVILLE ST., SUITE 1500 ALEIGH, NC 27601 (919) 882-7839 C FIRM LICENSE: C-1506	NO.         BY:         DATE:         NO.         BY:         DATE:           1         3         4         5         5	<b>S-4</b> fotal sheets 21

+

		LOAD AN	ND RE	SIS <sup>-</sup>	- ANCE	E FA(	CTOR	RAT	ING	(LRF	D) SI	JMMA	RY F	OR F	PRES	TRES	SSED	CON	CRET	EGI	rdef	25	
								STRE	ENGTH	I LIN	IIT ST	ATE				SE	ERVICE	III	LIMI	t sta	TE		
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING Load rating	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)
		HL-93(Inv)	N/A	1	1.055		1.75	0.275	1.23	55′	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55′	EL	27
DESTGN		HL-93(0pr)	NZA		1.591		1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N / A					
LOAD		HS-20(Inv)	36.000	2	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0.523	1.47	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27
RAIING	RAIING	HS-20(0pr)	36.000		1.9	68.396	1.35	0.275	1.99	55′	EL	27	0.523	1.9	55′	EL	5.4	N/A					
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55′	EL	27	0.523	4.17	55′	EL	5.4	0.80	0.275	2.78	55′	EL	27
		SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL	27	0.523	3.02	55′	EL	5.4	0.80	0.275	2.15	55′	EL	27
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3.03	55′	EL	27	0.523	2.83	55′	EL	5.4	0.80	0.275	2.08	55′	EL	27
		SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55′	EL	27	0.523	2.09	55′	EL	5.4	0.80	0.275	1.38	55′	EL	27
	S S S	SNAGGRS4	34.925		1.189	41.527	1.4	0.275	1.73	55′	EL	27	0.523	1.77	55′	EL	5.4	0.80	0.275	1.19	55′	EL	27
		SNS5A	35.550		1.16	41.255	1.4	0.275	1.69	55′	EL	27	0.523	1.82	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55′	EL	27	0.523	1.68	55′	EL	5.4	0.80	0.275	1.08	55′	EL	27
		SNS7B	42.000		1.028	43.175	1.4	0.275	1.5	55′	EL	27	0.523	1.67	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27
LOAD		TNAGRIT3	33.000		1.32	43.556	1.4	0.275	1.92	55′	EL	27	0.523	1.98	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27
RAIING		TNT4A	33.075		1.33	43.979	1.4	0.275	1.94	55′	EL	27	0.523	1.91	55′	EL	5.4	0.80	0.275	1.33	55′	EL	27
		TNT6A	41.600		1.101	45.811	1.4	0.275	1.6	55′	EL	27	0.523	1.83	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27
		TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27
		TNT7B	42.000		1.163	48.848	1.4	0.275	1.69	55′	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27
		TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55′	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.5	55′	EL	27	0.523	1.58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27
		TNAGT5B	45.000	3	1.013	45.582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27

![](_page_45_Figure_2.jpeg)

LRFR SUMMARY

FOR SPAN ``A''

DESIGN ENGINEER OF F	RECORD:
	Date : <u>11/2021</u>
ASSEMBLED BY : DIEGO A. AGU	JIRRE DATE : 11/2021
CHECKED BY : JACOB H. DU	JKE DATE : 11/2021
DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10	

DOCUMENT	NOT	CONSIDER	ЕD
FINAL	UNL	ESS ALL	
SIGNATU	res	COMPLETE	)

## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{\text{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

#### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

## (#) CONTROLLING LOAD RATING $\left<1\right>$ DESIGN LOAD RATING (HL-93) $\langle 2 \rangle$ design load rating (HS-20) 3 LEGAL LOAD RATING \*\* \* \* SEE CHART FOR VEHICLE TYPE GIRDER LOCATION

		OIND		LUCA	· \	-
Ι	_	INTERIOR	GIR	DER		

EL	-	EXTERIOR	LEFT (	GIRDER
ΓD			DTCUT	CTDDED

- ER EXTERIOR RIGHT GIRDER

NROL MAR	PROJECT NO. BP2.R018.1 PITT COUNTY STATION: 15+09.69 -L-
	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD LRFR SUMMARY FOR 55' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC) SPAN A

Docusigned by CARO/ Dicor & Waywork RO/ SEAL 048223 2/1 200 A KONET	5
& ASSOCIATES 301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	

DATE: NO. BY: DATE: 5-5 BY: total sheets 21

STD.NO.21LRFR1\_90S\_55L

ΈNΤ COM

										STRE	ENGTH	I LIN	AIT ST	ΤΑΤΕ				SE	RVICE	$\bot \bot \bot$	LIMI	t sta	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL LEVEL VEHICLE VEIGHT (W) TONS) MINIMUM	LEVEL		WEIGHT (W) (TONS)	CONTROLLING LOAD RATING ATNTMIIM	MINIMUM Rating factors (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.394		1.75	0.276	1.57	50′	EL	24.5	0.531	1.39	50′	EL	2.45	0.80	0.276	1.44	50′	EL	24.5	
DESTGN		HL-93(0pr)	NZA		1.807		1.35	0.276	2.03	50′	EL	24.5	0.531	1.81	50′	EL	2.45	N/A						
LOAD		HS-20(Inv)	36.000	2	1.667	60.007	1.75	0.276	1.95	50′	EL	24.5	0.531	1.67	50′	EL	2.45	0.80	0.276	1.79	50′	EL	24.5	
RAIING		HS-20(0pr)	36.000		2.161	77.787	1.35	0.276	2.52	50′	EL	24.5	0.531	2.16	50′	EL	2.45	N/A						
		SNSH	13.500		3.635	49.079	1.4	0.276	4.95	50′	EL	24.5	0.531	4.7	50′	EL	2.45	0.80	0.276	3.64	50′	EL	24.5	
		SNGARBS2	20.000		2.871	57.42	1.4	0.276	3.91	50′	EL	24.5	0.531	3.42	50′	EL	2.45	0.80	0.276	2.87	50′	EL	24.5	
		SNAGRIS2	22.000		2.778	61.109	1.4	0.276	3.78	50′	EL	19.6	0.531	3.21	50′	EL	2.45	0.80	0.276	2.78	50′	EL	24.5	
		SNCOTTS3	27.250		1.814	49.418	1.4	0.276	2.47	50′	EL	24.5	0.531	2.36	50′	EL	2.45	0.80	0.276	1.81	50′	EL	24.5	
	SV	SNAGGRS4	34.925		1.577	55.063	1.4	0.276	2.15	50′	EL	24.5	0.531	2.01	50′	EL	2.45	0.80	0.276	1.58	50′	EL	24.5	
		SNS5A	35.550		1.537	54.657	1.4	0.276	2.09	50′	EL	24.5	0.531	2.07	50′	EL	2.45	0.80	0.276	1.54	50′	EL	24.5	
		SNS6A	39.950		1.438	57.43	1.4	0.276	1.96	50′	EL	24.5	0.531	1.91	50′	EL	2.45	0.80	0.276	1.44	50′	EL	24.5	
I FGAI		SNS7B	42.000		1.370	57.54	1.4	0.276	1.87	50′	EL	24.5	0.531	1.91	50′	EL	2.45	0.80	0.276	1.37	50′	EL	24.5	
LOAD		TNAGRIT3	33.000		1.761	58.118	1.4	0.276	2.4	50′	EL	24.5	0.531	2.25	50′	EL	2.45	0.80	0.276	1.76	50′	EL	24.5	
RAIING		TNT4A	33.075		1.777	58.759	1.4	0.276	2.42	50′	EL	24.5	0.531	2.17	50′	EL	2.45	0.80	0.276	1.78	50′	EL	24.5	
		TNT6A	41.600		1.480	61.558	1.4	0.276	2.01	50′	EL	24.5	0.531	2.08	50′	EL	2.45	0.80	0.276	1.48	50′	EL	24.5	
	ST	TNT7A	42.000		1.502	63.087	1.4	0.276	2.05	50′	EL	24.5	0.531	1.94	50′	EL	2.45	0.80	0.276	1.50	50′	EL	24.5	
		TNT7B	42.000		1.566	65.773	1.4	0.276	2.13	50′	EL	24.5	0.531	1.84	50′	EL	2.45	0.80	0.276	1.57	50′	EL	24.5	
		TNAGRIT4	43.000		1.486	63.902	1.4	0.276	2.02	50′	EL	24.5	0.531	1.77	50′	EL	2.45	0.80	0.276	1.49	50′	EL	24.5	_
		TNAGT5A	45.000		1.388	62.47	1.4	0.276	1.89	50′	EL	24.5	0.531	1.8	50′	EL	2.45	0.80	0.276	1.39	50′	EL	24.5	
		TNAGT5B	45.000	3	1.360	61.206	1.4	0.276	1.85	50′	EL	24.5	0.531	1.68	50′	EL	2.45	0.80	0.276	1.36	50′	EL	24.5	

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DESIGN ENGINEER OF RECORD: DIEGO A. AGUIRREDATE :11/2021
ASSEMBLED BY : DIEGO A. AGUIRRE DATE : 11/2021 CHECKED BY : JACOB H. DUKE DATE : 11/2021
DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10

 $\langle 3 \rangle$ 

LRFR SUMMARY

FOR SPAN ``B''

_	
301 FAYETTEVILLE	DOCUMENT NOT CONSIDERED
RALEIGH, NC 2760	FINAL UNLESS ALL
NC FIRM LICENSE:	SIGNATURES COMPLETED

## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{\text{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

#### NOTES:

COI

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

## (#) CONTROLLING LOAD RATING $\left< 1 \right>$ DESIGN LOAD RATING (HL-93) $\langle 2 \rangle$ DESIGN LOAD RATING (HS-20) $\langle 3 \rangle$ LEGAL LOAD RATING \*\* \* \* SEE CHART FOR VEHICLE TYPE GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

DocuSigned by The AROL	PROJEC 	T NO PITT N:	BP2	2.R018 C0 9.69 -L-	.1 UNTY
SEAL 048223 2/14/200 A AGUN	DEPAF LR 50' (N0	STATE RTMENT S CFR SU CORE 90 N - INTE	OF NORTH CAR OF TRAN RALEIGH TANDARE JMMA JMMA SKE SKE	NSPORTA NSPORTA NRY FC AB UN W W	TION DR JIT IC)
KISINGER CAMPO		SP/	AN B		
301 FAYETTEVILLE ST., SUITE 1500	NO. BY:	DATE: N	0. BY:	DATE:	SHEET NU. S-6
RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	12		33  ]}		SHEETS 21

STD.NO.21LRFR1\_90S\_50L

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![](_page_47_Figure_1.jpeg)

2/14/2022 BP2.R018.1\_SMU\_CS01\_730015.dgn jduke

+

![](_page_48_Figure_1.jpeg)

DESIGN ENGINE <u>Diego A.</u>	er of f Aguirre	RECORI	D: ATE : _	11/2021
ASSEMBLED BY :	DIEGO A.	. Aguirr	E DATE :	11/2021
CHECKED BY :	JACOB	H. Duke	DATE :	11/2021
DRAWN BY : DGE	3/09	REV.	12/5/11	MAA/AAC
CHECKED BY : BCH	3/09	REV.	8/14	MAA/TMG

2/14/2022 BP2.R018.1\_SMU\_CS02\_730015.dgn jduke

18'-4"	18'-4"	
E GROUTED ISS DETAILS (TYP.)	10-#5 B14 IN VERTICAL CONCRETE BARRIER RAIL	
UTTERLINE		#5 S3 & #5 S4
12" Ø VOIDS (TYP.EA.SLAB UNIT)	<i>"</i> → <i>4″</i> (TYP.) P.)	3'-0" (TYP.)
1'-9″ SPLICE		
D.6''ØL.R. TRANSVERSE ST-TENSIONING STRAND N 2 <sup>i</sup> / <sub>2</sub> ''ØHOLE (TYP.)		
GUTTERLINE		
Q //2" EXP. JT. MAT'L. IN RAIL (TYP.)	10-#5 B14 IN VERTICAL CONCRE BARRIER RAIL	#4 S2
PAIRS (SPACED AS SHOWN IN DETAIL ``A'') (TYP.EA.L	INIT)	
3 (SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.UN PACED TO MATCH S3 IN VERTICAL CONCRETE BARRIEF	IIT) R RAIL)	
55'-0"	27'-6"	
PLAN OF UNIT		

![](_page_48_Figure_5.jpeg)

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![](_page_49_Figure_1.jpeg)

DESIGN ENGINEE <b>Diego A. A</b>	ER OF I Aguirre	RECORI	D: ATE :	11/2021
ASSEMBLED BY :	DIEGO A.	. AGUIRR	EDATE :	11/2021
CHECKED BY :	JACOB	H. DUKE	DATE :	11/2021
DRAWN BY : DGE	3/09	REV.	12/5/11	MAA/AAC
Checked by : BCH	3/09	REV.	8/14	MAA/TMG

2/14/2022 BP2.R018.1\_SMU\_CS03\_730015.dgn jduke

16′-8″	16'-8"	
- SEE GROUTED RECESS DETAILS (TYP.)	10-#5 B13 IN Vertical concrete Barrier Rail	
		•
GUTTERLINE -		( <u> </u>
12" Ø VOIDS 4" (TYP.EA.SLAB UNIT) (TYP.)	(TYP.)	
SPLICE		
Q 0.6'' Ø L.R. TRANSVERSE Post-tenstoning strand		
IN $2\frac{1}{2}^{\prime\prime}$ Ø HOLE (TYP.)		
GUTTERLINE		‡
Q 1/2" EXP.JT. Mat'l. in Rail (Typ.)	#4 S2	
2 PAIRS (SPACE AS SHOWN IN DETAIL ``A'')(TYP.EA.UNI I	[Ţ) ▶  <b>∢ ▶</b>	1'-0
 53 (SPACE AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.UNIT 5PACED TO MATCH S3 IN VERTICAL CONCRETE BARRIER 	r) RAIL)	21/2
	25'-0"	
50'-0" DIANIOE LINITT		
<u>AN OF UNLI</u> SPAN ``B´´		

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

![](_page_49_Figure_6.jpeg)

#### DocuSign Envelope ID: D547EA7D-66AA-42D5-9211-ACA3844919FF

Β7

S1

S2

\* S3 64

8

114

+

BILL OF MATERIAL FOR ONE 50' CORED SLAB UNIT									
EXTERIOR UNIT INTERIOR UNIT									
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT		
B6	4	#4	STR	25'-9"	69	25'-9"	69		
S1	8	#5	3	4'-3"	35	4'-3"	35		
S2	104	#4	3	5'-4"	371	5'-4"	371		
* S3	58	#5	1	5′-7″	338				
REINF	DRCING S	STEEL	LBS	5.	475		475		
* EPOX REIN	(Y COATE NFORCING	ED Gisteel	LBS	S.	338				
6500	P.S.I.CO	NCRETE	CU. YDS	) <u> </u>	7.1		7.1		
0.6″Ø	L.R. STR	ANDS	No	) .	19		19		

BILL OF MATERIAL FOR ONE 55' CORED SLAB UNIT

4'-3"

5′-4″

5'-7"

BAR NUMBER SIZE TYPE LENGTH WEIGHT

3

3

1

4 #4 STR <u>28'-3"</u>

#5

#4

#5

EXTERIOR UNIT INTERIOR UNIT

75

35

406

373

LENGTH WEIGHT

75

35

406

28'-3"

4'-3"

5′-4″

![](_page_50_Figure_2.jpeg)

![](_page_50_Figure_3.jpeg)

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

![](_page_50_Figure_5.jpeg)

			00110	
ASSEMBLED BY : CHECKED BY :	DIEGO A JACOB	. <b>Aguirre</b> date : <b>H. duke</b> date :	11/2021 11/2021	
DRAWN BY : DGE CHECKED BY : BCH	5/09 6/09	REV. 5/18	MAA/THC	DESIGN ENGINEER OF RECORD: DIEGO A. AGUIRRE DATE :11/2021
				2/14/2022 2/14/2022 BP2.R018.1_SMU_CS04_730015.dgn jduke jduke

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![](_page_50_Figure_8.jpeg)

TERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
5' UNITS	15⁄8″	3′-7 <sup>5</sup> ⁄8″

![](_page_50_Figure_10.jpeg)

![](_page_50_Figure_11.jpeg)

	BAR LIPES	NOTES
HICKNESS & RAIL HEIGHT		ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
@ MID-SPAN 1 <sup>5</sup> / <sub>8</sub> " 3'-7 <sup>5</sup> / <sub>8</sub> "		ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.
		RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.
	8 <sup>1</sup> / <sub>4</sub> " 6" 7 <sup>3</sup> / <sub>4</sub> "	THE $2^{1\!/_{2}''} \varnothing$ anchor bolt holes at fixed ends of slab sections shall be filled with non-shrink grout.
JEAD LOAD DEFLECTION AND CAMBER <u>3'-0" × 1'-9"</u>	S1,_ 1′-9′′	THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.
50' & 55' CORED SLAB UNIT     0.6"∅ L.R. STRAND       CAMBER (SLAB ALONE IN PLACE)     1½"       DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD     3%"       TNAL CAMBER     1¼"	S2 2'-8'' 1, -3', 1, -3', 1	WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.
* INCLUDES FUTURE WEARING SURFACE	ALL BAR DIMENSIONS ARE OUT TO OUT	ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.
BILL OF MATERIAL FOR VERTICA	L CONCRETE BARRIER RAIL	PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.
50' UNIT	I UTAL NU. SIZE ITME LENGIH WEIGHI	APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS. GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL
*B13     40       *S4     116       *EPOXY COATED REINFORCING STEEL	40 #5 STR 24'-7" 1026 116 #5 2 7'-2" 867 LBS. 1893	EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10
CLASS AA CONCRETE TOTAL VERTICAL CONCRETE BARRIER RAIL	CU.YDS. 12.8 LN.FT. 100.25	FEET IN LENGTH. FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT
BILL OF MATERIAL FOR VERTICA BAR BARS PER PAIR OF EXTERIOR UNITS 55' UNIT	L CONCRETE BARRIER RAIL Total no. size type length weight	THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE ``CONCRETE RELEASE STRENGTH'' TABLE.
*B14 40	40 #5 STR 27'-1" 1130	FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
* S4 128	128 #5 2 7'-2" 957	THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.
* EPOXY COATED REINFORCING STEEL CLASS AA CONCRETE TOTAL VERTICAL CONCRETE BARRIER RAIL	LBS. 2087 CU.YDS. 14.1 LN.FT. 110.25	THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.
CORED SLABS REQUIRED	CORED SLABS REQUIRED	THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.
INUMBERLENGIHIUIALLENGIH50' UNIT50'-0"100'-0"EXTERIOR C.S.250'-0"100'-0"	NUMBERLENGIHIUIALLENGIH55' UNIT55' -0"110'-0"EXTERIOR C.S.255'-0"110'-0"	THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.
INTERIOR C.S.       8       50'-0"       400'-0"         TOTAL       10       500'-0"       10	INTERIOR C.S. 8 55'-0" 440'-0" TOTAL 10 550'-0"	THE 78" Ø ANCHUR BULIS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.
2'-0"		WITH THE END BENT CAPS AND BENT CAP SHALL BE INCLUDED IN THE CORED SLAB PAY ITEM.
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	<b>#</b> 5 S3 & S4 ◄	
	 _▼	PROJECT NO. BP2.R018.1
		PITT COUNTY
	#5 S4 GRADE 270 STRANE	STATION: 15+09.69 -L-
$\begin{array}{c c} & & \hline \\ & & \hline \\ & & \\ & \\ & \\ & \\ & \\ &$	AREA	217 217
	#5 S3 (TYP.)ULTIMATE STRENGTH (LBS. PER STRAND)58,APPLIED PRESTRESS (LBS. PER STRAND)43,9	SEAL 048223 350 320 320 350 350 350 350 350 350 350 350 350 35
CONST. JT.	CONCRETE RELEASE STRE	NGTH PRESTRESSED CONCRETE
NSIDE VIEW	UNIT PSI 50/ 8 55/ UNITS 4000	CORED SLAB UNIT 90° SKEW SPANS ``A'' & ``R''
<u>END OF RAIL DETAILS</u>		KISINGER CAMPO     JI ANJ A & D       & ASSOCIATES     REVISIONS
	FINAL UNLESS A SIGNATURES COMPL	LUERED       301 FAYETTEVILLE ST., SUITE 1500       NO.       BY:       DATE:       NO.       BY:       DATE:       STOTAL         RALEIGH, NC 27601 (919) 882-7839       1       3       TOTAL       SHEETS         ETED       NC FIRM LICENSE: C-1506       2       4       21

		NOIES
HICKNESS & RAIL HEIGHT		ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
@ MID-SPAN     @ MID-SPAN       15%"     3'-75%"	$\begin{array}{c c} & & & & \\ \hline 1 & & & \\ \hline 1 & & & \\ \hline 2 & & & \\ \hline 2 & & & \\ \hline 4 & & \\ \hline 4 & & \\ \hline \end{array}$	ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.
		RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.
DEAD LOAD DEFLECTION AND CAMBER	8 <sup>1</sup> /4" 6" 7 <sup>3</sup> /4"	THE $2^{I}/_{2}$ $''$ <code>Ø</code> anchor bolt holes at fixed ends of slab sections shall be filled with non-shrink grout.
$\frac{3'-0'' \times 1'-9''}{0.6'' \varnothing L.R.}$	<u>S1 1'-9''</u>	THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.
SOLAD SEAD ONT     STRAND       AMBER (SLAB ALONE IN PLACE)     1½″ ↓       EFLECTION DUE TO     3%8″ ↓       UPERIMPOSED DEAD LOAD***     3%8″ ↓	1, -3, '       1, -3, '       1, -3, '       1, -4, '       25	WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.
INAL CAMBER 11/8″ ↓ ← INCLUDES FUTURE WEARING SURFACE	ALL BAR DIMENSIONS ARE OUT TO OUT	ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.
BILL OF MATERIAL FOR VERTICA	L CONCRETE BARRIER RAIL	PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.
BAR BARS PER PAIR OF EXTERIOR UNITS 1 50' UNIT	OTAL NO. SIZE TYPE LENGTH WEIGHT	APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.
*B13 40 *S4 116 *EPOXY COATED REINFORCING STEEL	40 #5 STR 24'-7" 1026 116 #5 2 7'-2" 867 LBS. 1893	GROOVED CONTRACTION JOINTS, 72" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10
CLASS AA CONCRETE Total vertical concrete barrier rail	CU.YDS. 12.8 LN.FT. 100.25	FEET IN LENGTH. FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT
BILL OF MATERIAL FOR VERTICA BAR BARS PER PAIR OF EXTERIOR UNITS T 55' UNIT	L CONCRETE BARRIER RAIL Total no. size type length weight	ALLOWED. THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE ``CONCRETE RELEASE STRENGTH'' TABLE.
*B14 40	40 #5 STR 27'-1" 1130	FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
* S4 128	128 #5 2 7'-2" 957	THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.
* EPOXY COATED REINFORCING STEEL CLASS AA CONCRETE TOTAL VERTICAL CONCRETE BARRIER RAIL	LBS.2087CU.YDS.14.1LN. FT.110.25	THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.
CORED SLABS REQUIRED	CORED SLABS REQUIRED	THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.
NUMBERLENGTH TOTALLENGTH50' UNIT	NUMBERLENGTHTOTALLENGTH55' UNIT55' -0"110' -0"EXTERIORC.S.255' -0"440' -0"INTERIORC.S.855' -0"440' -0"	THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS. THE 7/8" Ø ANCHOR BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM
TOTAL         10         500'-0"	TOTAL         10         550'-0"	A307. The cost of the 7/8″Ø anchor bolts, nuts, washers and plates cast
<		WITH THE END BENT CAPS AND BENT CAP SHALL BE INCLUDED IN THE CORED SLAB PAY ITEM.
$\frac{4^{+\pm}5 \times 3}{8 \times 4} = \frac{4^{-\pm}5 \times 4}{8 \times 4} = \frac{4^{-\pm}5 \times 4}{8 \times 4} = \frac{4^{-\pm}5 \times 4}{8 \times 4} = 4^$	<b>#</b> 5 S3 & S4 ►	
"     1"     FIELD BEND     6"CTS.     6"CTS.       "B" BARS           FIELD CUT     6"CTS.		BROUFOT NO BP2 R018 1
		PROJECT NO. DI ZII COUNTY
	#5 S4 GRADE 270 STRANE	STATION: 15+09.69 -L-
• FIELD • • • • • • • • • • • • • • • • • • •	AREA 0.6″S	ZL.R. Dicus A WANNER SHEET 4 OF 4
	#5 S3 (TYP.)(SQUARE INCHES)0.ULTIMATE STRENGTH (LBS. PER STRAND)58,APPLIED PRESTRESS (LBS. PER STRAND)43,9	SEAL 048223 050 SEAL 048223 STANDARD
CONST. JT.		NGTH PRESTRESSED CONCRETE
V SIDE VIEW	UNITE RELEASE SIRE	CORED SLAB UNIT
END OF RATI DETATIS	50' & 55' UNITS 4900	KISINGER CAMPO     SPANS `A'´&`B''       & ASSOCIATES     DEVICTORIC
	DOCUMENT NOT CONS FINAL UNLESS A SIGNATURES COMPL	IDERED       301 FAYETTEVILLE ST., SUITE 1500       NO.       BY:       DATE:       NO.       BY:       DATE:       SHEET NO.         ALL       RALEIGH, NC 27601 (919) 882-7839       1       3       SHEET NO.       S-10         ALL       NC FIRM LICENSE: C-1506       2       4       21
	· · · · · · · · · · · · · · · · · · ·	

CORED	SLABS	s req	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
50′UNIT			
EXTERIOR C.S.	2	50'-0"	100′-0″
INTERIOR C.S.	8	50'-0"	400'-0"
TOTAL	10		500'-0"

F		NOIES
		ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
	$(1) \begin{array}{c} \frac{1}{2} \\ \frac{1}{$	ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.
		RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.
$\neg$		THE 2 <sup>1</sup> /2″Ø ANCHOR BOLT HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE
	S1 1'-9''	THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER.SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.
-	S2     2'-8''       3     3	WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS.AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.
	ALL BAR DIMENSIONS ARE OUT TO OUT	ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.
		PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT
	DTAL NO.   SIZE   TYPE   LENGTH   WEIGHT	ENDS. Apply epoxy protective coating to cored slab unit ends.
		GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " in depth, shall be tooled in all exposed faces of the barrier bat, and th accordance with article
	40 #5 STR 24'-7" 1026 116 #5 2 7'-2" 867 BS: 1893	825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10
	CU.YDS. 12.8 LN.FT. 100.25	FEET IN LENGTH.
		ALLOWED.
	CONCRETE BARRIER RAIL         DTAL NO.       SIZE         TYPE       LENGTH	THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE ``CONCRETE RELEASE STRENGTH'' TABLE.
	40 #5 STR 27'-1" 1130	FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
	128 <b>#</b> 5 2 7'-2" 957	THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.
	LBS. 2087 CU.YDS. 14.1 LN.FT. 110.25	THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS.
	CORED SLARS REGULTRED	THE PERMITTED THREADED INSERTS MAT BE USED AS AN ALTERNATE. THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR
	NUMBER LENGTH TOTAL LENGTH	THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN
E I	SS UNIT       XTERIOR C.S.     2     55'-0"     110'-0"       NTERIOR C.S.     8     55'-0"     440'-0"	THE PRICE BID FOR THE PRECAST UNITS. THE $\frac{1}{8}$ " Ø anchor bolts shall conform to the requirements of astm
T	DTAL 10 550'-0"	A307. The cost of the 7/8″Ø anchor Bolts, nuts, washers and plates cast
		WITH THE END BENT CAPS AND BENT CAP SHALL BE INCLUDED IN THE CORED SLAB PAY ITEM.
►	<b>≠</b> 5 S3 & S4	
•	• •	PROJECT NO. BP2.R018.1
		PITT COUNTY
	#5 S4 GRADE 270 STRAND	S STATION: 15+09.69 -L-
•	AREA 0.6" Ø	DIL.R. DIL CAROLINA DIL CAROLINA DIL CAROLINA SHEET 4 OF 4
	ULTIMATE STRENGTH +5 S3 (I BC DED STRENGTH 58,6	SEAL DEPARTMENT OF TRANSPORTATION
	(TYP.) APPLIED PRESTRESS (LBS. PER STRAND) 43,9	50 048223 STANDARD
•		2/14/200 = A AGUNININ 3'-0'' X 1'-9''
W	CONCRETE RELEASE STREN	AGTH 
	50' & 55' UNITS 4900	KISINGER CAMPO   SPANS ``A´´ & ``B´´
	DOCUMENT NOT CONS	& ASSOCIATESREVISIONSSHEET NO.IDERED301 FAYETTEVILLE ST., SUITE 1500NO.BY:DATE:NO.BY:DATE:S-10
	FINAL UNLESS A SIGNATURES COMPL	L       RALEIGH, NC 27601 (919) 882-7839       1       3       TOTAL         ETED       NC FIRM LICENSE: C-1506       2       4       21

![](_page_50_Figure_16.jpeg)

![](_page_50_Figure_17.jpeg)

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![](_page_51_Figure_1.jpeg)

![](_page_51_Figure_3.jpeg)

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $\frac{7}{8}$ ' Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

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NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " hold down plate and 7 -  $\frac{7}{8}$ " Ø bolts with nuts and washers.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

![](_page_51_Figure_16.jpeg)

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![](_page_52_Figure_1.jpeg)

DESIGN ENGINEE	er of i	RECORD:		
DIEGO A. A	GUIRRE	DATE	:	11/2021
ASSEMBLED BY : CHECKED BY :	DIEGO A. JACOB	AGUIRRE DA H. DUKE DA	TE: TE:	11/2021 11/2021
DRAWN BY : DGE Checked by : MKT	01/10 01/10	REV. 4/15		MAA/TMG

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## NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR WING DETAILS, SEE SHEET 3 OF 4.

FOR DETAILS REGARDING THE  $\frac{7}{8}'' \varnothing$  anchor Bolts, see sheet S-10.

TOP ELE	OF PILE VATIONS
(1)	24.43
2	24.22
3	24.02
4	23.81
5	23.61

<u>)</u> )	DocuSigned & CAROL Dic So CAROL	PROJECT NO. BP2.R018.1 PITT COUNTY STATION: 15+09.69 -L-
	SEAL 048223 2/14/200 A AGUNUN	SUBSTRUCTURE
		END BENT No. 1
	& ASSOCIATES	REVISIONS SHEET NO.
SIDERED ALL 'LETED	301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	NO.         BY:         DATE:         NO.         BY:         DATE:         S-12           1         3

![](_page_53_Figure_1.jpeg)

2/14/2022 BP2.R018.1\_SMU\_E02\_730015.dgn jduke

## NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR WING DETAILS, SEE SHEET 3 OF 4.

FOR NOTES REGARDING THE  $7_{\!8}'' \varnothing$  anchor Bolts, see sheet S-10.

TOP ELE	OF PILE VATIONS
(1)	24.37
2	24.17
3	23.96
4	23.75
5	23.55

) DocuSign		PROJEC Static	T NO. <b>PITT</b> )N:	BP2 - 15+09	2.R018 C0 .69 -L-	.1 UNTY
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2/14/20	A AGUININ		SUB	STRUCT	URE	
K	CA		END	BENT	No. 2	
KISIN	GER CAMPO					
& A S	SOCIATES		REVIS	SIONS		SHEET NO.
SIDERED ALL LETED NC FIRM LI	TEVILLE ST., SUITE 1500 NC 27601 (919) 882-7839 CENSE: C-1506	NO. ВY: 1 2	DATE:	NO. ВҮ: З	DATE:	S-13 Total Sheets 21
			STD N			

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![](_page_54_Figure_1.jpeg)

![](_page_54_Figure_4.jpeg)

![](_page_55_Figure_1.jpeg)

DRAWN BY: DGE 12/09 CHECKED BY: MKT 01/10 REV. 4/17 MAA/THC

![](_page_55_Figure_3.jpeg)

![](_page_55_Figure_4.jpeg)

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v <sup>°</sup> w			R2	16	#4	STR	19'-1"	204
	<b>L</b>		B3	9	#4	STR	2'-5"	15
	7'-2"							
			D1	20	#6	STR	1'-6"	45
			H1	24	#4	2	7'-10"	126
			K 1	12	# /	STR	2'-11"	23
4	1/2" 2'-5" 4 <sup>1</sup> /2"	-	r \ 1					
			S1	46	#4	3	7'-5"	228
I	НК	1	S2	46	#4	4	3'-2"	97
	(4)		S3	10	#4	5	6'-6"	43
				4.0	# 4		A. 0.11	150
	1'-3'' LAF	C	VI	48	# 4	SIR	4'-8"	150
			REIN	FORCI	NG STE	EEL		
			(FOR	one e	END BE	NT)	1	965 LBS.
			CLASS	S A CO		E BREA	AKDOWN	
				(FOR (	JNE EN	U BENT		11 0 0 0
	1′-8″Ø		POUR	#1 C	AP, LOV F WING	VER PA GS & (	KI Collars	11.2 C.Y.
	. 1			U				
TONS A	RE OUT TO OUT		POUR	#2 U	PPER F	PART C	)F	1.8 C.Y.
LUNU A				VV	тирр			
			<b>T</b> ~ <b>T</b>				TE	
			I OTAL	L CLAS	SSAC	UNCRE	1 E	13.0 C.Y.
					END	BENT	No.1	
			12″ P	RFSTR	FSSFD		FTF PTIF	5
			$12^{\circ}$ PRESTRESSED CUNCRETE PILES NO. 5 I TN FT = 175					
				<u>ل</u>				
				PI	LE DRI	VING	EQUIPMEN <sup>-</sup>	Г
			1	12″ PRE	STRES:	EIUM F SED CC	NCRETE P	ILES
				. –	_			NO: 5
			PTIF	e RFDF	RIVES			N0: 0
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					END	BENT	No. 2	
			12" P	RESTR	ESSED	CONCR	ETE PILES	5
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			1	12″ PRE	STRES	SED CC	NCRETE P	ILES NO: 5
			PILE	e redf	RIVES			NO: O
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	SEAL	<b> </b> (	DEPAF	RTMEN	IT OF	TRAN	NSPORTA	TION
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	A AGUINT			30	וריי			
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			E	ND	RFN		0.1 <b>&amp;</b>	2
					DE <sup>-</sup>	ΓAIL	S	
	& ASSOCIATES			RF	VISTONS	- )		SHEET NO
ERFN	301 FAYETTEVILLE ST., SUITE 1500	NO.	BY:	DATE:	NO.	BY:	DATE:	S-15
	RALEIGH, NC 27601 (919) 882-7839	1			_ 3			TOTAL SHEETS
fed	NC FIRM LICENSE: C-1506	2			④			21

![](_page_56_Figure_1.jpeg)

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3

(4)

(5)

6

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jduke

![](_page_56_Figure_4.jpeg)

STD.NO. 16" PS\_BT\_30\_90S\_<60'

![](_page_57_Figure_1.jpeg)

DESIGN ENGINEER OF	RECORD:			
DIEGO A. AGUIRRE DATE : 11/2021				
ASSEMBLED BY : DIEGO A. A Checked by : Jacob H.	AGUIRRE DATE : Duke date :	11/2021 11/2021		
DRAWN BY : DGE 05/10 Checked by : Mkt 05/10	REV. 6/17	MAA/THC		

![](_page_57_Figure_5.jpeg)

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		ΒI	LL O	F MA	ATERIA	L
			FOR	ONE	BENT	
<u></u> 1'-3'' ΙΔΡ	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	B1	4	#10	1	34'-10"	600
	B2	4	#10	STR	32'-2"	554
	В3	4	#5	STR	32'-2"	134
	B4	8	#4	STR	17′-4″	93
((3))	B5	8	#4	STR	2'-11"	16
	D1	40	#6	STR	1'-6"	90
<u>2′-2″Ø</u> ►		32	#5	2	8'-1"	270
		14	#4	3	8'-1"	76
2'-10" U1	U1	4	#4	4	5'-10"	16
2'-0" U2, U4	U2	4	#4	4	5'-0"	13
2'-9" U3	U3	2	#9	4	10′-1″	69
	U4	2	#4	4	4'-2"	6
	REINFORCING STEEL (FOR ONE BENT) 1937 LE					1937 LBS
	CLASS A CONCRETE BREAKDOWN (FOR ONE BENT)					
	ТОТ	AL CLA	SS A CO	DNCRETE		9.3 C.Y.
	16″ F	PRESTRE (F	ESSED C OR ONE	ONCRET	E PILES	
T TO OUT.	No	. 7			LIN.	FT. 245
	PILE	E DRIV	ING EQL	JIPMENT	SETUP FOF	2
	16″ F	PRESTRE	ESSED C	ONCRET	E PILES	
					/	No.7
	PILE	E REDRI	IVES			NO: O
	CON CON THE	ICRETE ICRETE CONCF	DISPLA PILES I Rete qui	CED BY HAS BEE ANTITY,	THE 16″PRE En deducted	STRESSED ) FROM

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![](_page_58_Figure_1.jpeg)

QUANTITIES FOR ONE 12" PRESTRESSED PILE						
	CONCRETE	PILE WT.	ONE POIN	T PICK-UP	TWO POIN <sup>-</sup>	T PICK-UP
LENGTH	CU.YDS.	TONS	0.300L	0.700L	0.207L	0.586L
25'-0''	0.91	1.85	7'-6''	17'-6''		
30'-0''	1.10	2.22	9'-0''	21'-0''		
35'-0''	1.28	2.59	10'-6''	24'-6''		
40'-0''	1.46	2.96	12'-0''	28'-0''		
45'-0''	1.64	3.33	13'-6''	31′-6′′		
50'-0''	1.83	3.72	15'-0''	35'-0''		
55'-0''	2.01	4.09			11'-4 <sup>1</sup> /2''	32'-3''
60'-0''	2.19	4.46			12'-5''	35'-2''
65'-0''	2.38	4.81			13'-5 /2''	38'-1''
70'-0''	2.57	5.18			14'-6''	41'-0''

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![](_page_58_Picture_6.jpeg)

![](_page_58_Picture_7.jpeg)

PRESTRESSED CONCRETE STRENGTH : f'c = 7,500 PSI BUILD-UP CONCRETE STRENGTH : f'c = 7,500 PSI STRAND DATA:

SIZE	GRADE	AREA	ULTIMATE STRENGTH	APPLIED PRESTRESS FORCE
0.6″	270 L.R.	0.217	58,600# Per strand	43,940# PER STRAND

![](_page_58_Picture_10.jpeg)

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS CONFORMING TO AASHTO M203.STRAND SAMPLING REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED.

TRANSFER THE LOAD FROM THE ANCHORAGES TO THE PILE AFTER THE CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN PAIRS, EXCEPT WHERE 5 STRANDS ARE USED, THE LAST STRAND MAY BE BURNED SINGLY ACCORDING TO BURNING PATTERNS SHOWN. NOT MORE THAN 4 STRANDS MAY BE BURNED AT ANY ONE SECTION BEFORE THE SAME STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS ARE TO BE INDICATED WITH A 2" WIDE BLACK MARK.

DRIVE PILES USING A METHOD APPROVED BY THE ENGINEER, WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

DRIVING OF THE BUILT-UP PILE WILL NOT BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 5,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

## DOWEL INSTALLATION FOR OPTIONAL BUILD-UP

GROUT COMPRESSIVE STRENGTH: f'c= 5,000 PSI

BEFORE DRILLING DOWEL HOLES, REMOVE THE UPPER 3" OF CONCRETE FROM THE TOP OF THE PILE WITHOUT DAMAGE TO THE REINFORCING STEEL. THE REMOVAL PLANE SHOULD BE NORMAL TO THE EDGE OF THE PILE.

DOWEL HOLES SHALL BE POSITIONED TO MAINTAIN  $\frac{1}{2}$  clear to all EXISTING PRESTRESSING STRANDS IN THE CONCRETÉ PILE.

FIELD DRILLED HOLES SHALL BE CLEAN AND FREE OF ANY OBSTRUCTIONS BEFORE GROUTING OF DOWELS. DOWEL BARS SHALL BE INSTALLED AND GROUTED WITH AN APPROVED NON-SHRINK GROUT.

THE SPIRAL REINFORCING IN ALL BUILD-UPS SHALL BE W4.0 COLD DRAWN WIRE WHICH SHALL BE SECURED TO THE LONGITUDINAL REINFORCEMENT TO MAINTAIN PITCH.

THE SPIRAL REINFORCING IN THE BUILD-UP AND THE PRESTRESSED CONCRETE PILE SHALL BE SPLICED BY OVERLAPPING A MIN. OF ONE TURN.

DocuSigned by SARO	PROJECT NO. BP2.R018.1 PITT COUNTY STATION: 15+09.69 -L-
SEAL 048223 2/14/200 A AGUNUU	DEPARTMENT OF TRANSPORTATION RALEIGH 12" PRESTRESSED CONCRETE PILE
KISINGER CAMPO & ASSOCIATES	REVISIONS SHEET NO.
) 301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	NO.         BY:         DATE:         NO.         BY:         DATE:         S-18           1         3         TOTAL SHEETS         TOTAL SHEETS         21
	STD. NO. PCP1

![](_page_58_Picture_27.jpeg)

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![](_page_59_Figure_1.jpeg)

iduke

![](_page_59_Figure_4.jpeg)

QUANTITIES FOR ONE 16"PRESTRESSED PILE						
	CONCRETE	PILE WT.	ONE POIN	T PICK-UP	TWO POIN <sup>-</sup>	T PICK-UP
LENGTH	CU. YDS.	TONS	0.300L	0.700L	0.207L	0.586L
25'-0"	1.63	3.31	7′-6″	17′-6″	5′-2″	14′-8″
30'-0"	1.96	3.97	9'-0"	21'-0"	6'-2 <sup> </sup> /2"	17′-7″
35'-0"	2.29	4.63	10'-6"	24'-6"	7'-3"	20'-6"
40'-0"	2.61	5.29	12'-0"	28'-0"	8′-3 <sup> </sup> /2″	23'-5"
45'-0"	2.94	5.95	13'-6"	31'-6"	9'-4"	26'-4"
50'-0"	3.27	6.61	15'-0"	35′-0″	10′-4″	29'-4"
55'-0"	3.59	7.28	16'-6"	38′-6″	11′-4 <sup> </sup> /2″	32'-3"
60'-0"	3.92	7.94			12'-5"	35′-2″
65'-0"	4.25	8.60			13′-5 <sup> </sup> /2″	38'-1"
70′-0″	4.57	9.26			14′-6″	41'-0"
75'-0"	4.90	9.92			15′-6 <sup> </sup> /2″	43′-11″
80′-0″	5.23	10.58			16′-7″	46'-10"

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BURNING	S	RAN	DS		
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SIGNATU	res	СОМ	PLETED

## NOTES

PRESTRESSED CONCRETE STRENGTH : f'c = 7,500 PSI BUILD-UP CONCRETE STRENGTH : f'c = 7,500 PSI STRAND DATA:

SIZE	GRADE	AREA	ULTIMATE STRENGTH	APPLIED PRESTRESS FORCE
0.6″	270 L.R.	0.217	58,600# Per strand	43,940# PER STRAND

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS CONFORMING TO AASHTO M203. STRAND SAMPLING REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED. TRANSFER THE LOAD FROM THE ANCHORAGES TO THE PILE AFTER THE CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN OPPOSITE PAIRS AS INDICATED IN THE TYPICAL PATTERN SHOWN. FOR ANY NUMBER OF STRANDS, BURN IN OPPOSITE PAIRS AND SYMMETRICALLY ABOUT BOTH THE VERTICAL AND HORIZONTAL AXES. STRANDS 1-1 SHALL BE BURNED BEFORE 2-2, ETC. NOT MORE THAN 4 STRANDS, SAY 3-3 AND 4-4, MAY BE BURNED AT ANY ONE SECTION BEFORE THESE SAME PAIRS OF STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED. OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS ARE TO BE INDICATED WITH A 2" WIDE BLACK MARK.

DRIVE PILES USING A METHOD APPROVED BY THE ENGINEER, WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

DRIVING OF THE BUILT-UP PILE WILL NOT BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 5,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

DOWEL INSTALLATION FOR OPTIONAL BUILD-UP

GROUT COMPRESSIVE STRENGTH: f'c= 5,000 PSI

BEFORE DRILLING DOWEL HOLES, REMOVE THE UPPER 3" OF CONCRETE FROM THE TOP OF THE PILE WITHOUT DAMAGE TO THE REINFORCING STEEL. THE REMOVAL PLANE SHOULD BE NORMAL TO THE EDGE OF THE PILE.

DOWEL HOLES SHALL BE POSITIONED TO MAINTAIN  $\frac{1}{2}$ " clear to all EXISTING PRESTRESSING STRANDS IN THE CONCRETE PILE.

FIELD DRILLED HOLES SHALL BE CLEAN AND FREE OF ANY OBSTRUCTIONS BEFORE GROUTING OF DOWELS. DOWEL BARS SHALL BE INSTALLED AND GROUTED WITH AN APPROVED NON-SHRINK GROUT.

THE SPIRAL REINFORCING IN ALL BUILD-UPS SHALL BE W4.0 COLD DRAWN WIRE WHICH SHALL BE SECURED TO THE LONGITUDINAL REINFORCEMENT TO MAINTAIN PITCH.

THE SPIRAL REINFORCING IN THE BUILD-UP AND THE PRESTRESSED CONCRETE PILE SHALL BE SPLICED BY OVERLAPPING A MIN. OF ONE TURN.

DocuSigned by Dicgo H. W. William	PROJEC	CT NO. <b>PITT</b> DN:	BP2	2.R018 C0 9.69 -L-	.1 UNTY
SEAL 048223 2/14/200 A AGUININ 2/14/200 A AGUININ KISINGER CAMPO	DEPA	STAT RTMENT ST 16" PR CON(	e of North Car OF TRAN RALEIGH ANDAR ESTR CRETE	OLINA NSPORTA D ESSEI E PILE	TION
& ASSOCIATES	REVISIONS SHEET NO.				
301 FAYETTEVILLE ST., SUITE 1500	NO. BY:	DATE:	NO. BY:	DATE:	S-19
RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	1		3 4		total sheets 21
			STD.N	NO. PCP	2

![](_page_60_Figure_1.jpeg)

2/14/2022 BP2.R018.1\_SMU\_RR\_730015.dgn jduke

		PROJECT NO. BP2.R018.1
GROUND LINE		PITT COUNTY
		STATION:15+09.69 -L
WIN MIN MIN	DocuSigned by CAROL	
	SEAL 048223	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
ΤΥ)	2/14/00 A AGUINT	STANDARD
	KCA	RIP RAP DETAILS
	KISINGER CAMPO & ASSOCIATES	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED	301 FAYETTEVILLE ST., SUITE 1500 RALEIGH, NC 27601 (919) 882-7839 NC FIRM LICENSE: C-1506	NO.         BY:         DATE:         NO.         BY:         DATE:         S-20           1         3         TOTAL SHEETS         21
		STD.NO.RR1 <sup>(Sht 2)</sup>

NO	tes	o o				
FOR	BERM	WIDTH	DIMENSIONS,	SEE	GENERAL	DRAWING.

ESTIMATED QUANTITIES				
E @ 5+09.69 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE For drainage		
	TONS	SQUARE YARDS		
BENT 1	152.3	170		
BENT 2	135.7	151		

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![](_page_61_Figure_1.jpeg)

2/14/2022 BP2.R018.1\_SMU\_AS\_730015.dgn iduke

![](_page_61_Figure_3.jpeg)

![](_page_61_Figure_4.jpeg)

BE PAVED. SEE ROADWAY PLANS.

![](_page_61_Figure_7.jpeg)

![](_page_61_Figure_9.jpeg)

![](_page_61_Figure_10.jpeg)

![](_page_61_Figure_12.jpeg)

![](_page_61_Figure_13.jpeg)

SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

STD.NO.BAS\_30\_90S

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![](_page_62_Figure_1.jpeg)

2/14/2022 BP2.R018.1\_SMU\_AS\_730015.dgn iduke

![](_page_62_Figure_3.jpeg)

![](_page_62_Figure_4.jpeg)

BE PAVED. SEE ROADWAY PLANS.

![](_page_62_Figure_7.jpeg)

![](_page_62_Figure_9.jpeg)

![](_page_62_Figure_10.jpeg)

![](_page_62_Figure_12.jpeg)

![](_page_62_Figure_13.jpeg)

SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	1'-11"	1'-7"
#5	2'-5"	2'-0"
#6	3'-7"	2'-5"

STD.NO.BAS\_30\_90S

#### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED  $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A  $\frac{1}{4}$  RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

## STANDARD NOTES

#### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES. DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS. AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FÁLSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ "  $\varnothing$  shear studs for the  $\frac{3}{4}$ " Ø studs specified on the plans. This substitution shall be made at THE RATE OF 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES. SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø studs based on the ratio of 3 -  $\frac{7}{8}$ " Ø STUDS FOR 4 -  $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY, AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/6" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY  $V_{16}$ INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

![](_page_63_Picture_33.jpeg)