

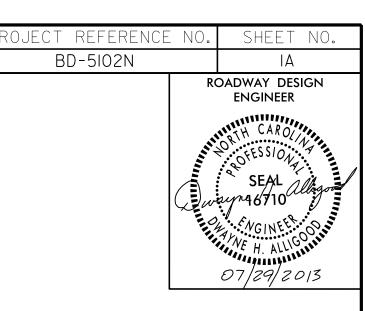


STATE	STATE	TOTAL SHEETS		
N.C.	BE	1		
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION
453	348.1.14	BRZ-1091(10)	PE	
453	48.2.14	BRZ-1091(10)	RW	
453	48.3.14	BRZ-1091(10)	CON	ST

INDEX OF SHEETS 2012 ROADWAY ENGLISH STANDARD DRAWINGS GENERAL NOTES: The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design TITLE SHEET Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are 1 A INDEX OF SHEETS, GENERAL NOTES, STANDARD DRAWINGS GRADE LINE: applicable to this projectand by reference hereby are considered a part of these plans: GRADING AND SURFACING: 1 B CONVENTIONAL SYMBOLS STD.NO. TITLE TYPICAL SECTIONS SUMMARY OF QUANTITIES DIVISION 2 - EARTHWORK 200.02 Method of Clearing - Method II 3 A SUMMARY OF DRAINAGE, GUARDRAIL AND EARTHWORK 225.02 Guide for Grading Subgrade - Secondary and Local QUANTITIES 225.04 Method of Obtaining Superelevation - Two Lane Pavement PLAN AND PROFILE SHEET 4 CLEARING: TMP1-TMP2 TRAFFIC MANAGEMENT PLANS DIVISION 3 - PIPE CULVERTS 300.01 Method of Pipe Installation EC1-EC3 EROSION CONTROL SHEETS BY METHOD II. CROSS-SECTION SUMMARY X 1 A DIVISION 4 - MAJOR STRUCTURES CROSS-SECTIONS X 1 SUPERELEVATION: 422.10 Reinforced Bridge Approach Fills S1-S18 STRUCTURE PLANS (BRIDGE) ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH DIVISION 5 - SUBGRADE, BASES AND SHOULDERS 560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I DIVISION 8 - INCIDENTALS 815.03 Pipe Underdrain and Blind Drain Concrete Base Pad for Drainage Structures SHOULDER CONSTRUCTION: 840.00 Frames and Narrow Slot Flat Grates 840.29 840.35 Traffic Bearing Grated Drop Inlet 840.45 Precast Drainage Structure 840.66 Drainage Structure Steps UNDERDRAINS: 862.01 Guardrail Placement Guardrail Installation 862.02 862.03 Structure Anchor Units Guide for Rip Rap at Pipe Outlets 876.02 DRIVEWAYS: GUARDRAIL: END BENTS: UTILITIES: RIGHT-OF-WAY MARKERS: ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

- STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01. UNDERDRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.03 AT LOCATIONS DIRECTED BY ENGINEER. DRIVEWAYS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. 848.02 USING 3' RADII OR RADII AS SHOWN ON THE PLANS, LOCATIONS OF DRIVES WILL BE AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER. THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL. THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE. ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS, EXCEPT AS SHOWN ON THE PLANS.
- CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED
- THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS, GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

2012 SPECIFICATIONS EFFECTIVE: 01-17-12 REVISED: 11/01/11



## Note: Not to Scale \*S.U.E. = Subsurface Utility Engineering

# CONVENTIONAL PLAN SHEET SYMBO

## **BOUNDARIES AND PROPERTY:**

State Line ————————————————————————————————————	
County Line	
Township Line	
City Line	
Reservation Line	
Property Line	
Existing Iron Pin	
Property Corner	—×
Property Monument	
Parcel/Sequence Number (23)	
Existing Fence Line	-X-
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	
Existing Endangered Plant Boundary	
BUILDINGS AND OTHER CULTURE:	
Gas Pump Vent or U/G Tank Cap — O	
Sign O	
Well	
Small Mine 🔶 🛠	
Foundation	
Area Outline	
Cemetery †	]
Building	

## HYDROLOGY:

School

Church

Dam -

Stream or Body of Water	
Hydro, Pool or Reservoir	
Jurisdictional Stream	
Buffer Zone 1	— — BZ 1 — — —
Buffer Zone 2	BZ 2
Flow Arrow	~~~~~
Disappearing Stream	
Spring	-0
Wetland	— ¥
Wetland Boundary	WLB
Proposed Lateral, Tail, Head Ditch	
False Sump	

RAILRO

Standard C RR Signal A Switch — RR Abando RR Disman *RIGHT* Baseline C Existing Rig Existing Rig Proposed R

Proposed I Iron Pir Proposed I

Concret Existing Co

Proposed ( Existing Ea

Proposed <sup>-</sup>

Proposed 7

Proposed I Proposed I

Proposed <sup>-</sup>

Proposed F Iron Pin

## ROADS AND RELATED FEATURES:

Existing Ed Existing Cu Proposed S Proposed S Proposed C Existing Ma Proposed C Existing Co Proposed C Equality Sy Pavement R *VEGETA* Single Tree Single Shru

Single Tree Single Shru Hedge — Woods Line Orchard — Vineyard —

EUISVUKEENENBU-DIØZN\_Ureene 4/NBUDIØZN\_ddcZ\_psn\_Ib.dgn ERNAME\$\$\$\$

## STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

DADS:	
Gauge	$-\frac{1}{CSX} + \frac{1}{TRANSPORTATION}$
Milepost	_ 💮 MILEPOST 35
	– SWITCH
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ntled	
OF WAY:	
Control Point	•
ight of Way Marker	$\bigtriangleup$
ight of Way Line	
Right of Way Line	
Right of Way Line with in and Cap Marker	
Right of Way Line with	
Control of Access	( <u>Ĉ</u> )
Control of Access	
asement Line	——————————————————————————————————————
Temporary Construction Easement –	E
Temporary Drainage Easement —	TDE
Permanent Drainage Easement ——	PDE
Permanent Utility Easement	PUE
Temporary Utility Easement	TUE
Permanent Easement with n and Cap Marker	$\langle \diamond \rangle$

dge of Pavement	
Curb	
Slope Stakes Cut	<u>C</u>
Slope Stakes Fill	F
Curb Ramp	CR
Netal Guardrail ————————————————————————————————————	<u> </u>
Guardrail ————	<u> </u>
Cable Guiderail	
Cable Guiderail	
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Removal	$\boxtimes$
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	Vineyard

## **EXISTING STRUCTURES:**

MAJOR:		
Bridge, Tunnel or Box Culvert		CONC
Bridge Wing Wall, Head Wall and End Wall	_	) CONC WW
MINOR:		
Head and End Wall		CONC HW
Pipe Culvert		
Footbridge	$\succ$	
Drainage Box: Catch Basin, DI or JB ———		СВ
Paved Ditch Gutter		
Storm Sewer Manhole		S
Storm Sewer		S

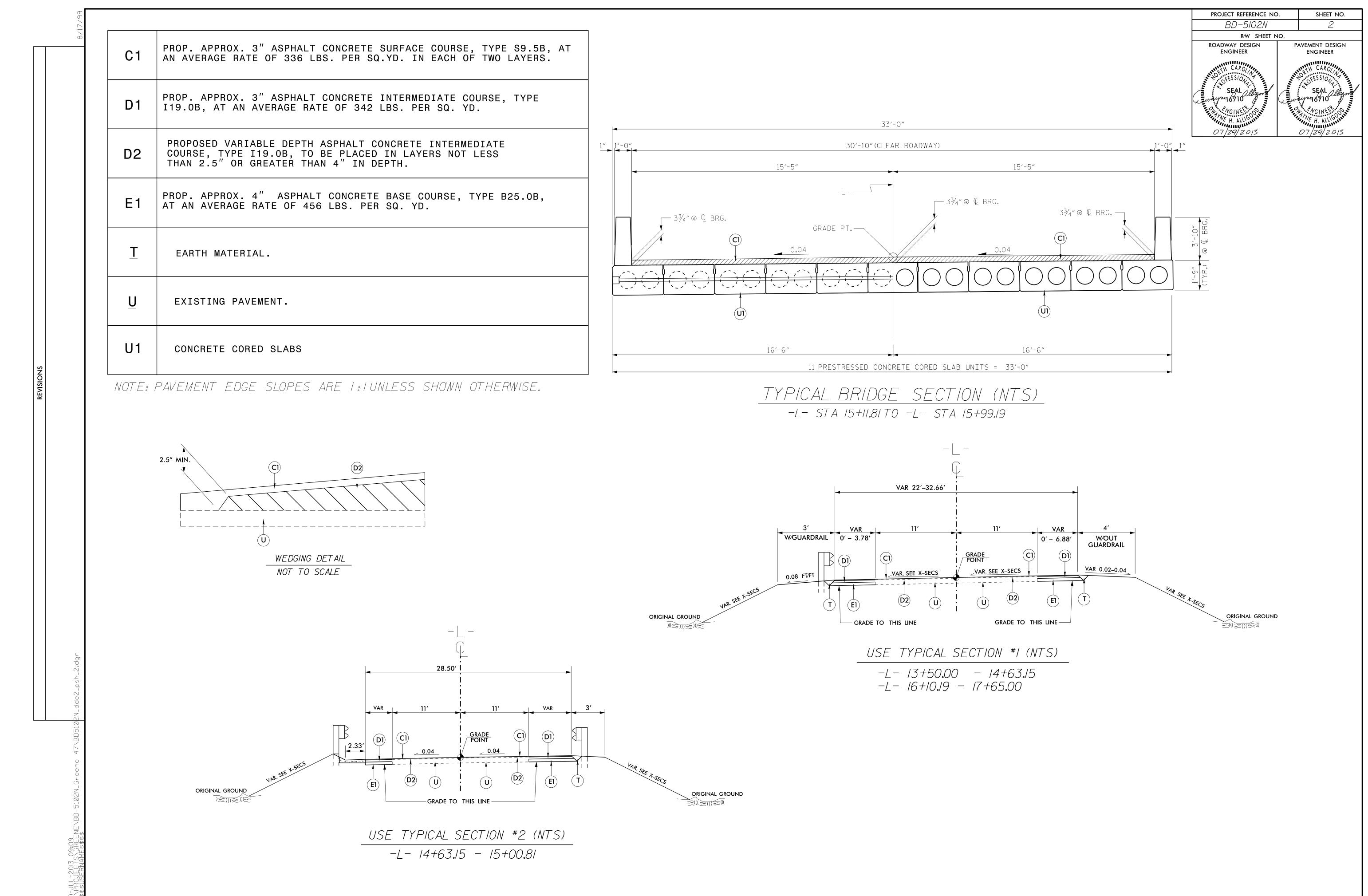
### **UTILITIES:**

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••
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— — P — -

### TELEPHONE:

Existing Telephone Pole	
Proposed Telephone Pole	-0-
Telephone Manhole	$\bigcirc$
Telephone Booth	٦
Telephone Pedestal	$\Box$
Telephone Cell Tower	$\sqrt{\Phi}_{y}$
U/G Telephone Cable Hand Hole ———	HH
Recorded U/G Telephone Cable	T
Designated U/G Telephone Cable (S.U.E.*) $-$	— — — T — -
Recorded U/G Telephone Conduit	TC
Designated U/G Telephone Conduit (S.U.E.* <del>)</del>	— — — — TC— ·
Recorded U/G Fiber Optics Cable	T F0
Designated U/G Fiber Optics Cable (S.U.E.* <del>)</del>	— — — — T FO—

	PROJECT REFERENCE NO.	SHEET
	BD-5102N	<i>\</i> [
_S		
WATER:		
Water Manhole		Ŵ
Water Meter		$\bigcirc$
Water Valve		×
Water Hydrant		÷
Recorded U/G Water Line —		——— w ————
Designated U/G Water Line (S.	U.E.*) — — — — —	w w
Above Ground Water Line ——	A	/G Water
TV:		
TV Satellite Dish ————		$\ltimes$
TV Pedestal		C
TV Tower		$\bigotimes$
U/G TV Cable Hand Hole		Нн
Recorded U/G TV Cable ——		TV
Designated U/G TV Cable (S.U		
Recorded U/G Fiber Optic Cabl		
Designated U/G Fiber Optic Ca		
Gas Valve Gas Meter		$\diamondsuit$
Recorded U/G Gas Line		G
Designated U/G Gas Line (S.U.	E.*)	- — C — — —
Above Ground Gas Line		A/G Gas
SANITARY SEWER:		
Sanitary Sewer Manhole		$\oplus$
Sanitary Sewer Cleanout		(†)
U/G Sanitary Sewer Line ——		÷
Above Ground Sanitary Sewer		
Recorded SS Forced Main Line		
Designated SS Forced Main Lin		
MISCELLANEOUS:		-
Utility Pole		
Utility Pole with Base		·
Utility Located Object		$\odot$
Utility Traffic Signal Box		S
Utility Unknown U/G Line —		
U/G Tank; Water, Gas, Oil	L	
A/G Tank; Water, Gas, Oil	L	
U/G Test Hole (S.U.E.*)		
Abandoned According to Utility	Records	ATUR
		E.O.I.



### STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS SUMMARY OF QUANTITIES

ITEM	SECT	QUANTITY	UNIT	ITEM DESCRIPTION	ITEM	SECT	QUANTITY	UNIT	ITEM DESCRIPTION
1	800	1	LS	MOBILIZATION	30	SP	350	LF	SAFETY FENCE
2	801	1	LS	CONSTRUCTION SURVEYING	3/	1630	5	СҮ	SILT EXCAVATION
3	SP	1	LS	REINFORCED BRIDGE APPROACH FILL,-L- STA 15+55.50	32	1631	460	SY	MATTING FOR EROSIO
4	226	1	LS	GRADING	33	1632	70	LF	¼" HARDWARE CLOTH
5	226	200	СҮ	UNDERCUT EXCAVATION	34	SP	80	SY	FLOATING TURBIDITY
6	300	10	TON	FOUNDATION CONDITIONING MATERIAL, MINOR STRUCTURES	35	SP	200	LF	WATTLE
7	300	30	SY	FOUNDATION CONDITIONING GEOTEXTILE	36	1660	1	ACRE	SEEDING AND MULCHI
8	310	68	LF	15" R.C.PIPE CULVERTS, CLASS III	37	1661	50	LB	SEED FOR REPAIR S
9	610	60	TON	ASPHALT CONCRETE BASE COURSE,TYPE B25.0B	38	1661	0.2	TON	FERTILIZER FOR REI
10	610	210	TON	ASPHALT CONCRETE INTERMEDIATE COURSE, TYPE 119.0B					
//	610	275	TON	ASPHALT CONCRETE SURFACE COURSE,TYPE S9.5B					
12	620	30	TON	ASPHALT BINDER FOR PLANT MIX	39	402	1	LS	REMOVAL OF EXISTIN
13	840	2	EA	MASONRY DRAINAGE STRUCTURES	40	450	1	ΕA	PDA TESTING
14	840	2	ΕA	FRAME WITH GRATE,STD 840.29	41	412	1	LS	UNCLASSIFIED STRUC
15	846	35	LF	SHOULDER BERM GUTTER	42	420	39.4	СҮ	CLASS A CONCRETE (
16	862	124	LF	STEEL BEAM GUARDRAIL, SHOP CURVED	43	422	1	LS	BRIDGE APPROACH S
17	862	10	ΕA	ADDITIONAL GUARDRAIL POSTS	44	425	64/6	LB	REINFORCING STEEL
18	862	4	EA	GUARDRAIL ANCHOR UNITS, TYPE III	45	450	630	LF	HP 12 X 53 STEEL H
19	862	4	EA	GUARDRAIL ANCHOR UNITS, TYPE 350	46	450	560	LF	HP 14 X 73 GALVANIZ
20	876	5	TON	RIP RAP,CLASS I	47	450	8	ΕA	STEEL PILE POINTS
21	876	5	TON	RIP RAP,CLASS B	48	450	12	ΕA	PILE REDRIVES
22	876	5	SY	GEOTEXTILE FOR DRAINAGE	49	460	170.50	LF	CONCRETE BARRIER
23	1605	700	LF	TEMPORARY SILT FENCE	50	876	247	TON	RIP RAP,CLASS II (2'-
24	1610	5	TON	STONE FOR EROSION CONTROL,CLASS B	5/	876	275	SY	GEOTEXTILE FOR DR.
25	1610	5	TON	SEDIMENT CONTROL STONE	5 <i>2</i>	430	1	LS	ELASTOMETRIC BEARI
26	1615	1	ACRE	TEMPORARY MULCHING	54	430	935	LF	3'-0" X  '-9" PRESTRE
27	1620	50	LB	SEED FOR TEMPORARY SEEDING					
28	1620	0.2	TON	FERTILIZER FOR TEMPORARY SEEDING					
29	1622	200	LF	TEMPORARY SLOPE DRAINS					

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PROJECT REFERENCE NO.	SHEET NO.
BD-5/02N	3

SAFETY FENCE SILT EXCAVATION ATTING FOR EROSION CONTROL "HARDWARE CLOTH LOATING TURBIDITY CURTAIN VATTLE SEEDING AND MULCHING SEED FOR REPAIR SEEDING ERTILIZER FOR REPAIR SEEDING

EMOVAL OF EXISTING STRUCTURE AT -L- STA 15+55.50 PDA TESTING INCLASSIFIED STRUCTURE EXCAVATION LASS A CONCRETE (BRIDGE) BRIDGE APPROACH SLABS REINFORCING STEEL (BRIDGE) HP 12 X 53 STEEL PILES HP 14 X 73 GALVANIZED STEEL PILES STEEL PILE POINTS PILE REDRIVES ONCRETE BARRIER RAIL RIP RAP, CLASS II (2'-O" THICK) EOTEXTILE FOR DRAINAGE LASTOMETRIC BEARINGS "-0" X I'-9" PRESTRESSED CONCRETE CORED SLABS

COMPUTED BY: <u>VT</u> CHECKED BY: <u>LJ</u> NOTE: Invert See "S		DA	re: <u>3–15–11</u> re: <u>3–15–11</u> for Bid P ications	ourpose						<sup>-</sup> constr	uction			ST (	OF P	<b>IPES</b>	I	DIVIS	OF 1 Sion I <i>lls</i> ,	OF	HI	GHW	AYS	)		ઐ	UNI	DER)								PR	OJECT REFER	
STATION (LT,RT, OR CL)		VATION	ELEVATION	ELEVATION	CRITICAL	(RCP,	DRAINAG CSP, CAAP,	E PIPE HDPE, or P	VC)			RUGATED INUM PIP	E		R.C. PIPE (CLASS II	)		R.C. PIF (CLASS	PE IV)	CONTRACTOR DESIGN PIPE	CONTRACTOR DESIGN PIPE	ST ST	NDWALLS D. 838.01, D. 838.11 OR D. 838.80 UNLESS NOTED THERWISE)		* TOTAL L.F. FOR PAY AUANTITY SHALL BE COL. 'A' + (1.3 X COL.'B')		FRAME, G AND H STANDARD	DOD	CONCRETE TRANSITIONAL SECTION	TE STD. 840.22	) GRATES STD. 840.22 GRATE STD. 840.29	TWO GRATES STD. 840.24	D. & SIZE	C.Y. STD 840.72	UG, C.Y. STD. 840.71		C.B. N.D.I. D.I. G.D.I. G.D.I. (N	ABBREVIATIONS CATCH BASIN NARROW DROP INLET DROP INLET GRATED DROP INLET S.) GRATED DROP INLET (NARROW SLOT)
SIZE SIZE OLY OUT	FROM	TOP ELEV	INVERT E	INVERT E	0 12" 15	" 18" 24	" 30" 36"		DO NOT USE RCP DO NOT USE CSP DO NOT USE CAAP	NOT USE HDP					24" 30"	36" 42" 48	" 12" 15"	18" 24" 30	0" 36" 42"	**" R. C. PIPE (CLASS V)	ט   נ	15" SIDE DRAIN PIPE 18" SIDE DRAIN PIPE		ER EACH (0' THRU	5.0' THRU 10.0' > 10.0' AND ABOVE 9	C.B. STD. 840.01 OR S1	TYPE OF		CATCH BASIN DROP INLET	G.D.I. FRAME WITH GRA	G.D.I. FRAME WITH TWO G.D.I. (N.S.) FRAME WITH		CORR. STEEL ELBOWS NO	CONC. COLLARS CL. "B"	CONC. & BRICK PIPE PLI	PIPE REMOVAL LIN.FT.	J.B. M.H. T.B.D.I. T.B.J.B.	JUNCTION BOX MANHOLE TRAFFIC BEARING DROP I TRAFFIC BEARING JUNCTI REMARKS
-L- 14+68.30 LT	1	39.93	36.86																					1							1	1						
-L- 14+96.25 LT	2	39.98	36.98																					1							1	1						
-L- 14+82.32 LT			36.98											24							_																	
_L_ 14+68.30 LT	1 OL	TL	36.86	36.64										44																								
TOTALS														68										2							2	2						

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT. FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL. W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL. G = GATING IMPACT ATTENUATOR TYPE 350

NG = NON-GATING IMPACT ATTENUATOR TYPE 350

SURVEY LINE	BEG. STA.	END STA.	LOCATION	(WITH	LENGTH ANCHOR DEDI	JCTIONS)	WARRAN	t point	"N" DIST.	TOTAL SHOULDER	FLARE	LENGTH	w	/				ANCHORS	ATT T	IMPACT TENUATOR TYPE 350	R SINGLE FACED	REMOVE AND	REMOVE AND STOCKPILE	REMARKS
LINE	BEG. STA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	TYPE 350	TYPE III	CAT-1			PERMITTEI	GUARDRAIL	AND RESET EXISTING GUARDRAIL	EXISTING GUARDRAIL	KEMAKKS
-L-	14+10.90	15+12.81	LT	0	30			15+12.81	2.4	5.33		50		1.0	1	1								
-L-	14+17.37	15+12.81	RT	0	32		15+12.81		2.4	5.33	50		1.0		1	1								
-L-	15 + 98.19	17 + 00.95	LT	0	30		15+98.19		2.4	5.33	50		1.0		1	1								
-L-	15 + 98.19	16+94.53	RT	0	32			15+98.19	2.4	5.33		50		1.0	1	1								
TOTAL				0	124										4	4								

## SUMMARY OF EARTHWORK IN CUBIC YARDS

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT + %	BORROW	WASTE
-L-14+00.00-15+11.00	20	0	44	22	0
-L- 16+00.00 - 17+00.00	29	0	79	50	0
UNDERCUT (CONTINGENCY)		200	240	240	200
UNCLASSIFIED STRUCTURE EXCAVATION	100	0	0	0	100
SUB TOTAL	149	200	363	312	300
SAY	150	200	370	320	300

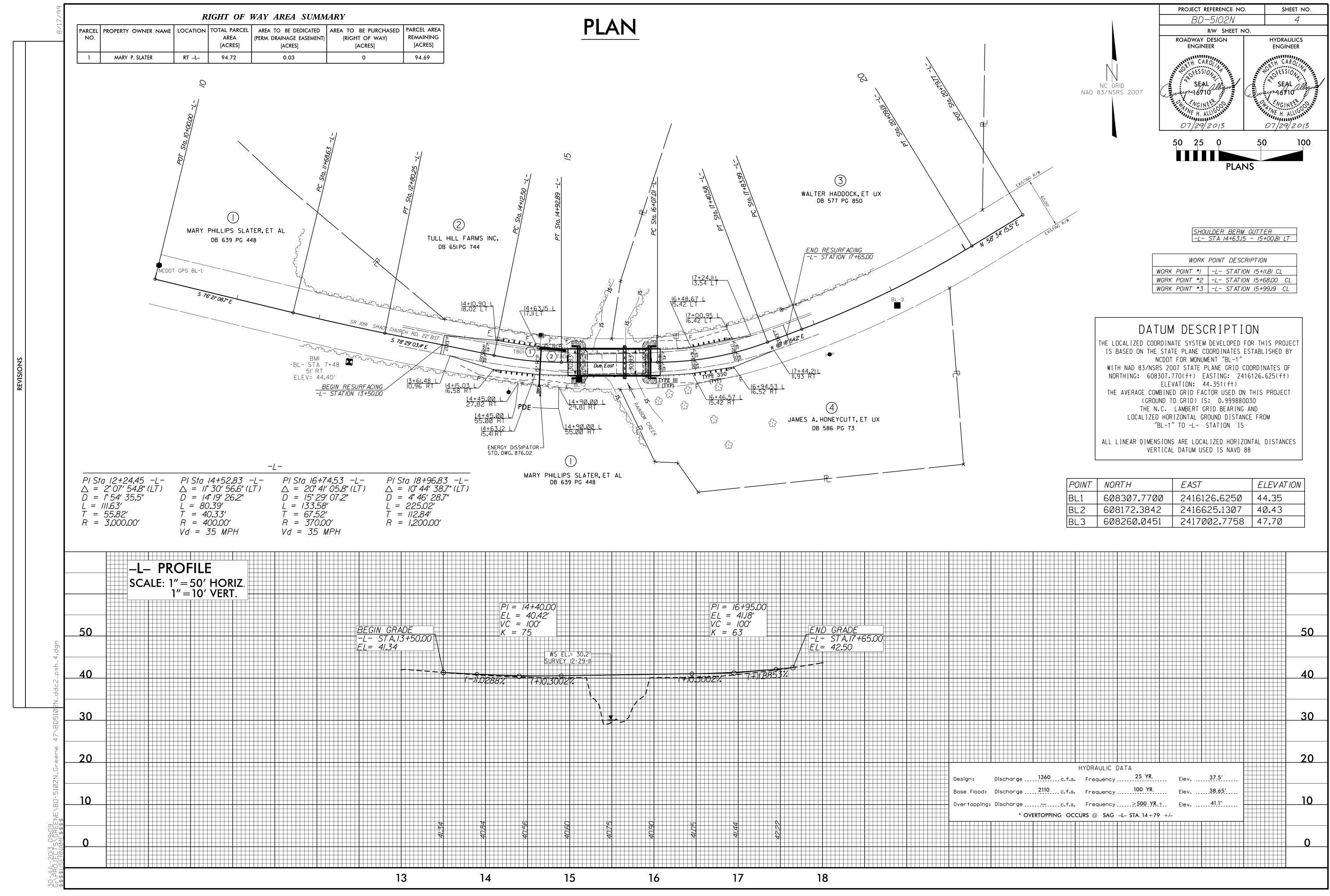
## GUARDRAIL SUMMARY

## PAVEMENT REMOVAL SUMMARY IN SQUARE YARDS

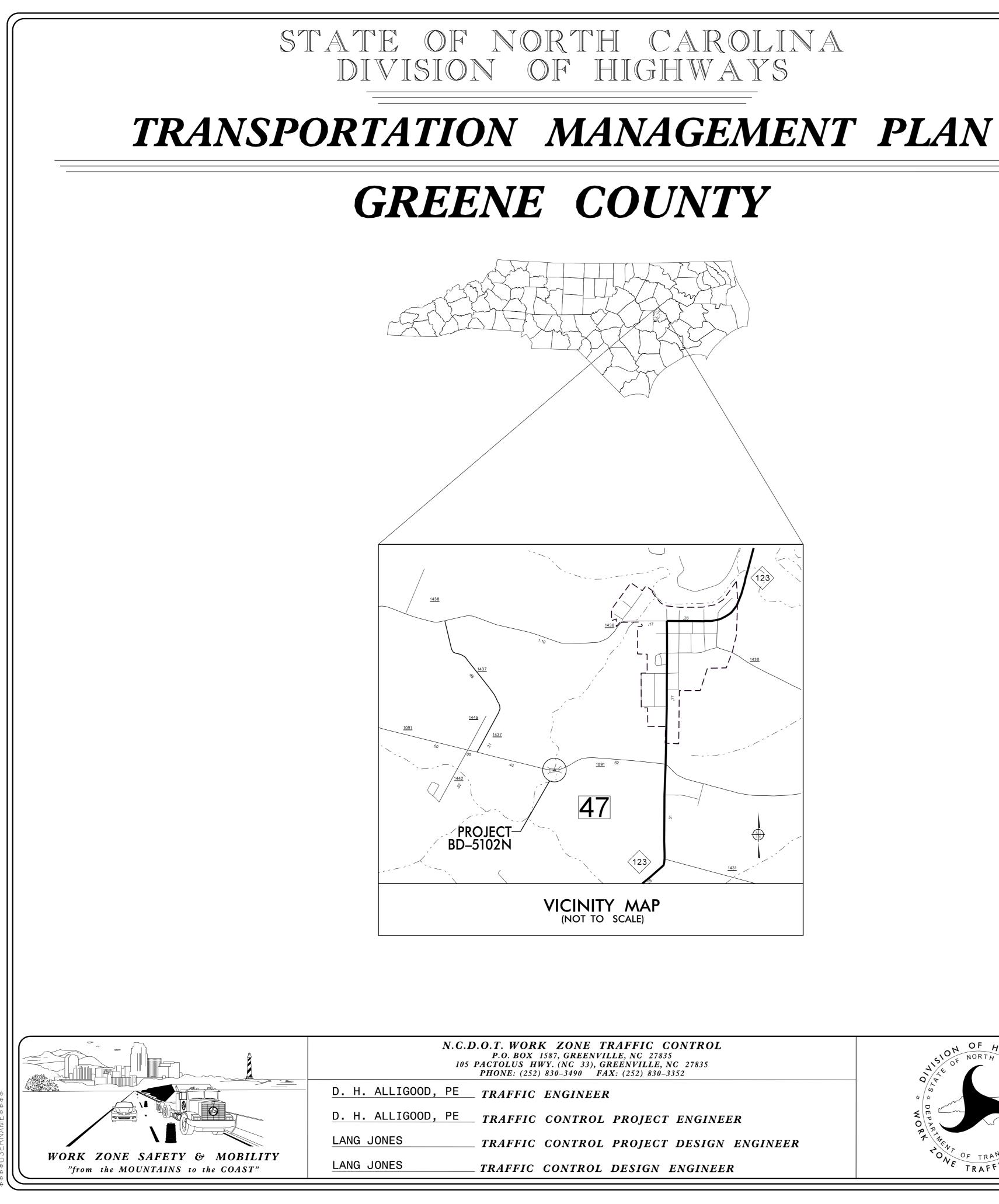
LINE	STATION – STATION	LOCATION	REMOVAL (SY)
-L-	15+00 - 15+23	CL	60
-L-	15+93 - 16+10	CL	44
TOTAL			104
		SAY	110

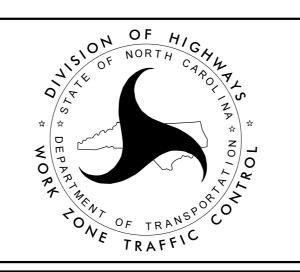
## NOTE:

APPROXIMATE QUANTITIES ONLY, UNCLASSIFIED EXCAVATION, STRUCTURE EXCAVATION, BORROW EXCAVATION, FINE GRADING, CLEARING AND GRUBBING, BREAKING OF EXISTING PAVEMENT AND REMOVAL OF EXISTING PAVEMENT WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR "GRADING."



POINT	NORTH	EAST	ELEVATION
BL1	608307.7700	2416126.6250	44.35
BL2	608172.3842	2416625.1307	40.43
BL3	608260.0451	2417002.7758	47.70





SHEET NO.

TMP - 1

TMP-2

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS"-HIGHWAY DESIGN BRANCH-N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C. DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

## STD. NO.

1101.11 1110.01 1145.01

DIRECTION OF TRAFFIC FLOW DIRECTION OF PEDESTRIAN TRAFFIC FLOW NORTH ARROW — PROPOSED PVMT. ----- EXIST. PVMT.

NDEX O	F SHEETS
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## TITLE

TITLE SHEET WITH VICINITY MAP & INDEX OF SHEETS LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS, AND LEGEND.

SHEET NO.

TMP-1

5102N

PROJECT NOTES, DETOUR AND PLANS.

## ROADWAY STANDARD DRAWINGS

1101.03 (SHT. 1 OF 9)

## TITLE

TEMPORARY ROAD CLOSURES TRAFFIC CONTROL DESIGN TABLES STATIONARY WORK ZONE SIGNS BARRICADES (TYPE III)

## LEGEND

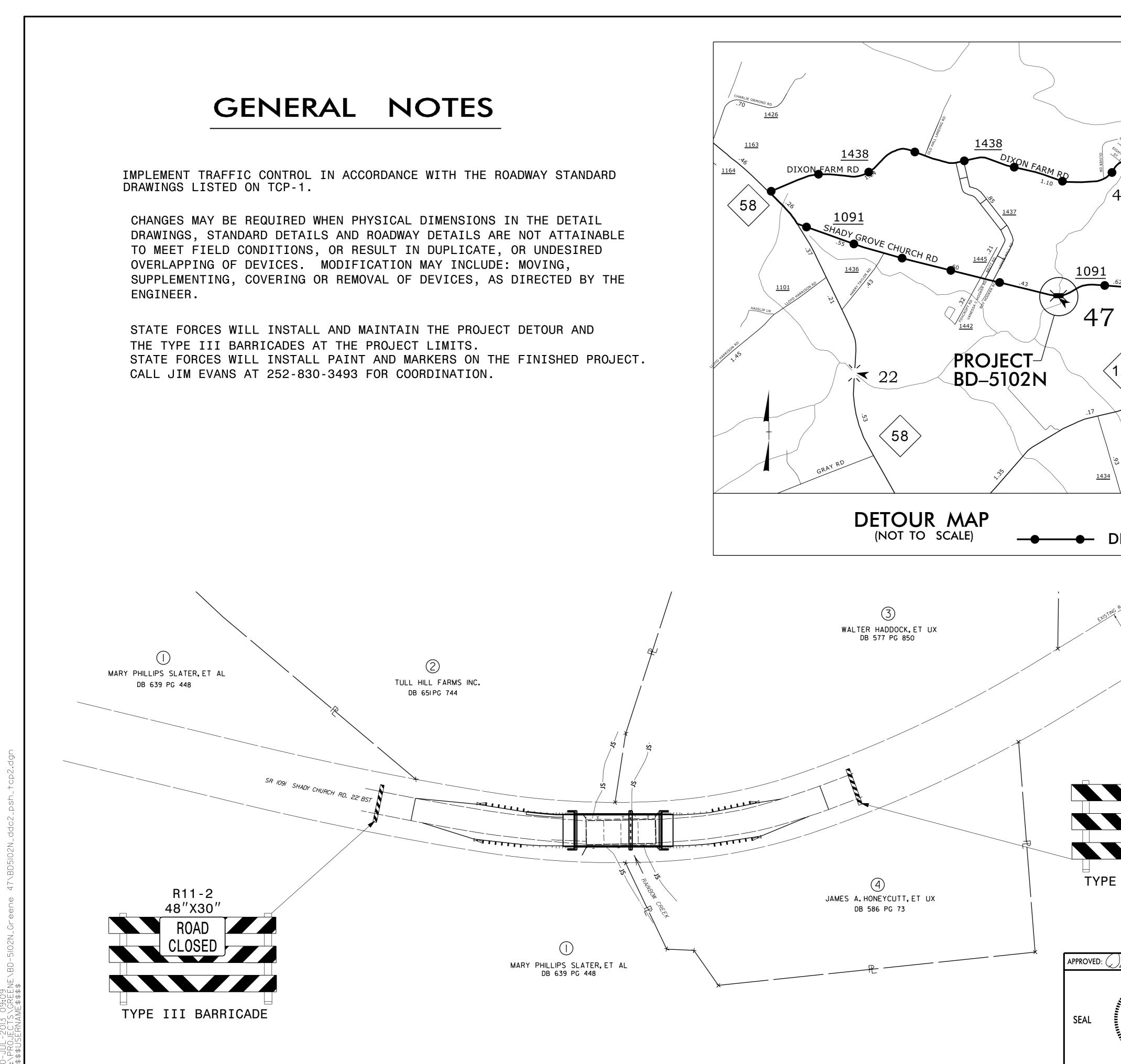
## GENERAL

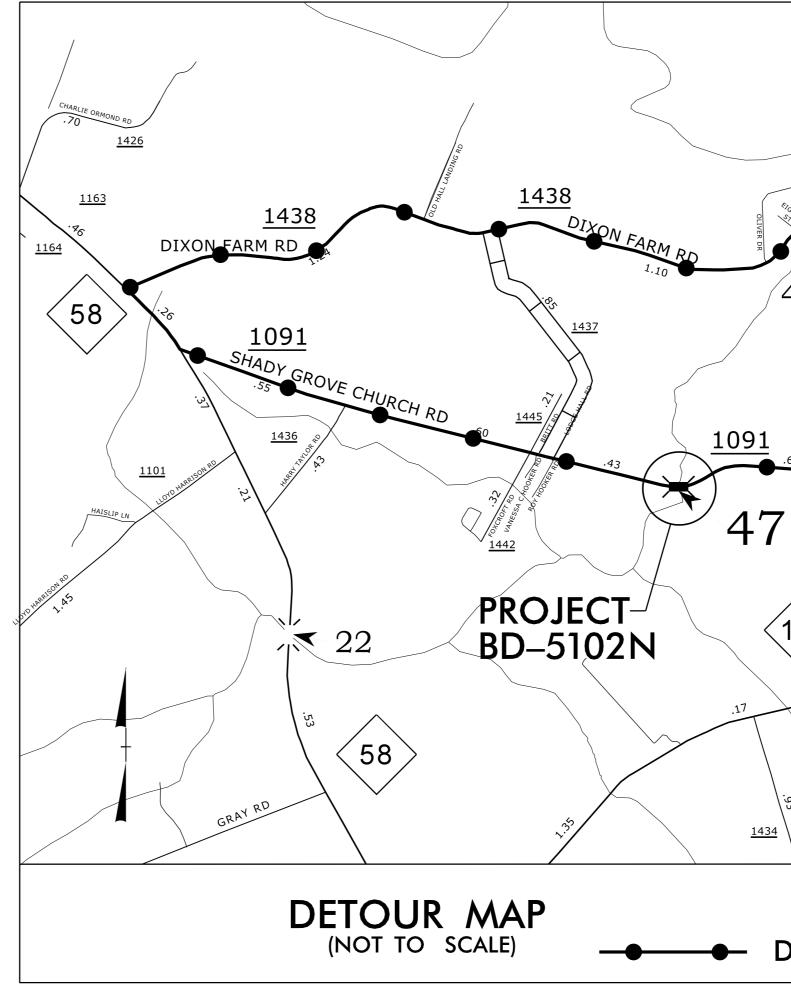
WORK AREA

## TRAFFIC CONTROL DEVICES

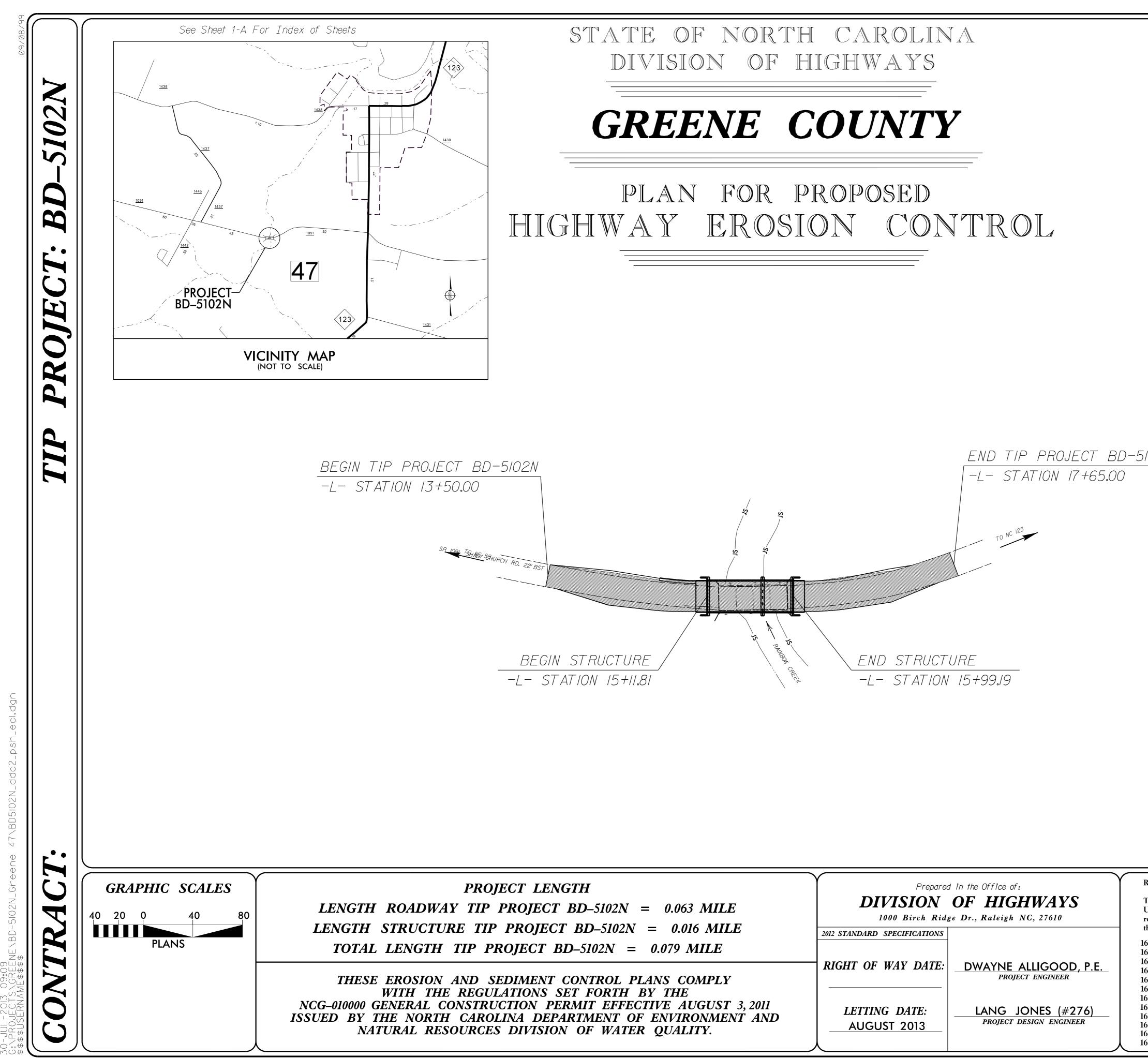
BARRICADE (TYPE III)

APPROVED: wayne 17. allegood DATE: 07/29/2013	
SEAL SEAL 16710 SEAL	





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S ≷					
.62 VILLAGE PARK DR					
.62 VILLAGE PARK DR HOOKER DR HUGO R					
	D				
123					
· 2 <sup>9</sup>	15				
	40				
.9. 2.					
<u><u>1432</u></u>					
DETOUR ROUTE					
C R/W					
6.22					
EXISTING R/W					
R11-2 48″X30″		I N NC GF			
ROAD		NAD 83/NSF			
E III BARRICADE					
) wayne H. allegood	P	ROJEC	T NOT	ΈS,	
THE CAROLINE	DET	OUR	AND F	<b>PLA</b> N	1S
SFAL S				_	EVISIONS
SEAL 16710	ATE: 6/2/12				
SEAL 16710	wg. by: LJ esign by: LJ				
07/29/2013 RI	EVIEWED BY: BB			CADD FILE	

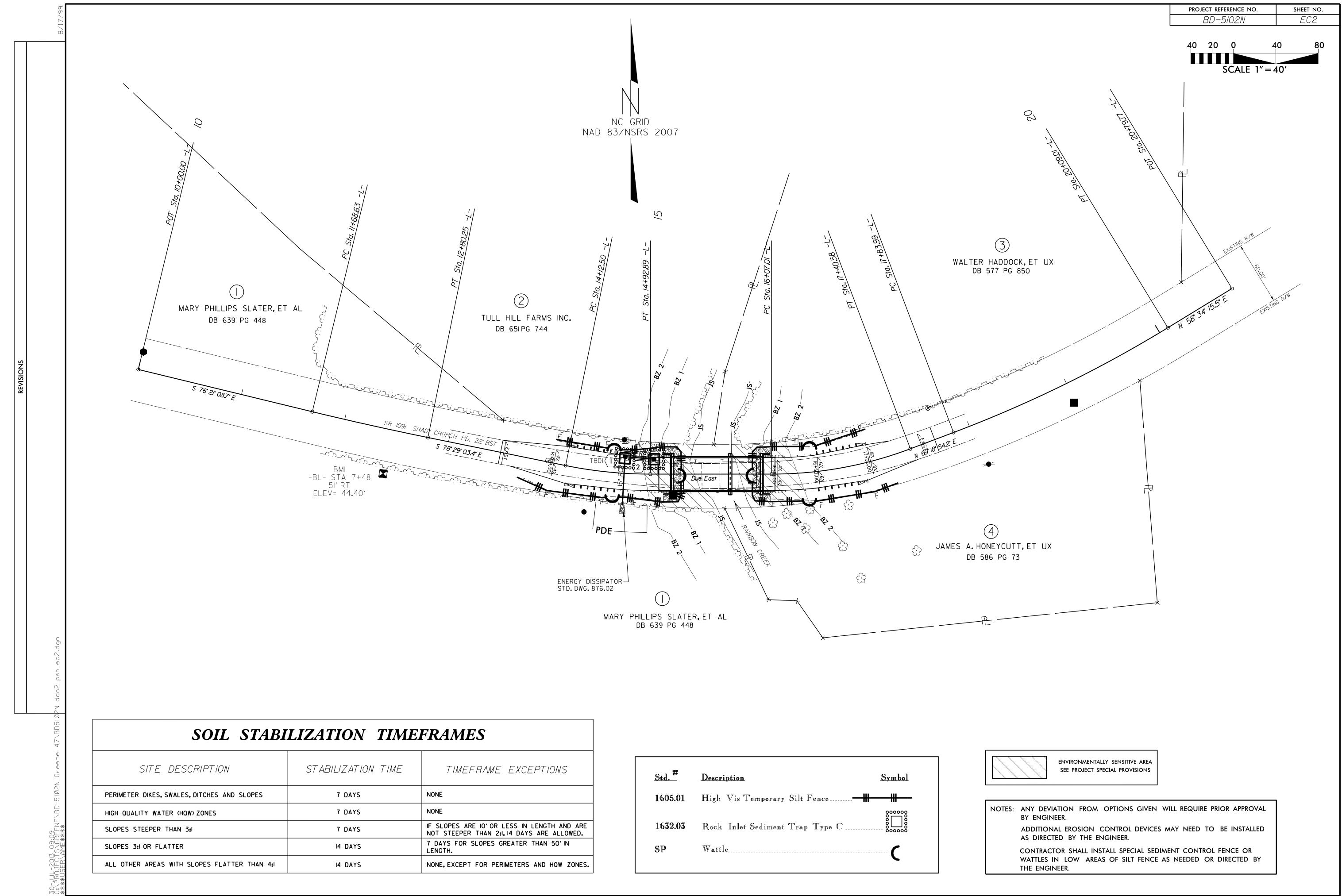


DJECT LENGTH P PROJECT BD-5102N = 0.063 MILE P PROJECT BD-5102N = 0.016 MILE	DIVISION	d in the Office of: <b>OF HIGHWAYS</b> ge Dr., Raleigh NC, 27610		
PROJECT BD-5102N = 0.079 MILE $PROJECT BD-5102N = 0.079 MILE$	2012 STANDARD SPECIFICATIONS RIGHT OF WAY DATE:	DWAYNE ALLIGOOD, P.E.		
SEDIMENT CONTROL PLANS COMPLY ULATIONS SET FORTH BY THE CUCTION PERMIT EFFECTIVE AUGUST 3, 2011 OLINA DEPARTMENT OF ENVIRONMENT AND ES DIVISION OF WATER QUALITY.	LETTING DATE: AUGUST 2013	PROJECT ENGINEER		

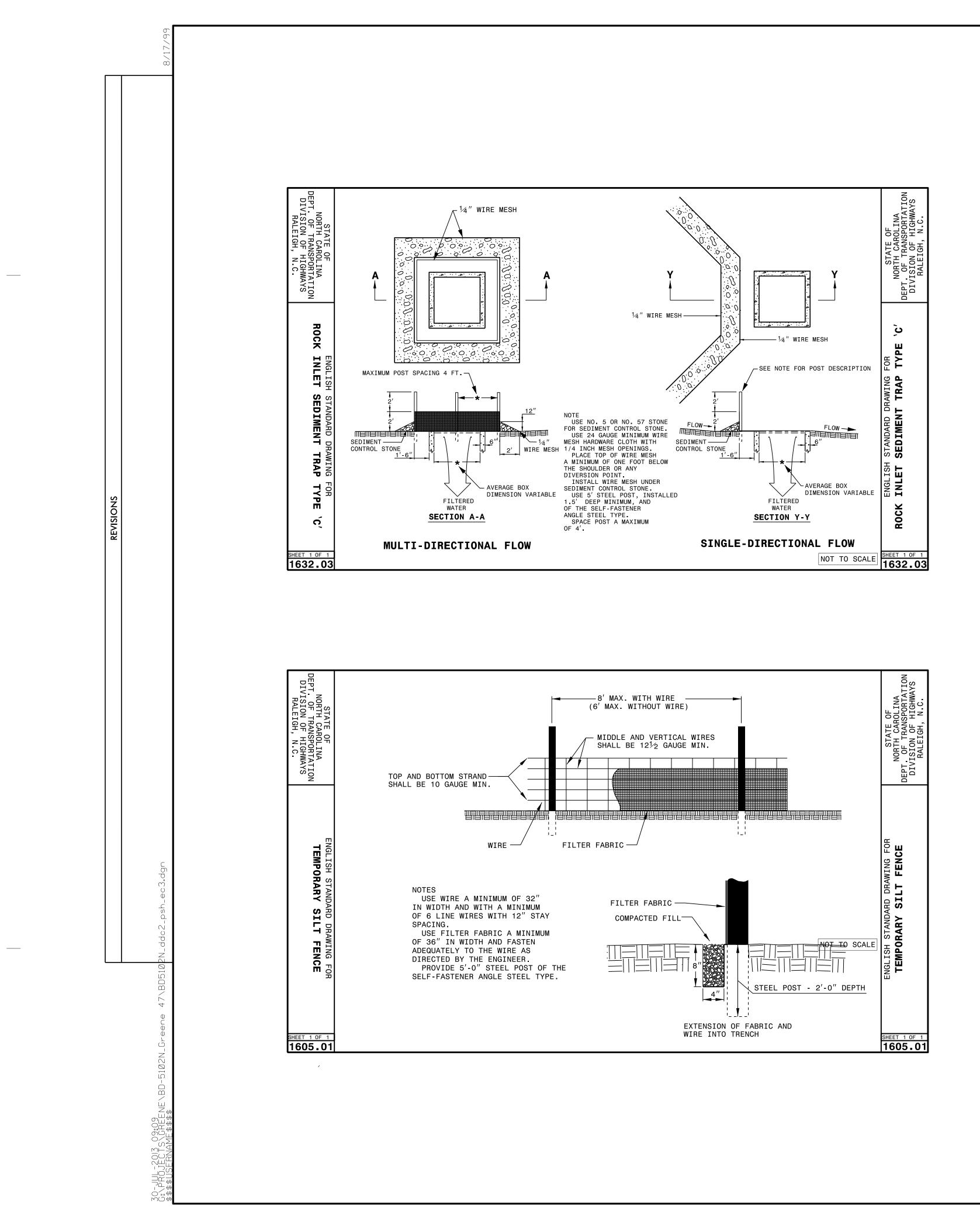
	STATE		PROJECT REFERENCE NO.	SHEET TO' NO. SHI
	N.C.	BE	D-5102N	
	STATE P 4534		F. A. PROJ. NO. BRZ-1091(10)	DESCRIPTION PE
	4534		BRZ-1091(10)	RW
	4534	3.3.14	BRZ-1091(10)	CONST
				I
EROSION	I AND SED	IMENT	CONTROL MEA	ASURES
<u>Std.</u> #	Description			Symbol
	_			
1605.01	High Vis Te	mporary	Silt Fence —+	
1632.03	Rock Inlet S	Sediment '	Frap Type C	
an	ו , צאוא			80000008
SP	Wattle			(
		TH	IS PROJECT CON	NTAINS
		ERC	<b>DSION CONTROL</b>	PLANS
			FOR CLEARING A	AND
			GRUBBING PHASE	
$\mathbb{N}$				
			CONSTRUCTION	N.
		E E	NVIRONMENTA	LLY
		SEN	SITIVE AREA(S	) EXIST
			ON THIS PRÒJ	<i>′</i>
			efer To E. C. Special P	
N			for Special Considera	uions.
I   N NC GRID				
83/NSRS 20	07		THE DECIECT I	
		_	THIS PROJECT I	
			BEEN DESIGNED	
		S S	ENSITIVE WATER	SHED
			STANDARDS.	
ay Standard D				

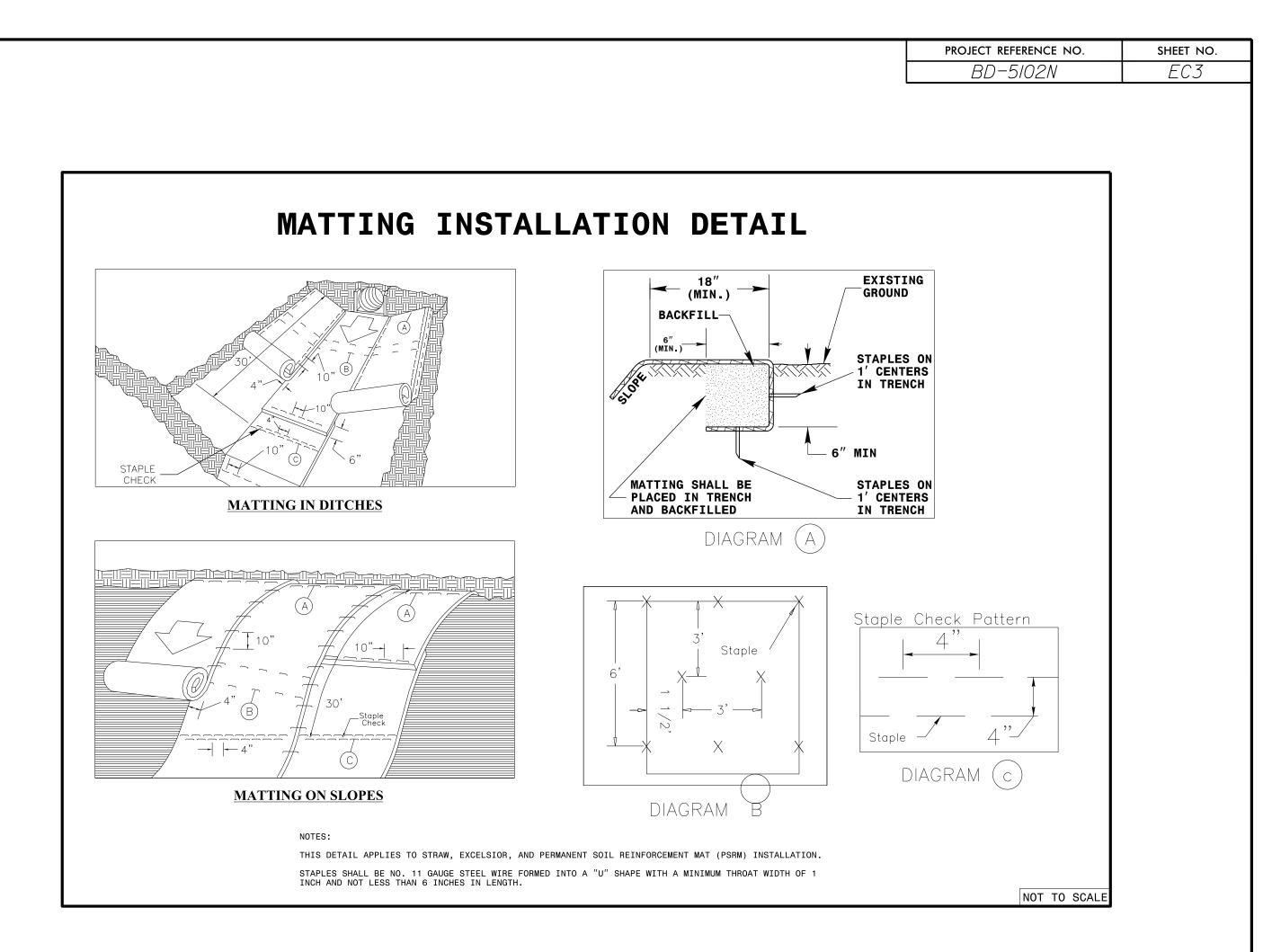
1632.01Rock Inlet Sediment TrapType A1632.02Rock Inlet Sediment TrapType B 1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1632.03 Rock Inlet Sediment Trap Type C 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1633.01 Temporary Rock Silt Check Type A 1622.01 Temporary Berms and Slope Drains 1633.02 Temporary Rock Silt Check Type B 1630.01 Riser Basin 1634.01 Temporary Rock Sediment Dam Type A 1630.02 Silt Basin Type B 1634.02Temporary Rock Sediment Dam Type B1635.01Rock Pipe Inlet Sediment Trap Type A 1630.03 Temporary Silt Ditch 1635.02Rock Pipe Inlet Sediment Trap Type B1640.01Coir Fiber Baffle 1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06Special Stilling Basin1631.01Matting Installation 1645.01 Temporary Stream Crossing

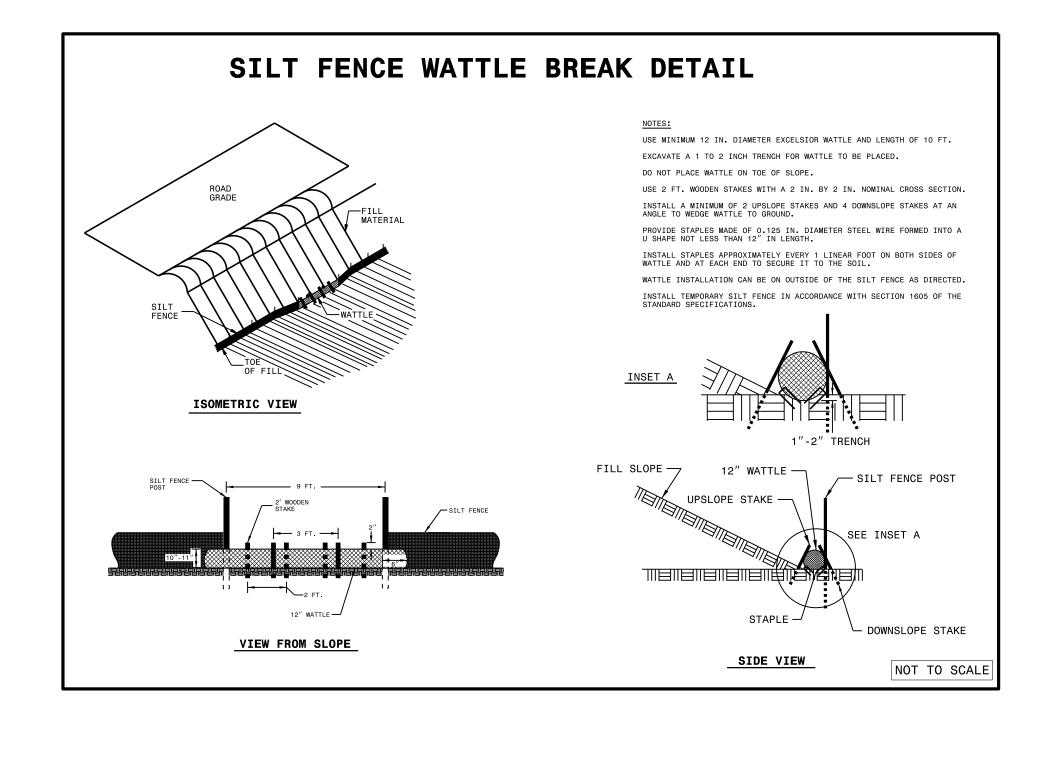
these plans.



<b>S</b>			
RAME EXCEPTIONS	<u>Std.</u> #	Description	Symbol
	1605.01	High Vis Temporary Silt Fence	
E IO' OR LESS IN LENGTH AND ARE THAN 2:1, 14 DAYS ARE ALLOWED.	1632.03	Rock Inlet Sediment Trap Type C	
PES GREATER THAN 50' IN	SP	Wattle	
OR PERIMETERS AND HOW ZONES.			







NOTE: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract Lump Sum price for "Grading".

## DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

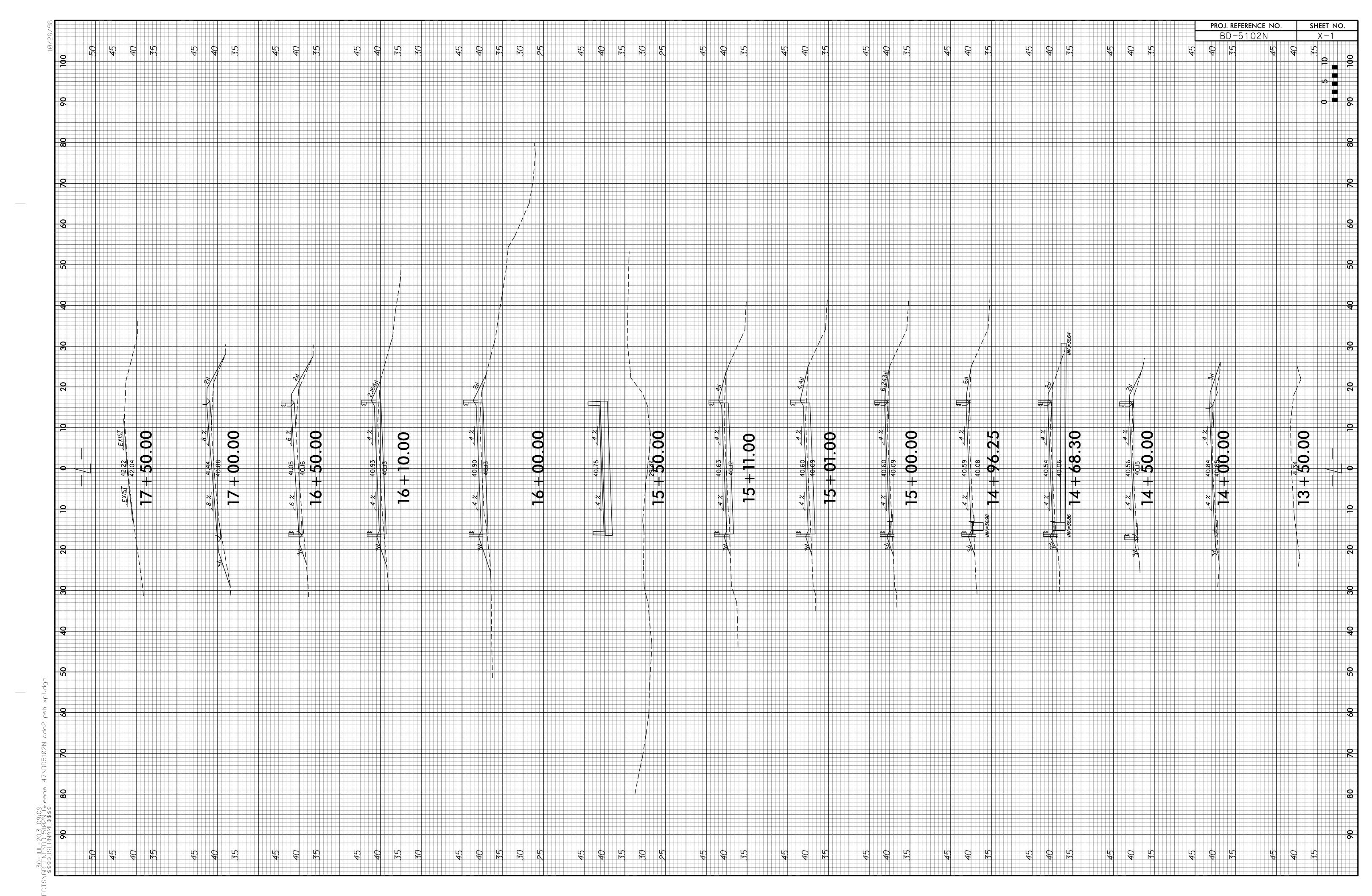
# **CROSS-SECTION SUMMARY**

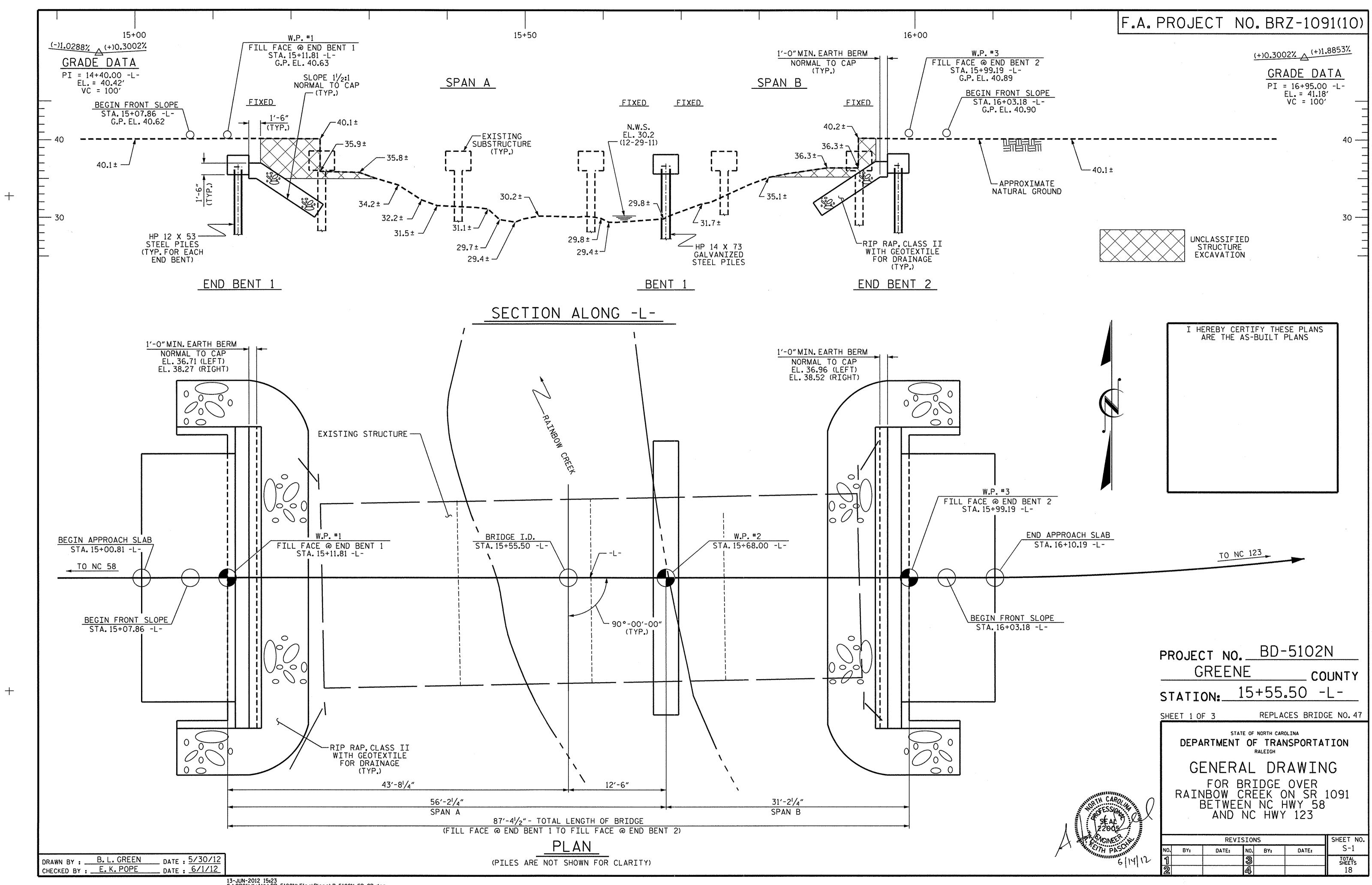
IN CUBIC YARDS

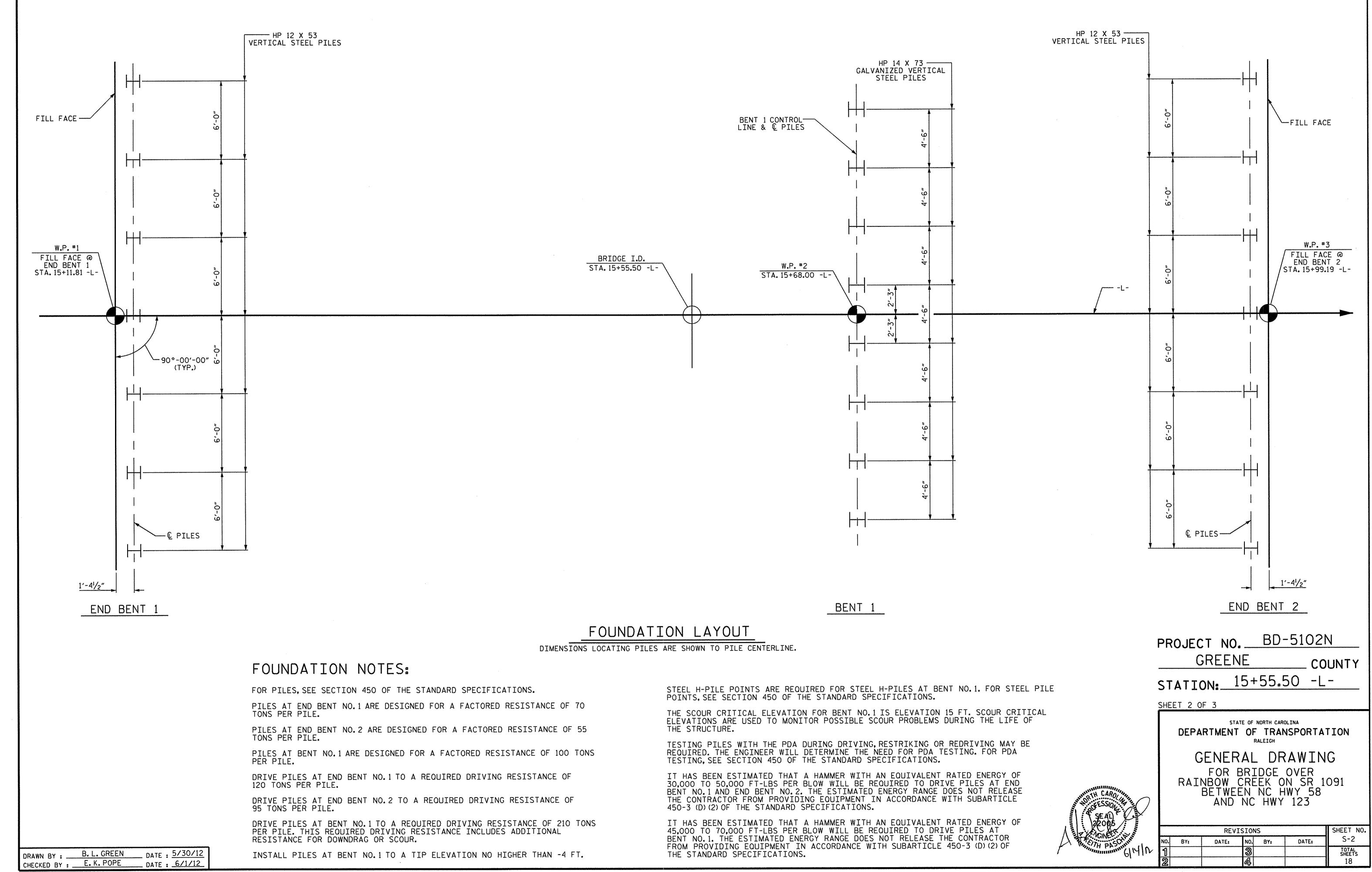
## NOTE: EMBANKMENT COLUMN DOES NOT INCLUDE BACKFILL FOR UNDERCUT.

LOCATION (-L-)	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBANKMENT
14+00.00	0	0	0
14 + 50.00	4	0	18
14+68.30	1	0	5
14 + 96.25	1	0	9
15+00.00	0	0	1
15+01.00	1	0	0
15+11.00	13	0	4
	BRID	GE	
16+00.00	0	0	0
16+10.00	10	0	6
16 + 50.00	19	0	23
17+00.00	0	0	36

PROJ. REFERENCE NO.	SHEET NO.
BD-5102N	X-1 A

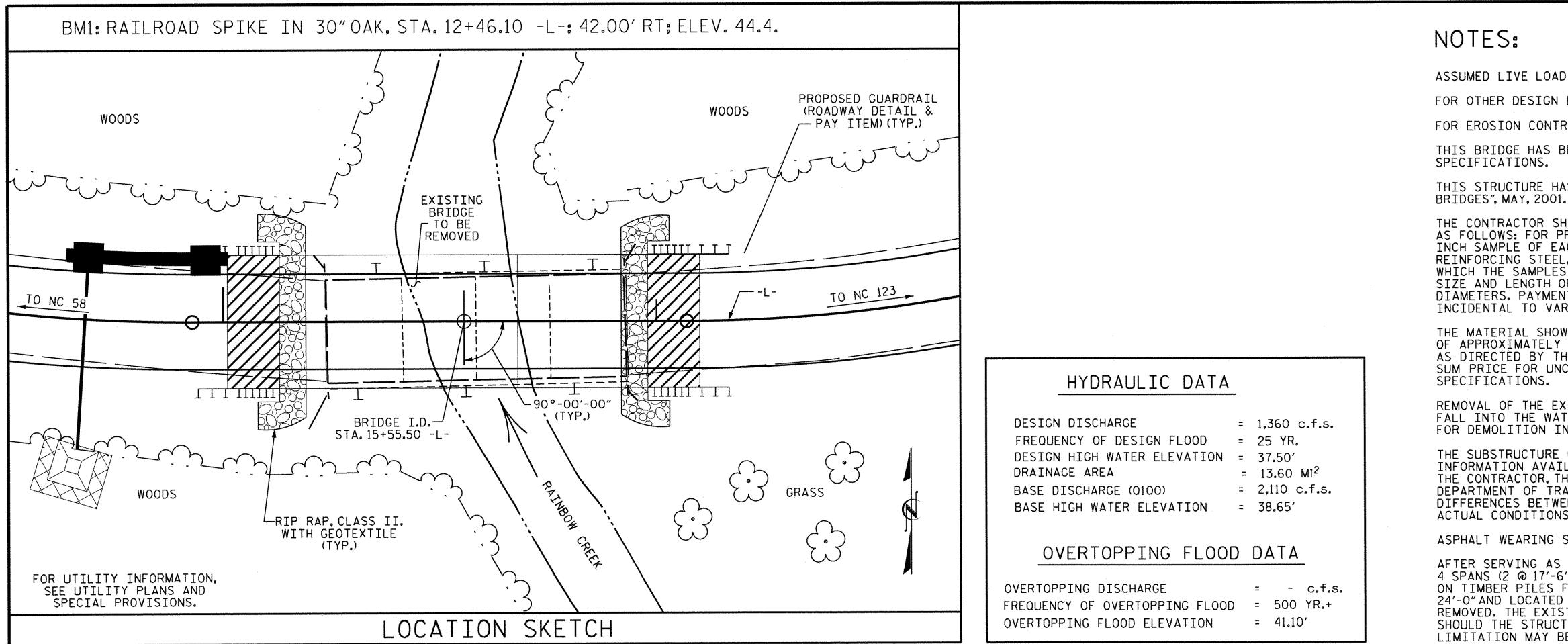






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					- TOTA	۱L	BIL	Ļ	OF	MATE	RIAL	*****					
	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORCING STEEL	HP STE	12 X 53 EL PILES	GAL	14 X 73 VANIZED EL PILES	STEEL PILE POINTS	PILE REDRIVES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE	
	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN.FT.	NO.	LIN.FT.	EACH	EACH	LIN.FT.	TONS	SO.YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE		LUMP SUM		LUMP SUM								170.50		·	LUMP SUM	22	935.00
END BENT NO. 1			14.3		2,127	7	315				4		121	135			
BENT NO. 1	1		10.8		2,162			8	560	8	4						
END BENT NO. 2			14.3		2,127	7	315				4		126	140			
TOTAL	1	LUMP SUM	39.4	LUMP SUM	6,416	14	630	8	560	8	12	170.50	247	275	LUMP SUM	22	935.00

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, EVALUATING SCOUR AT

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEN BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE, PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS. PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF APPROXIMATELY 25 FT. EACH SIDE OF CENTERLINE ROADWAY AT END BENTS #1 AND #2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

AFTER SERVING AS A TEMPORARY STRUCTURE, THE EXISTING STRUCTURE CONSISTING OF 4 SPANS (2 @ 17'-6", 2 @ 17') REINFORCED CONCRETE DECK/TIMBER JOISTS, TIMBER CAPS ON TIMBER PILES FOR END BENTS AND INTERIOR BENTS WITH A CLEAR ROADWAY WIDTH OF 24'-O" AND LOCATED AT THE APPROXIMATE SITE OF THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE, THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

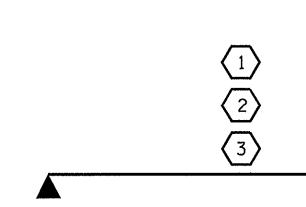
FOR CRANE SAFETY. SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

	PROJEC	T NO. BREEN			
	STATI	DN:1	5+55	.50 -	<u> </u>
	SHEET 3 0	F 3			
	DEPA		OF NORTH CARC OF TRAN RALEIGH	NSPORTA	TION
	GE			AWIN	G
TH CAROLAN		NBOW C		ON SR	
RTH CARO			NC HW	HWY 58 Y 123	
1 22005					
THE SUNCE OF PLAN		REVIS		~ 4 T T	SHEET NO. S-3
8.213	NO. BY:	DATE:	NO. BY:	DATE:	TOTAL
<i>v</i> ·	1		<u>ଏ</u>		SHEETS 18

										STRE	NGTH	I LIN	IIT ST	ATE				SE	RVICE	III	LIMI	T STA	TE
				MOMENT SHEAR									MOMENT										
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	L I VEL OAD F AC TORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)
·····		HL-93(Inv)	N/A	1	1.055		1.75	0.275	1.23	55'	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55′	EL	27
DESIGN		HL-93(0pr)	N/A		1.591		1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N/A					
LOAD	ľ	HS-20(Inv)	36.000	2	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0.523	1.47	55′	EL	5.4	0.80	0.275	1.32	55'	EL	27
RATING		HS-20(0pr)	36.000		1.9	68.396	1.35	0.275	1.99	55′	EL	27	0.523	1.9	55′	EL	5.4	N/A					
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55′	EL	27	0.523	4.17	55'	EL	5.4	0.80	0.275	2.78	55′	EL	27
		SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL	27	0.523	3.02	55′	EL	5.4	0.80	0.275	2.15	55'	EL	27
		SNAGRIS2	22.000		2.079	45.734	1.4	0.275	3.03	55′	EL	27	0.523	2.83	55′	EL	5.4	0.80	0.275	2.08	55′	EL	27
		SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55′	EL	27	0.523	2.09	55′	EL	5.4	0.80	0.275	1.38	55'	EL	27
	S S	SNAGGRS4	34.925		1.189	41.527	1.4	0.275	1.73	55′	EL	27	0.523	1.77	55′	EL	5.4	0.80	0.275	1.19	551	EL	27
		SNS5A	35.550		1.16	41.255	1.4	0.275	1.69	55′	EL	27	0.523	1.82	55′	EL	5.4	0.80	0.275	1.16	55'	EL	27
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55′	EL	27	0.523	1.68	55′	EL	5.4	0.80	0.275	1.08	55'	EL	27
LEGAL		SNS7B	42.000		1.028	43.175	1.4	0.275	1.5	55′	EL	27	0.523	1.67	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27
OAD		TNAGRIT3	33.000		1.32	43.556	1.4	0.275	1.92	55′	EL	27	0.523	1.98	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27
ATING		TNT4A	33.075		1.33	43.979	1.4	0.275	1.94	55′	EL	27	0.523	1.91	55′	EL	5.4	0.80	0.275	1.33	55′	EL	27
		TNT6A	41.600		1.101	45.811	1.4	0.275	1.6	55′	EL	27	0.523	1.83	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27
	ST	TNT7A	42.000		1.114	46.804	1.4	0.275	1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27
		TNT7B	42.000		1.163	48.848	1.4	0.275	1.69	55′	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27
		TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55′	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL .	27
		TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.5	55′	EL	27	0.523	1.58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27
		TNAGT5B	45.000	3	1.013	45.582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27



LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY : CHECKED BY :	E.K.PO G.KOUCH		5-23-12 5-24-12
DRAWN BY : CVC CHECKED BY : DNS	6710 6710		

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## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	Ŷ <sub>DW</sub>
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

## NOTES:

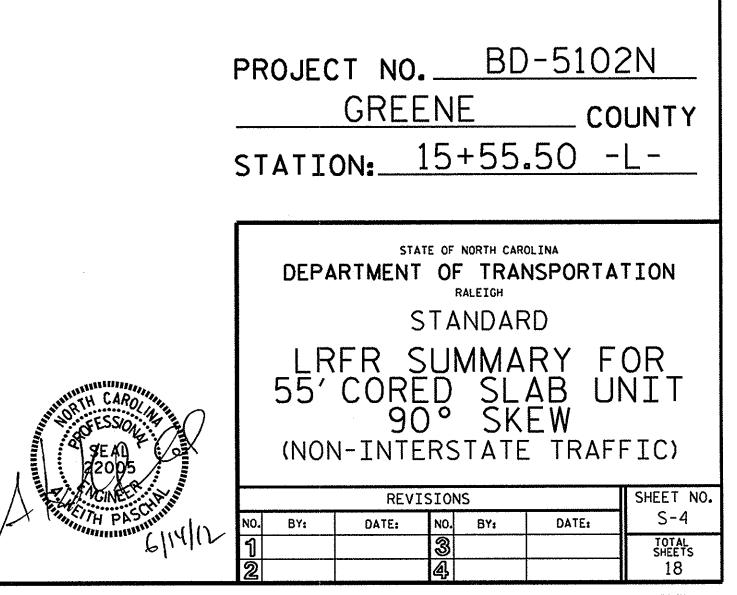
NUMBER

COMMENT

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

### COMMENTS:

- 1. 2.
- 3.
- 4.
- (#) CONTROLLING LOAD RATING  $\left(1\right)$  DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING \*\* \* \* SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER ER - EXTERIOR RIGHT GIRDER



										STRE	NGTH	I LIN	NIT ST	ATE				SE	RVICE	III	LIMIT	STA	ΤE
										MOMENT					SHEAR						MOMENT		
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f†)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	L I VELOAD F ACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)
		HL-93(Inv)	N/A	1	1.037		1.75	0.283	1.83	30′	EL	14.5	0.574	1.04	30'	EL	1.45	0.80	0.283	1.58	30′	EL	14.5
DESIGN		HL-93(0pr)	N/A		1.344	<b></b> ,	1.35	0.283	2.38	30′	EL	14.5	0.574	1.34	30′	EL	1.45	N/A					<b>₩</b>
LOAD		HS-20(Inv)	36.000	2	1.183	42.587	1.75	0.283	2.53	30′	EL	11.6	0.574	1.18	30'	EL	1.45	0.80	0.283	2.20	30′	EL	11.6
RATING		HS-20(0pr)	36.000	·····	1.533	55.205	1.35	0.283	3.28	30'	EL	11.6	0.574	1.53	30′	EL	1.45	NZA					
		SNSH	13.500		2.895	39.081	1.4	0.283	5.18	30'	EL	14.5	0.574	2.89	30′	EL	1.45	0.80	0.283	3.56	30′	EL	14.5
		SNGARBS2	20.000	·	2.240	44.792	1.4	0.283	4.53	30′	EL	11.6	0.574	2.24	30′	EL	1.45	0.80	0.283	3.15	30′	EL	11.6
		SNAGRIS2	22.000		2.157	47.463	1.4	0.283	4.6	30′	EL	11.6	0.574	2.16	30′	EL	1.45	0.80	0.283	3.20	30′	EL	11.6
		SNCOTTS3	27.250		1.462	39.849	1.4	0.283	2.6	30′	EL	14.5	0.574	1.46	30′	EL	1.45	0.80	0.283	1.79	30′	EL	14.5
	SV	SNAGGRS4	34.925		1.346	46.999	1.4	0.283	2.5	30′	EL	14.5	0.574	1.35	30′	EL	1.45	0.80	0.283	1.72	30′	EL	14.5
		SNS5A	35.550		1.427	50.733	1.4	0.283	2.42	30′	EL	14.5	0.574	1.43	30′	EL	1.45	0.80	0.283	1.67	30′	EL	14.5
		SNS6A	39.950		1.341	53.59	1.4	0.283	2.29	30′	EL	14.5	0.574	1.34	30′	EL	1.45	0.80	0.283	1.58	30′	EL	14.5
LEGAL		SNS7B	42.000		1.369	57.505	1.4	0.283	2.23	30'	EL	14.5	0.574	1.37	30′	EL	1.45	0.80	0.283	1.53	30′	EL	14.5
LOAD		TNAGRIT3	33.000		1.593	52.58	1.4	0.283	2.97	30′	EL	14.5	0.574	1.59	30′	EL	1.45	0.80	0.283	2.04	30′	EL	14.5
RATING		TNT4A	33.075	<b></b> .	1.483	49.043	1.4	0.283	2.82	30′	EL	14.5	0.574	1.48	30′	EL	1.45	0.80	0.283	1.94	30′	EL	14.5
		TNT6A	41.600		1.433	59.622	1.4	0.283	2.56	30'	EL	14.5	0.574	1.43	30′	EL	1.45	0.80	0.283	1.76	30′	EL	14.5
	ST	TNT7A	42.000		1.363	57.264	1.4	0.283	2.64	30′	EL	14.5	0.574	1.36	30′	EL	1.45	0.80	0.283	1.82	30′	EL	14.5
	TT	TNT7B	42.000		1.331	55.915	1.4	0.283	2.49	30′	EL	14.5	0.574	1.33	30′	EL	1.45	0.80	0.283	1.72	30′	EL	14.5
		TNAGRIT4	43.000	· • • • •	1.287	55.356	1.4	0.283	2.58	30′	EL	14.5	0.574	1.29	30′	EL	1.45	0.80	0.283	1.78	30′	EL	14.5
		TNAGT5A	45.000		1.381	62.151	1.4	0.283	2.5	30′	EL	14.5	0.574	1.38	30′	E.L.	1.45	0.80	0.283	1.72	30′	EL	14.5
		TNAGT5B	45.000	3	1.212	54.54	1.4	0.283	2.41	30'	EL	11.6	0.574	1.21	30′	EL	1.45	0.80	0.283	1.66	30′	EL	11.6

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(1) (2) (3)

FOR SPAN 'B'

ASSEMBLED BY : CHECKED BY :	E.K.POPE G.KOUCHEKI	DATE : 5-23-12 DATE : 5-24-12
DRAWN BY : CVC CHECKED BY : DNS	6/10 6/10	

## LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\rm DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

### NOTES:

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COMMENT

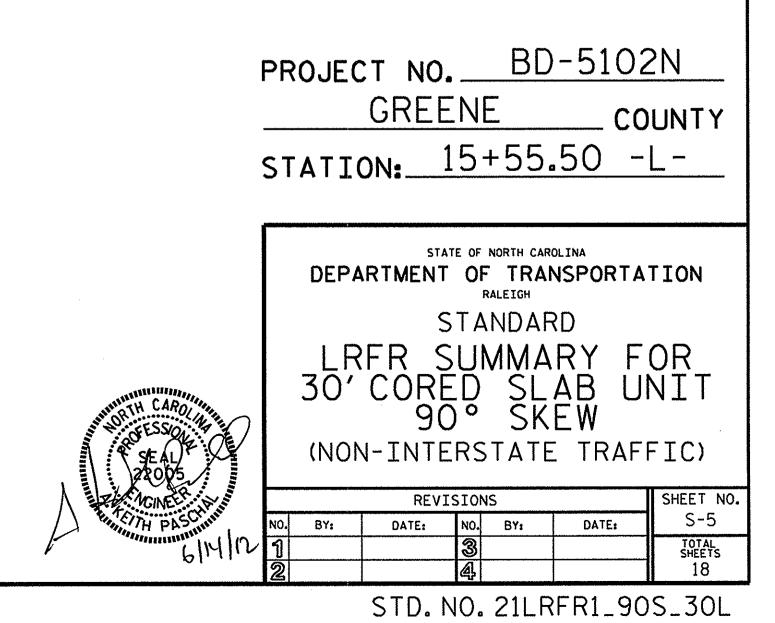
MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

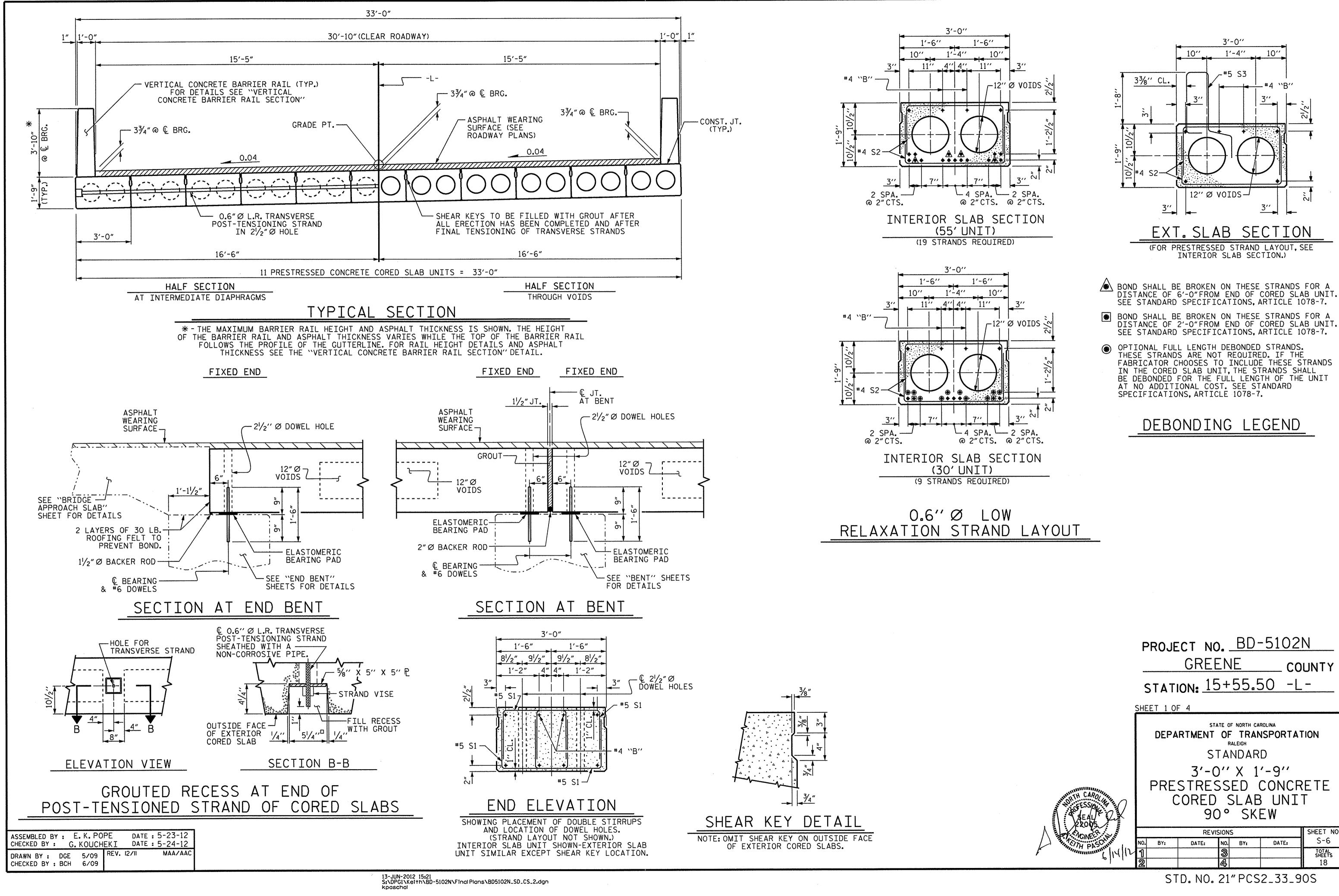
### COMMENTS:

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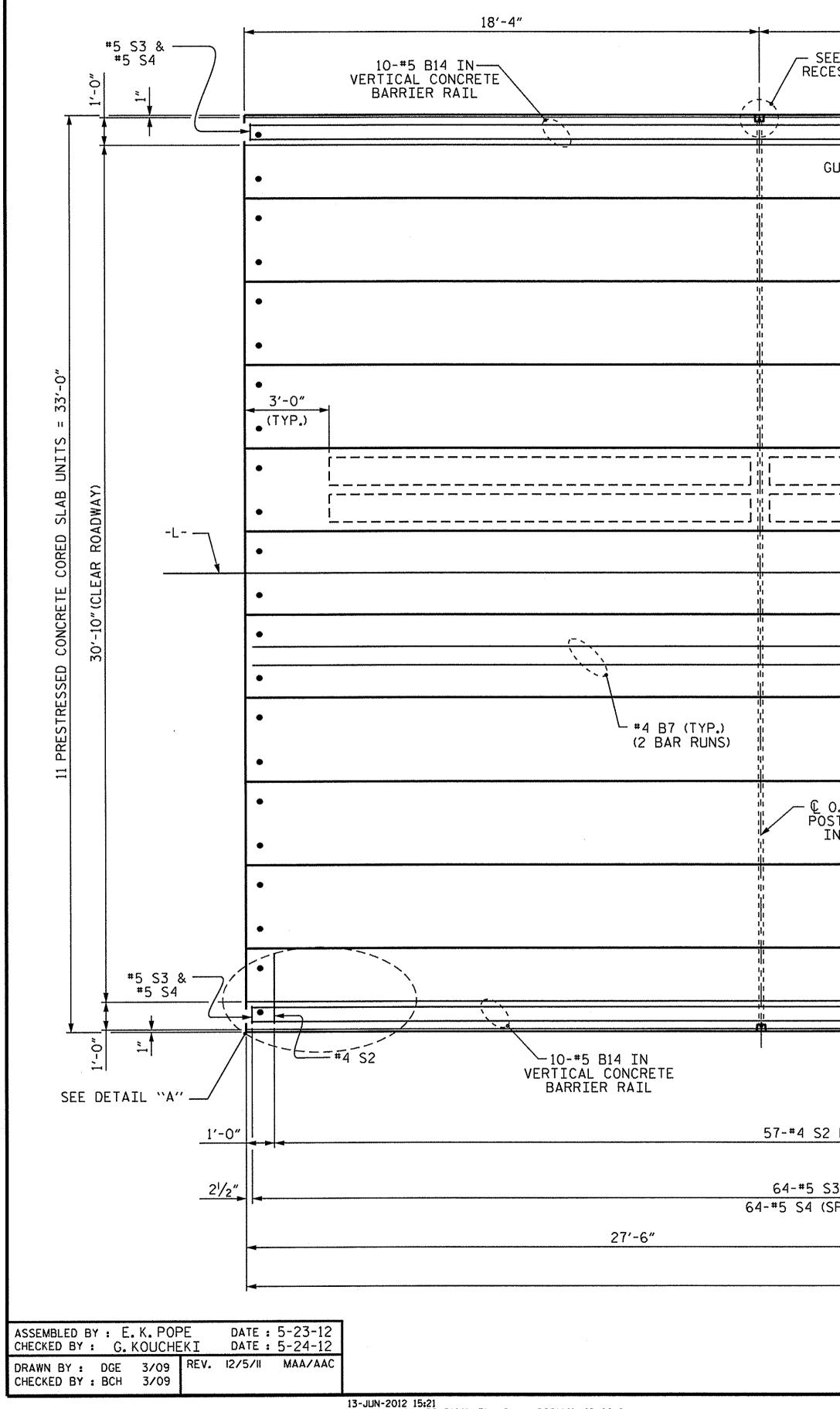
- 4.
- CONTROLLING LOAD RATING 1 DESIGN LOAD RATING (HL-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING \*\* \*\* SEE CHART FOR VEHICLE TYPE GIRDER LOCATION I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER





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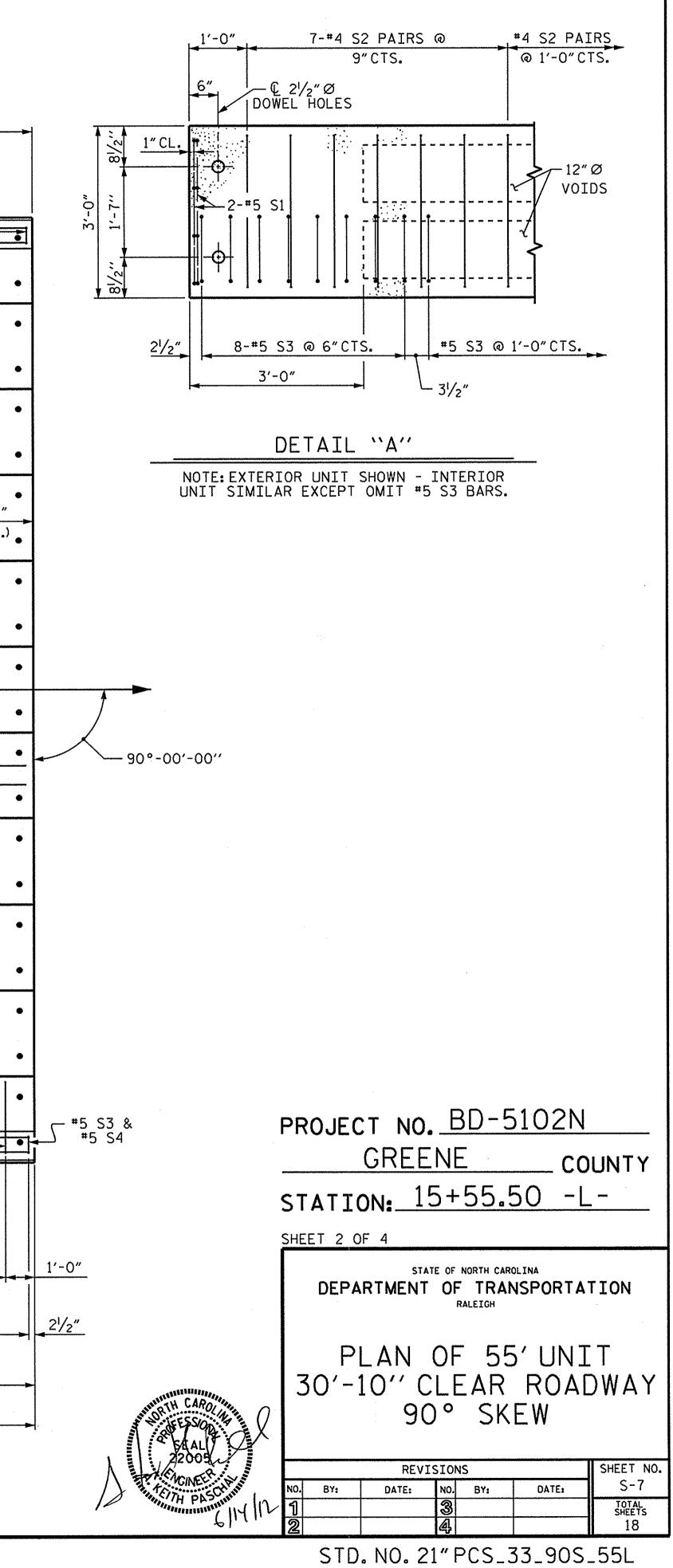


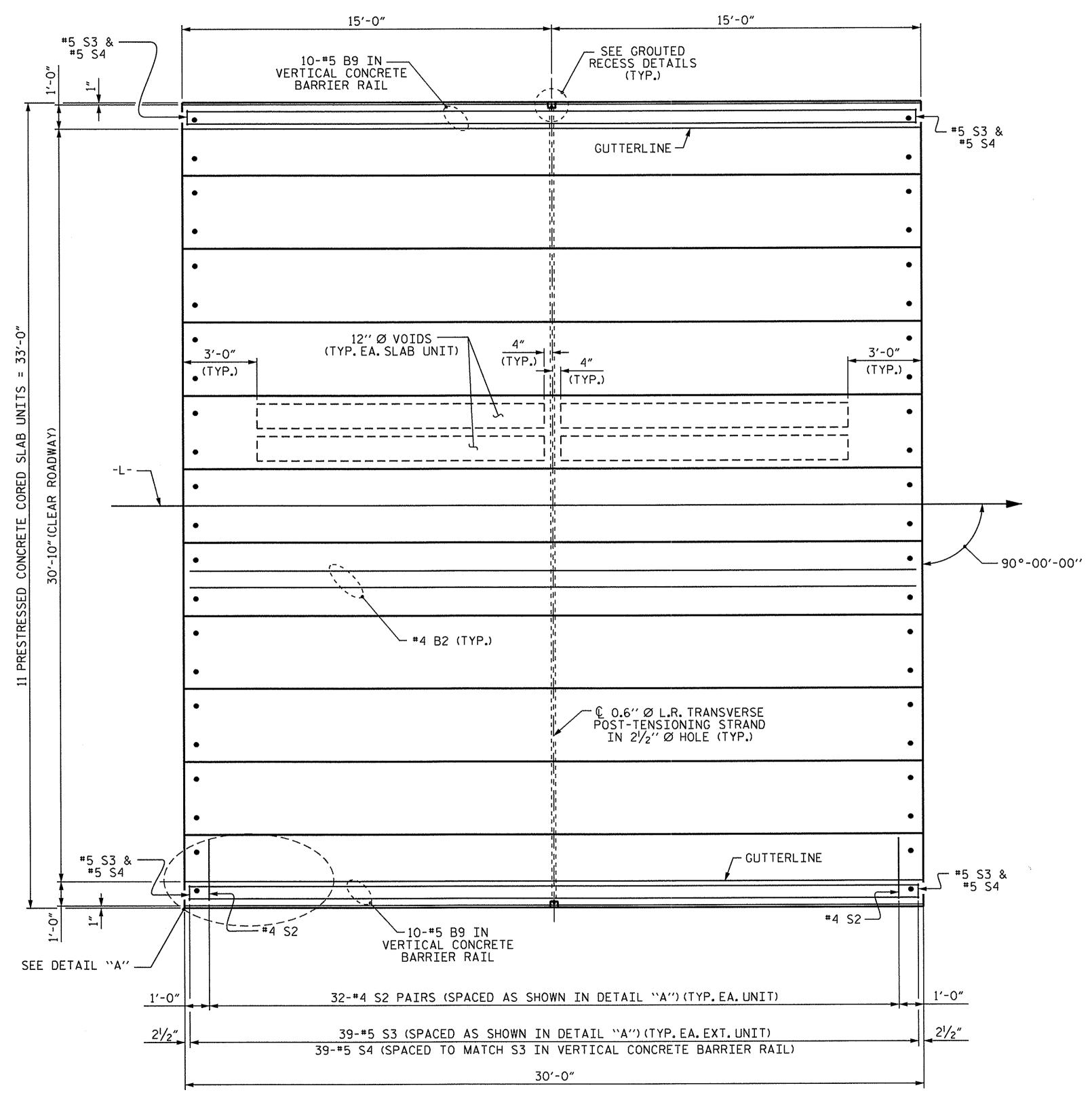
13-JUN-2012 15:21 S:\DPG1\Keith\BD-5102N\FinalPlans\BD5102N\_SD\_CS\_2.dgn kpaschal

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18'-4"	18'-4"	
E GROUTED ESS DETAILS (TYP.)	10-#5 B14 IN VERTICAL CONCRETE BARRIER RAIL	
UTTERLINE -		#5 S3 & #5 S4
	'1 	
	$\frac{1}{1}$	<b>New York Constant of Constant</b>
	4" (TYP.)	3'-0 (TYP
1'-9" SPLICE	1)r 1)r 1 1 1 1 1 1 1 1 1 1 1 1 1	
	1 1 1 1 1 1 1 1 1 1 1 1 1 1	
	<sup>8</sup> 2( 	
0.6" Ø L.R. TRANSVERSE	1); 1,1 1,1 1,1 1,1 1,1 1,1 1,1 1,	• · · ·
D.6" Ø L.R. TRANSVERSE ST-TENSIONING STRAND N 2 <sup>1</sup> / <sub>2</sub> " Ø HOLE (TYP.)	2 2 1 2 1 1 1 2 1 2 1 2 2 1 2 2 2 2 2 2	
	Lp£  1  3  3  1  3  1  1	
GUTTERLINE		
€ ½″ EXP. JT. MAT'L. IN RAIL (TYP.)	-10-#5 B14 IN VERTICAL CONCRETE BARRIER RAIL	#4 S2
PAIRS (SPACED AS SHOWN IN DETAIL ``A'') (TYP.EA.	UNIT)	
3 (SPACED AS SHOWN IN DETAIL ``A'')(TYP.EA.EXT.U PACED TO MATCH S3 IN VERTICAL CONCRETE BARRIE		
55'-0"		
PLAN OF UNIT		

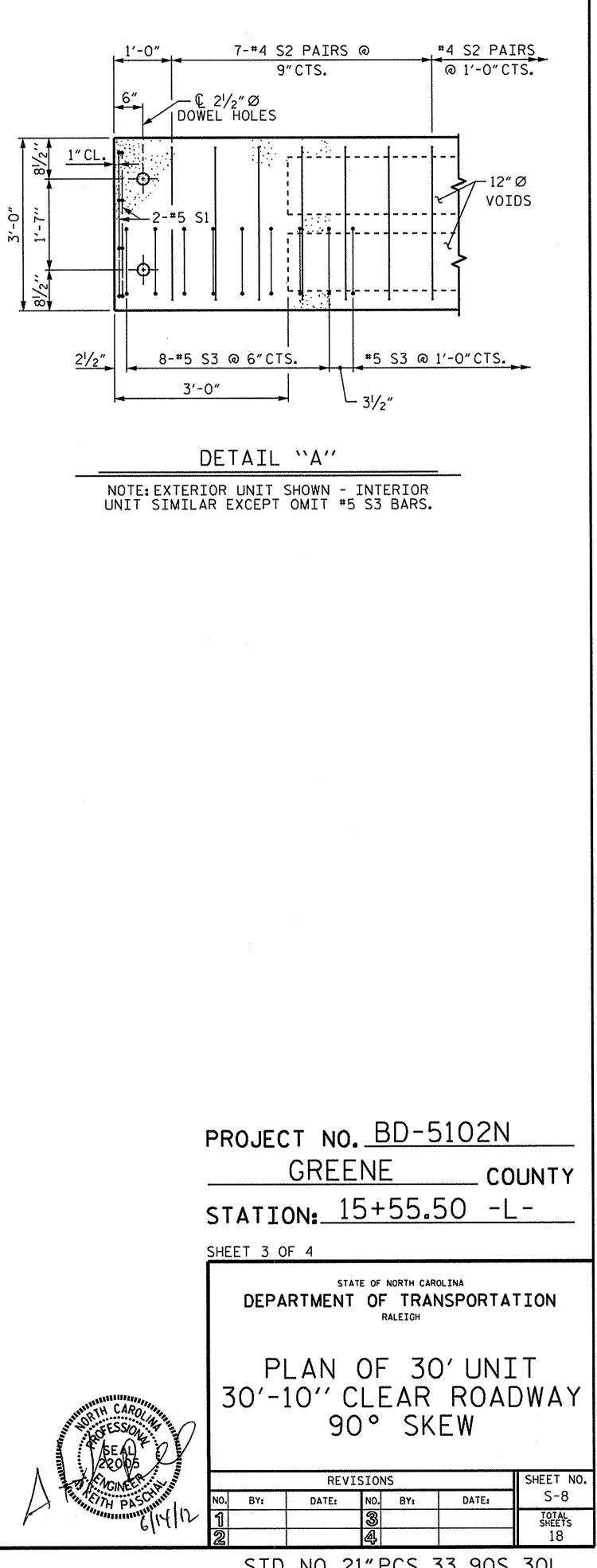




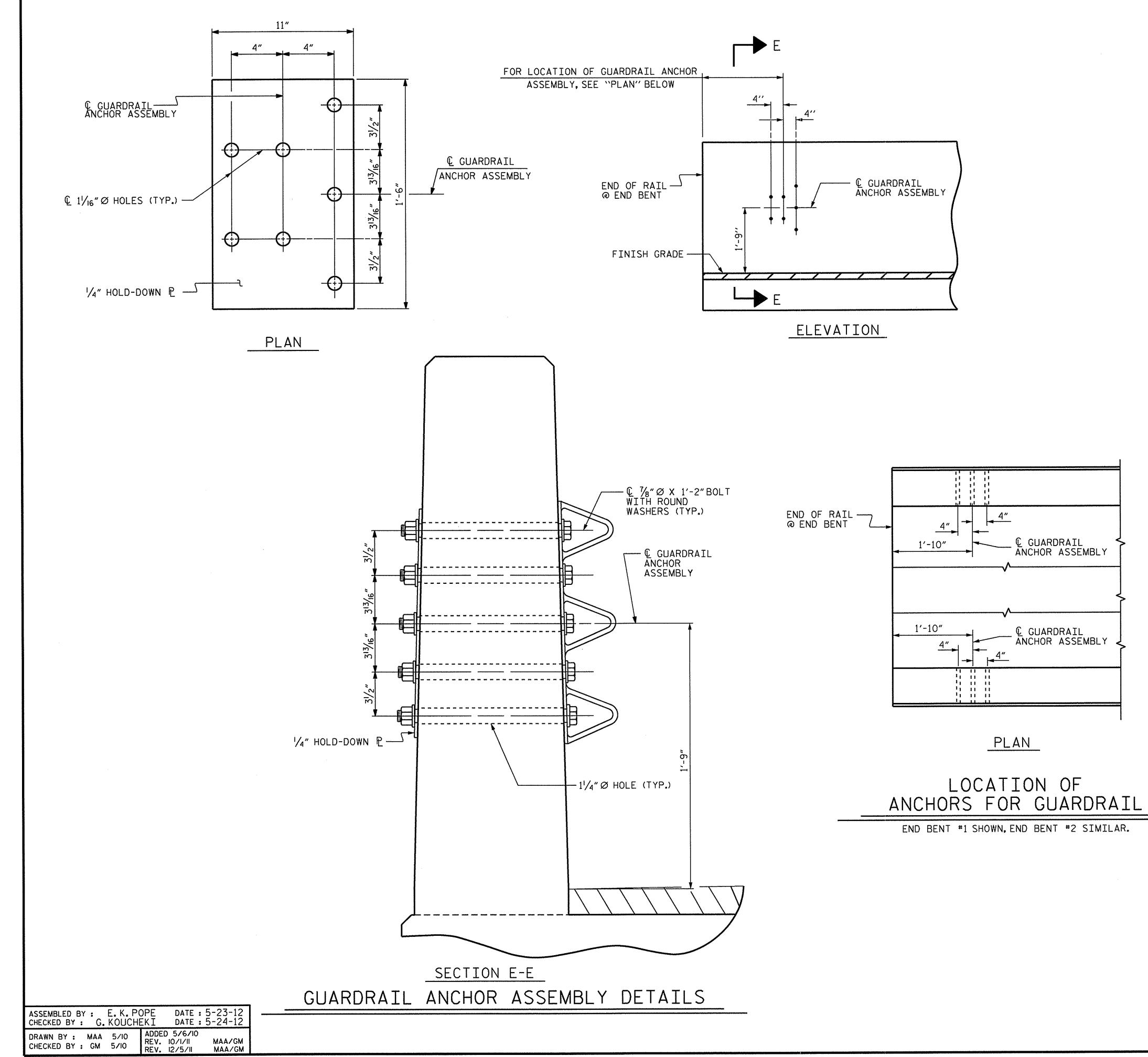
ASSEMBLED BY : E CHECKED BY : G.	. K. POF KOUCHE	PE		5-23-12 5-24-12
DRAWN BY : DGE CHECKED BY : BCH	3/09 3/09	REV.	12/5/11	MAA/AAC

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PLAN OF UNIT

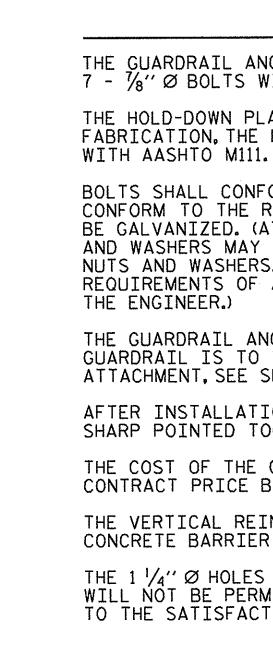


STD. NO. 21" PCS\_33\_90S\_30L



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THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 -  $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE  $\frac{7}{8}$ " Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

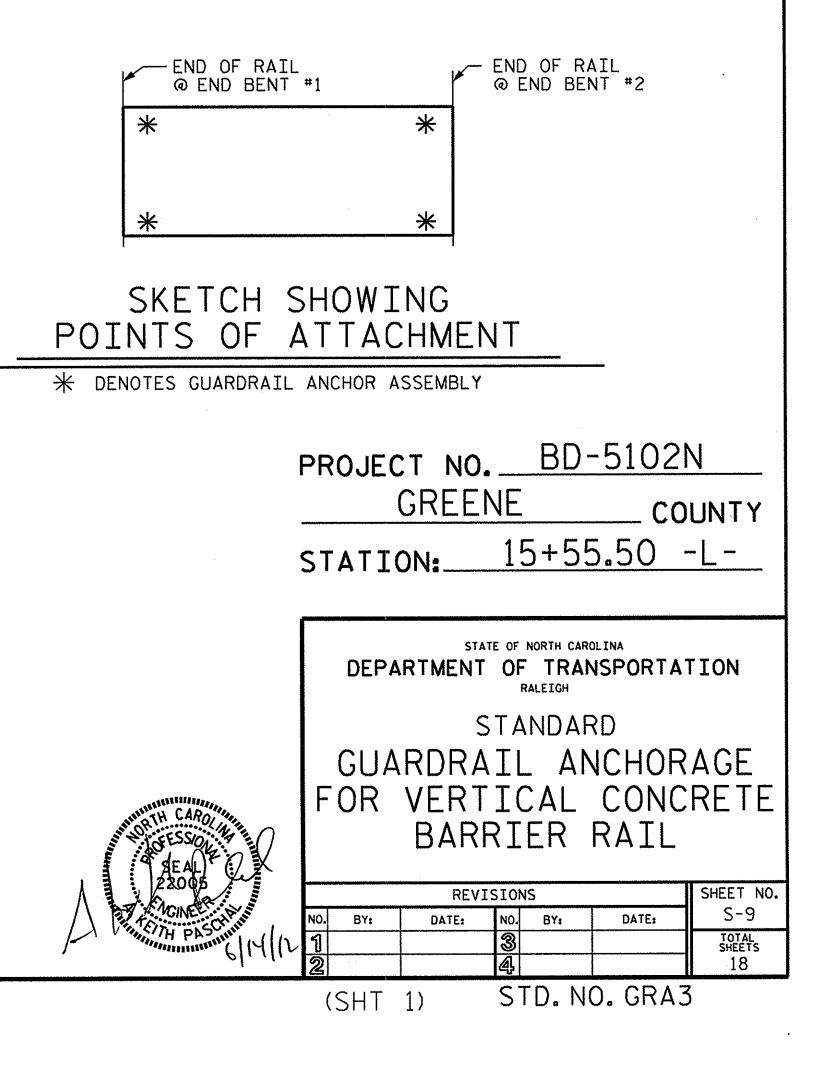
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL.FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



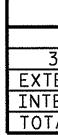
		BILL 5	OF N 5' COR	ATERIA ED SLA	FOR O UNIT	INE		BILL OF MATERIAL FOR ONE 30' CORED SLAB UNIT								
				EXTERIO	OR UNIT	INTERI	OR UNIT					EXTERIO	OR UNIT	INTERI	OR UNIT	
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGH	
B7	4	#4	STR	28'-3"	75	28'-3"	75	<u>B2</u>	2	#4	STR	29'-8"	40	29'-8"	40	
S1	8	*5	3	4'-3"	35	4'-3"	35	S1	8	#5	3	4'-3"	35	4'-3"	35	
<u>S2</u>	114	#4	3	5'-4"	406	5'-4"	406	S2	64	#4	3	5'-4"	228	5'-4"	228	
<u>* S3</u>	64	#5	1	6'-2"	412			<u>* S3</u>	39	<b>#</b> 5	1	6'-2"	251			
REINF	ORCING	STEEL	LBS	5.	516		516	REINF	ORCING	STEEL	LBS	)a	303		303	
	(Y COATE		LB	S.	412				XY COATE		LBS		251			
6500	P.S.I.CO	NCRETE	CU. YDS	à	7.8		7.8	5000	P.S.I. CO	NCRETE	CU. YDS	) <u>.</u>	4.4	<u></u>	4.4	
0.6″Ø	L.R. STR	ANDS	Nc	>.	19		19	0.6″Ø	L.R. STR	ANDS	Nc	).	9		9	

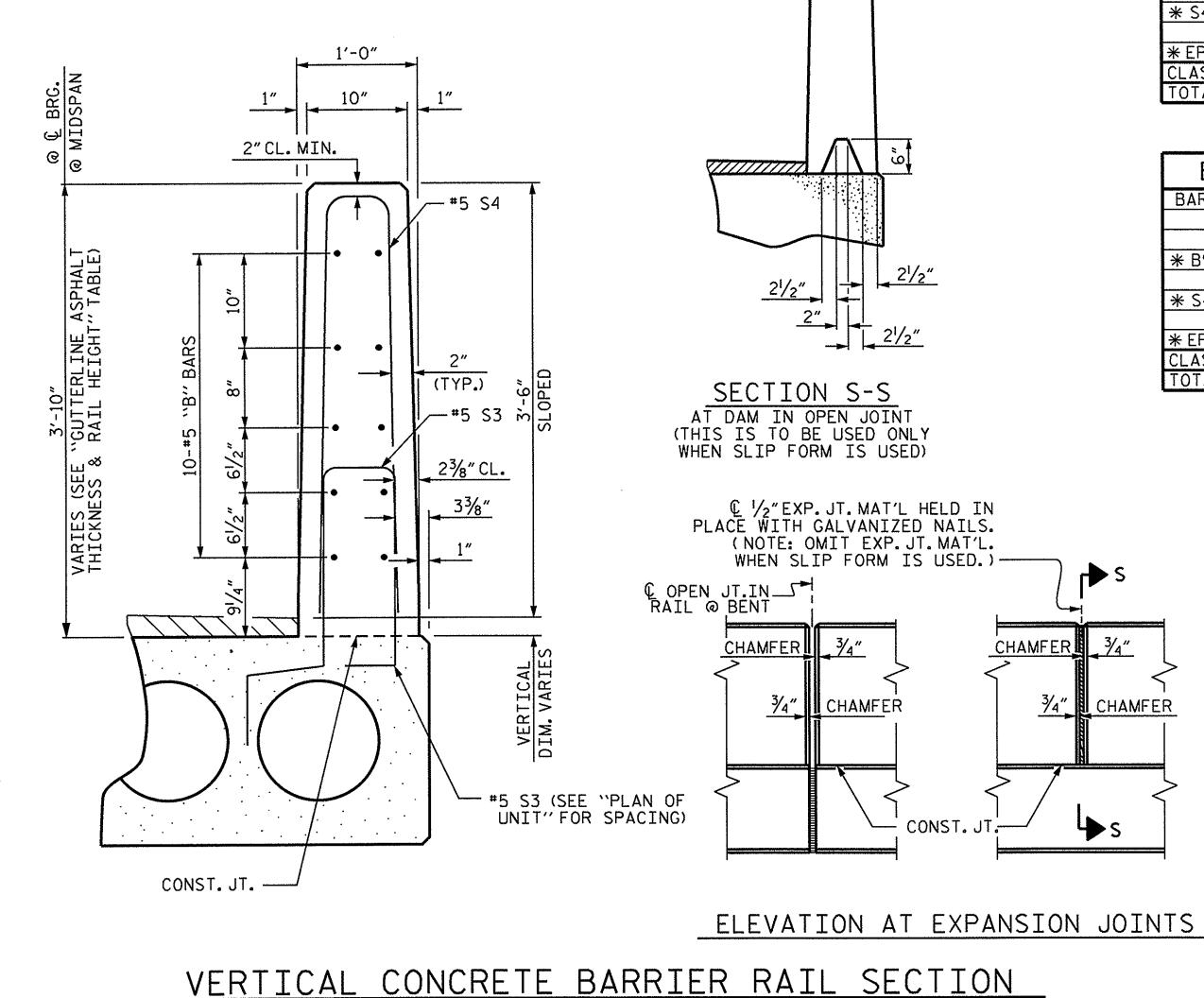
GRADE 270 S	TRANDS
	0.6″ØL.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

+

+

CORED	SLABS REQUIRED					
	NUMBER	LENGTH	TOTAL LENGTH			
55' UNIT						
EXTERIOR C.S.	2	55'-0"	110'-0"			
INTERIOR C.S.	9	55'-0"	495'-0"			
TOTAL	11	:	605′-0″			





ASSEMBLED BY : E.K. CHECKED BY : G.KO		: 5-23-12 : 5-24-12
DRAWN BY : DGE 5/0 CHECKED BY : BCH 6/	9 REV. 12711 09	MAA/AAC

## BAR TYPES

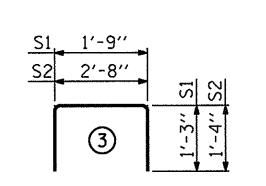
6″

2

73/4"

CORED	) SLABS REQUIRED					
	NUMBER	LENGTH	TOTAL LENGTH			
30' UNIT						
ERIOR C.S.	2	30'-0"	60'-0"			
ERIOR C.S.	9	30'-0″	270'-0"			
AL	11		330'-0"			

	7″	
81/4	1	1'-11"
NIN. 81/4"	6"	



ALL BAR DIMENSIONS ARE OUT TO OUT

BIL	L OF MATERIAL FOR VERTIC	CAL CONCR	ETE I	BARR	IER RA	<b>IL</b>
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	55' UNIT					
*B14	40	40	#5	STR	27'-1"	1130
<del>*</del> \$4	128	128	#5	2	7'-2″	957
* EPOX	Y COATED REINFORCING STEEL			LBS.		2087
CLASS	AA CONCRETE			CU.YDS.	•	14.4
TOTAL		······		LN.FT.		110.25

BI	LL OF MATERIAL FOR VERTI	CAL CONCI	RETE	BARR	IER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	30' UNIT					· · · · · · · · · · · · · · · · · · ·
<u>* 89</u>	20	20	#5	STR	29'-7"	617
* S4	78	78	#5	2	7'-2"	583
₩ EPOX	Y COATED REINFORCING STEEL		<u> </u>	LBS.		1200
CLASS	AA CONCRETE			CU.YDS.	•	7.9
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN.FT.		60.25

GUTTERLINE ASPI	HALT THICKNESS & RAI	L HEIGHT
30'-10" CLEAR ROADWAY	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
	SUPERED SECTION	
55' UNITS	11/2"	3'-7 <sup>3</sup> /4"
30' UNITS	33⁄8″	3′-9 <sup>5</sup> ⁄8″

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
55' CORED SLAB UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	21/2″ 🕴
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	<sup>I</sup> ∕4″ ♥
FINAL CAMBER	2 <sup>1</sup> /4″ 🕴
** INCLUDES FUTURE WEARING SURF	ACE

				-	<u> </u>
		UN	IT		
	5	5'	UN	Ι	ΤS
	3	0′	UN	II	TS

DEAD LOAD DEFLECTION AN	ND CAMBER			
	3'-0" × 1'-9"			
30' CORED SLAB UNIT	0.6″ØL.R. STRAND			
CAMBER (SLAB ALONE IN PLACE)	!∕2″ ∔			
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	¹∕8″ ↓			
FINAL CAMBER	3∕8″ ∔			
** INCLUDES FUTURE WEARING SURFACE				



## NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

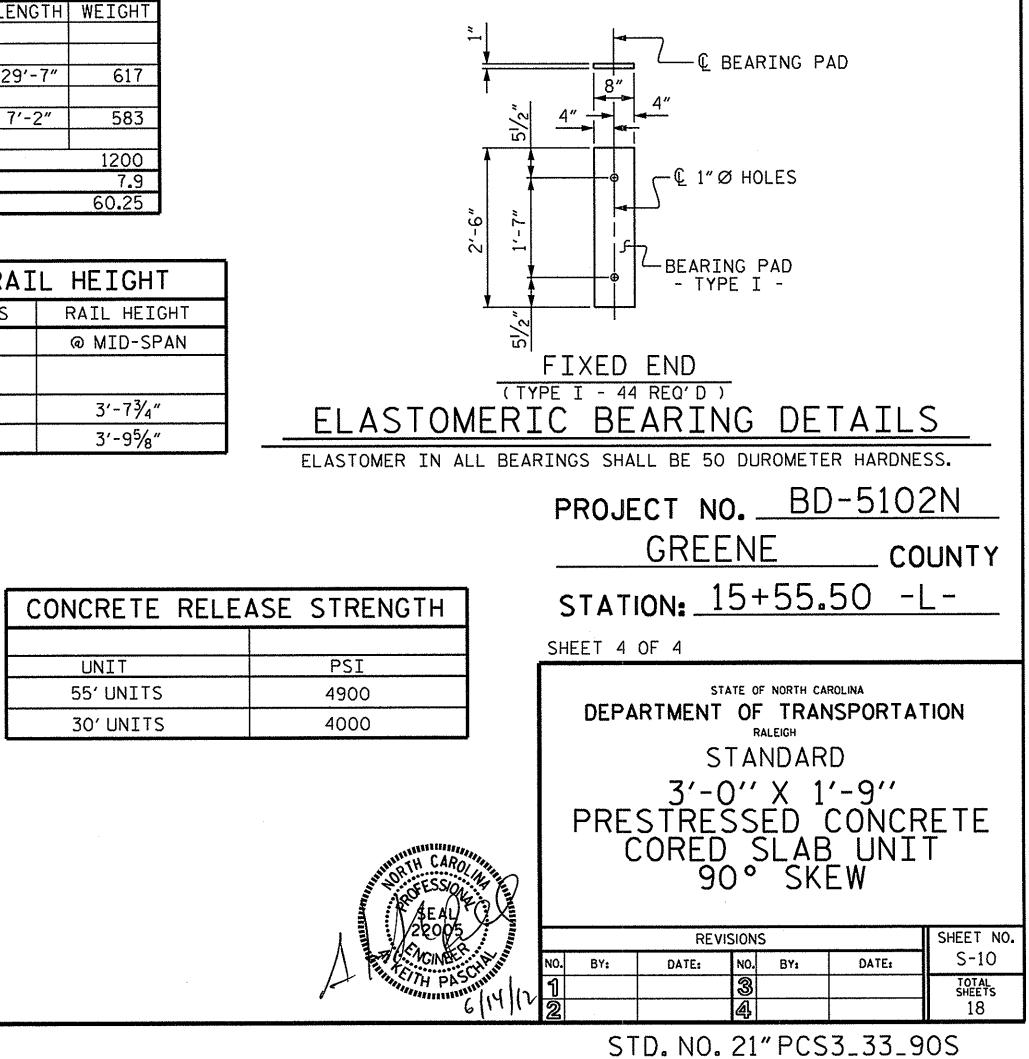
APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

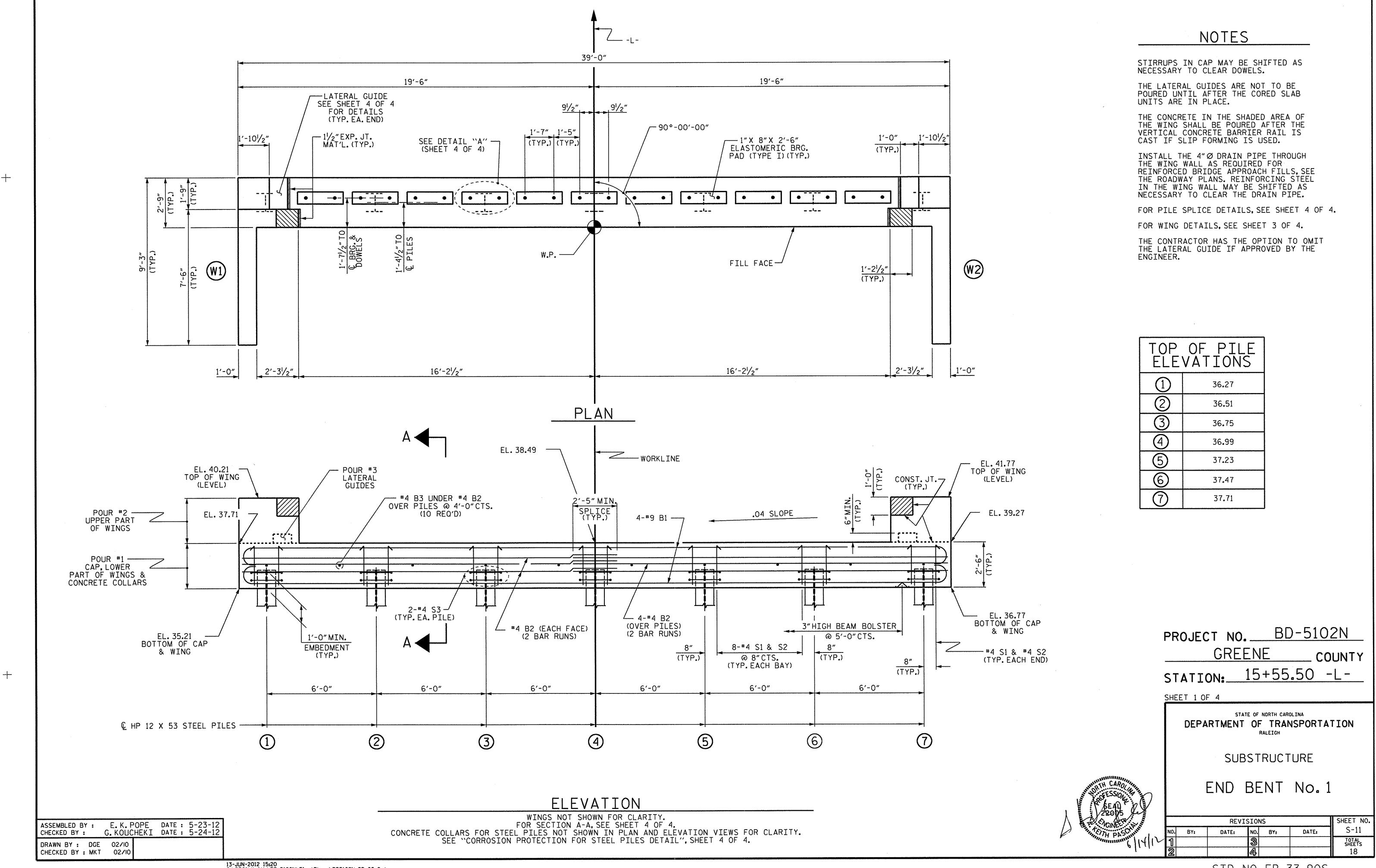
GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.



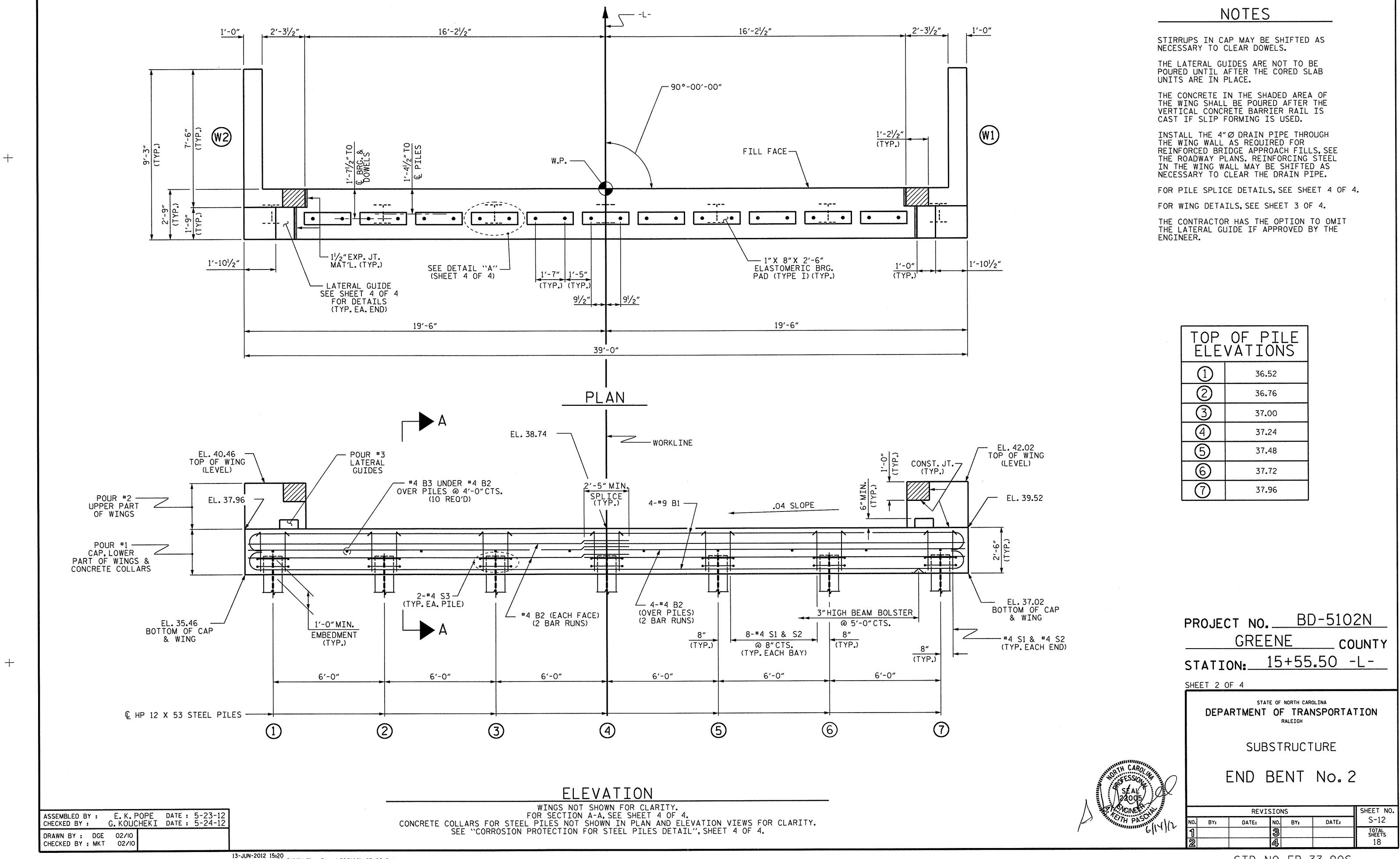


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TOP OF PILE ELEVATIONS			
	36.27		
2	36.51		
3	36.75		
4	36.99		
5	37.23		
6	37.47		
	37.71		

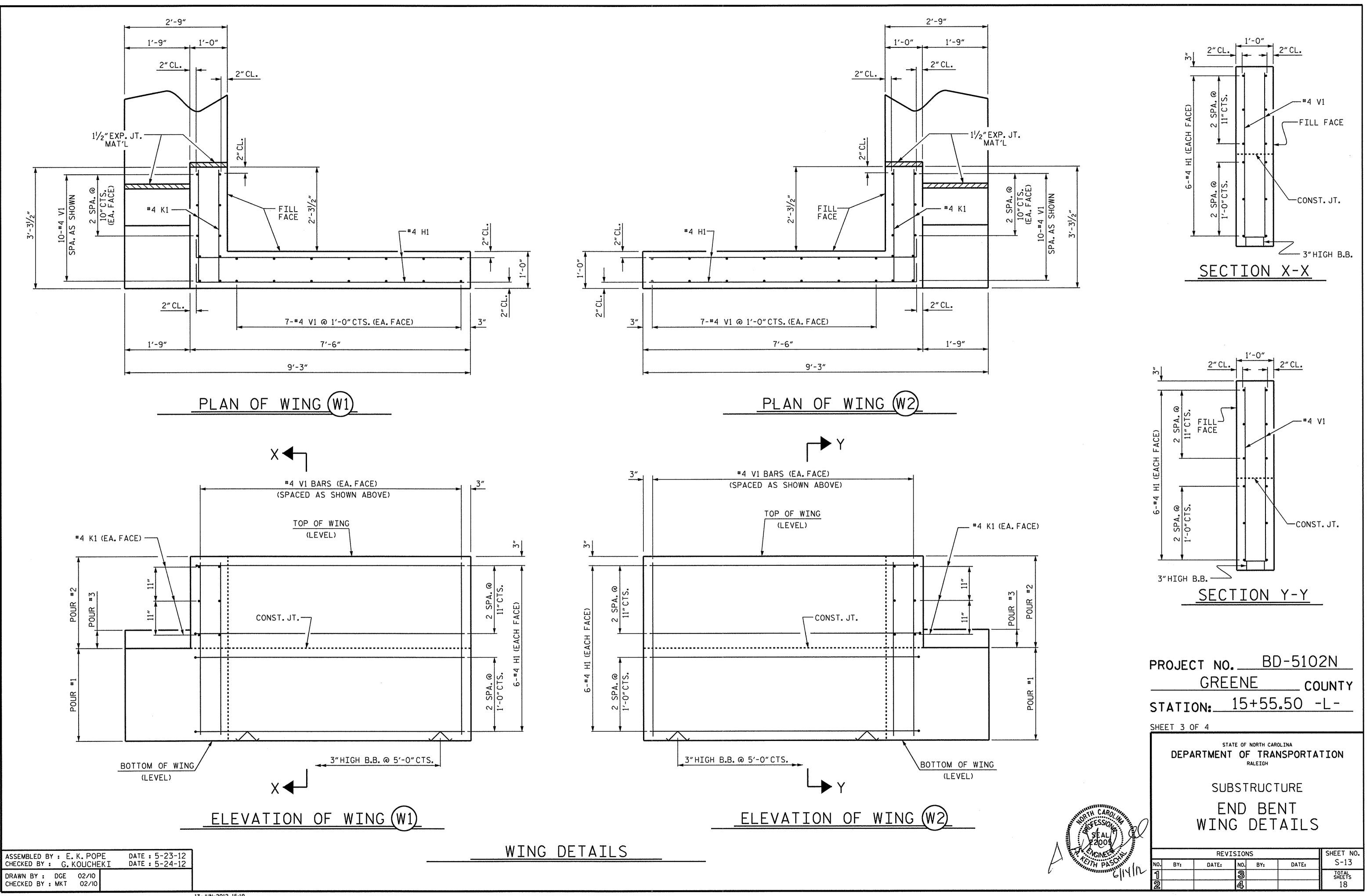
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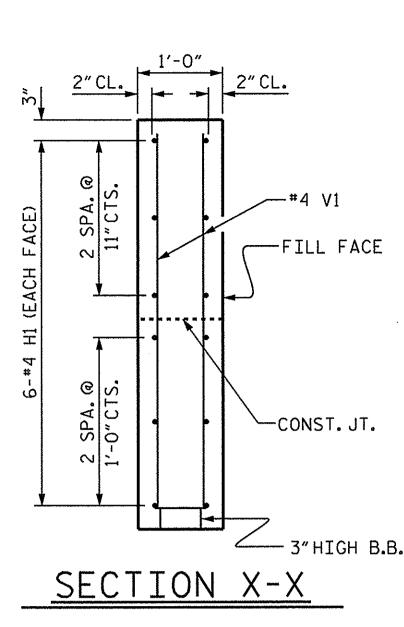
TOP OF PILE ELEVATIONS			
	36.52		
2	36.76		
3	37.00		
4	37.24		
5	37.48		
6	37.72		
	37.96		

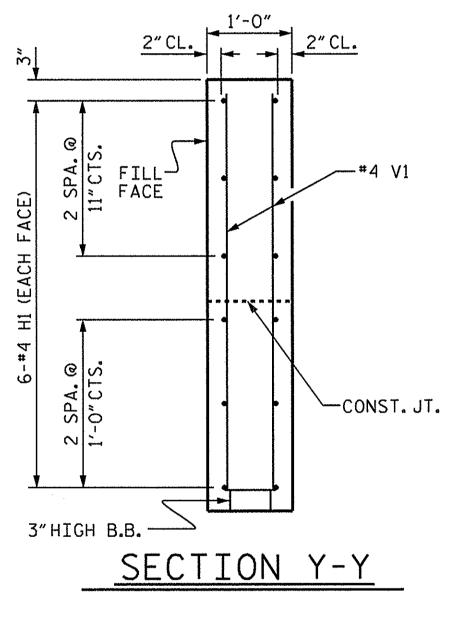
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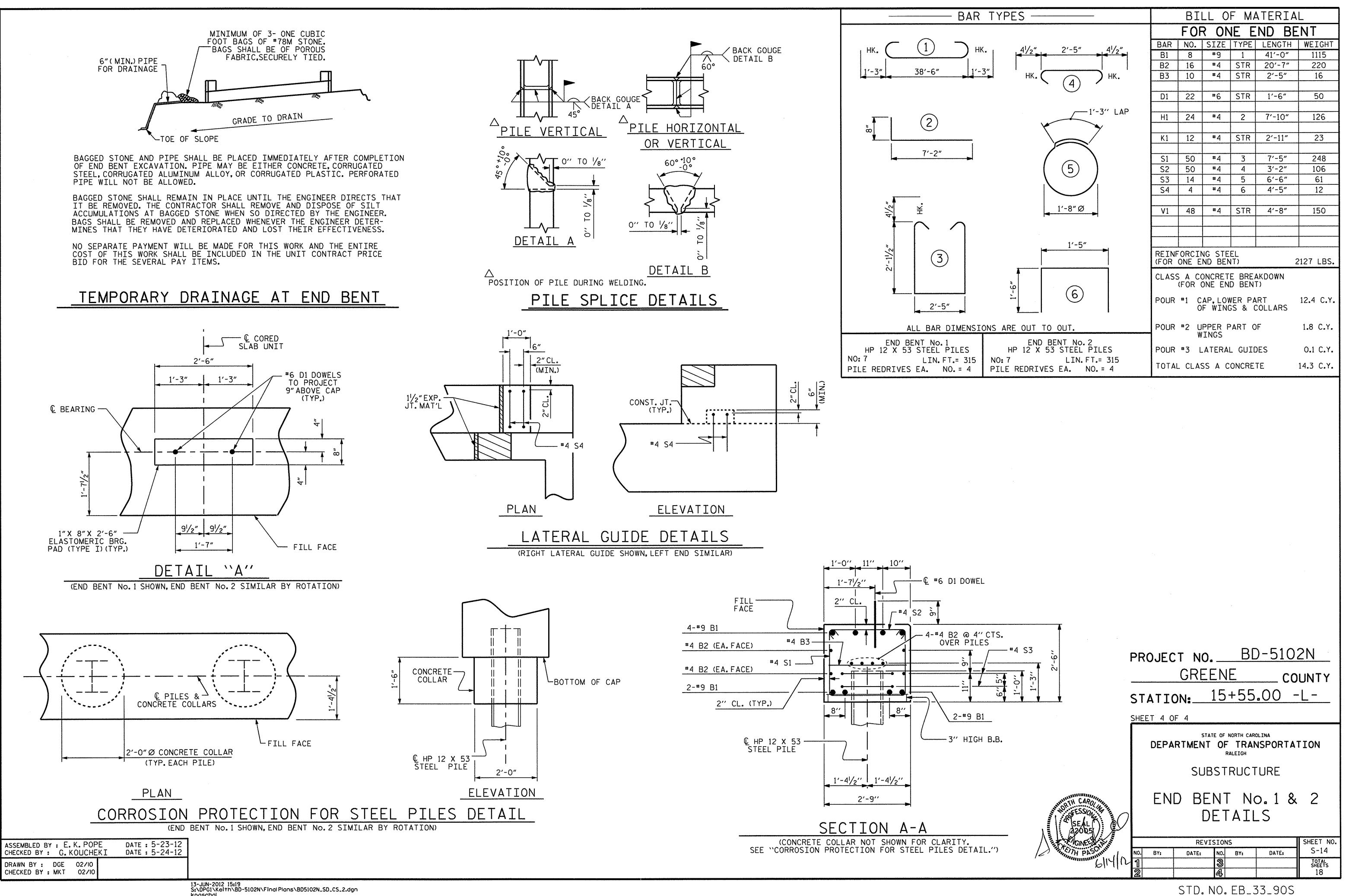
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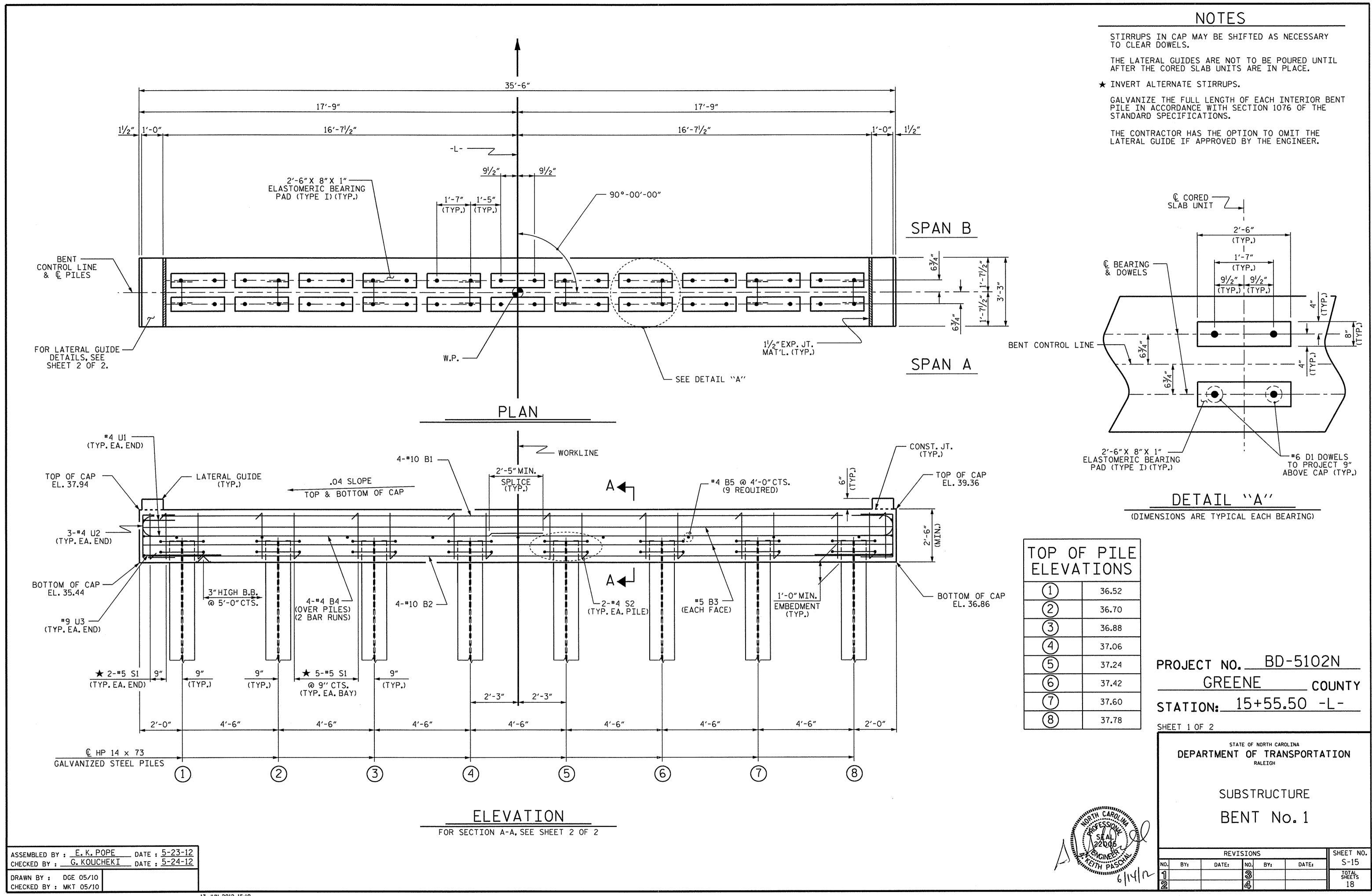


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R TYPES		BI	LL O	F MA	ATERIA		
		FOF	R ON	IE E	ND BI	ENT	
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT	
HK. $4^{1/2}$ $2'-5''$ $4^{1/2}$	B1	8	#9	1	41'-0"	1115	
	B2	16	#4	STR	20'-7"	220	
нк. Нк. Нк.	B3	10	#4	STR	2'-5"	16	
4	D1	22	<b>*</b> 6	STR	1'-6"	50	
/1'-3" LAP							
	H1	24	#4	2	7'-10″	126	
	К1	12	#4	STR	2'-11"	23	
	S1	50	#4	3	7'-5″	248	
((5))	S2	50	#4	4	3'-2"	106	
	S3	14	#4	5	6′-6″	61	
	S4	4	#4	6	4′-5″	12	
1'-8"Ø	V1	48	#4	STR	4'-8"	150	
	· · · ·				· · · · · · · · · · · · · · · · · · ·		
<u>1'-5″</u>	REINF (FOR	ORCIN ONE E	NG STE ND BEN	L EL IT)			
1,-e"		CLASS A CONCRETE BREAKDOWN (FOR ONE END BENT)					
		POUR #1 CAP, LOWER PART 12.4 C OF WINGS & COLLARS				12.4 C.Y.	
IONS ARE OUT TO OUT.	POUR		PPER F INGS	PART C	١F	1.8 C.Y.	
END BENT NO. 2 HP 12 X 53 STEEL PILES	POUR			_ GUIC	DES	0.1 C.Y.	
NO: 7 LIN. FT.= 315 PILE REDRIVES EA. NO. = 4	TOTAL	_ CLAS	SS A C	ONCRE	TE	14.3 C.Y.	



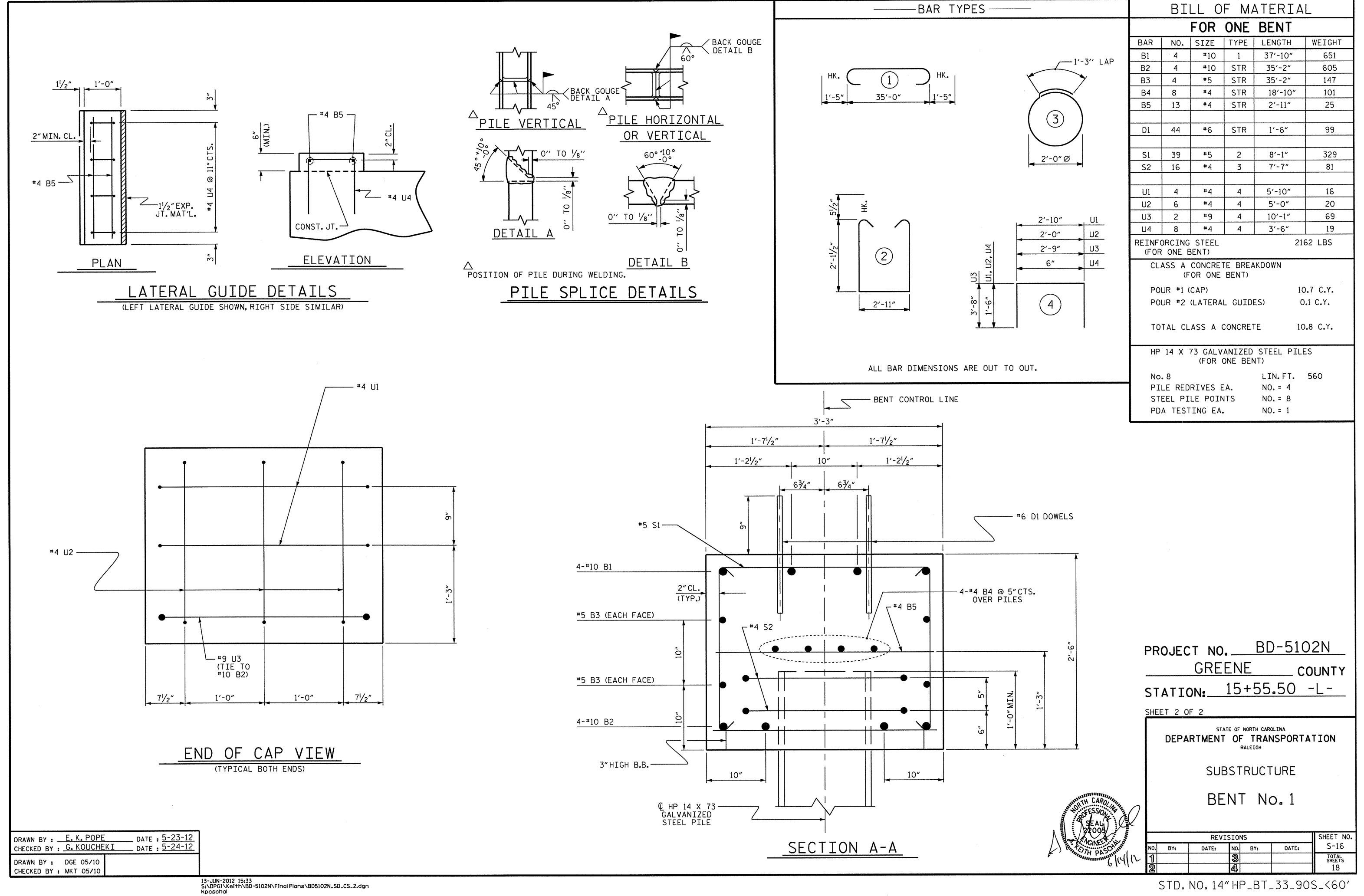
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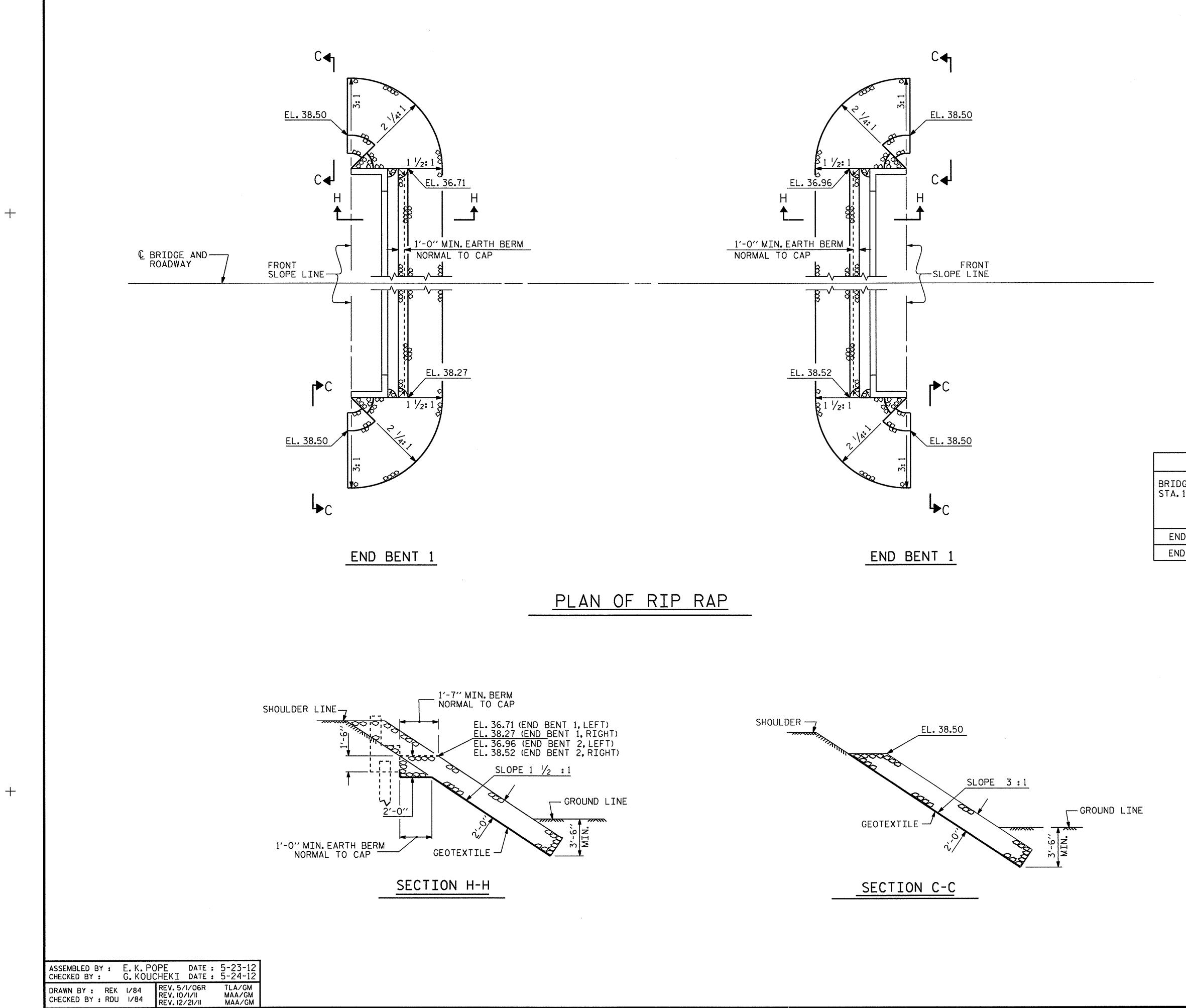
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STD. NO. 14" HP\_BT\_33\_90S\_<60'



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		BI	LL O	FMA	ATERIA	L
			FOR	ONE	BENT	
	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
	B1	<sup>-1</sup> 4	#10	1	37'-10″	651
1'-3'' LAP	B2	4	#10	STR	35′-2″	605
$\langle \cdot \rangle$	B3	4	#5	STR	35'-2″	147
	B4	8	#4	STR	18'-10″	101
	B5	13	#4	STR	2'-11"	25
(3)						
	D1	44	#6	STR	1′-6″	99
					· · · · · · · · · · · · · · · · · · ·	
2'-0"Ø	S1	39	#5	2	8'-1"	329
	S2	16	#4	3	7'-7"	81
	U1	4	#4	4	5'-10″	16
	U2	6	#4	4	5'-0"	20
2'-10" U1	U3	2	#9	4	10'-1"	69
2'-0" U2	U4	8	#4	4	3'-6"	19
	REINFORCING STEEL 2162 LBS (FOR ONE BENT)					62 LBS
2'-9" U3 6" U4	CLASS A CONCRETE BREAKDOWN (FOR ONE BENT)					
	PO	UR #1 (	CAP)		1(	D.7 C.Y.
	POUR #1 (CAP)10.7 C.Y.POUR #2 (LATERAL GUIDES)0.1 C.Y.					
TOTAL CLASS A CONCRETE 10.8 C.					).8 C.Y.	
	HP	14 X		ANIZED	STEEL PIL	ES
IT TO OUT.		LE RED	RIVES		LIN.FT. NO. = 4	560
			LE POIN TING EA		NO. = 8 NO. = 1	



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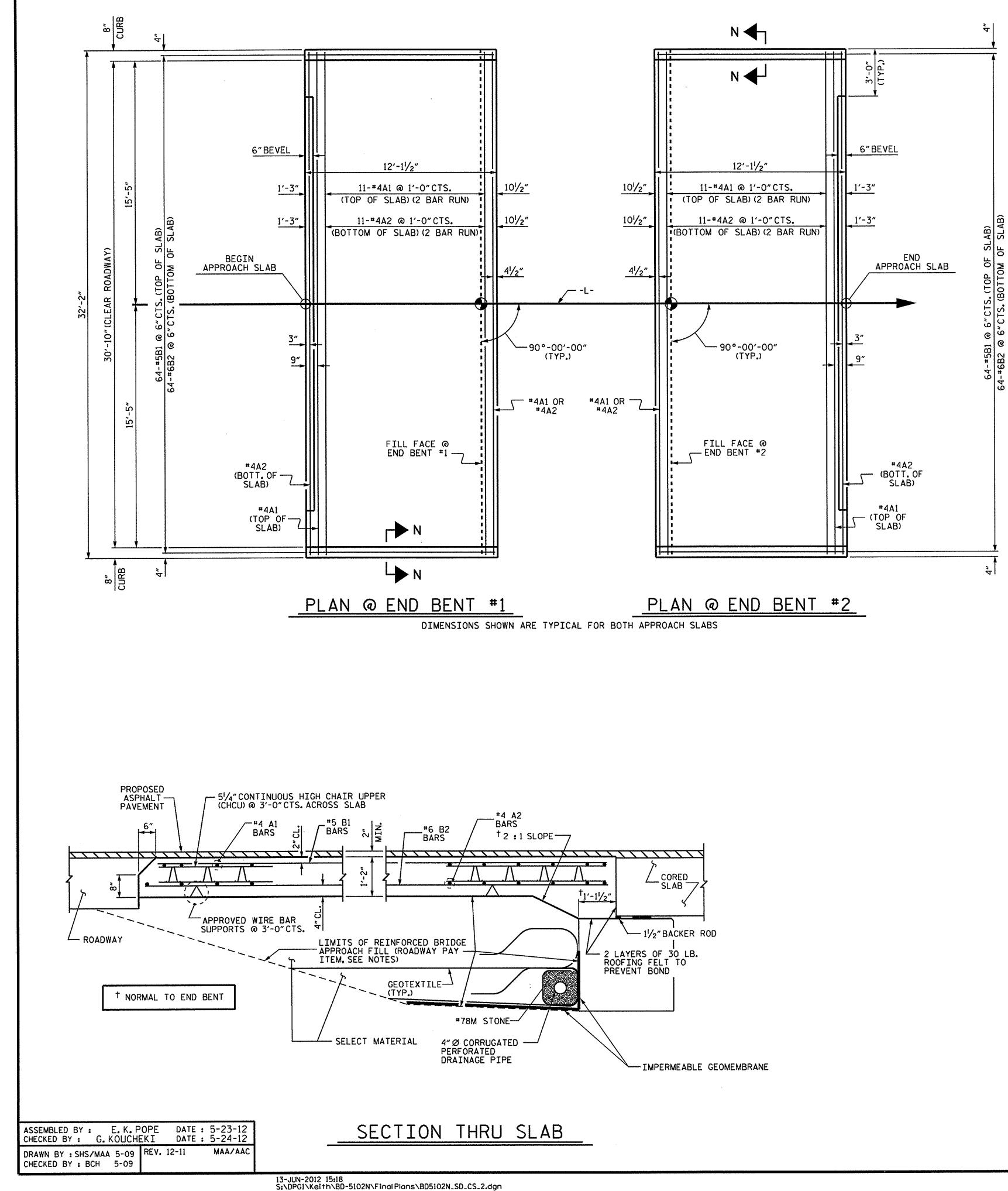
NOTES : FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.

ESTIMATED QUANTITIES					
)GE @ 15+55.50 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE			
	TONS	SQUARE YARDS			
ID BENT 1	121	135			
ID BENT 2	126	140			

	STATIO	GREE DN: 1		5.50		UNTY
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD					
SELAL PZOOS	— R]	[P R	٩P	DET	AIL	S —
	REVISIONS SHEET NO.					
THE TOWNER (10)	NO. BY:	DATE:		Y1	DATE:	S-17
Manning MIC	1		3 4			total sheets 18
	<b></b>					(Sht 2)

PROJECT NO. BD-5102N

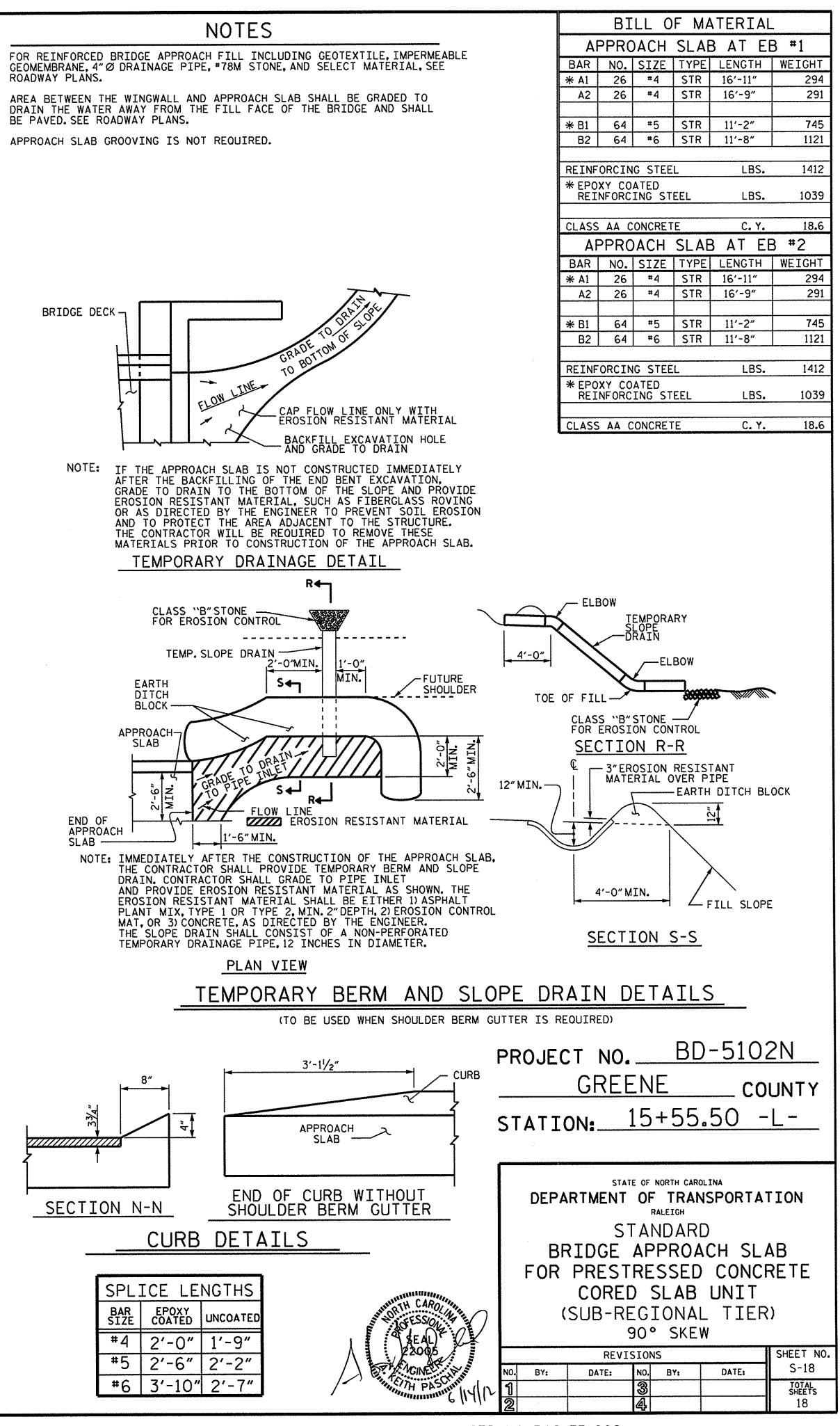
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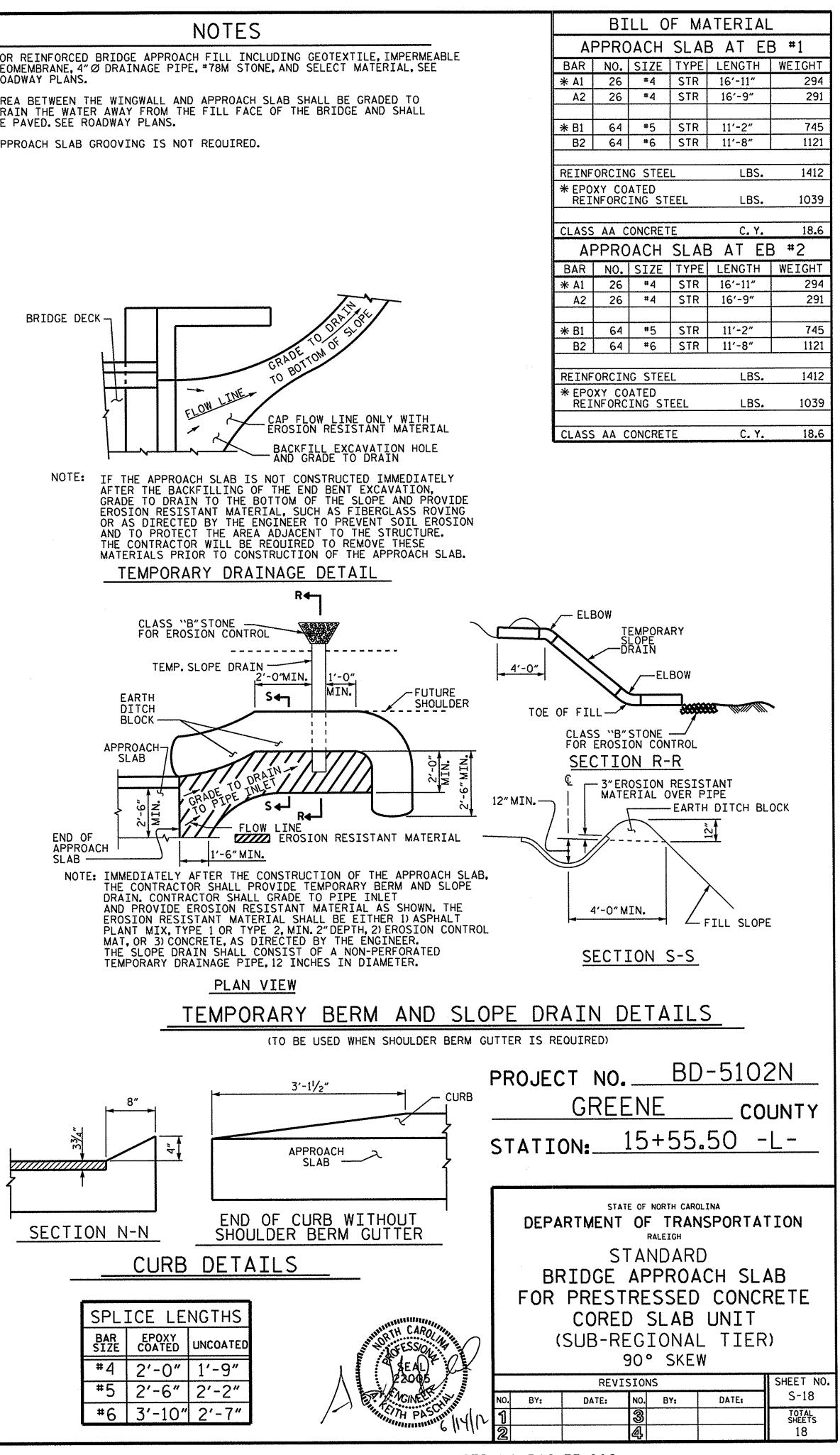


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SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATE
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3'-10"	2'-7"

STD. NO. BAS\_33\_90S

### STANDARD NOTES

### DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF	
STRUCTURAL STEEL - AASHTO M270 GRADE 36 -	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W -	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50 -	27,000 LBS.PER SO.IN.
REINFORCING STEEL IN TENSION	
GRADE 60	24,000 LBS.PER SO.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR	
UNTREATED - EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT.

### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS, DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE

INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE %4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE %4" Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0". EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED. WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES,ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

OR METALLIZING.

SPECIAL NOTES:

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### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

