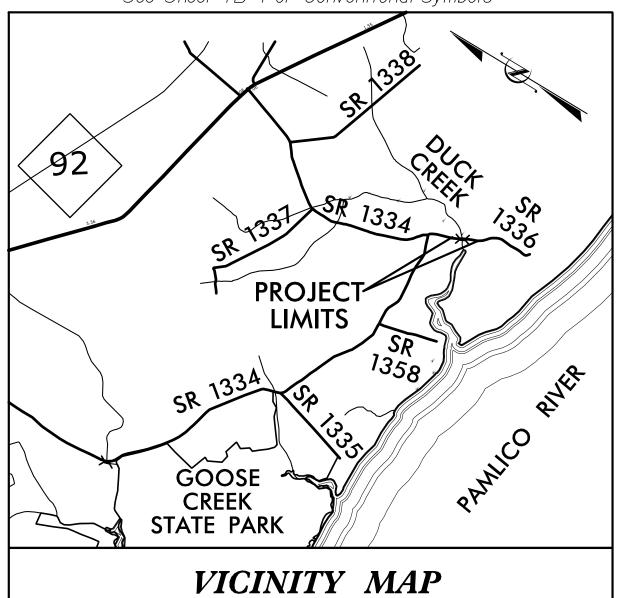
# This electronic collection of documents is provided for the convenience of the user and is Not a Certified Document –

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This file or an individual page shall not be considered a certified document.

IE

See Sheet 1A For Index of Sheets See Sheet 1B For Conventional Symbols

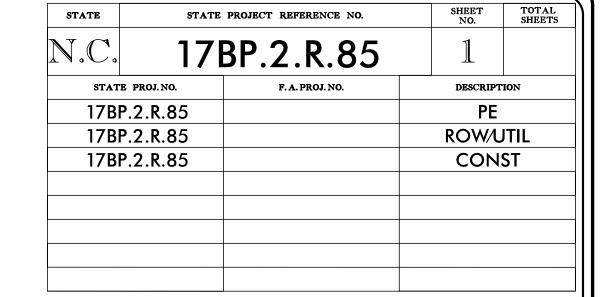


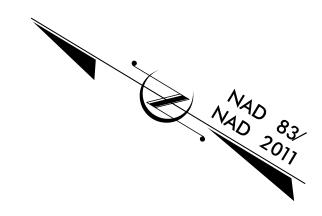
# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

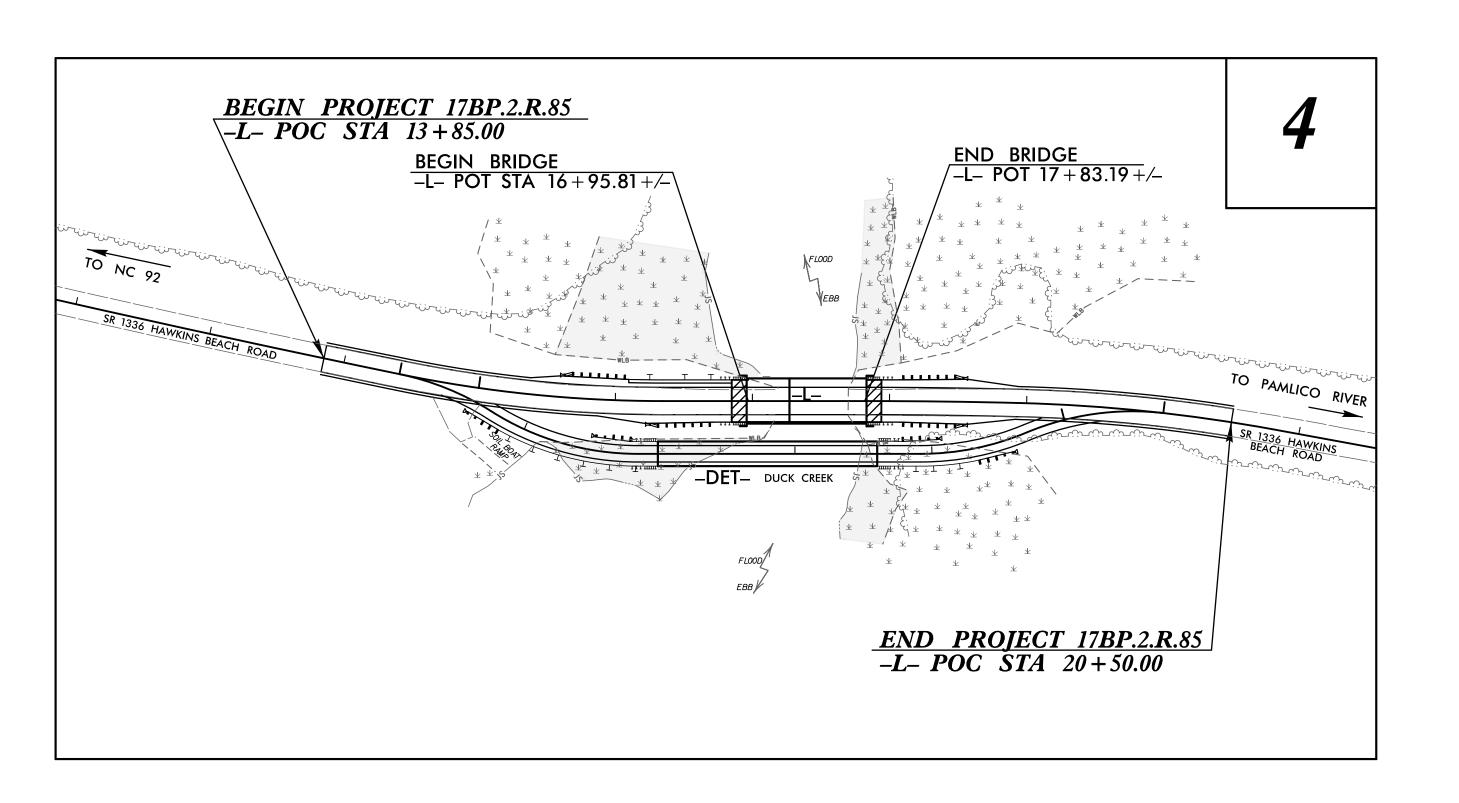
# BEAUFORT COUNTY

LOCATION: REPLACE BRIDGE NO. 249 OVER DUCK CREEK ON SR 1336 (HAWKINS BEACH ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURE







# **NOTES**:

- I. CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.
- 2. THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES.
- 3. SEE SHEET 2B-1 FOR TEMPORARY DETOUR.

DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED** 

# GRAPHIC SCALES **PLANS** PROFILE (HORIZONTAL) PROFILE (VERTICAL)

# DESIGN DATA

ADT 2011 = 530ADT 2031 = 1060K = 10 %D = 60 %

> T = 6V = 60 MPH

\* TTST =2% DUAL 4%FUNC CLASS = LOCAL

SUB-REGIONAL TIER

## PROJECT LENGTH

LENGTH OF ROADWAY PROJECT 17BP.2.R.85 = 0.109 MILES LENGTH OF STRUCTURE PROJECT 17BP.2.R.85 = 0.017 MILES

TOTAL LENGTH OF PROJECT 17BP.2.R.85 = 0.126 MILES Prepared in the Office of:

HNTB NORTH CAROLINA, P.C.
343 E. Six Forks Road, Suite 200
Raleigh, North Carolina 27609
NC License No: C-1554

2018 STANDARD SPECIFICATIONS

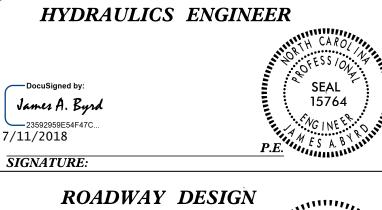
RIGHT OF WAY DATE: JANUARY 31, 2018

LETTING DATE: AUGUST 22, 2018

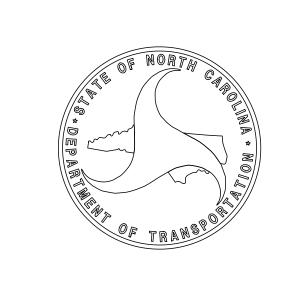
DOUGLAS WHEATLEY, PE

PROJECT ENGINEER ROY H. TELLIER, PE PROJECT DESIGN ENGINEER

HEATHER C. LANE, PE NCDOT CONTACT



**ENGINEER** SEAL 7 36786 Douglas M. Wheatley 7/11/2018 **SIGNATURE**:



SHEET NUMBER

1 SHEET

TITLE SHEET

A INDEX OF SHEETS, GENERAL NOTES & LIST OF STANDARDS

1B SYMBOLOGY SHEET
1C-1 SURVEY CONTROL SHEETS

2A-1 THRU 2A-2 TYPICAL SECTIONS
2B-1 DETOUR DETAIL

2G-1 GEOTEXTILE FOR EMBANKMENT STABILIZATION DETAIL
2G-2 ROCK EMBANKMENT AND ROCK PLATING DETAILS

3B–1 THRU 3B–2 ROADWAY SUMMARY SHEETS

3G–1 GEOTECHNICAL SUMMARY SHEET

4 PLAN & PROFILE SHEET

TMP-1 THRU TMP-4

EC-1 THRU EC-4

EROSION CONTROL PLANS

RF-1 THRU RF-2

REFORESTATION PLANS

UC-1 THRU UC-4
UTILITY CONSTRUCTION PLANS
U0-1 THRU UO-2
UTILITIES BY OTHER PLANS
X-1 THRU X-11
CROSS SECTION SHEETS
S-1 THRU S-17
STRUCTURE PLANS

GENERAL NOTES: 2018 SPECIFICATIONS

EFFECTIVE: 01–16–2018

REVISED:

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF

SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.02

SUBSURFACE DRAINS:

SUBSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD. NO. 815.02 AT

LOCATIONS DIRECTED BY THE ENGINEER.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

TEMPORARY SHORING:

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC WILL BE PAID FOR AS "EXTRA

WORK" IN ACCORDANCE WITH SECTION 104-7.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS— SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION

APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

POWER – CITY OF WASHINGTON
PHONE – CENTURYLINK
FIBER OPTIC – TRI COUNTY COMMUNICATIONS
WATER – BEAUFORT COUNTY WATER AND SEWER

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS, EXCEPT AS SHOWN ON PLANS

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

PROJECT REFERENCE NO. SHEET NO.

17BP.2.R.85

ROADWAY DESIGN ENGINEER

ROADWAY DESIGN ENGINEER

STH CAROLL

OFESSION

SEAL

36786

Durch Charles

OFESSION

SEAL

36786

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

EFF. 01–16–2018

REV.

#### 2018 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch – N. C. Department of Transportation – Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE

DIVISION 2 – EARTHWORK

200.02 Method of Clearing – Method II

225.04 Method of Obtaining Superelevation – Two Lane Pavement

DIVISION 3 – PIPE CULVERTS

300.01 Method of Pipe Installation

310.10 Driveway Pipe Construction

DIVISION 4 - MAJOR STRUCTURES

422.02 Bridge Approach Fills – Type II Modified Approach Fill

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.02 Method of Shoulder Construction – High Side of Superelevated Curve – Method II

DIVISION 8 – INCIDENTALS

815.02 Subsurface Drain

840.29 Frames and Narrow Slot Flat Grates

840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates

840.66 Drainage Structure Steps

846.01 Concrete Curb, Gutter and Curb & Gutter

846.04 Drop Inlet Installation in Shoulder Berm Gutter

862.01 Guardrail Placement

862.02 Guardrail Installation (Special Detail for Sheet 6 of 8)

Structure Anchor Units (Special Detail for Type III Anchor Units Sheets 1 of 7 and 2 of 7)

876.01 Rip Rap in Channels

876.02 Guide for Rip Rap at Pipe Outlets

PROJECT REFERENCE NO.	SHEET NO.
17BP.2.R.85	1B

# STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

CONVENTIONAL PLA	N SHEET	SYMBOLS
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BOUNDARIES AND PROPERT	<b>'Y</b> :	Note: Not to S	Scale *S.	U.E. = Subsurface Utility Engineering	
State Line —		RAILROADS:			000000000000000000000000000000000000000
County Line		Standard Gauge	CSX TRANSPORTATION	Hedge ———————————————————————————————————	
Township Line		RR Signal Milepost	MILEPOST 35	Woods Line	
City Line		Switch	SWITCH	Orchard —	& & & & &
Reservation Line		RR Abandoned	<del></del>	Vineyard ————————————————————————————————————	Vineyard
Property Line —		RR Dismantled		EXISTING STRUCTURES:	
Existing Iron Pin	<u> </u>			MAJOR:	
Computed Property Corner	×	RIGHT OF WAY & PROJECT CO	ONTROL:	Bridge, Tunnel or Box Culvert ————	CONC
Property Monument		Secondary Horiz and Vert Control Point ——	•	Bridge Wing Wall, Head Wall and End Wall –	) CONC WW (
Parcel/Sequence Number		Primary Horiz Control Point		MINOR:	
Existing Fence Line	×××_	Primary Horiz and Vert Control Point	•	Head and End Wall	CONC HW
Proposed Woven Wire Fence		Exist Permanent Easment Pin and Cap ———	$\Diamond$	Pipe Culvert	
Proposed Chain Link Fence		New Permanent Easement Pin and Cap —	<b>♦</b>	Footbridge ————————————————————————————————————	
Proposed Barbed Wire Fence		Vertical Benchmark		Drainage Box: Catch Basin, DI or JB	СВ
Existing Wetland Boundary		Existing Right of Way Marker	$\triangle$	Paved Ditch Gutter	
Proposed Wetland Boundary ————		Existing Right of Way Line		Storm Sewer Manhole ————	(\$)
	EAB	New Right of Way Line	$\frac{R}{W}$	Storm Sewer —————	s
Existing Endangered Ammar Boomdary	EPB	New Right of Way Line with Pin and Cap—	$\frac{R}{W}$	ITTI TTIEC.	
Existing Endangered Plant Boundary —— Existing Historic Property Boundary ——	HPB ———	,	<b>w</b> –	UTILITIES:	
		New Right of Way Line with  Concrete or Granite R/W Marker	$\frac{\mathbb{R}}{\mathbb{R}}$	POWER:	1
Known Contamination Area: Soil		New Control of Access Line with		Existing Power Pole ————————————————————————————————————	•
Potential Contamination Area: Soil		Concrete C/A Marker		Proposed Power Pole	O
Known Contamination Area: Water		Existing Control of Access		Existing Joint Use Pole ————	<b>-←</b> -
Potential Contamination Area: Water —		New Control of Access		Proposed Joint Use Pole	-0-
Contaminated Site: Known or Potential -		Existing Easement Line	——E——	Power Manhole ————————————————————————————————————	(P)
BUILDINGS AND OTHER CU	LIURE:	New Temporary Construction Easement –	——Е——	Power Line Tower ————————————————————————————————————	
Gas Pump Vent or U/G Tank Cap	O	New Temporary Drainage Easement ——	—— TDE ——	Power Transformer ———————————————————————————————————	$\square$
Sign	<u> </u>	New Permanent Drainage Easement ——	PDE	U/G Power Cable Hand Hole	
Well		New Permanent Drainage / Utility Easement	DUE	H–Frame Pole ————————————————————————————————————	•—•
Small Mine	<b>─</b>	New Permanent Utility Easement ———	PUE	U/G Power Line LOS B (S.U.E.*)	P
Foundation —		New Temporary Utility Easement ———	TUE	U/G Power Line LOS C (S.U.E.*)	——————————————————————————————————————
Area Outline		New Aerial Utility Easement	AUE	U/G Power Line LOS D (S.U.E.*)	P
Cemetery				TELEPHONE:	
Building —		ROADS AND RELATED FEATUR	RES:		
School		Existing Edge of Pavement		Existing Telephone Pole	-
Church		Existing Curb		Proposed Telephone Pole	<del>-</del> 0-
Dam —		Proposed Slope Stakes Cut	<u>C</u>	Telephone Manhole	
HYDROLOGY:		Proposed Slope Stakes Fill —————	<u> </u>	Telephone Pedestal	Ш
Stream or Body of Water —————		Proposed Curb Ramp	(CR)	Telephone Cell Tower	••
Hydro, Pool or Reservoir ————————————————————————————————————		Existing Metal Guardrail		U/G Telephone Cable Hand Hole	H <sub>H</sub>
Jurisdictional Stream		Proposed Guardrail —————	<u> </u>	U/G Telephone Cable LOS B (S.U.E.*)	
Buffer Zone 1	BZ 1	Existing Cable Guiderail		U/G Telephone Cable LOS C (S.U.E.*)	
Buffer Zone 2	BZ 2	Proposed Cable Guiderail		U/G Telephone Cable LOS D (S.U.E.*)	
Flow Arrow		Equality Symbol	•	U/G Telephone Conduit LOS B (S.U.E.*) ——	
Disappearing Stream ————————————————————————————————————	<b>&gt;</b>	Pavement Removal —————		U/G Telephone Conduit LOS C (S.U.E.*)——	
Spring —		VEGETATION:	r ∨ ∨ ∨ ∨ ∨ )	U/G Telephone Conduit LOS D (S.U.E.*)——	
Wetland ————————————————————————————————————	<u> </u>		- ::	U/G Fiber Optics Cable LOS B (S.U.E.*) ——	T FO
Proposed Lateral, Tail, Head Ditch ———	FLOW	Single Tree  Single Shrub	- & - &	U/G Fiber Optics Cable LOS C (S.U.E.*)——	T FO
False Sump		Single Shrub	W	U/G Fiber Optics Cable LOS D (S.U.E.*)——	T FO

U.E. = Subsurface Utility Engineering	
Hedge —	
Woods Line	()()()()()
Orchard —	සි සි සි සි
Vineyard ————————————————————————————————————	Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall	- ) CONC WW (
MINOR:	
Head and End Wall ——————————————————————————————————	
Pipe Culvert	
Footbridge ————————————————————————————————————	>
Drainage Box: Catch Basin, DI or JB ———	СВ
Paved Ditch Gutter	
Storm Sewer Manhole —————	(\$)
Storm Sewer —	s
UTILITIES:	
POWER:	
Existing Power Pole ————	•
Proposed Power Pole ————	6
Existing Joint Use Pole ————	
Proposed Joint Use Pole	<del>-</del> \$-
Power Manhole —————	P
Power Line Tower	$\boxtimes$
Power Transformer ———————————————————————————————————	$\overline{\mathcal{M}}$
U/G Power Cable Hand Hole	
H_Frame Pole	•—•
U/G Power Line LOS B (S.U.E.*)	P
U/G Power Line LOS C (S.U.E.*)	
U/G Power Line LOS D (S.U.E.*)	P
TELEPHONE:	
	•
Existing Telephone Pole	<del></del>
Proposed Telephone Pole	
Telephone Manhole	
Telephone Pedestal	
Telephone Cell Tower	
U/G Telephone Cable LOS B (S.U.E.*)	
U/G Telephone Cable LOS B (S.U.E.*)	
U/G Telephone Cable LOS C (S.U.E.*)	
U/G Telephone Cable LOS D (S.U.E.*)	
U/G Telephone Conduit LOS B (S.U.E.*)	
U/G Telephone Conduit LOS C (S.U.E.*)	
U/G Telephone Conduit LOS D (S.U.E.*)	
U/G Fiber Optics Cable LOS B (S.U.E.*)	
U/G Fiber Optics Cable LOS C (S.U.E.*)	——————————————————————————————————————

WATER:	
Water Manhole	. W
Water Meter	
Water Valve	⊗
Water Hydrant	·
U/G Water Line LOS B (S.U.E*)	
U/G Water Line LOS C (S.U.E*)	
U/G Water Line LOS D (S.U.E*)	
Above Ground Water Line	
TV: TV Pedestal	· [C]
TV Tower —	
U/G TV Cable Hand Hole	
U/G TV Cable Hand Hole  U/G TV Cable LOS B (S.U.E.*)	
U/G TV Cable LOS C (S.U.E.*)	
U/G TV Cable LOS D (S.U.E.*)	
U/G Fiber Optic Cable LOS B (S.U.E.*)	
U/G Fiber Optic Cable LOS C (S.U.E.*)	
U/G Fiber Optic Cable LOS D (S.U.E.*)	- TV FU
GAS:	
Gas Valve	
Gas Meter	$\Phi$
U/G Gas Line LOS B (S.U.E.*)	
U/G Gas Line LOS C (S.U.E.*)	
U/G Gas Line LOS D (S.U.E.*)	
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	•
Sanitary Sewer Cleanout	<b>(</b>
U/G Sanitary Sewer Line ————————————————————————————————————	ss
Above Ground Sanitary Sewer —	A/G Sanitary Sewer
SS Forced Main Line LOS B (S.U.E.*)	FSS
SS Forced Main Line LOS C (S.U.E.*)	——————————————————————————————————————
SS Forced Main Line LOS D (S.U.E.*)	FSS
MISCELLANEOUS:	
Utility Pole	
Utility Pole with Base ————————————————————————————————————	
Utility Located Object ————————————————————————————————————	<u>—</u>
Utility Traffic Signal Box —	
Utility Unknown U/G Line LOS B (S.U.E.*)	
U/G Tank; Water, Gas, Oil —————	
Underground Storage Tank, Approx. Loc. ——	
A/G Tank; Water, Gas, Oil ———————————————————————————————————	<del></del>
Geoenvironmental Boring	
U/G Test Hole LOS A (S.U.E.*)	•
Abandoned According to Utility Records ——	_
End of Information —	, , , , , , , , , , , , , , , , , , , ,
	E.U.I.

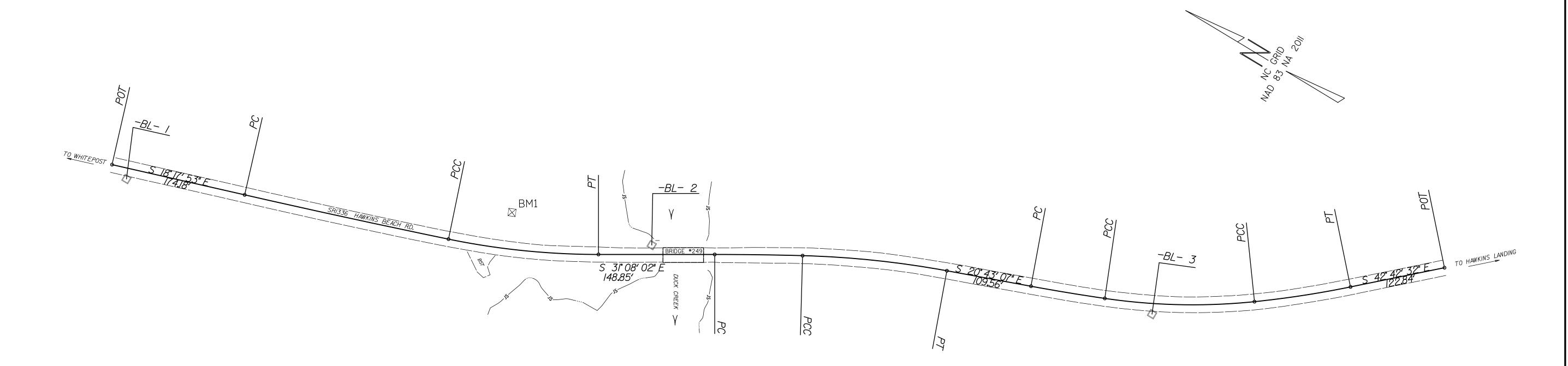


PROJECT REFERENCE NO. SHEET NO

06-0249 1C-1

Location and Surveys

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION



## **BASELINE**

BL				
POINT	N	E	BEARING	DIST
POT	63Ø874.487	26386Ø7.927		
LINE			S 23°57′48.4" E	677.92
POT	630255.000	2638883.267		
LINE			S 23°16′36.4" E	646.76
POT	629660.885	2639138.848		
LINE			S 36°26′Ø9.Ø" E	904.46
POT	628933.228	2639676,026		

## **BENCHMARK**

BM1 ELEVATION = 3.73 N 630430 E 2638827 RR SPIKE SET IN 24" PINE

## EXISTING ALIGNMENT

EL									
POINT	N	E	BEARING	DIST	DELTA	D	L	T	R
POT	63Ø899.832	2638614.115							
LINE			S 18°17′53.3" E	174.18					
PC	63Ø734.463	2638668.800							
CURVE			S 18°53′Ø1.4" E	267.Ø9	Ø1°1Ø′16.3"(LT)	ØØ°26′18.6"	267.Ø9	133.55	13Ø66.29
PCC	630481.750	2638755.242							
CURVE			S 25°18′Ø6.2" E	193.Ø8	11°39′53.3"(LT)	Ø6°Ø1′52.1"	193.41	97.04	950.00
PT	63Ø3Ø7.196	2638837.760							
LINE			S 31°Ø8′Ø2.9" E	148.85					
PC	63Ø179.789	2638914.720							
CURVE			S 30°21′56.6" E	112.54	Ø1°32′12.5"(RT)	Ø1°21′56.1"	112.54	56.27	4195.71
PCC	630082.692	2638971.6Ø9							
CURVE			S 25°Ø9′29.1" E	185.76	Ø8°52′42.6"(RT)	Ø4°46′28.7"	185.95	93.16	1200.00
PT	629914.549	2639Ø5Ø.581							
LINE			S 20°43′07.8" E	109.56					
PC	629812.Ø78	2639Ø89.34Ø							
CURVE			S 21°45′38.7" E	95.79	Ø2°Ø5′Ø1.9"(LT)	Ø2°1Ø′31.Ø"	95.80	47.90	2633.96
PCC	629723.112	2639124.853							
CURVE			S 29°54′25.2" E	191.70	14°12′3Ø.9"(LT)	Ø7°23′34.8"	192.19	96.59	775.00
PCC	629556.941	2639220.432							
CURVE			S 39°51′36.4" E	124.41	Ø5°41′51.5"(LT)	Ø4°34′4Ø.8"	124.46	62.28	1251.54
PT	629461.446	2639300.166							
LINE			S 42°42′32.1" E	122.84					
POT	629371.180	2639383.487							

## DATUM DESCRIPTION

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "BL-4"

WITH NAD 83/NA 2011 STATE PLANE GRID COORDINATES OF NORTHING: 628933.228(f+) EASTING: 2639676.026(f+) ELEVATION: 21.148(f+)

THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.99988391

THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "BL-4" TO -L- STATION IS

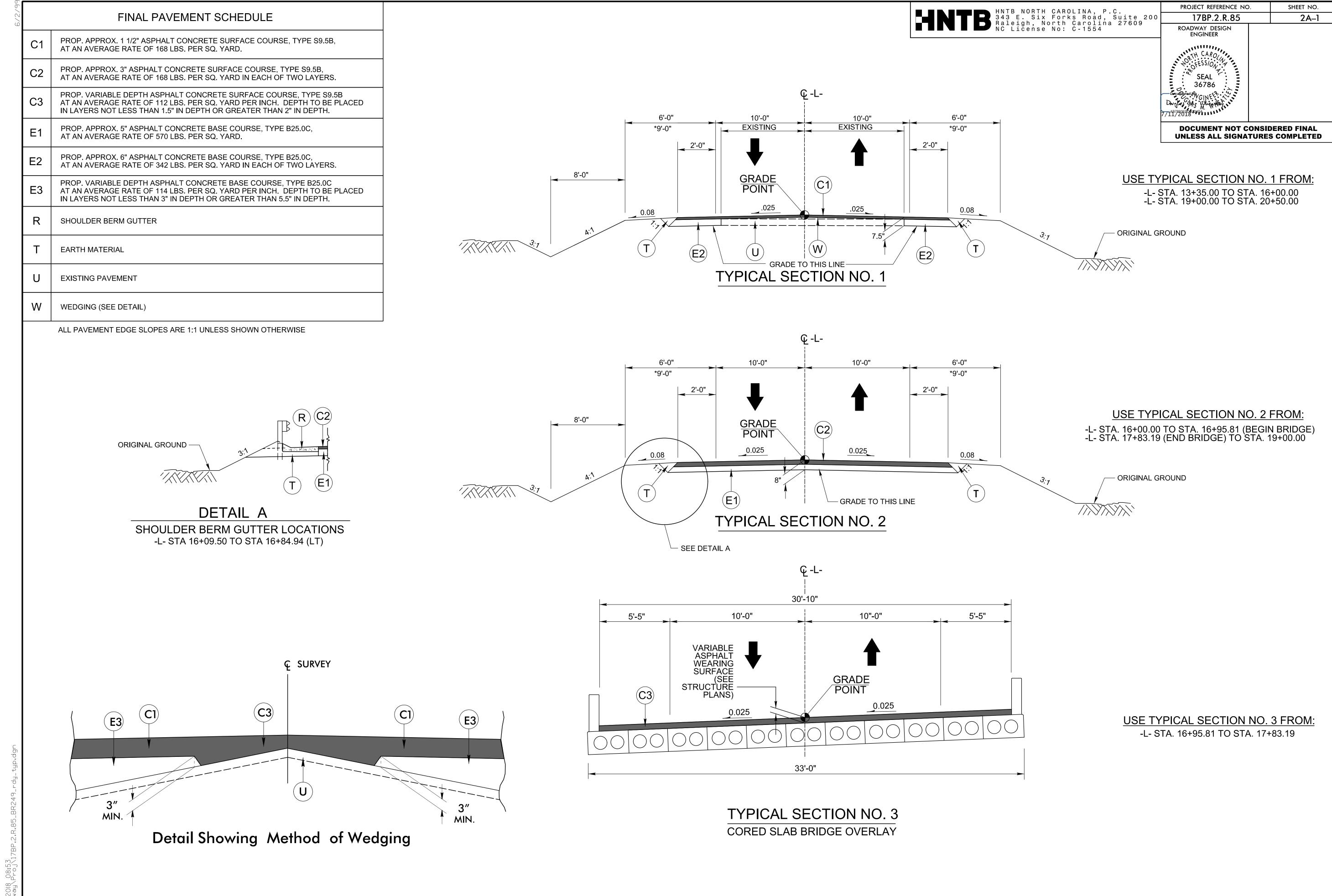
ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES

VERTICAL DATUM USED IS NAVD 88

#### NOTES:

- I. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

/BP.Z.K.85\_Beaufort BK249\トınal Survey\Ø6-Ø249\_LS\_1 TB

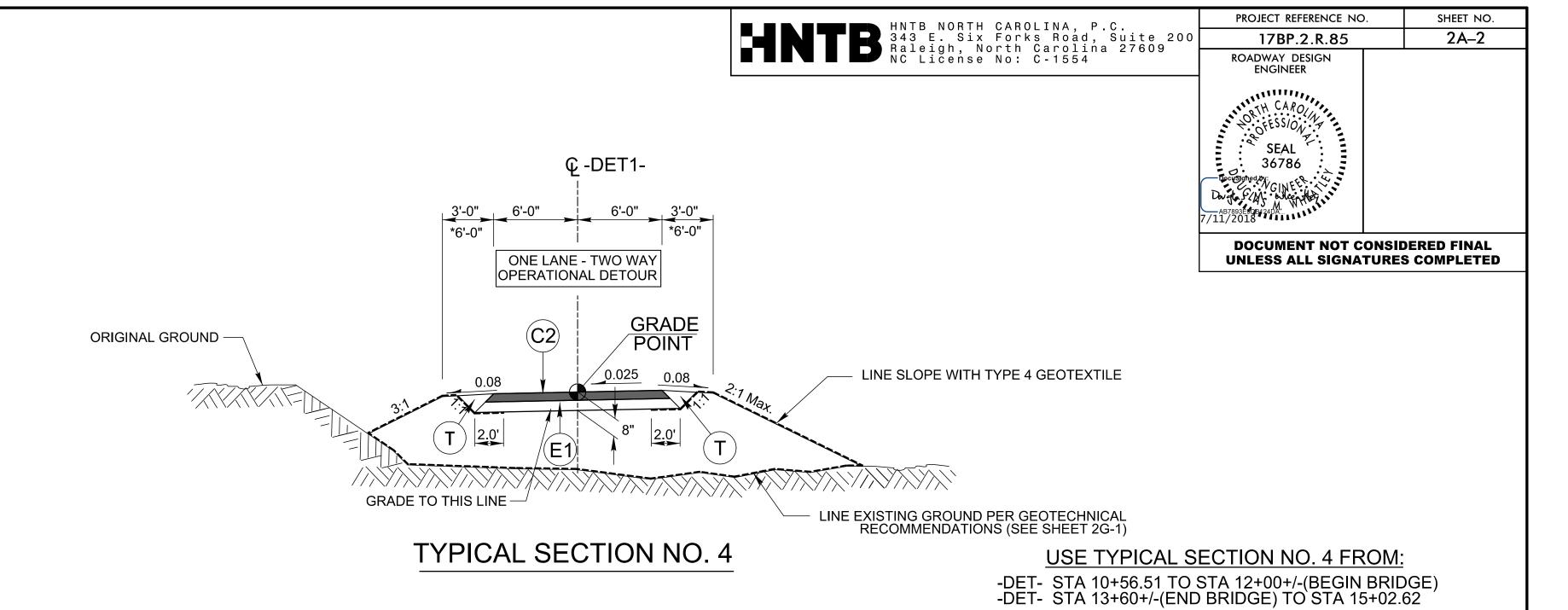


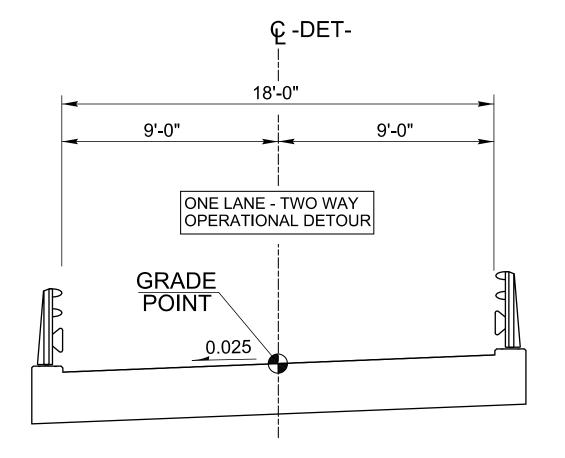
**NOTES:** 

\* SHOULDER WIDTH INCREASED 3' WITH THE USE OF GUARDRAIL

	FINAL PAVEMENT SCHEDULE
C1	PROP. APPROX. 1 1/2" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YARD.
C2	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 168 LBS. PER SQ. YARD IN EACH OF TWO LAYERS.
C3	PROP. VARIABLE DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B AT AN AVERAGE RATE OF 112 LBS. PER SQ. YARD PER INCH. DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 1.5" IN DEPTH OR GREATER THAN 2" IN DEPTH.
E1	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 570 LBS. PER SQ. YARD.
E2	PROP. APPROX. 6" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 342 LBS. PER SQ. YARD IN EACH OF TWO LAYERS.
E3	PROP. VARIABLE DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C AT AN AVERAGE RATE OF 114 LBS. PER SQ. YARD PER INCH. DEPTH TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH.
R	SHOULDER BERM GUTTER
Т	EARTH MATERIAL
U	EXISTING PAVEMENT
W	WEDGING (SEE DETAIL)

ALL PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE

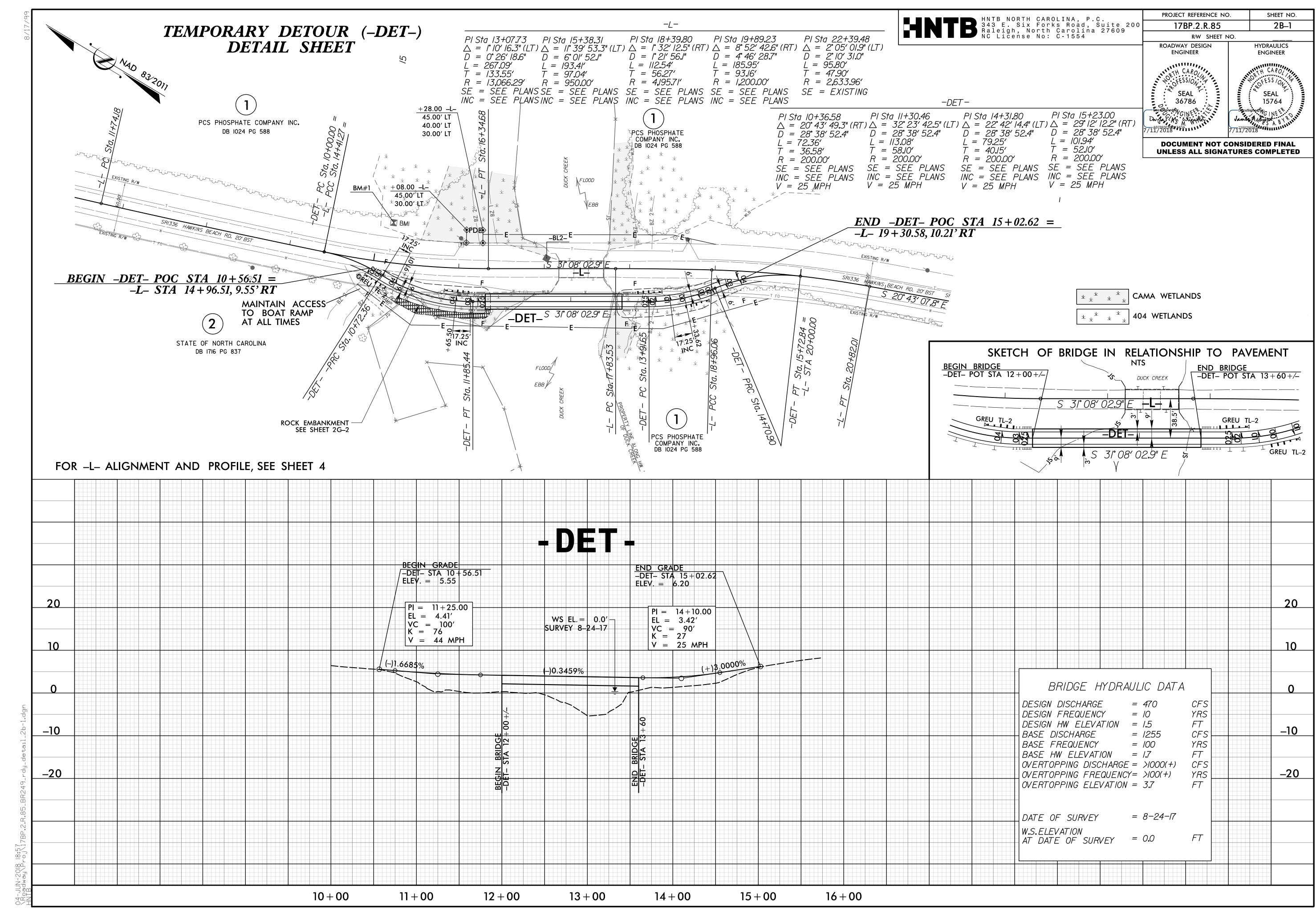


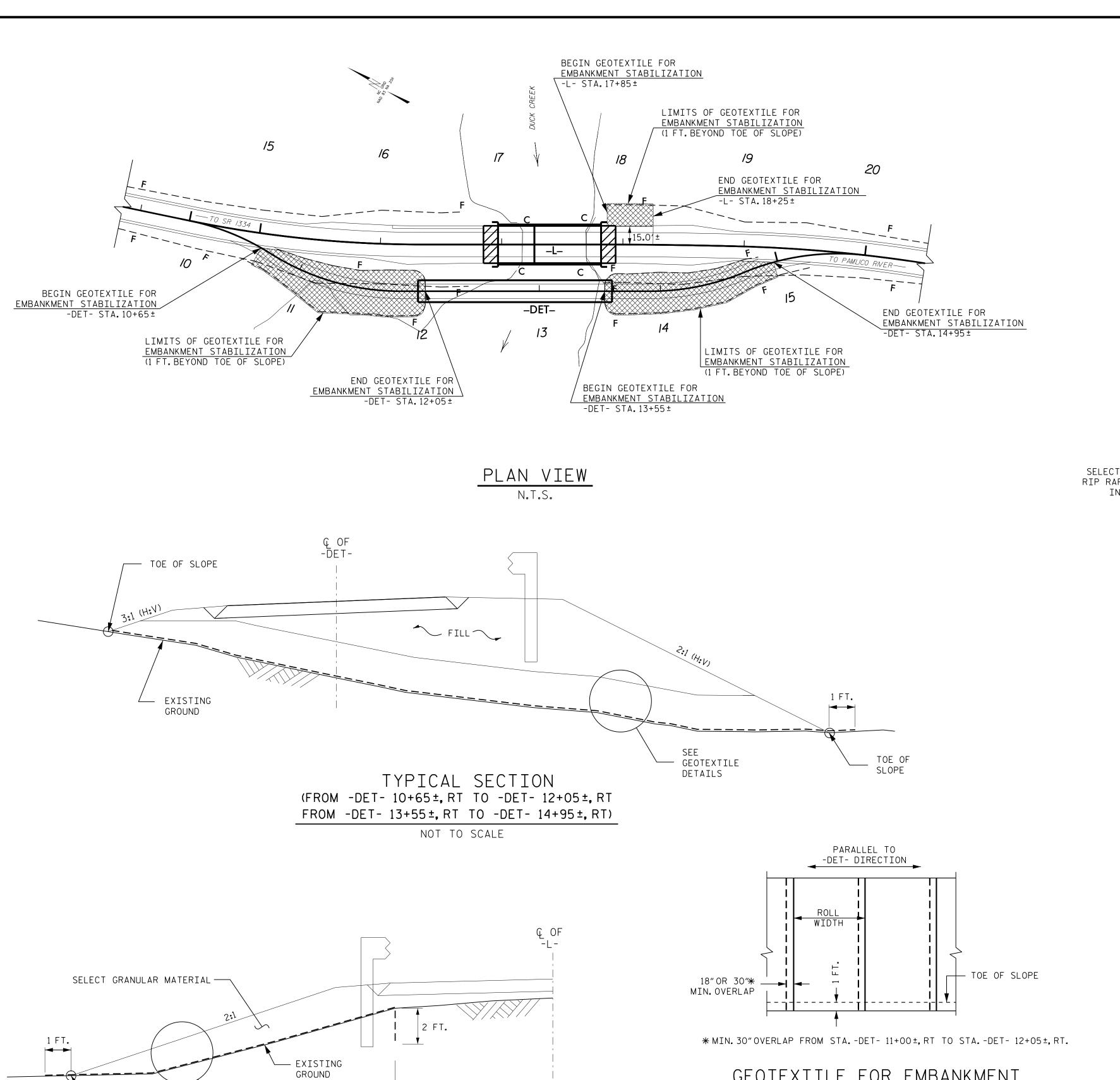


USE TYPICAL SECTION NO. 5 FROM:

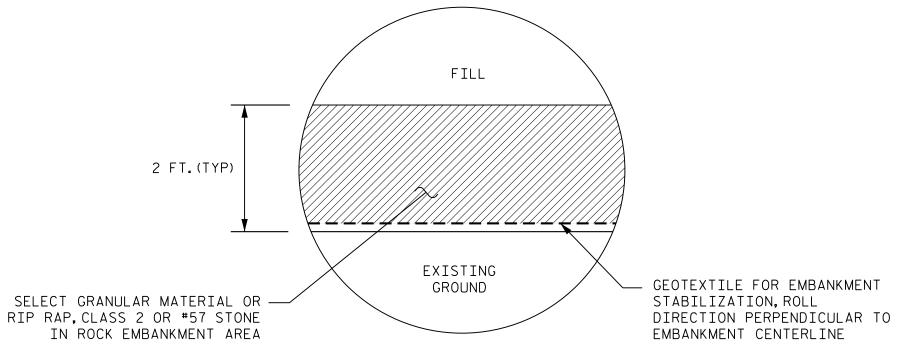
-DET- STA 12+00+/- TO STA 13+60+/-

TYPICAL SECTION NO. 5





PROJECT REFERENCE NO. | SHEET NO. 17BP.2.R.85 2G-1 GEOTECHNICAL **ENGINEER ENGINEER** DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED** 



# GEOTEXTILE DETAILS

#### NOTES

- 1. DO NOT GRUB, ONLY CLEAR THE AREA WITHIN THE LIMITS OF THE GEOTEXTILE FOR EMBANKMENT STABILIZATION.
- 2. PLACE GEOTEXTILE FOR EMBANKMENT STABILIZATION PERPENDICULAR TO EMBANKMENT CENTERLINE ON THE EXISTING GROUND AS SHOWN IN THE PLAN OR AS DIRECTED BY THE ENGINEER.
- 3. PLACE THE GEOTEXTILE WITHOUT ANY WRINKLES OR CREASES.
- 4. PLACE 2 FT.OF SELECT GRANULAR MATERIAL, RIP RAP, CLASS 2 OR #57 STONE ON THE GEOTEXTILE FOR EMBANKMENT STABILIZATION.
- 5. IN ROCK EMBANKMENT AREAS, RIP RAP, CLASS 2 OR #57 STONE SHALL BE PLACED ON THE GEOTEXTILE FOR EMBANKMENT STABILIZATION. SEE ROCK EMBANKMENT DETAILS.
- 6. THE TERMS ROLL AND MACHINE DIRECTION ARE USED INTERCHANGEABLY.
- 7. NO SEAMS OR JOINTS ARE ALLOWED IN THE MACHINE DIRECTION OF GEOTEXTILE.
- 8. ALL JOINTS IN THE CROSS MACHINE DIRECTION MUST BE OVERLAPPED A MINMUM OF 30 INCHES FROM STA.-DET- 11+00±, RT TO STA.-DET- 12+05±, RT OR 18 INCHES ELSEWHERE.
- 9. FOR GEOTEXTILE FOR EMBANKMENT STABILIZATION, SEE GEOTEXTILE FOR EMBANKMENT STABILIZATION (SPECIAL) PROVISION.

#### QUANTITIES GEOTEXTILE FOR EMBANKMENT STABILIZATION 1,100 SY# SELECT GRANULAR MATERIAL 600 CY

\* GEOTEXTILE FOR EMBANKMENT STABILIZATION ESTIMATED QUANTITY DOES NOT INCLUDE OVERLAPS OR WASTE AND NO MEASUREMENT WILL BE MADE FOR OVERLAPPING GEOTEXTILE



GEOTEXTILE FOR EMBANKMENT

STABILIZATION OVERLAP DETAILS

(PLAN VIEW, NOT TO SCALE)

**GEOTECHNICAL** ENGINEERING UNIT

## GEOTEXTILE FOR **EMBANKMENT STABILIZATION DETAILS**

**REVISIONS** BY DATE NO. DATE 3 | | 4 |

NOT TO SCALE

TYPICAL SECTION

(FROM -L- 17+85±, LT TO -L- 18+25±, LT)

SEE - GEOTEXTILE

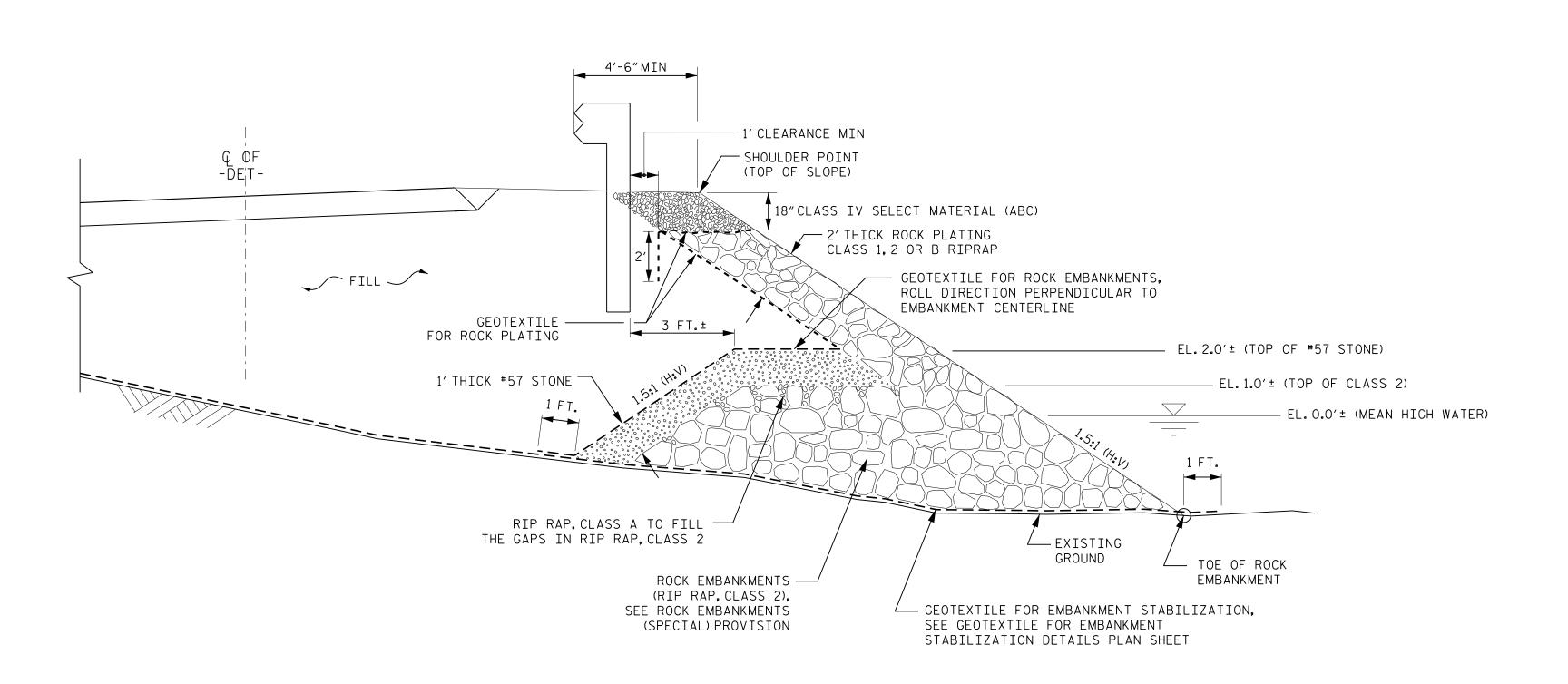
DETAILS

15 FT**.**±

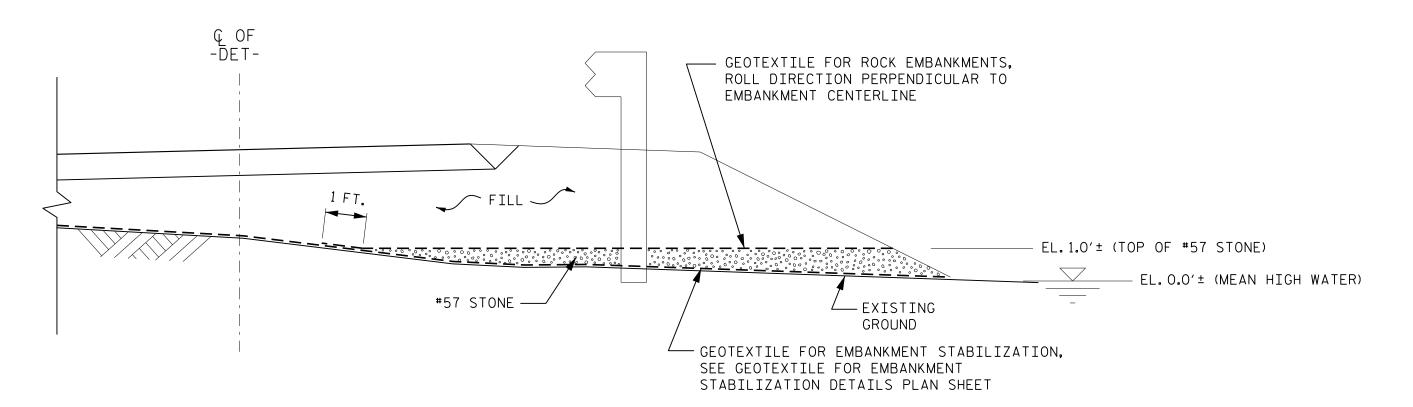
DATE: 04/2018 PREPARED BY: J. PARK DATE: 04/2018 REVIEWED BY: J. BATTS

TOE OF

SLOPE

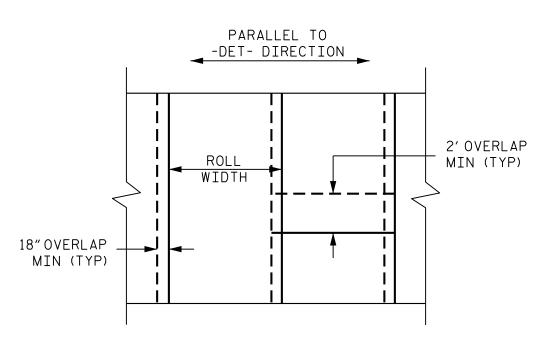


# TYPICAL SECTION (FROM -DET- 11+00±, RT TO -DET- 12+05±, RT) NOT TO SCALE



# TYPICAL SECTION (FROM -DET- 13+55±, RT TO -DET- 14+20±, RT)

NOT TO SCALE



# GEOTEXTILE FOR ROCK EMBANKMENT AND GEOTEXTILE FOR ROCK PLATING OVERLAP DETAILS

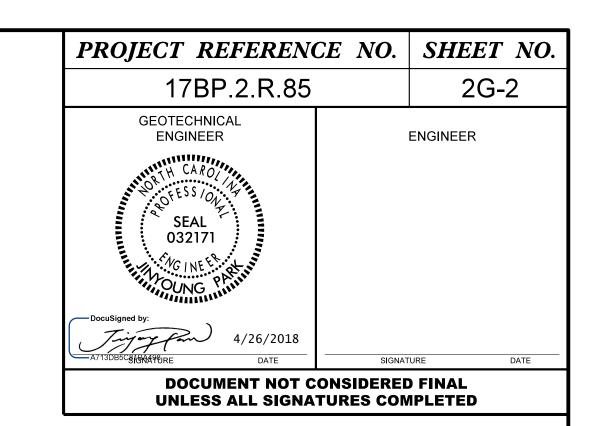
(PLAN VIEW, NOT TO SCALE)

PREPARED BY: J. PARK

DATE: 04 / 2018

REVIEWED BY: J. BATTS

DATE: 04 / 2018

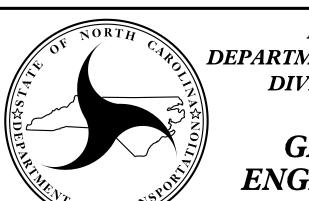


ESTIMATED QUANTITIES	
RIP RAP, CLASS 2	250 TONS*
RIP RAP, CLASS A	50 TONS
#57 STONE (SELECT MATERIAL, CLASS VI)	150 TONS <del>*</del>
GEOTEXTILE FOR ROCK EMBANKMENTS	200 SY**
ROCK PLATING	80 SY

- \* ESTIMATED QUANTITIES FOR RIP RAP, CLASS 2 AND #57 STONE INCLUDE ADDITIONAL QUANTIES FOR SETTLEMENTS.
- \*\*\* GEOTEXTILE FOR EMBANKMENT STABILIZATION ESTIMATED QUANTITY DOES NOT INCLUDE OVERLAPS OR WASTE AND NO MEASUREMENT WILL BE MADE FOR OVERLAPPING GEOTEXTILE

#### NOTES

- 1. FOR ROCK EMBANKMENTS, SEE ROCK EMBANKMENTS (SPECIAL) PROVISIONS.
- 2. INSTALL ROCK EMBANKMENTS USING CLASS 2 RIP RAP AS SHOWN IN THE PLAN AND TO 1.0 FT ABOVE THE MEAN SEA LEVEL.
- 3. FILL VOIDS IN THE TOP OF ROCK EMBANKMENTS WITH RIP RAP, CLASS A.
- 4. PLACE #57 STONE (SELECT MATERIAL, CLASS VI) 1 FT. (TYP.)
  ABOVE RIP RAP, CLASS 2 AS SHOWN IN THE PLAN.
- 5. INSTALL GEOTEXTILE FOR ROCK EMBANKMENT ON TOP OF #57 STONE.
- 6. CONSTRUCT ROCK PLATING ABOVE ROCK EMBANKMENTS. FOR ROCK PLATING, SEE SECTION 275 OF STANDARD SPECIFICATIONS.



NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

#### ROCK EMBANKMENTS AND ROCK PLATING DETAILS

	REVISIONS								
10.	BY	DATE	NO.	BY	DATE				
1	J. PARK	03 / 2018	3						
2			4						

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# PROJECT REFERENCE NO. SHEET NO. 3B–1

# SUMMARY OF EARTHWORK

STATION	STATION	UNCL. EXCAV.	EMBANK. +%	BORROW	WASTE
-DET- STA 10+56.51	-DET- STA 12 + 00(BRIDGE)	1	529	528	
-DET- STA 13+60(BRIDGE)	-DET- STA 15+02.62	5	273	268	
SUBTOTAL	S:	6	802	796	
-L- STA 13+85.00	-L- STA 16+95.81 (BRIDGE)	6	481	475	
-L- STA 17+83.19 (BRIDGE)	-L- STA 20+50.00	7	326	319	
SUBTOTAL		13	807	794	
	REMOVAL	440			
-DET- STA 10+56.51 -DET- STA 13+60(BRIDGE)	-DET- STA 12+00(BRIDGE) -DET- STA 15+02.62	462 191			462 191
SUBTOTAL	S:	653			653
PROJEC	CT TOTALS:	672	1609	1590	653
5% TO REPL	ACE BORROW			80	
GRANI	O TOTALS:	672	1609	1670	653
SAY:		700		1750	

## PAVEMENT REMOVAL SUMMARY

SURVEY LINE	STATION	STATION	LOCATION LT/RT/CL	YD <sup>2</sup>
-L-	16 + 00	17 + 17	CL	243.92
	17 + 70	19 + 00	CL	281.05
–DET– REMOVAL	10 + 63	12 + 00	CL	182.89
	13 + 60	14 + 97	CL	183.18
			TOTAL:	891.04
			SAY:	935

## SHOULDER BERM GUTTER SUMMARY

SURVEY LINE	STATION	STATION	LENGTH (FT)
-L-	16+09.50	16 + 84.94	75.44′
		TOTAL:	75.44′
		SAY:	80′
·			

# ROW AREA DATA SUMMARY

	21077 1221			<b>O D 1 1 1 1 1</b>		
PARCEL NO.	PROPERTY OWNERS NAMES	PROP. R⁄W	PERM. UTILTIY EASE.	PERM. DRAIN. EASE.	PERM. DRAINAGE UTILITY EASE.	CONST. EASE.
1	PCS PHOSPHATE COMPANY INC.			288.79 S.F.		6452.44 S.F.
2	STATE OF NORTH CAROLINA					4364.74 S.F.

# Note: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading."

Earthwork quantities are calculated by the Roadway Design Unit. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

EST. 300 CY UNDERCUT EXCAVATION (FROM NCDOT GEOTECH)
EST. 900 CY SELECT GRANULAR MATERIAL. CLASS III (FROM NCDOT GEOTECH)

# LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

STATION	A (LT,RT, OR CL) STRUCTURE NO.	ATION	EVATION	EVATION	ıtıcal.	CA	CAAP		В	ITUMINOUS (UNLES:	S COATED S NOTED	C.S. PIPE T OTHERWISE)	YPE B		ALUMINIZE	SS III R.C. P OR OD C.S. PIPE OR IPE, TYPE S	E, TYPE IR			STD. 838.6 STD. 838.6 OR STD. 838.6 (UNLESS NOTED OTHERWIS	QUANTITIES FOR DRAINAGE	* TOTAL L.F. FOR PAY    Z	S	FRAME, GRATES AND HOOD STANDARD 840.03	TD. 840.15	. 840.16	.18 .19 .TS =T	TES S	TWO GRATES STD. 840.24	0.32 ′ STD. 840.35	TWO GRATES STD. 840.29	D. & SIZE C.Y. STD 840.72	UG, C.Y. STD. 840.71	C.B. N.D.I. D.I. G.D.I. G.D.I. (N	ABBREVIATIONS  CATCH BASIN NARROW DROP INLET DROP INLET GRATED DROP INLET N.S.) GRATED DROP INLET (NARROW SLOT)
SIZE	LOCATION	TOP ELEV	INVERT EL	INVERT EL	SLOPE CR	15" 18" 24	4" 30" 36'	" 42" 48"	12" 15" 1	8" 24"	30"	36"	42"	48" 1	2" 15" 18"	24" 30" 36	6" 42" 4	48" Ha	PIPE	CU. YDS	IRU 5.0')	*FT.	5		4 OR SI	GRATE STD	" STD. 840 " STD. 840	WITH TWO	AME WITH	1 OR 840	RAME AND	LBOWS NO	XK PIPE PL	J.B. M.H.	(NARROW SLOT)  JUNCTION BOX  MANHOLE
THICKNESS OR GAUGE	FROM	2				064		.109	.064	.064	620.	620.	901.	2.				IDE DRAIN	IDE DRAIN	R.C.P.	EACH (0' TF	THRU 10.0' AND ABOV		TYPE OF GRATE	STD. 840.1.	FRAME & C	G.D.I. TYPE "B" G.D.I. TYPE "D'	D.I. FRAME V	ヹ゠ヹ゠	STD. 840.3 GRATED D.	.D.I. (N.S.) FI	CORR. STEEL EL	NC. & BRIC	T.B.J.B.	TRAFFIC BEARING DROP INLET TRAFFIC BEARING JUNCTION BOX
																		15″ S	18" S 24" S		PER B	5.0′ T	E E	F G	D.I.	D.I. G.I	0.0	3.0	ີ່ ອີ   ອີ	J.B.	T.B.	8 8	CO		REMARKS
_L_ 16+17.70	LT 0401	4.59																			1									1	1				
	0401 O	UT	1.16	1.14											16								_												
																							_												
C 0																																			
0° E 70																							+												
6-																																			
6 4																																			
BRZ																																			
.85																																			
N L																																			
1 A B B B B B B B B B B B B B B B B B B																																			
© O L TOTAL															16						1									1	1				

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO. 17BP.2.R.85 3B-2

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.
G = GATING IMPACT ATTENUATOR TYPE 350
NG = NON-GATING IMPACT ATTENUATOR TYPE 350

# GUARDRAIL SUMMARY

NG =	NON-GATING IMPACT	ATTENUATOR TYPE 350									_		_		_								
SURVEY	BEG. STA.	ENID STA	LOCATION		LENGTH		WARRA	NT POINT	"N" DIST.	TOTAL SHOUL.	FLARE I	LENGTH	,	W				ANCHO	RS	IMPACT ATTENUATOR SINGLE REMOVE 350 FACED EXISTING	REMOVE AND STOCKPILE	DEMARKS	
LINE	BEG. SIA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	TEMP. GREU TL-2	TYPE III	GREU TL-3	TEMP. TYPE III		350 FACED GUARDRAIL GUARDRA	EXISTING GUARDRAIL	REMARKS	
	STA. 16 + 20.81	STA. 16 + 95.81 (BRIDGE)	RT	75′		S1	TA 16 + 95.81 (BRIDGE)		5.42′	9′	50′		1′			1	1						
-L-	STA. 15 + 58.31	STA. 16 + 95.81 (BRIDGE)	LT	137.50′				STA 16 + 97(BRIDGE)	5.42'	9′		50′		1′		1	1						
-L-	STA. 17 + 83.19 (BRIDGE	STA. 18 + 58.19	RT	75′				STA 17+82(BRIDGE	5.42′	9'		50′		1′		1	1						
-L-	STA 17+83.19 (BRIDGE	E) STA 18+57.00	LT	75'			STA 17 + 82(BRIDGE)		5.42′	9′	50′		1′			1	1						
			SUBTOTAL:	362.50′																			
		ANCHO	OR DEDUCTIONS:																				
			REU TL-3: 4@50'	-200′																			
			YPE III: 4@18.75'	<b>–75</b> ′																			
			<u> </u>	07.50/																			
			TOTAL:	87.50′																			
			SAY:	100′						1										++++			
			ADDITIONAL POST:	. 5																			
	TELLBORA BY GUAR	DRAW SOR DETOUR																					
	TEMPORARY GUAR	RDRAIL FOR DETOUR																					
-DET-	STA 10+56.25	STA 12 + 00(BRIDGE)	RT	143.75′		S	STA 12 + 00(BRIDGE)		3′	6′	25′		0.5′		1			1				REMOVE WHEN DETOUR IS REMOVED	
_DET_	STA 11 + 50.00	STA 12 + 00(BRIDGE)	LT	50′				STA 12 + 00(BRIDGE)	) 3′	6′		25′		0.50′	1			1				REMOVE WHEN DETOUR IS REMOVED	
-DET-	STA 13 + 60(BRIDGE)	STA 14+60.00	RT	100′				STA 13 + 60(BRIDGE)	3′	6′		25′		0.50′	1			1				REMOVE WHEN DETOUR IS REMOVED	
_DET_	STA 13 + 60(BRIDGE)	STA 14+10.00	LT	50′		s	TA 13+60(BRIDGE)		3′	6′	25′		0.5′		1			1				REMOVE WHEN DETOUR IS REMOVED	
			SUBTOTAL:	343.75′																			
		ANCHO	OR DEDUCTIONS:																				
			REU TL-2: 4@25'	<b>–100</b> ′						1													
		TY	YPE III: 4@18.75'	<b>–75</b> ′																			
			TOTAL:	168.75′																			
			SAY:	175′																			

COMPUTED BY: _	Tyler C. Bottoms_	DATE: <u>3/12/18</u>
<b>CHECKED BY:</b>	DAT	E:

# (1-16-18)

PROJECT NO.	SHEET NO.
17BP.2.R.85	3G-1

# STATE OF NORTH CAROLINA **DIVISION OF HIGHWAYS**

#### SUMMARY OF GEOTEXTILE SUMMARY OF SUBSURFACE DRAINAGE FOR PAVEMENT STABILIZATION

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
	CONTIN	IGENCY		SD	200
				TOTAL LF:	200

\*UD = Underdrain \*BD = Blind Drain

\*SD = Subsurface Drain

LINE	Station	Station	Geotextile for Pavement Stabilization SY	Class IV Subgrade Stabilization TONS
	CONTINGENC	Y		
	TOT	AL SY/TONS:	0	0*

\*Total tons of "Class IV Subgrade Stabilization" is only the estimated quantity for pavement stabilization and may only represent a portion of the subgrade stabilization quantity shown in the Item Sheets of the

#### SUMMARY OF ROCK PLATING

LINE	Beginning Slope (H:V)	Approx. Station	Ending Slope (H:V)	Approx. Station	Location LT/RT	Rock Plating Detail No. 1/2/3/4	Riprap Class* 1/2/B	Rock Plating SY
-DET-	1.75:1	11+00±	2:1	12+05±	RT			80
	1							
							TOTAL SY:	80

\*Use Class 1, 2 or B riprap if riprap class is not shown for rock plating location.

## SUMMARY OF PRE-SPLITTING OF ROCK

LINE	Beginning Rock Cut Slope (H:V)	Approx. Station	Ending Rock Cut Slope (H:V)	Approx. Station	Location LT/RT	Pre-splitting of Rock SY
		_			TOTAL SY:	0

#### SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION

LINE	Station	Station	Aggregate Type* ASU/AST	Aggregate Thickness INCHES	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Soil Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
	CONTINGENC	Ϋ́							
							_		
			TOTAL	CY/TONS/SY:	0	0**	0**	0	0

\*ASU = Aggregate Subgrade

\*AST = Aggregate Stabilization

#### SUMMARY OF REINFORCED SOIL SLOPES AND SLOPE EROSION CONTROL

LINE	Beginning Slope/ RSS (H:V)	Approx. Station	Ending Slope/ RSS (H:V)	Approx. Station	Location LT/RT	Reinforced Soil Slope (RSS) SY	Geocells SY	Coir Fiber Mat SY	Matting for Erosion Control SY
					TOTAL SY:	0	0	0*	0**

\*Total square yards of "Coir Fiber Mat" is only the estimated quantity for slopes steeper than 2:1 (H:V) and may only represent a portion of the coir fiber mat quantity shown in the Item Sheets of the Proposal.

## SUMMARY OF SURCHARGES AND SURCHARGE WAITING PERIODS

LINE	Station	Station	Surcharge Height FT	MONTHS

## SUMMARY OF SETTLEMENT GAUGES

Gaugo	LINE	Offset			
No.	Gauge and Station		Direction LT/RT		
	TOTAL GAI	JGES (EACH):			

## SUMMARY OF EMBANKMENT WAITING PERIODS

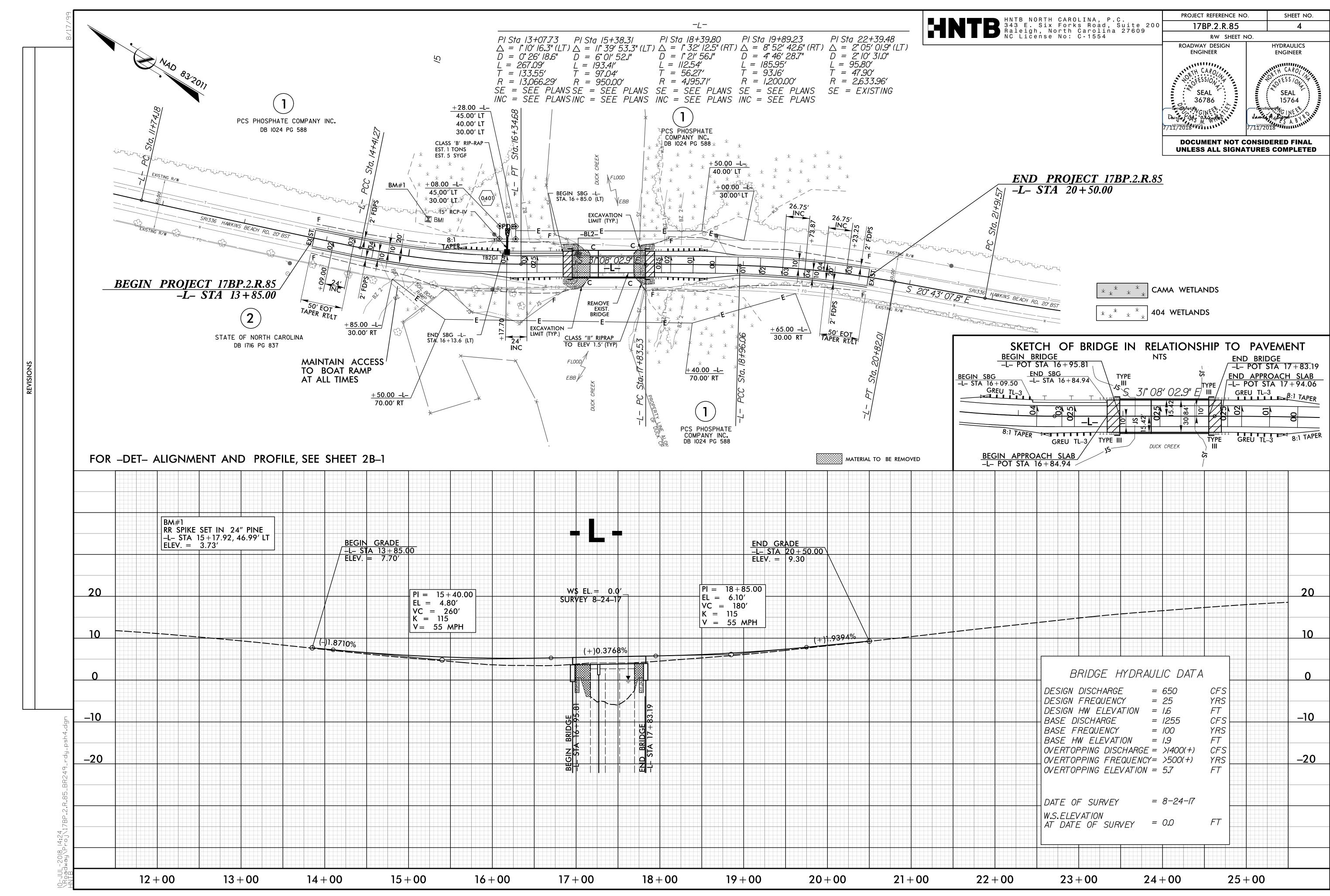
#### SUMMARY OF BRIDGE WAITING PERIODS

LINE	Station	Station	MONTHS
-DET-	10+65±	12+05±	1
-DET-	13+55±	14+95±	1

Bridge Description	End Bent/ Bent No.	MONTHS

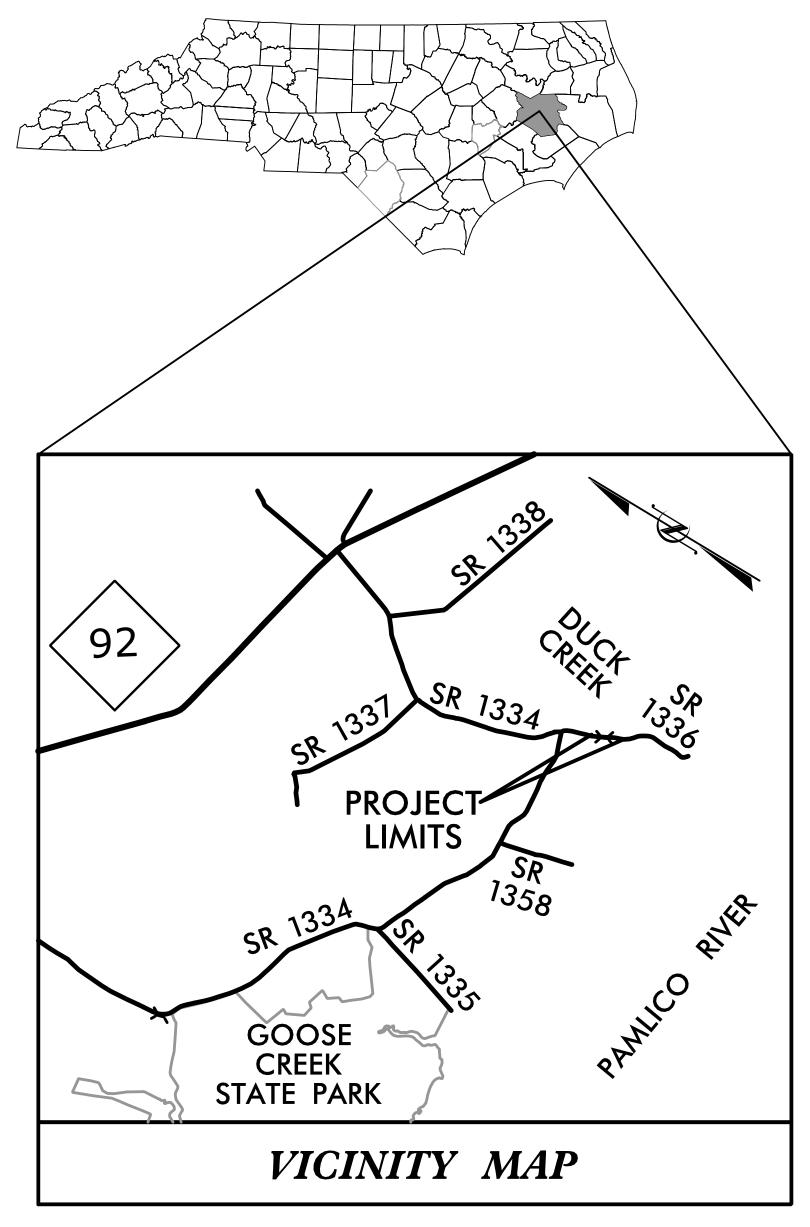
<sup>\*\*</sup>Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Soil Stabilization" are only the estimated quantities for ASU/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.

<sup>\*\*</sup>Total square yards of "Matting for Erosion Control" is only the estimated quantity for RSS and may only represent a portion of the matting quantity shown in the Item Sheets of the Proposal.



# TRANSPORTATION MANAGEMENT PLAN

# BEAUFORT COUNTY



LOCATION: REPLACE BRIDGE NO. 249 OVER DUCK CREEK ON SR 1336 (HAWKINS BEACH ROAD)

PLANS PREPARED BY: HNTB

R.B. EARLY, P.E. PROJECT ENGINEER

J. A. PHILLIPS PROJECT DESIGN TECHNICIAN NCDOT CONTACTS:

S. J. HAMILTON, PE, CPM DIVISION TRAFFIC ENGINEER



TITLE SHEET, VICINITY MAP AND INDEX OF SHEETS

ROADWAY STANDARD DRAWINGS, LEGEND & TEMPORARY PAVEMENT MARKING SCHEDULE TMP-1A

GENERAL NOTES AND PHASING PHASE I STEP 2 DETAIL TMP-3

SHEET NO.

TMP - 1

PHASE I STEPS 3 AND 4 DETAIL

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 



HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Ste 200 Raleigh, North Carolina 27609 NC License No: C-1554

APPROVED: Rhonda B. Early
DATE:

SEAL



TMP-1

PROJ. REFERENCE NO.	SHEET NO.
17BP.2.R.85	TMP-1A

# ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	TITLE
----------	-------

1101.01	WORK ZONE ADVANCE WARNING SIGNS
1101.02	TEMPORARY LANE CLOSURES
1101.03	TEMPORARY ROAD CLOSURES
1101.05	WORK ZONE VEHICLE ACCESSES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1130.01	DRUM
1135.01	CONES
1145.01	BARRICADES
1150.01	FLAGGING DEVICES
1180.01	SKINNY-DRUM
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO-LANE AND MULTI-LANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	RAISED PAVEMENT MARKERS - INSTALLATION SPACING
1251.01	RAISED PAVEMENT MARKERS - PERMANENT AND TEMPORARY

# **LEGEND**

## <u>GENERAL</u>

DIRECTION OF TRAFFIC FLOW

DIRECTION OF PEDESTRIAN TRAFFIC FLOW

----- EXIST. PVMT.

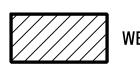
NORTH ARROW

— PROPOSED PVMT.

TEMP. SHORING (LOCATION PURPOSES ONLY)

WORK AREA

REMOVAL



#### SIGNALS







#### PAVEMENT MARKINGS

——EXISTING LINES ——TEMPORARY LINES

## TRAFFIC CONTROL DEVICES

BARRICADE (TYPE III) 

DRUM SKINNY DRUM TEMPORARY CRASH CUSHION

FLAGGER

LAW ENFORCEMENT

TRUCK MOUNTED ATTENUATOR (TMA)

#### TEMPORARY SIGNING

PORTABLE SIGN

STATIONARY SIGN

#### PAVEMENT MARKERS

CRYSTAL/CRYSTAL

CRYSTAL/RED

YELLOW/YELLOW

## PAVEMENT MARKING SYMBOLS

PAVEMENT MARKING SYMBOLS

#### TEMPORARY PAVEMENT MARKING

SYMBOL DESCRIPTION PAY ITEM

PAVEMENT MARKING LINES

WHITE EDGELINE

DOUBLE YELLOW

WHITE STOP BAR

CRYSTAL/CRYSTAL

PAINT (24")

PAINT (4")

PAVEMENT MARKERS

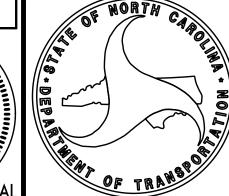
TEMPORARY RAISED

NOTE: FOR EACH PAINT PAVEMENT MARKING ITEM, REFER TO

GENERAL NOTES FOR NUMBER OF APPLICATIONS.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED Rhonda B. Early ----F34CAF5AC6BF48A... SEAL 023521

6/18/2018



TRANSPORTATION MANAGEMENT PLAN ROADWAY STANDARD DRAWINGS, LEGEND & TEMPORARY PAVEMENT MARKING SCHEDULE

HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Suite 200 Raleigh, North Carolina 27609 NC License No: C-1554

## **MANAGEMENT STRATEGIES**

MAINTENANCE OF TRAFFIC FOR THIS PROJECT HAS BEEN DIVIDED INTO TWO PHASES USING TEMPORARY SIGNALS AND ONE-LANE, TWO-WAY TRAFFIC PATTERNS.

DURING PHASE I, USING FLAGGERS AS NEEDED, CONSTRUCT TEMPORARY BRIDGE PROVIDING SMOOTH TIE FROM EXISTING TO PROPOSED. SHIFT TRAFFIC TO TEMPORARY PATTERN. TRAFFIC WILL BE IN A ONE-LANE, TWO-WAY PATTERN ON THE ONSITE DETOUR WITH THE USE OF TEMPORARY PORTABLE SIGNALS WHILE THE PROPOSED BRIDGE AND ROADWAY IS CONSTRUCTED.

DURING PHASE II, USING FLAGGERS, TRAFFIC IS RETURNED TO ORIGINAL PATTERN WHILE TEMPORARY DETOUR IS REMOVED AND COMPLETED BY PLACING FINAL LAYER OF SURFACE COURSE, FINAL PAVEMENT MARKINGS & MARKERS AND PLACE TRAFFIC IN FINAL PATTERN.

#### GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

#### LANE AND SHOULDER CLOSURE REQUIREMENTS

- A) REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED OR AS DIRECTED BY THE ENGINEER.
- B) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.
- C) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING ON THE SHOULDER ADJACENT TO AN UNDIVIDED FACILITY AND WITHIN 5 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN TRAVEL LANE USING ROADWAY STANDARD DRAWING NO. 1101.02 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL.
- D) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN A LANE OF TRAVEL OF AN UNDIVIDED OR DIVIDED FACILITY. CLOSE THE LANE ACCORDING TO THE TRAFFIC CONTROL PLANS, ROADWAY STANDARD DRAWINGS, OR AS DIRECTED BY THE ENGINEER. CONDUCT THE WORK SO THAT ALL PERSONNEL AND/OR EQUIPMENT REMAIN WITHIN THE CLOSED TRAVEL LANE.

#### PAVEMENT EDGE DROP OFF REQUIREMENTS

E) BACKFILL AT A 6:1 SLOPE UP TO THE EDGE AND ELEVATION OF EXISTING PAVEMENT IN AREAS ADJACENT TO AN OPENED TRAVEL LANE THAT HAS AN EDGE OF PAVEMENT DROP-OFF AS FOLLOWS:

BACKFILL DROP-OFFS THAT EXCEED 2 INCHES ON ROADWAYS WITH POSTED SPEED LIMITS OF 45 MPH OR GREATER.

BACKFILL DROP-OFFS THAT EXCEED 3 INCHES ON ROADWAYS WITH POSTED SPEED LIMITS LESS THAN 45 MPH.

BACKFILL WITH SUITABLE COMPACTED MATERIAL, AS APPROVED BY THE ENGINEER, AT NO EXPENSE TO THE DEPARTMENT.

## GENERAL NOTES

F) DO NOT EXCEED A DIFFERENCE OF 2 INCHES IN ELEVATION BETWEEN OPEN LANES OF TRAFFIC FOR NOMINAL LIFTS OF 1.5 INCHES. INSTALL ADVANCE WARNING "UNEVEN LANES" SIGNS (W8-11) 350 IN ADVANCE AND A MINIMUM OF EVERY HALF MILE THROUGHOUT THE UNEVEN AREA.

#### TRAFFIC PATTERN ALTERATIONS

- G) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.
- H) NOTIFY THE OVERSIZE/OVERWEIGHT PERMIT GROUP FOURTEEN (14) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

#### SIGNING

- I) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- J) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

#### TRAFFIC CONTROL DEVICES

- K) WHEN LANE CLOSURES ARE NOT IN EFFECT SPACE CHANNELIZING DEVICES IN WORK AREAS NO GREATER IN FEET THAN TWICE THE POSTED SPEED LIMIT (MPH) EXCEPT, 10 FT ON-CENTER IN RADII, AND 3 FT OFF THE EDGE OF AN OPEN TRAVELWAY. REFER TO STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES SECTIONS 1130 (DRUMS), 1135 (CONES) AND 1180 (SKINNY DRUMS) FOR ADDITIONAL REQUIREMENTS.
- L) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

#### PAVEMENT MARKINGS AND MARKERS

M) INSTALL TEMPORARY PAVEMENT MARKINGS AND TEMPORARY PAVEMENT MARKERS ON INTERIM LAYERS OF PAVEMENT AS FOLLOWS:

ROAD NAME	MARKING	MARKER
1. SR 1336	PAINT	TEMPORARY RAISED

- N) PLACE ONE APPLICATION OF PAINT FOR TEMPORARY TRAFFIC PATTERNS. PLACE A SECOND APPLICATION OF PAINT SIX (6) MONTHS AFTER THE INITIAL APPLICATION AND EVERY SIX MONTHS AS DIRECTED BY THE ENGINEER.
- O) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.
- P) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS BY THE END OF EACH DAY'S OPERATION.
- Q) PASSING ZONES WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.

#### **MISCELLANEOUS**

R) ENSURE THE OVERSIZE/OVERWEIGHT PERMIT UNIT (919) 733-4740 HAS BEEN ADVISED OF THE ONGOING TRAFFIC OPERATIONS THROUGH THE DIVISION OFFICE. SEE ALSO GENERAL NOTE "H".

## **PHASING**

COMPLETE ANY PROPOSED WIDENING IN SUCH A MANNER THAT PONDING OF WATER WILL NOT OCCUR IN THE TRAVEL LANE. THIS MAY REQUIRE TEMPORARY DITCHES.

SHEET NO.

TMP-2

PROJ. REFERENCE NO. 17BP.2.R.85

THE TERM "RSD" REFERS TO ROADWAY STANDARD DRAWINGS.

ALL REFERENCES TO CONSTRUCTION INCLUDE PAVING UP TO BUT NOT INCLUDING FINAL LAYER OF SURFACE COURSE UNLESS SPECIFICALLY CALLED FOR.

#### PHASE I

#### STEP 1:

INSTALL ADVANCED WORK ZONE WARNING SIGNS. USING INSET "TYPICAL FOR SIGN PLACEMENT" ON SHEET TMP-4, INSTALL ALL SIGNS AND COVER ALL EXCEPT THE ADVANCED WORK ZONE WARNING SIGNS.

#### STEP 2:

USING RSD 1101.02, (SHEET 1 OF 14) AND FLAGGERS AS NEEDED, CONSTRUCT DETOUR FROM -DET- STA 10+33+/- TO STA 15+25+/-. (SEE SHEET TMP-3.)

#### STEP 3:

INSTALL TEMPORARY SIGNALS, PAVEMENT MARKING, MARKERS, DEVICES, UNCOVER SIGNS AS SHOWN ON TMP-4 AND SHIFT TRAFFIC TO ONE LANE-TWO WAY TRAFFIC PATTERN. (SEE SPECIAL PROVISION FOR TEMPORARY PORTABLE SIGNAL.)

#### STEP 4:

AWAY FROM TRAFFIC, REMOVE EXISTING STRUCTURE (REFER TO STRUCTURE PLANS) AND CONSTRUCT PROPOSED BRIDGE. (SEE TMP-4)

AWAY FROM TRAFFIC CONSTRUCT THE FOLLOWING:

- \* -L- FROM STA 15+50+/- TO BRIDGE
- \* -L- FROM BRIDGE TO STA 19+00+/-

#### PHASE II (NOT SHOWN)

#### STEP 1:

USING RSD 1101.02 (SHEET 1 OF 14) AND FLAGGERS AS NEEDED, MAINTAINING TRAFFIC ON TEMPORARY DETOUR. CONSTRUCT NB SIDE OF -L- FROM STA 13+85+/-TO STA 15+50+/- AND FROM STA 19+00+/- TO STA 20+50+/- UP TO BUT NOT INCLUDING THE FINAL LAYER OF SURFACE COURSE.

#### STEP 2:

USING RSD 1101.02 (SHEET 1 OF 14) AND FLAGGERS AS NEEDED, SHIFT TRAFFIC TO -L- NB IN A ONE LANE, TWO WAY PATTERN, WORK IN A CONTINUOUS MANNER TO CONSTRUCT SB SIDE OF -L- FROM STA 13+85+/- TO STA 15+50+/- AND FROM STA 19+00+/- TO STA 20+50+/- UP TO BUT NOT INCLUDING THE FINAL LAYER OF SURFACE COURSE. PLACE PAINT PAVEMENT MARKING IN PROPOSED PATTERN, REMOVE STOP BARS AND OPEN -L- TO TWO-LANE, TWO WAY PATTERN.

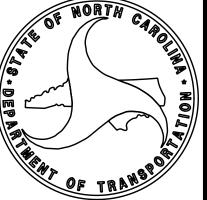
#### STEP 3:

USING RSD 1101.02 (SHEET 1 OF 14) AND FLAGGERS AS NEEDED, REMOVE TEMPORARY SIGNAL, REMOVE DETOUR BRIDGE AND TEMPORARY PAVEMENT.

#### STEP 4:

USING RSD 1101.02 (SHEET 1 OF 14), PLACE FINAL LAYER OF SURFACE COURSE ON -L- FROM STA 13+85+/- TO STA 20+50+/-. PLACE PAVEMENT MARKING (PAINT) IN FINAL PATTERN.

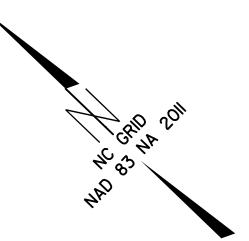
DOCUMENT NOT CONSIDERED FINAL **UNLESS ALL SIGNATURES COMPLETED** Rhonda B. Early SEAL 023521 6/18/2018

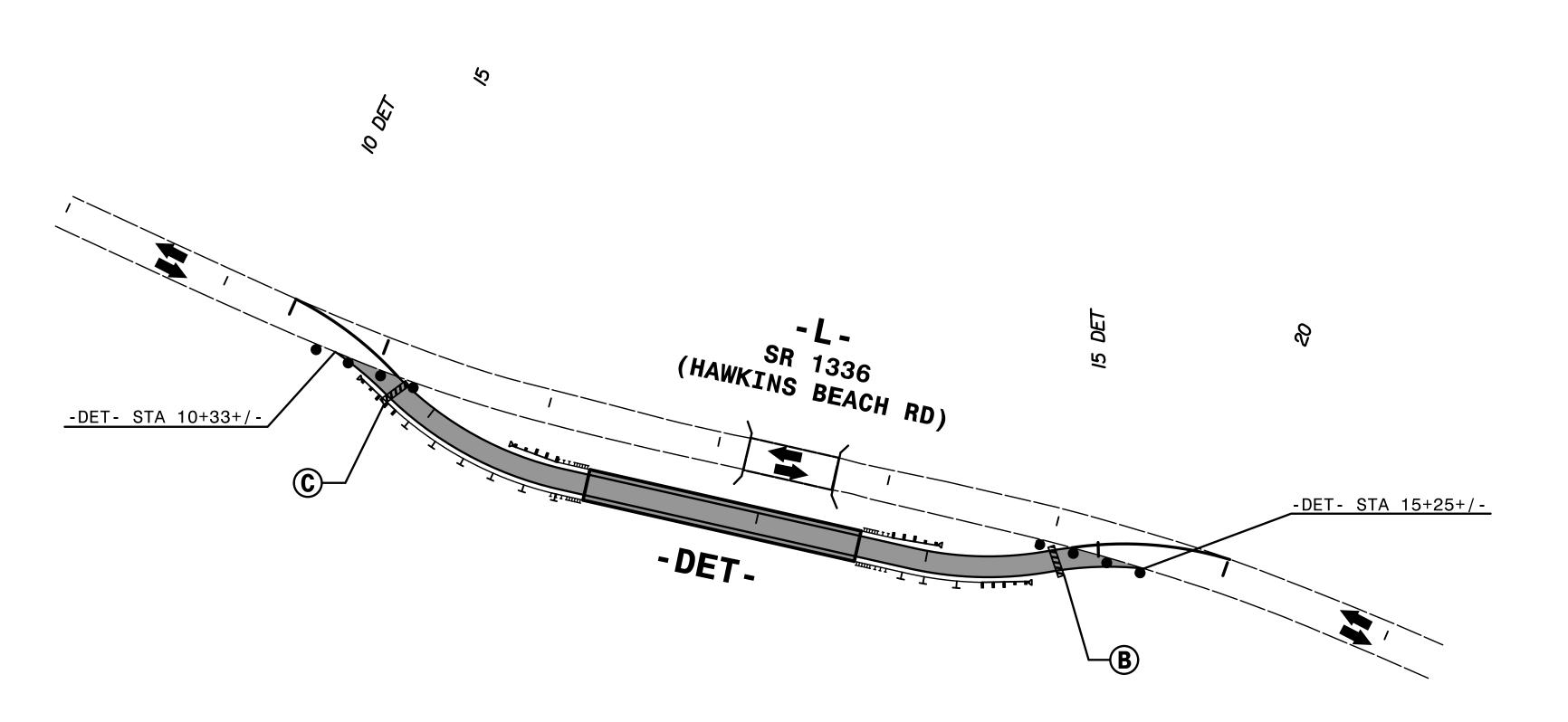


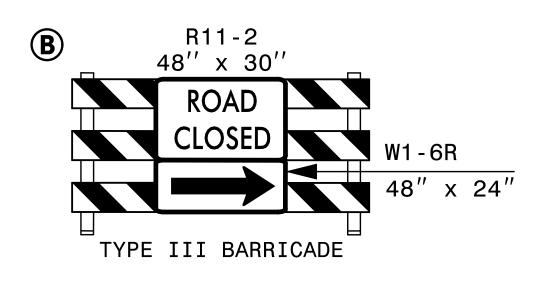
TRANSPORTATION MANAGEMENT PLAN

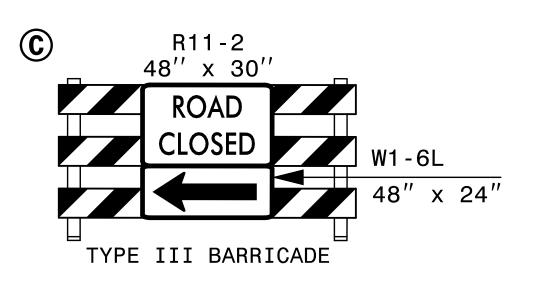
TRANSPORTATION OPERATIONS PLAN, NOTES AND PHASING

PROJ. REFERENCE NO. SHEET NO. 17BP.2.R.85 TMP-3









DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

APPROXED A CARDY CAROUTE SOLUTION OF ESSION AND CONTROL OF ESSION AND

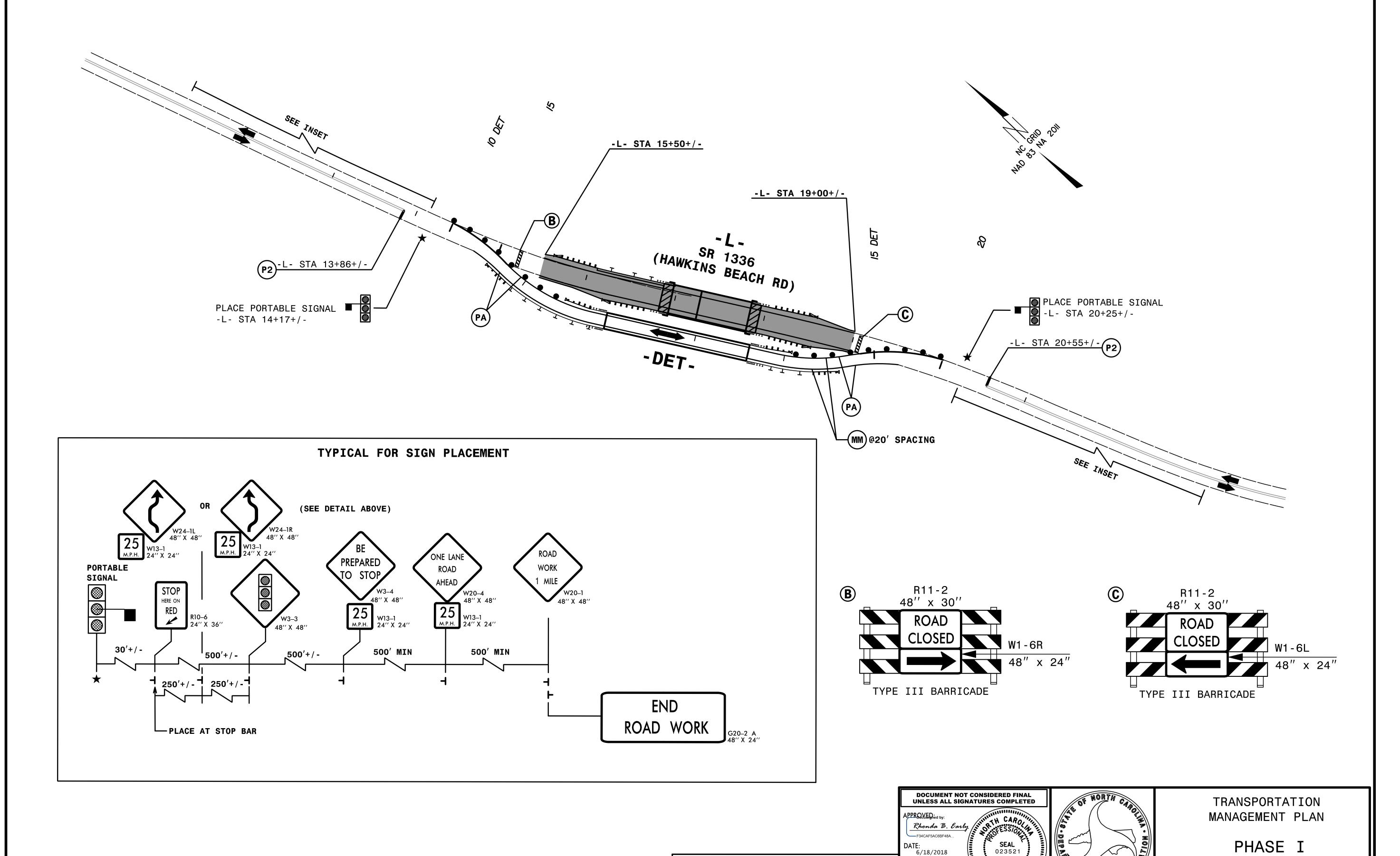


TRANSPORTATION MANAGEMENT PLAN

PHASE I STEP 2

PROJ. REFERENCE NO. TMP-4

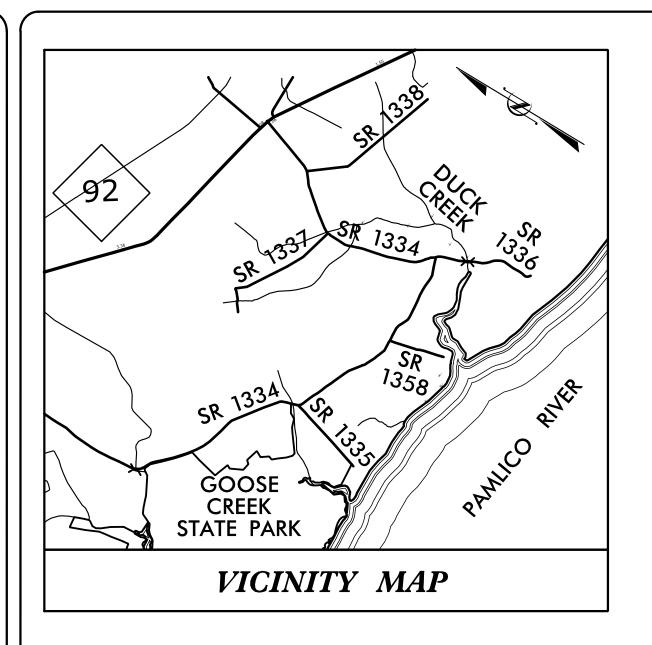
STEPS 3 AND 4



DATE:

HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Suite 200 Raleigh, North Carolina 27609 NC License No: C-1554

6/18/2018



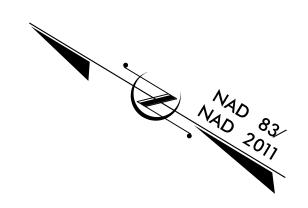
# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

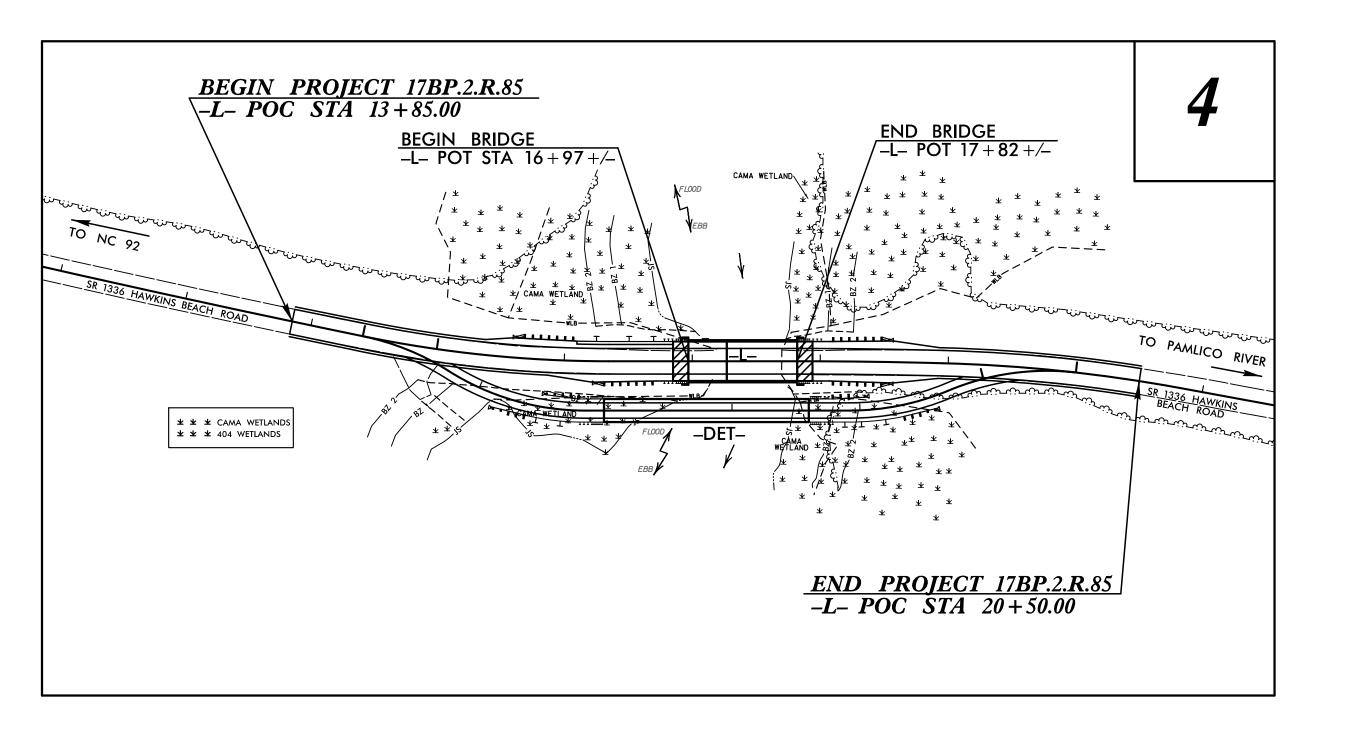
PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

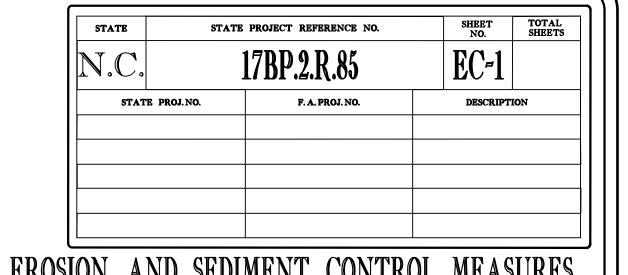
# BEAUFORT COUNTY

LOCATION: REPLACE BRIDGE NO. 249 OVER DUCK CREEK ON SR 1336 (HAWKINS BEACH ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING AND STRUCTURE







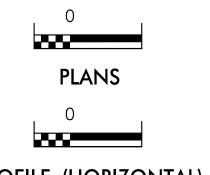
	MIND SEPTIMENT CONTINUE PIERSUNES
<u>Std.</u> #	Description Symbol
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion TD
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Drains
1630.02	Silt Basin Type B
1633.01	Temporary Rock Silt Check Type-A.
	Temporary Rock Silt Check Type A with Matting and Polyacrylamide (PAM)
1633.02	Temporary Rock Silt Check Type-B
	Wattle / Coir Fiber Wattle
	Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)
1634.01	Temporary Rock Sediment Dam Type-A
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A 8
1635.02	Rock Pipe Inlet Sediment Trap Type-B
1630.04	Stilling Basin
1630.06	Special Stilling Basin
	Rock Inlet Sediment Trap:
1632.01	Туре А
1632.02	Туре В
1632.03	Туре С
	Skimmer Basin
	Tiered Skimmer Basin
	Infiltration Basin

#### **ENVIRONMENTALLY** SENSITIVE AREA(S) EXIST ON THIS PROJECT

Refer To E. C. Special Provisions for Special Considerations.

THIS PROJECT HAS **JEEN DESIGNED TO** SENSITIVE WATERSHED STANDARDS.

# GRAPHIC SCALE



PROFILE (HORIZONTAL)

PROFILE (VERTICAL)

ROADSIDE ENVIRONMENTAL UNIT **DIVISION OF HIGHWAYS** STATE OF NORTH CAROLINA

> THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 1, 2016 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

# Prepared in the Office of: HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Suite 200 Raleigh, North Carolina 27609 NC License No: C-1554

2018 STANDARD SPECIFICATIONS

KASE SCHALOIS **EROSION CONTROL** LEVEL III CERTIFICATION #4079

#### Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drains 1630.01 Riser Basin 1630.02 Silt Basin Type B

1630.03 Temporary Silt Ditch 1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin

1631.01 Matting Installation

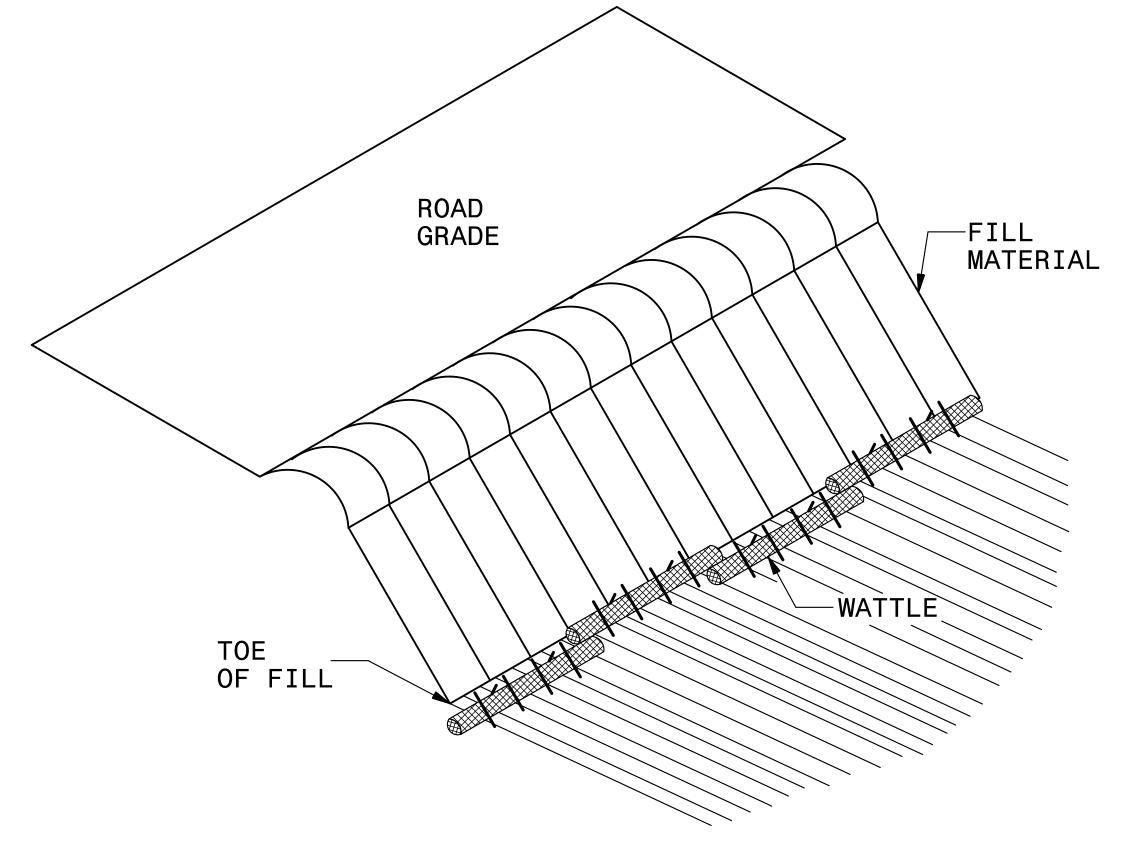
1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A 1634.02 Temporary Rock Sediment Dam Type B

1635.01 Rock Pipe Inlet Sediment Trap Type A 1635.02 Rock Pipe Inlet Sediment Trap Type B 1640.01 Coir Fiber Baffle

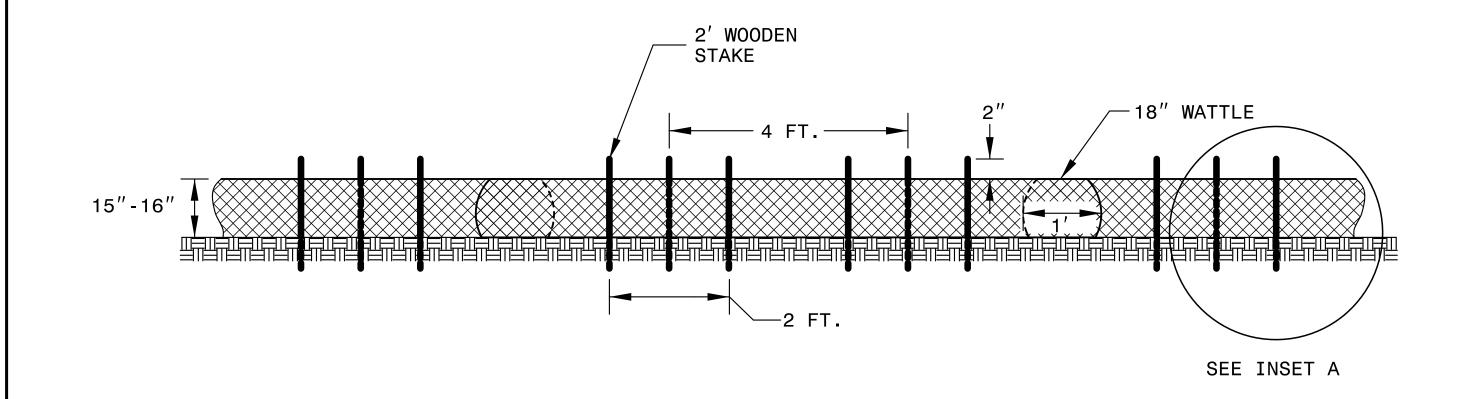
1645.01 Temporary Stream Crossing

# COIR FIBER WATTLE BARRIER DETAIL

PROJECT REFERENCE NO.		SHEET NO.	
17BP.2.R.85		EC-2	1
R/W SHEET N	10.		
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	



**ISOMETRIC VIEW** 



FRONT VIEW

#### NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

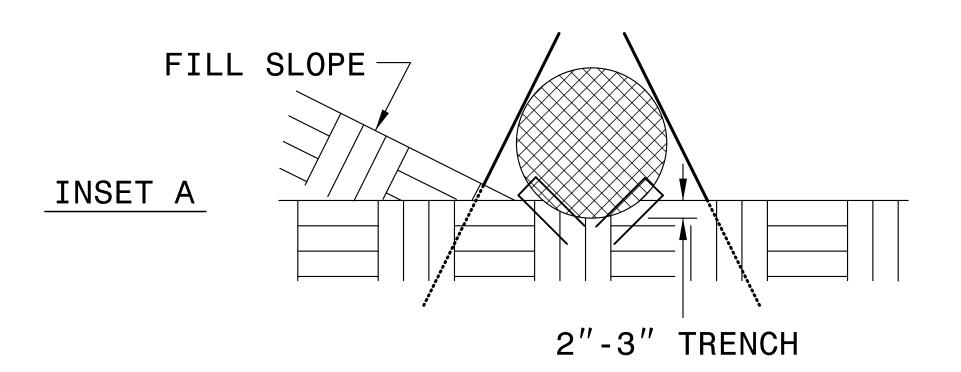
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

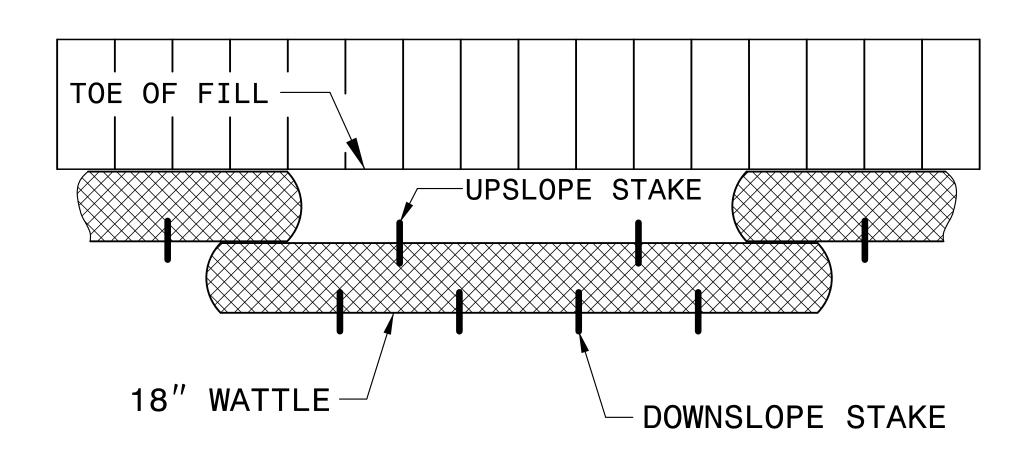
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 25 FT.

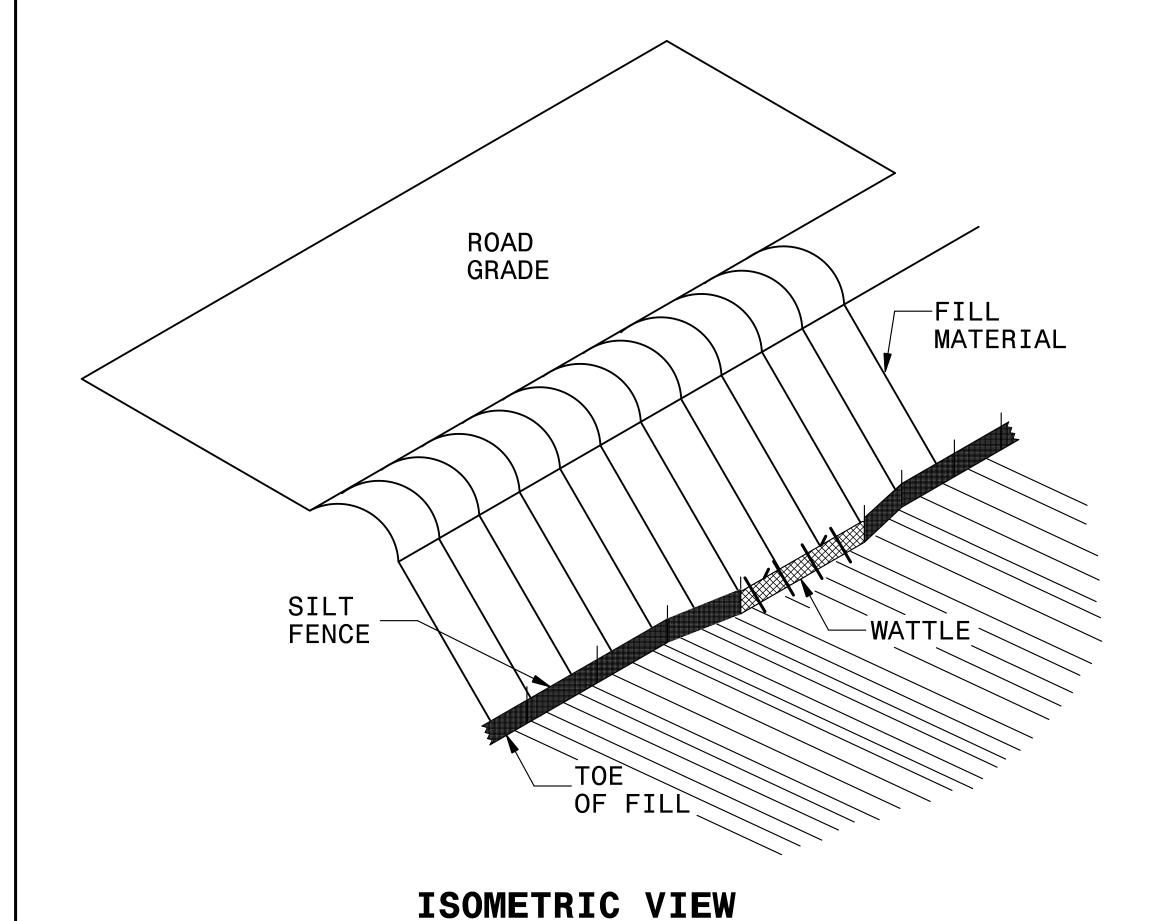


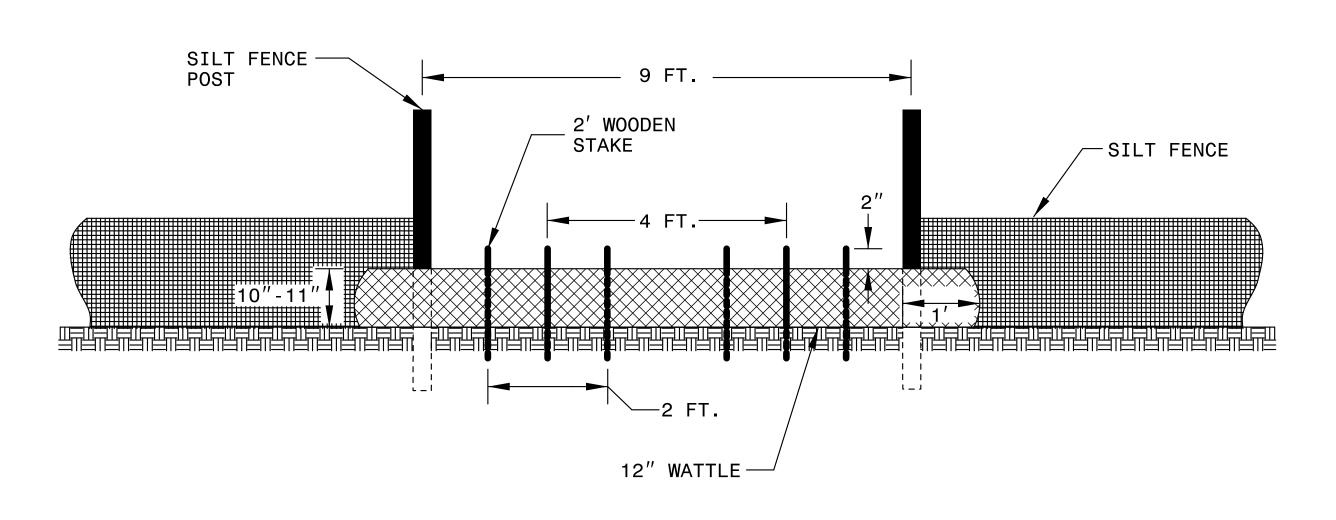


TOP VIEW

# SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO.  ITBP.2.R.85  RW SHEET NO.  ROADWAY DESIGN ENGINEER  HYDRAULICS ENGINEER			
R/W SHEET NO.  ROADWAY DESIGN HYDRAULICS	PROJECT REFERENCE NO	PROJECT REFERENCE NO.	
ROADWAY DESIGN HYDRAULICS	17BP.2.R.85		EC-2A
I I	R/W SHEET N	١٥.	





VIEW FROM SLOPE

#### NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

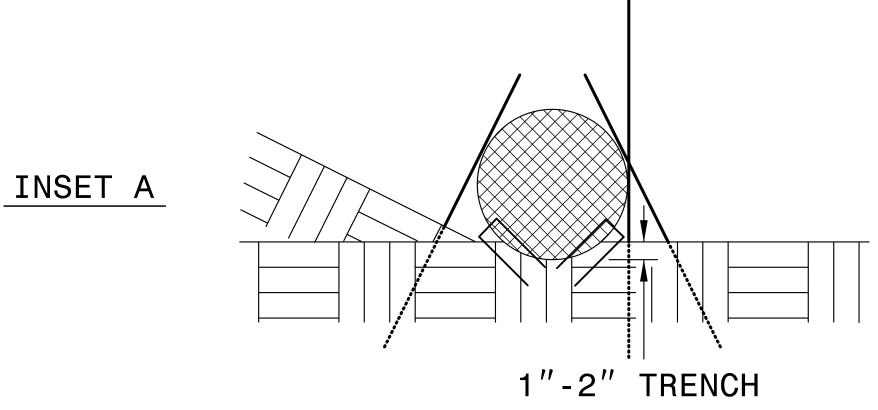
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

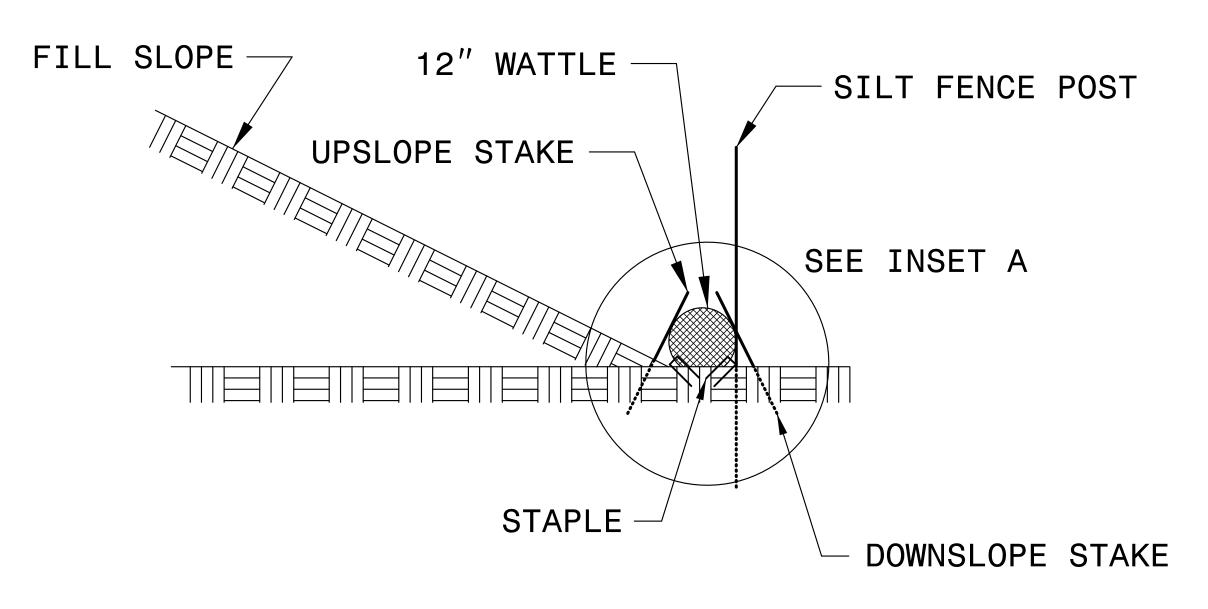
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





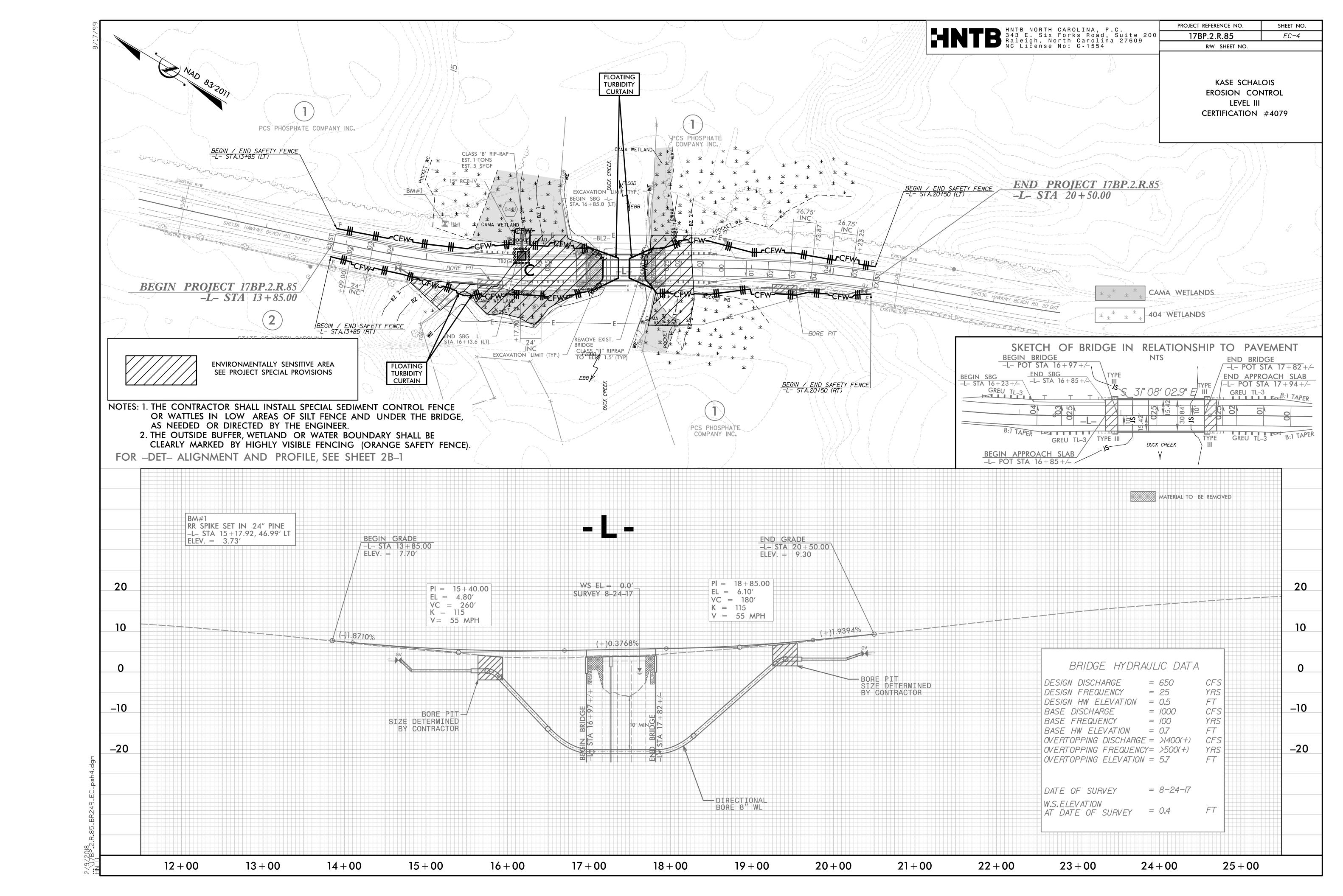
SIDE VIEW

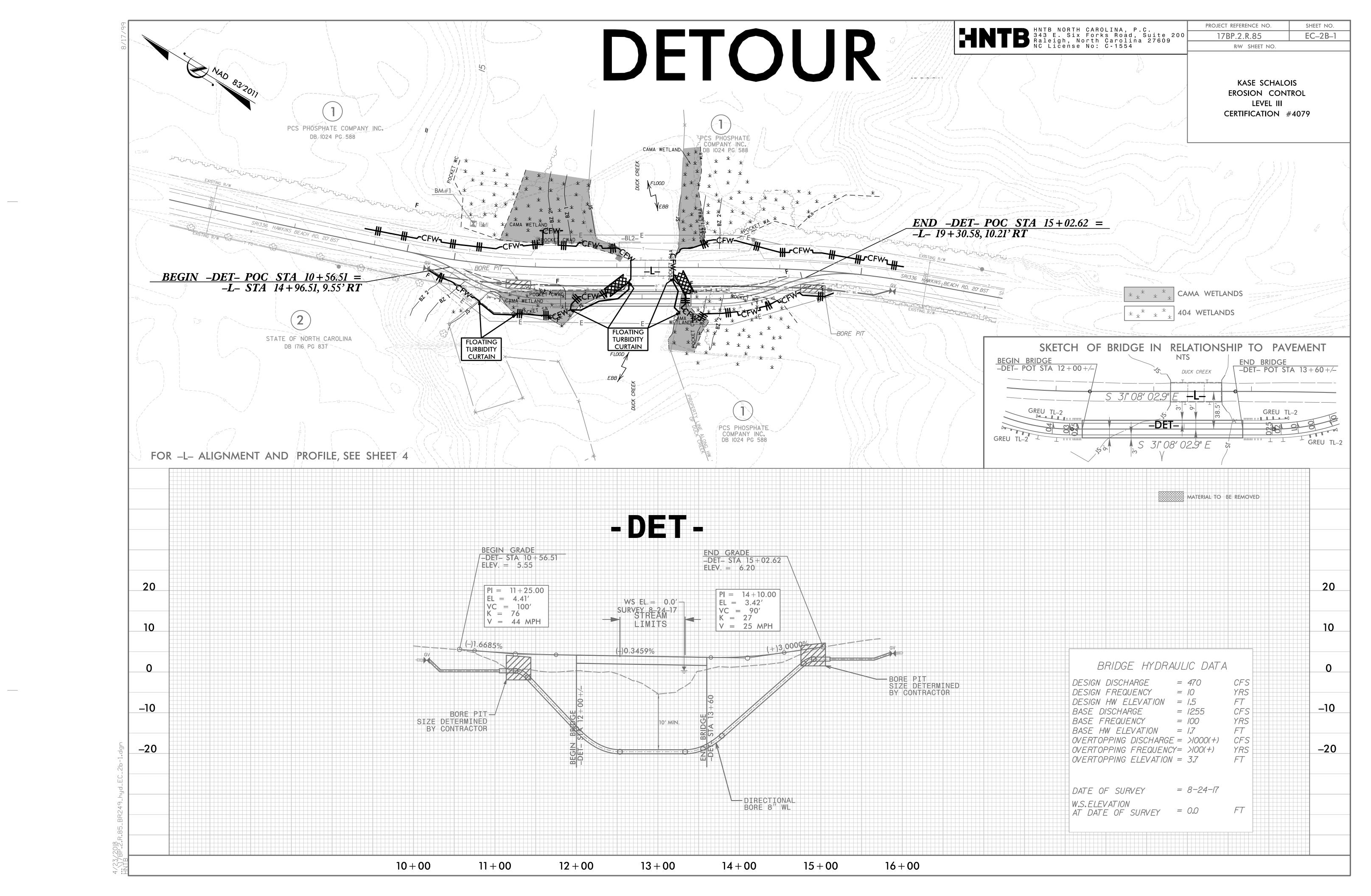
# DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

PROJECT REFERENCE NO.		SHEET NO.
17BP.2.R.85		EC-3
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

# SOIL STABILIZATION TIMEFRAMES

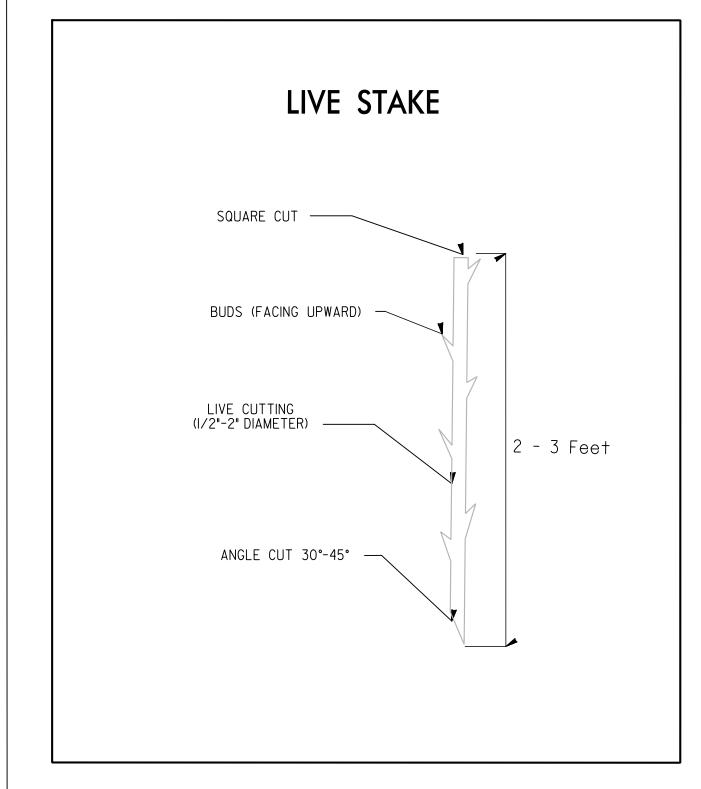
SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS			
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE			
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE			
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.			
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.			
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.			

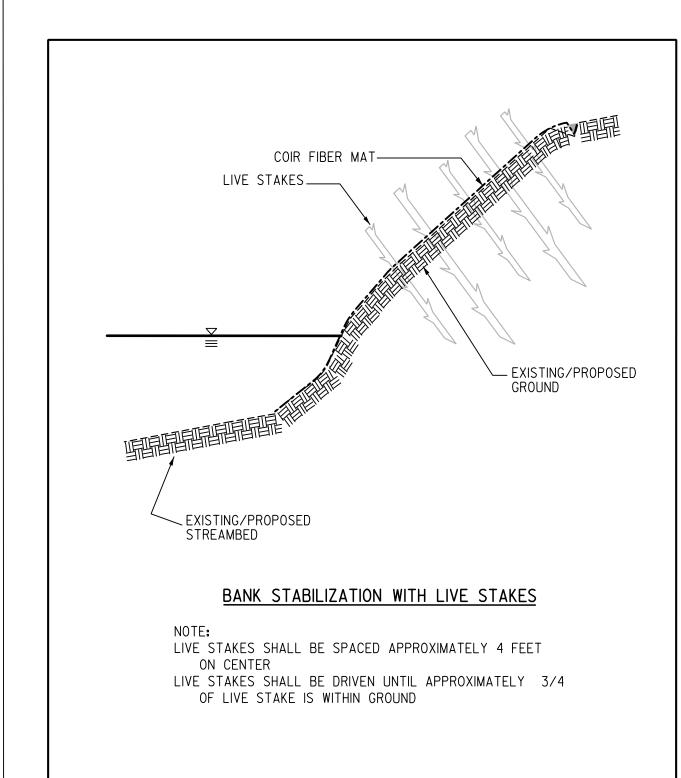




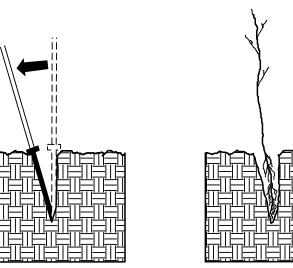
# PLANTING DETAILS

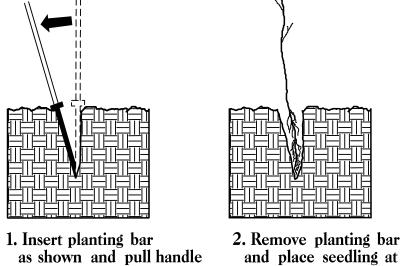
#### LIVE STAKES PLANTING DETAIL

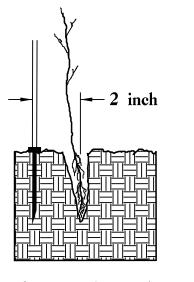




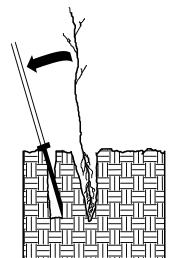
#### BAREROOT PLANTING DETAIL DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR

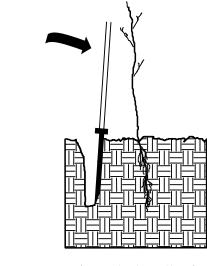


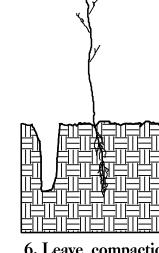




3. Insert planting bar 2 inches toward planter







4. Pull handle of bar toward planter, firming soil at bottom.

5. Push handle forward firming soil at top.

6. Leave compaction hole open. Water

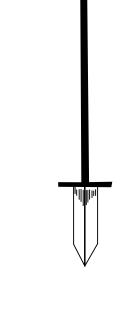
## PLANTING NOTES:

PLANTING BAG During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR Planting bar shall have a blade with a triangular cross section, and shall be 12 inches long, 4 inches wide and I inch thick at center.

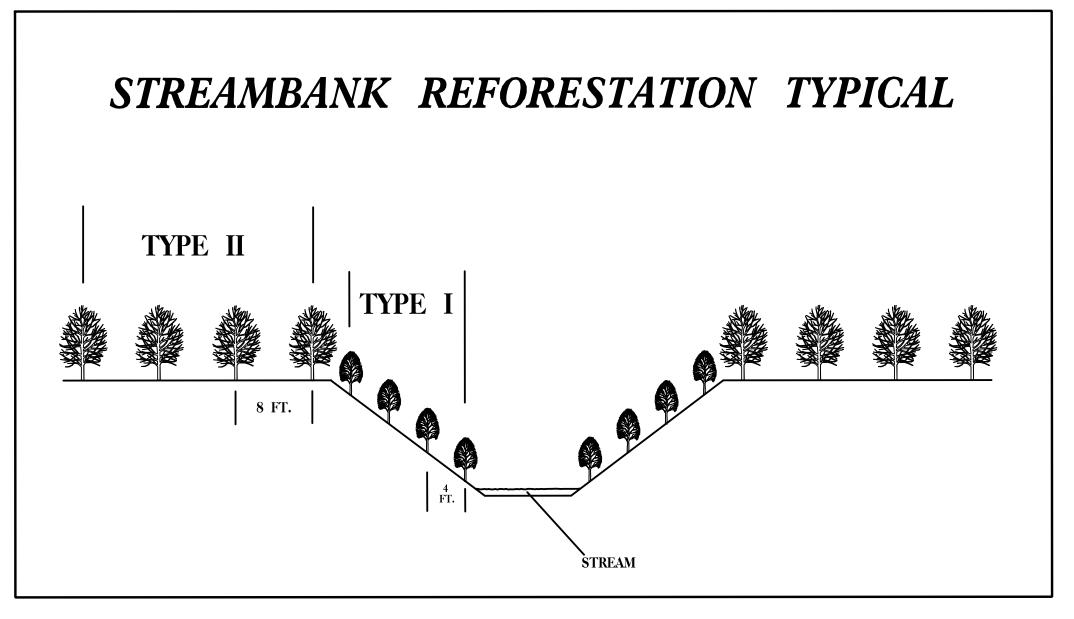
ROOT PRUNING All seedlings shall be root pruned, if necessary, so that no roots extend more than 10 inches below the root collar.



TYPE 1 STREAMBANK REFORESTATION SHALL BE PLANTED 3 FT. TO 5 FT. ON CENTER, RANDOM SPACING, AVERAGING 4 FT. ON CENTER, APPROXIMATELY 2724 PLANTS PER ACRE.

TYPE 2 STREAMBANK REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

NOTE: TYPE 1 AND TYPE 2 STREAMBANK REFORESTATION SHALL BE PAID FOR AS "STREAMBANK REFORESTATION"



STREAMBANK REFORESTATION		
MIXTURE, TYPE, SIZE, AND FURNISH SHALL (	CONFORM TO THE FOLLOWING:	
TYPE 1		
25% SALIX NIGRA	BLACK WILLOW	2 ft - 3 ft LIVE STAKES
25% CORNUS AMOMUM	SILKY DOGWOOD	2 ft - 3 ft LIVE STAKES
25% SPARTINA CYNOSUROIDES	BIG CORDGRASS	12 in – 18 in BR
25% JUNCUS ROEMERIANUS	BLACK NEEDLE RUSH	12 in – 18 in BR
TYPE 2		
25% LIRIODENDRON TULIPIFERA	YELLOW POPLAR	12 in – 18 in BR
25% PLATANUS OCCIDENTALIS	SYCAMORE	12 in – 18 in BR
25% QUERCUS LAURIFOLIA	LAUREL OAK	12 in – 18 in BR
25% BETULA NIGRA	RIVER BIRCH	12 in – 18 in BR

SEE PLAN SHEETS FOR AREAS TO BE PLANTED

# STREAMBANK REFORESTATION DETAIL SHEET 1 OF 2

PROJECT REFERENCE NO. 17BP.2.R.85

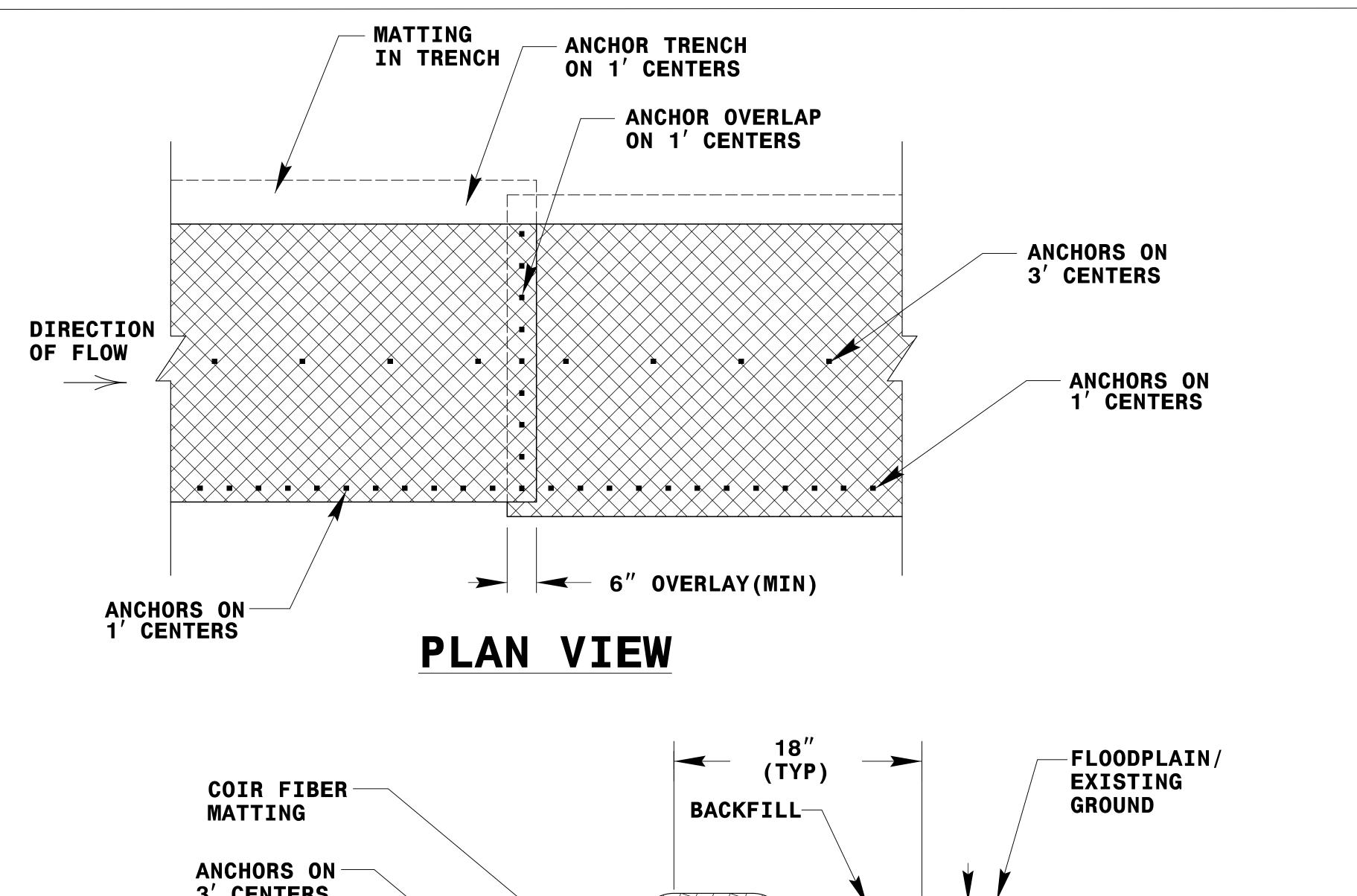
ROADWAY DESIGN ENGINEER

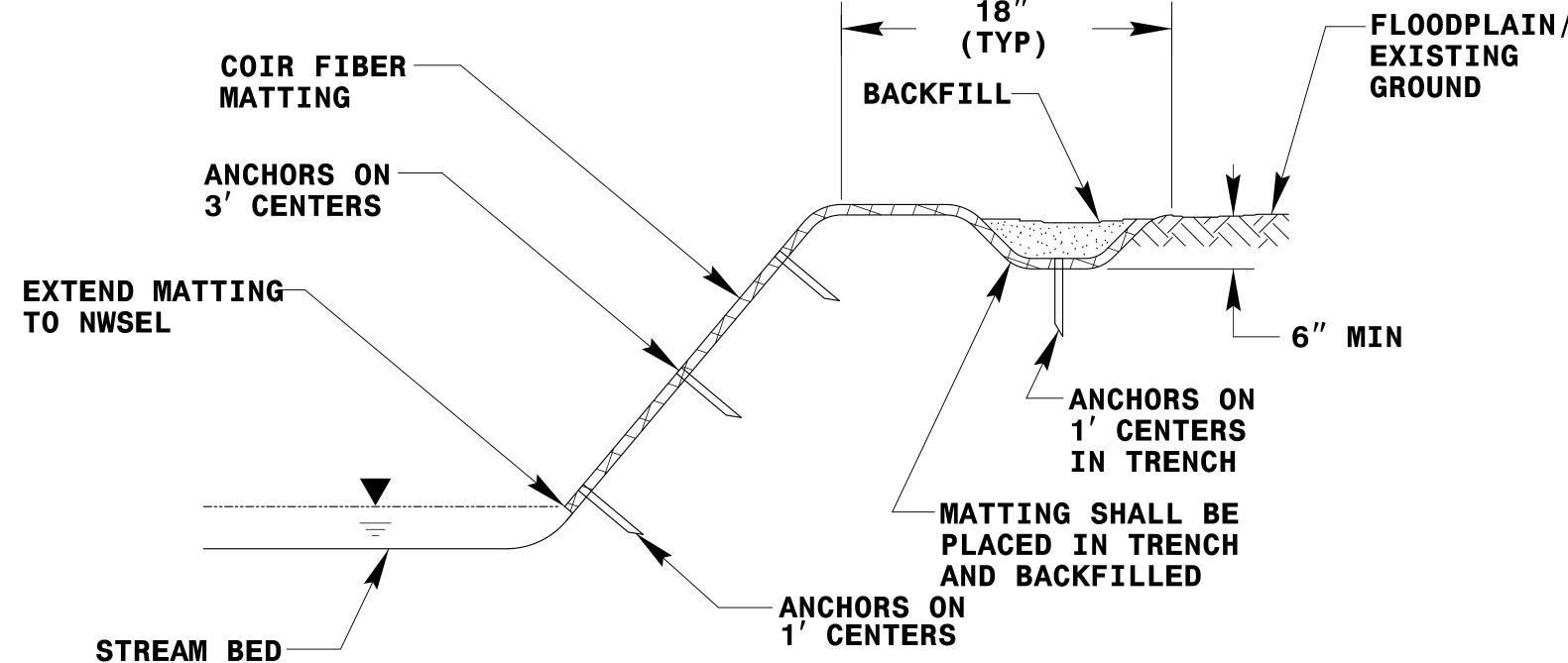
R/W SHEET NO.

RF-I

HYDRAULICS ENGINEER

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

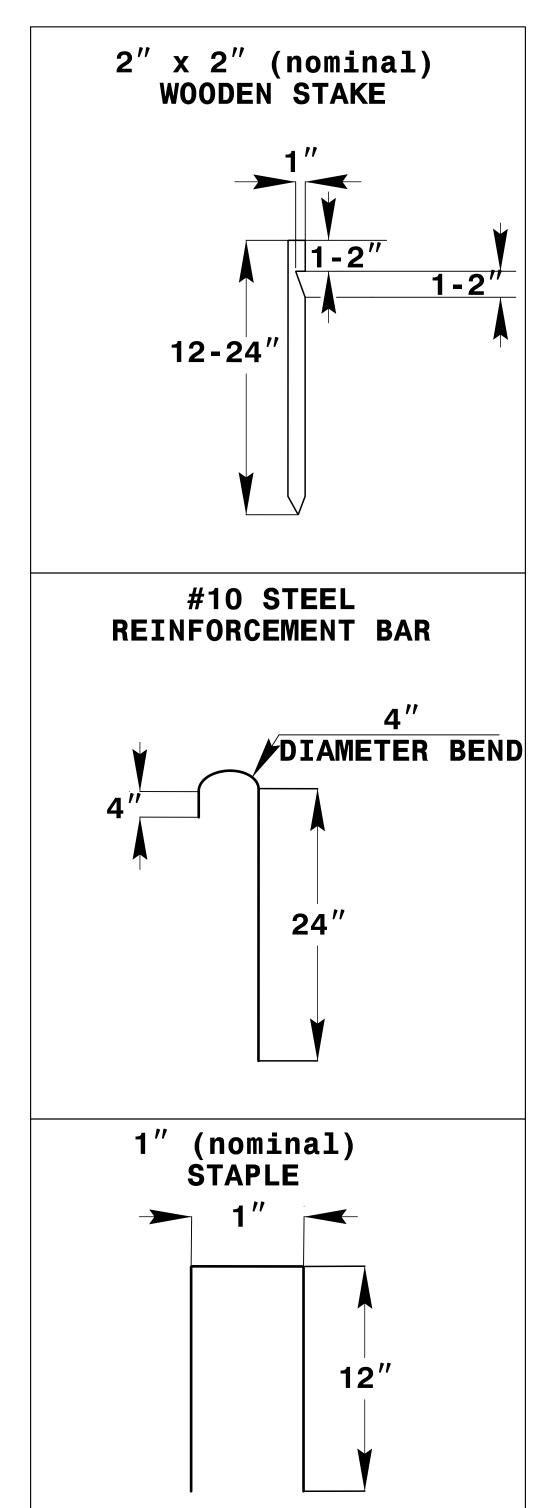




TYPICAL CROSS SECTION

COIR FIBER MATTING DETAIL

NOT TO SCALE



ANCHOR OPTIONS

STREAMBANK REFORESTATION

DETAIL SHEET 2 OF 2

N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT

PROJECT REFERENCE NO. 17BP.2.R.85

ROADWAY DESIGN ENGINEER

R/W SHEET NO.

RF-2

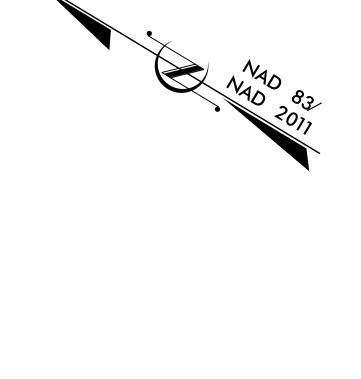
HYDRAULICS ENGINEER STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

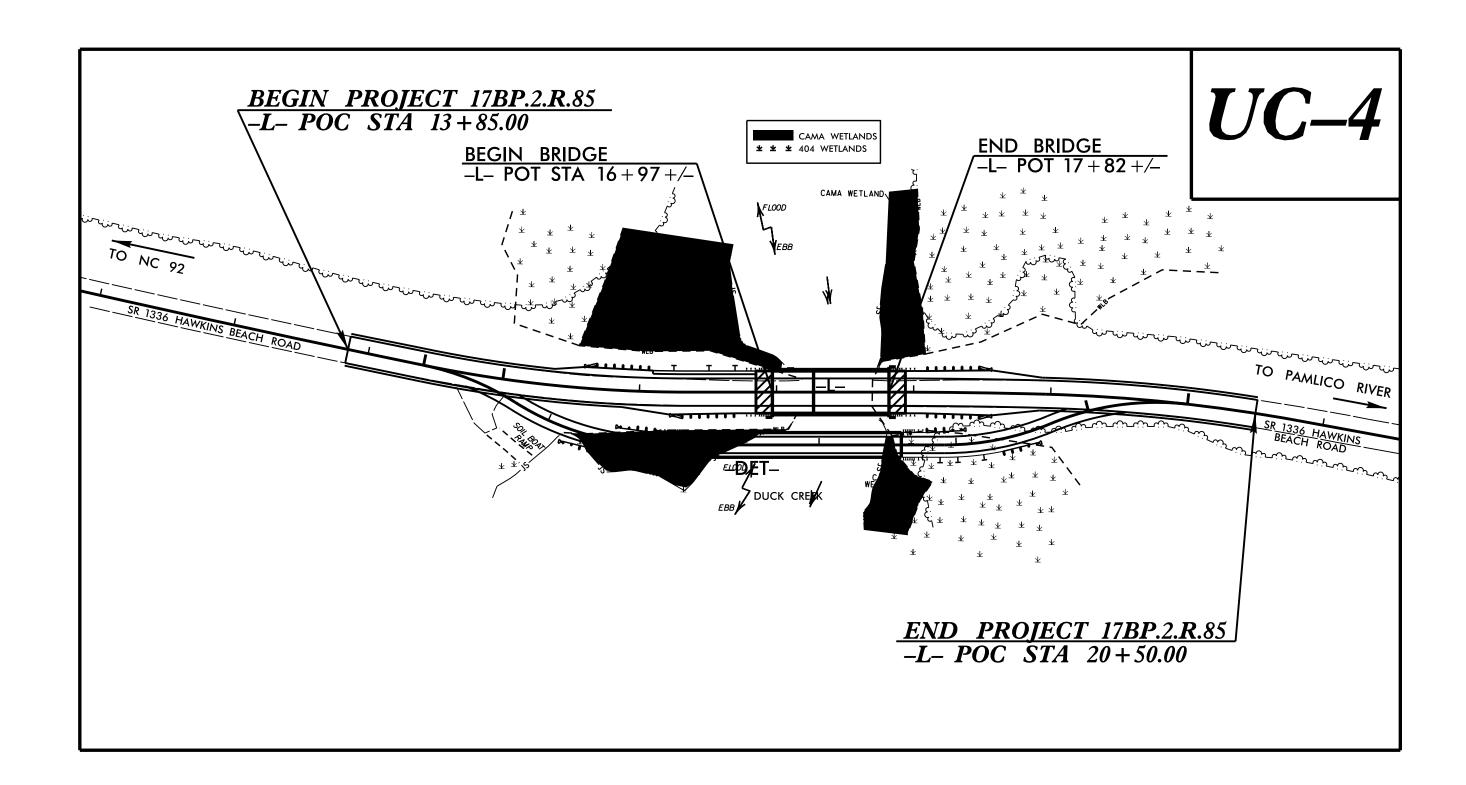
T.I.P. NO.	SHEET	NO.
17BP.2.R.85	UC-	_1

# UTILITY CONSTRUCTION PLANS BEAUFORT COUNTY

LOCATION: BRIDGE 249 OVER DUCK CREEK ON SR 1336 (HAWKINS BEACH ROAD)

TYPE OF WORK: WATER LINE RELOCATION

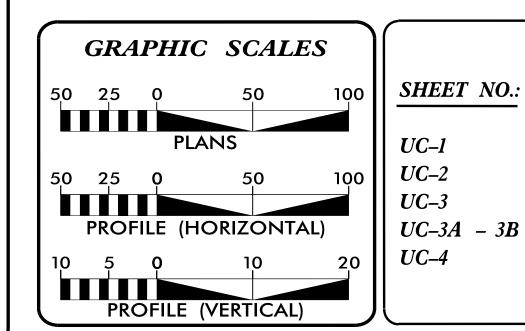




NOTE:

1. THIS PROJECT IS NOT WITHIN ANY MUNICIPAL BOUNDARIES.

DOCUMENT NOT CONSIDERED FINAL UNTIL ALL SIGNATURES ARE COMPLETED



**CREEK** 

VICINITY MAP

# INDEX OF SHEETS

**DESCRIPTION:** 

TITLE SHEET UTILITY SYMBOLOGY **NOTES DETAILS** 

UTILITY PLAN / PROFILE SHEET

WATER AND SEWER OWNERS ON PROJECT

(A) WATER - BEAUFORT COUNTY WATER DEPT

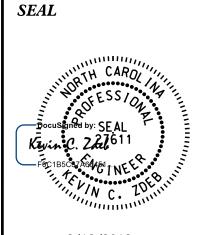
PREPARED IN THE OFFICE OF M A Engineering
Consultants, Inc.

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Phone: 919.297.0220 Fax: 919.297.0221
NC License: F-0160

HNTB NORTH CAROLINA, P.C. 343 E. Six Forks Road, Suite 200 Raleigh, North Carolina 27609 NC License No: C-1554

KEVIN ZDEB, PE PROJECT ENGINEER

SAM FORSTER PROJECT DESIGN ENGINEER





DIVISION OF HIGHWAYS HIGHWAY DIVISION 2

105 PACTOLUS HIGHWAY (NC 33) PO BOX 1587 GREENVILLE NC 27835 PHONE (252) 439–2800 FAX (252) 830–3352

JEFF CABANISS, PE

DIVISION PROJECT DEVELOPMENT ENGINEER

DIVISION BRIDGE PROGRAM MANAGER HEATHER LANE, PE DWAYNE SMITH DIV. UTILITY COORDINATOR

\_\_\_ UTILITY COORDINATOR

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# UTILITIES PLAN SHEET SYMBOLS

# PROPOSED WATER SYMBOLS

# Water Line (Sized as Shown) 11½ Degree Bend 22½ Degree Bend 45 Degree Bend 90 Degree Bend Plug Tee · Cross. Reducer Gate Valve Butterfly Valve Tapping Valve Line Stop Line Stop with Bypass Blow Off Fire Hydrant ··· Relocate Fire Hydrant REM FH Remove Fire Hydrant Water Meter Relocate Water Meter Remove Water Meter Water Pump Station RPZ Backflow Preventer DCV Backflow Preventer Relocate RPZ Backflow Preventer Relocate DCV Backflow Preventer PROPOSED SEWER SYMBOLS

# PROPOSED MISCELLANOUS UTILITIES SYMBOLS

Power Pole	Thrust Block ·····
Telephone Pole ····································	Air Release Valve ····································
Joint Use Pole	Utility Vault
Telephone Pedestal ····································	Concrete Pier
Utility Line by Others(Type as Shown)	Steel Pier
Trenchless Installation	Plan Note
Encasement by Open Cut	Pay Item Note
Encasement ·····	PAY ITEM

# EXISTING UTILITIES SYMBOLS

Power Pole ······ •	*Underground Power Line ····································
Telephone Pole ····································	*Underground Telephone Cable ····································
Joint Use Pole ────────────────────────────────────	*Underground Telephone Conduit····································
Utility Pole	*Underground Fiber Optics Telephone Cable ———— T FO
Utility Pole with Base	*Underground TV Cable
H-Frame Pole ····································	*Underground Fiber Optics TV Cable ················—————————————————————
Power Transmission Line Tower ───────────────────────────────	*Underground Gas Pipeline ····································
Water Manhole	Aboveground Gas Pipeline ——————————————————————————————
Power Manhole ····· ®	*Underground Water Line ····································
Telephone Manhole ®	Aboveground Water Line————————————————————————————————
Sanitary Sewer Manhole	*Underground Gravity Sanitary Sewer Line ————ss———
Hand Hole for Cable ™	Aboveground Gravity Sanitary Sewer Line A/G Sanitary Sewer
Power Transformer	*Underground SS Forced Main Line···················
Telephone Pedestal	Underground Unknown Utility Line—ขน
CATV Pedestal	SUE Test Hole ·································
Gas Valve ····································	Water Meter 🗢
Gas Meter 💠	Water Valve ⊗
Located Miscellaneous Utility Object o	Fire Hydrant ····································
Abandoned According to Utility Records AATUR	Sanitary Sewer Cleanout ⊕
End of Information E.O.I.	

	*For	Existing	Utilities
--	------	----------	-----------

Gravity Sewer Line

Force Main Sewer Line ....

(Sized as Shown)

(Sized as Shown)

(Sized per Note)

Sewer Pump Station

Manhole

# UTILITY CONSTRUCTION

## **GENERAL NOTES:**

- 1. THE PROPOSED UTILITY CONSTRUCTION SHALL MEET THE APPLICABLE REQUIREMENTS OF THE NC DEPARTMENT OF TRANSPORTATION'S "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" DATED JANUARY 2018.
- 2. THE EXISTING WATER LINE UTILITIES BELONG TO BEAUFORT COUNTY.

CONTACT: ERICK JENNINGS PHONE: 252-975-0720

- 3. ALL WATER LINES TO BE INSTALLED WITHIN COMPLIANCE OF THE RULES AND REGULATIONS OF THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENTAL AND NATURAL RESOURCES, DIVISION OF ENVIRONMENTAL HEALTH.
- 4. THE UTILITY OWNER OWNS THE EXISTING UTILITY FACILITIES AND WILL OWN THE NEW UTILITY FACILITIES AFTER ACCEPTANCE BY THE DEPARTMENT. THE DEPARTMENT OWNS THE CONSTRUCTION CONTRACT AND HAS ADMINISTRATIVE AUTHORITY. COMMUNICATIONS AND DECISIONS BETWEEN THE CONTRACTOR AND UTILITY OWNER ARE NOT BINDING UPON THE DEPARTMENT OR THIS CONTRACT UNLESS AUTHORIZED BY THE ENGINEER. AGREEMENTS BETWEEN THE UTILITY OWNER AND CONTRACTOR FOR THE WORK THAT IS NOT PART OF THIS CONTRACT OR IS SECONDARY TO THIS CONTRACT ARE ALLOWED, BUT ARE NOT BINDING UPON THE DEPARTMENT.
- 5. PROVIDE ACCESS FOR THE DEPARTMENT PERSONNEL AND THE OWNER'S REPRESENTATIVES TO ALL PHASES OF CONSTRUCTION. NOTIFY DEPARTMENT PERSONNEL AND THE UTILITY OWNER TWO WEEKS PRIOR TO COMMENCEMENT OF ANY WORK AND ONE WEEK PRIOR TO SERVICE INTERRUPTION. KEEP UTILITY OWNERS' REPRESENTATIVES INFORMED OF WORK PROGRESS AND PROVIDE OPPROTUNITY FOR INSPECTION OF CONSTRUCTION AND TESTING.

- 6. THE PLANS DEPICT THE BEST AVAILABLE INFORMATION FOR THE LOCATION, SIZE, AND TYPE OF MATERIAL FOR ALL EXISTING UTILITIES. MAKE INVESTIGATIONS FOR DETERMINING THE EXACT LOCATION, SIZE, AND TYPE MATERIAL OF THE EXISTING FACILITIES AS NECESSARY FOR THE CONSTRUCTION OF THE PROPOSED UTILITIES AND FOR AVOIDING DAMAGE TO EXISTING FACILITIES. REPAIR ANY DAMAGE INCURRED TO EXISTING FACILITIES TO THE ORIGINAL OR BETTER CONDITION AT NO ADDITONAL COST TO THE DEPARTMENT.
- 7. MAKE FINAL CONNECTIONS OF THE NEW WORK TO THE EXISTING SYSTEM WHERE INDICATED ON THE PLANS, AS REQUIRED TO FIT THE ACTUAL CONDITIONS, OR AS DIRECTED.
- 8. MAKE CONNECTIONS BETWEEN EXISTING AND PROPOSED UTILITIES AT TIMES MOST CONVENIENT TO THE PUBLIC, WITHOUT ENDANGERING THE UTILITY SERVICE, AND IN ACCORDANCE WITH THE UTILITY OWNER'S REQUIREMENTS. MAKE CONNECTIONS ON WEEKENDS, AT NIGHT, AND ON HOLIDAYS IF NECESSARY.
- 9. ALL UTILITY MATERIALS SHALL BE APPROVED PRIOR TO DELIVERY TO THE PROJECT. SEE 1500-7, "SUBMITTALS AND RECORDS" IN SECTION 1500 OF THE STANDARD SPECIFICATIONS.
- 10. CONTRACTOR SHALL NOT OPERATE ANY VALVES ON THE EXISTING UTILITY SYSTEMS. CONTRACTOR SHALL CONTACT THE UTILITY OWNER TO CONDUCT STRATEGIC OPERATION OF VALVES FOR SERVICE INTERRUPTION IN ORDER TO PERFORM SPECIFIC WORK.

# PROJECT SPECIFIC NOTES:

- 1. PROPOSED OPEN TRENCH WATER LINE SHALL BE 6" DUCTILE IRON PIPE, CLASS 350, WITH GRIP RINGS.
- 2. PROPOSED WATER LINE FOR DIRECTIONAL DRILLING SHALL BE 200 PSI PRESSURE PIPE D.I.P.S. 8" HDPE SDR-9 WITH MATERIAL DESIGNATION PE 3408 / 3608 THAT CONFORMS TO NSF-61.
- 3. ALL WATER LINE FITTINGS, 4-INCHES THROUGH 12-INCHES IN DIAMETER, SHALL BE DUCTILE IRON.
- 4. CONTRACTOR'S ATTENTION IS DIRECTED TO SECTIONS 102, 107, AND 1550 OF THE STANDARD SPECIFICATIONS CONCERNING TRENCHLESS INSTALLATION. IT IS CONTRACTOR'S RESPONSIBILITY TO HAVE BORE DESIGNED AND SEALED BY A LICENSED NORTH CAROLINA PROFESSIONAL ENGINEER. NO DAMAGE IS ALLOWED TO RIVER, STREAM, CREEK, WETLANDS, OR BUFFER ZONES.
- 5. ALL PROPOSED FITTINGS (BENDS, TEES, CROSSES, REDUCERS, PLUGS, ETC.) SHALL BE ADEQUATELY RESTRAINED BY THE USE OF RESTRAINED JOINT CONSTRUCTION AND/OR CAST IN PLACE CONCRETE THRUST RESTRAINTS AS DETAILED ON THESE DRAWINGS, OR AS DIRECTED BY THE RESIDENT ENGINEER.

# PROJECT REFERENCE NO. SHEET NO. 17BP.2.R.85 DESIGNED BY: SHF DRAWN BY: SHF CHECKED BY: KCZ APPROVED BY: KCZ REVISED: NORTH CAROLINA DEPARTMENT OF TRANSPORTATION UTILITIES ENGINEERING SEC. PHONE: (919)707-6690 FAX: (919)250-4151 SHEET NO. UC-3 DOCUMENT OF CAROLINA DEPARTMENT OF TRANSPORTATION UTILITY CONSTRUCTION PLANS ONLY

#### | UTILITY CONSTRUCTION

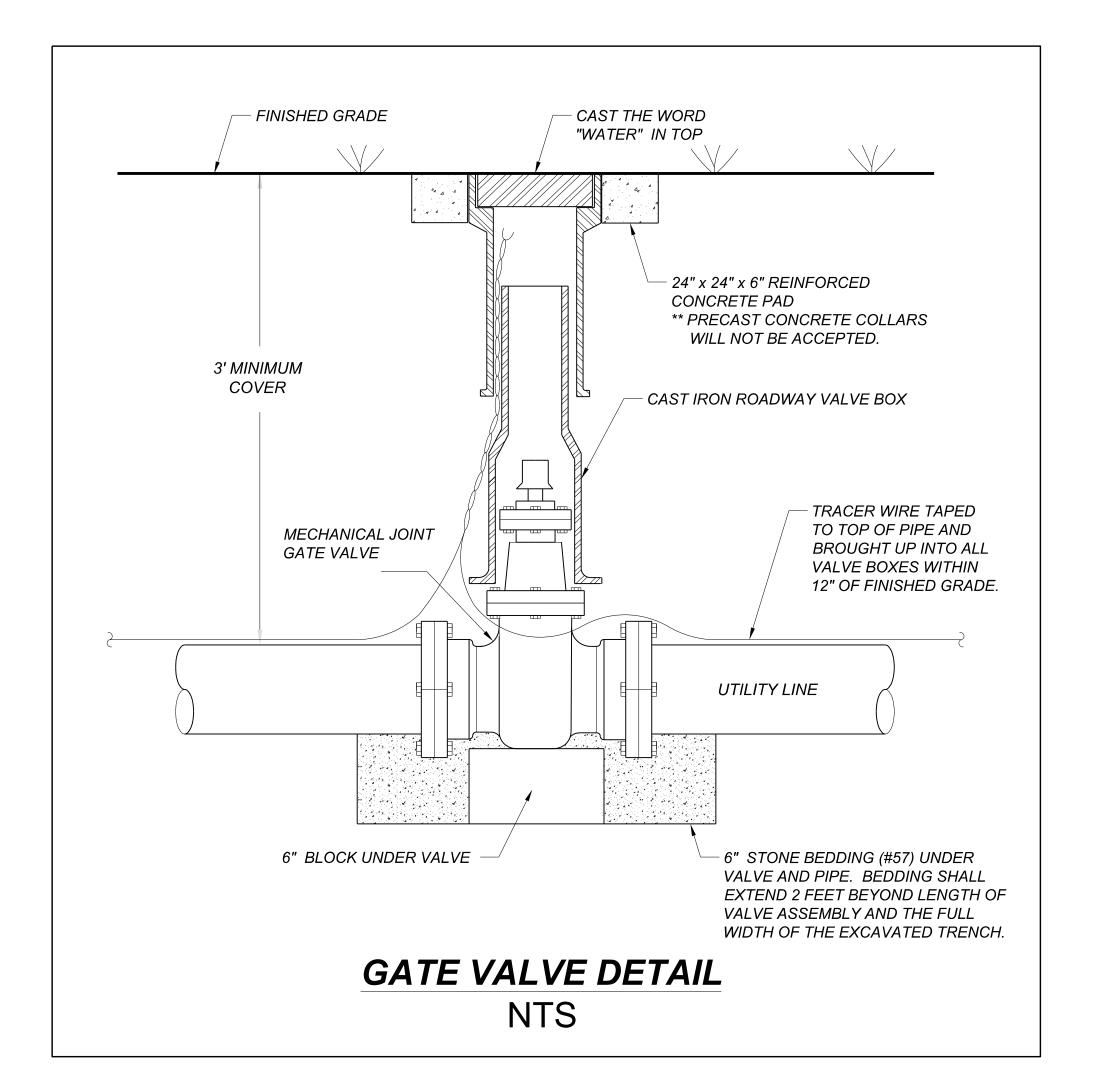
M A Engineering
Consultants, Inc.

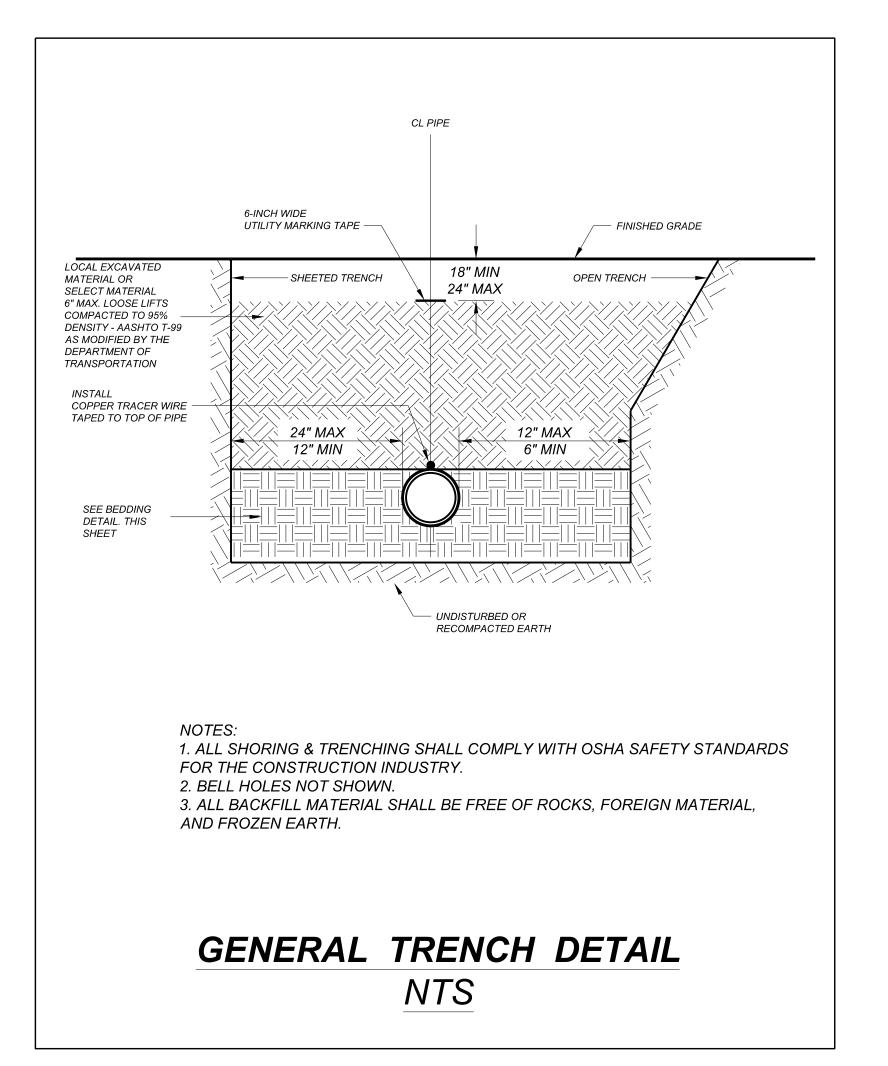
See East Chatham Street - Suite 137
Cary, NC 27511
Phone: 919.297.0220 Fax: 919.297.022
NC License: F-0160

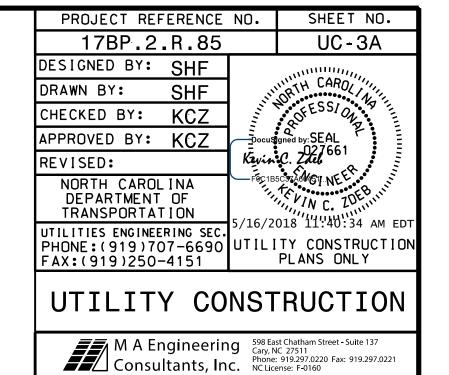
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# PROJECT QUANTITIES

ITEM NUMBER	DESCRIPTION	QUA	NTITY
5325600000-E	6" WATER LINE	184	LF
5325800000-E	8" WATER LINE	392	LF
5329000000-E DUCTILE IRON WATER PIPE FITTINGS			POUNDS
5540000000-E	6" VALVE	2	EA
5800000000-E	ABANDON 6" UTILITY PIPE	584	LF
5871500000-E	DIRECTIONAL DRILLING OF 8"	392	LF



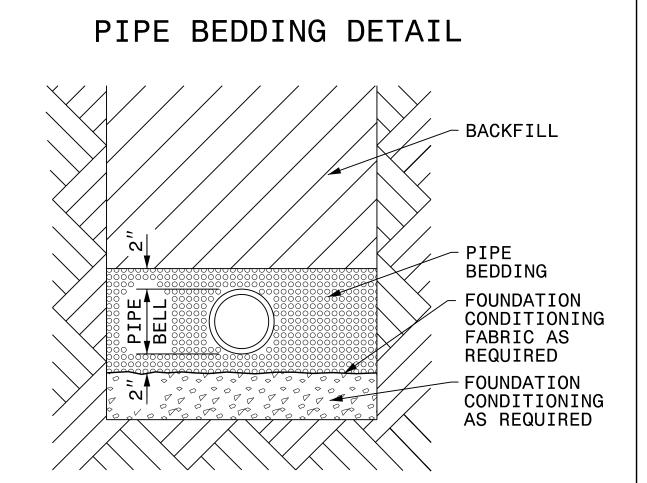




DOCUMENT NOT CONSIDERED FINAL

UNTIL ALL SIGNATURES ARE COMPLETED

MAXIMUM OPEN TRENCH WIDTH AT TOP OF PIPE								
NOMINAL		NOMINAL						
PIPE SIZE	TRENCH WIDTH	PIPE SIZE	TRENCH WIDTH					
(INCHES)	(INCHES)	(INCHES)	(INCHES)					
4	28	20	44					
6	3Ø	24	48					
8	32	3Ø	54					
1Ø	34	36	60					
12	36	42	66					
14	38	48	72					
16	40	54	78					
18	42							



PLACE FOUNDATION CONDITIONING MATERIAL BELOW BEDDING IF REQUIRED, AS DIRECTED BY ENGINEER. PIPE BEDDED IN SELECT MATERIAL, CLASS II (TYPE 1) OR CLASS III. TRENCH BACKFILLED IN LOOSE 6" LAYERS COMPACTED TO TOP OF TRENCH USING LOCAL EXCAVATED MATERIAL IF APPROVED BY THE ENGINEER, OR SELECT MATERIAL. ALL MATERIAL SHALL BE FREE OF ROCKS, FOREIGN MATERIAL, AND FROZEN EARTH. COMPACTION SHALL BE TO APPROXIMATELY 95% DENSITY IN ACCORDANCE WITH AASHTO T-99 AS MODIFIED BY THE DEPARTMENT OF TRANSPORTATION.

2018 18/50010887/2018 | 10887/20867/2087/1088/2087/1088/2084 | 2497/11+11+1088/2084 | 85 11+ 116-234 | 411 | 420

#### BASED ON TEST PRESSURE OF 200 P.S.I. HORIZONTAL RESTRAINT VERTICAL RESTRAINT (ALL AREAS GIVEN ARE IN SQUARE FEET) (ALL VOLUMES GIVEN ARE IN CUBIC YARDS)\*\* ALLOWABLE SOIL BEARING (PSF) PIPE RESTRAINING RODS DEGREE OF BEND SIZE | OF BEND | THRUST \* | 1000 | 2000 | 3000 | 4000 | 5000 | 6000 | 7000 | 8000 | 2 | 1/2" | 0.25 | 0.50 | 0.75 2 1/2" 0.50 1.0 1.75 2 5/8" 0.75 1.50 3.0 2 3/4" 1.25 2.25 4.50 2 7/8" 1.75 3.25 6.50 5/8" 2.25 4.50 8.75 4 3/4" 3.0 6.0 11.50 \*\*INCLUDES I.50 SAFETY FACTOR INCLUDES 1.25 SAFETY FACTOR GENERAL NOTES: I. CONCRETE SHALL BE CLASS "B". 2. CONCRETE SHALL NOT CONTACT BOLTS ENDS OF MECHANICAL JOINT FITTINGS. 3. CONSULT WITH ENGINEER FOR CONCRETE REQUIREMENTS ON MAINS LARGER THAN 16 INCHES. (FOR VERTICAL & HORIZONTAL BENDS) 4. ALLOWABLE SOIL BEARING SHALL BE DETERMINED BY THE ENGINEER. NO. DATE DESCRIPTION SHEET 2 OF 2 THRUST RESTRAINT FOR WATER MAINS

## **DUCTILE IRON PIPE RESTRAINED JOINT DESIGN TABLE**

FITTING	REQUIRED RESTRAINED LENGTH (FT) OF BARE D.I. PIPE BY DEPTH OF COVER							
HORIZONTAL BENDS	3 FT	4 FT	5 FT	6 FT	7 FT	8 FT	9 FT	10 FT
6 INCH DIA - 11.25 DEG	3	2	2	2	2	1	1	1
6 INCH DIA - 22.5 DEG	5	4	4	3	3	3	3	2
6 INCH DIA - 45 DEG	11	9	8	7	7	6	5	5
6 INCH DIA - 90 DEG	26	22	19	17	16	14	13	12
VERTICAL DOWN BENDS	3 FT	4 FT	5 FT	6 FT	7 FT	8 FT	9 FT	10 FT
6 INCH DIA - 11.25 DEG	7	6	6	5	4	4	4	3
6 INCH DIA - 22.5 DEG	15	13	11	10	9	8	8	7
6 INCH DIA - 45 DEG	31	27	23	21	19	17	16	15
VERTICAL UP BENDS	3 FT	4 FT	5 FT	6 FT	7 FT	8 FT	9 FT	10 FT
6 INCH DIA - 11.25 DEG	3	2	2	2	2	1	1	1
6 INCH DIA - 22.5 DEG	5	4	4	3	3	3	3	2
6 INCH DIA - 45 DEG	11	9	8	7	7	6	5	5

**ASSUMPTIONS** 

LAYING CONDITION = TYPE 4 DESIGN PRESSURE = 200 PSI (TEST PRESSURE)

SOIL DESIGNATION = GC = COHESIVE-GRANULAR SAFETY FACTOR = 1.5

#### **NOTES**

- 1. RESTRAINED LENGTH IS MEASURED FROM THE CENTER OF THE BEND AS FOLLOWS:
- A. HORIZONTAL AND VERTICAL BENDS: ALONG EACH SIDE OF BEND.
- B. HORIZONTAL AND VERTICAL BENDS OFFSET OR COMBINED: ALONG THE OUTER SIDE OF EACH BEND.
  ALL PIPE BETWEEN THE TWO BENDS SHALL BE RESTRAINED JOINT WHEN THE DISTANCE BETWEEN THEM IS
  EQUAL TO OR LESS THAN THE REQUIRED RESTRAINED LENGTH. WHEN THE DISTANCE BETWEEN BENDS IS
  LESS THAN REQUIRED, THE BALANCE OF THE REQUIRED RESTRAINED LENGTH SHALL BE ADDED ON TO THE
  LENGTH ALONG THE OUTSIDE OF EACH BEND RESPECTIVELY TO MAKE UP FOR THE DEFICIENCY IN THAT DIRECTION.
  HORIZONTAL BEND EXAMPLE...

INSTALL A 8 INCH 45 DEG BEND AND A 22.5 DEG BEND WITH 10 FEET BETWEEN BENDS AND 4 FEET OF COVER. THE CONTRACTOR SHALL PROVIDE AN ADDITIONAL 1 FOOT OF RESTRAINED LENGTH BEYOND THE 45 DEGREE BEND (FOR A TOTAL OF 13 FEET) AND AN ADDITIONAL 7 FEET OF RESTRAINED LENGTH BEYOND THE 22.5 DEGREE BEND (FOR A TOTAL OF 13 FEET).

2. WHEN IT IS NOT POSSIBLE TO INSTALL THE RESTRAINED LENGTHS AS NOTED BY THIS TABLE, THE CONTRACTOR SHALL INSTALL THE APPROPRIATE CONCRETE THRUST RESTRAINTS AS PER THE DETAILS HEREIN.

PROJECT REFERENCE NO. SHEET NO.

17BP.2.R.85

DESIGNED BY: SHF

DRAWN BY: SHF

CHECKED BY: KCZ

APPROVED BY: KCZ

REVISED:

NORTH CAROLINA
DEPARTMENT OF
TRANSPORTATION

UTILITIES ENGINEERING SEC.
PHONE: (919)707-6690
FAX: (919)250-4151

SHEET NO.

UC-3B

UC-3B

Designed by: SEAL

Kain C. 2027661

FEC: 185 C. 7AM CINE

FOC: 185 C. 7AM CINE

5/16/2018 11:41:15 AM EDT

UTILITY CONSTRUCTION
PLANS ONLY

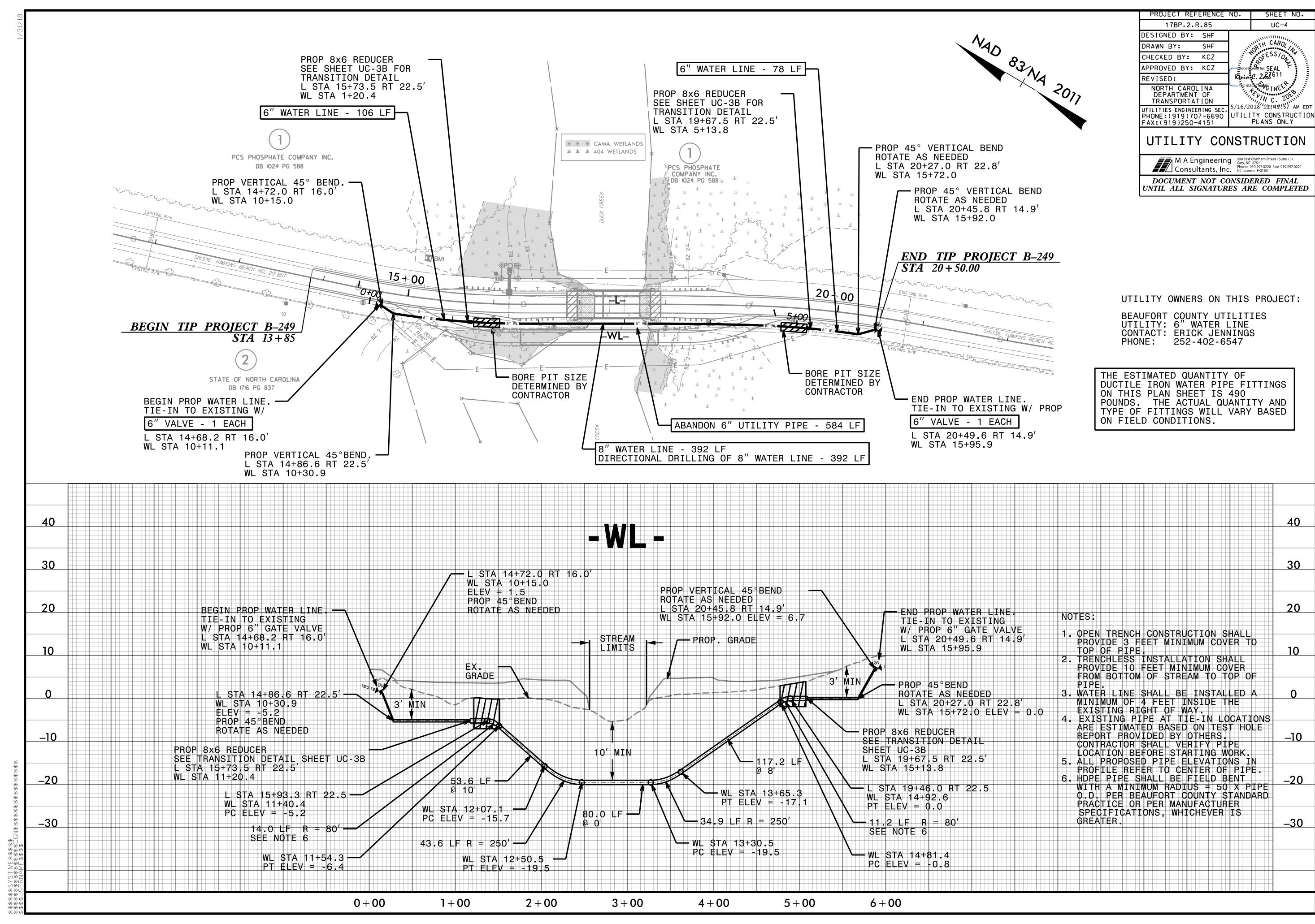
#### UTILITY CONSTRUCTION

M A Engineering Consultants, Inc.

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NC License: F-0160

DOCUMENT NOT CONSIDERED FINAL UNTIL ALL SIGNATURES ARE COMPLETED

MECHANICAL JOINT - 8" X 6" MJDI REDUCER (COMPACT C-153 CLASS 350) CONTRACTOR TO PROVIDE A MINIMUM OF PVC PIPE 20' HDPE AT 0% GRADE FOR PROPER ALIGNMENT TO CONNECT TO DIP. D.I. FITTING -MECHANICAL JOINT 3' MIN. COVER WITH GRIP RING OVER TOP OF PIPE RESTRAINT (TYPICAL) 6" DUCTILE IRON PIPE 3 FULL 18' JOINTS LAID HORIZONTAL 8" HDPE DR-9 CONTRACTOR TO FUSE ON MJ ADAPTER — NO DEFLECTION IN THESE JOINTS WITH STIFFENING INSERT BACKFILL DIP WITH SELECT OFFSITE GRANULAR MATERIAL OR CRUSHER RUN FOR FULL PIPE LENGTH FROM 6" BELOW PIPE INVERT TO TOP OF PIPE 8" HDPE X 6" DIP TRANSITION DETAIL NOT TO SCALE



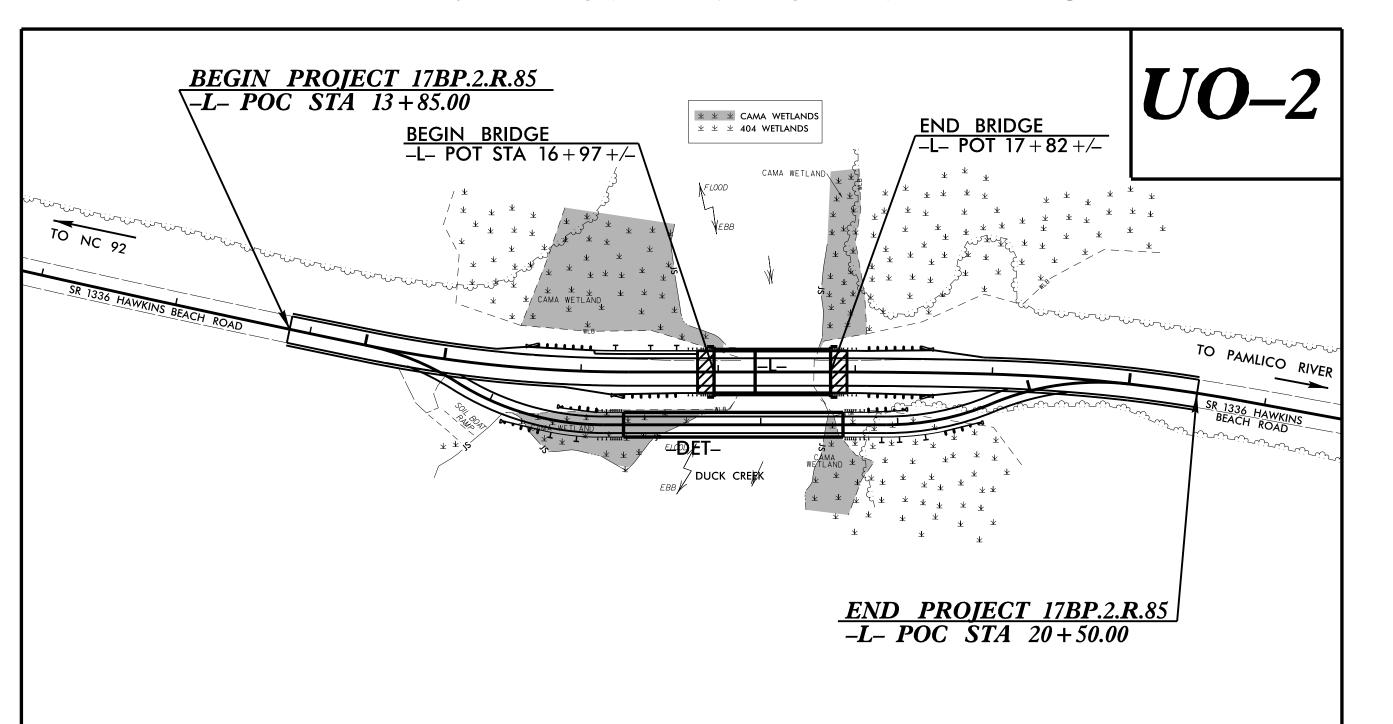
GOOSE CREEK VICINITY MAP

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# UTILITIES BY OTHERS PLANS BEAUFORT COUNTY

LOCATION: REPLACE BRIDGE NO 249 OVER DUCK CREEK ON SR 1336 (HAWKINS BEACH RD)

TYPE OF WORK: RELOCATE COMMUNICATION LINES



T.I.P. NO.

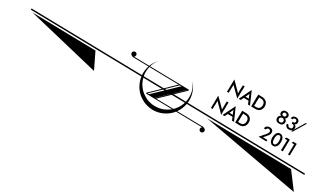
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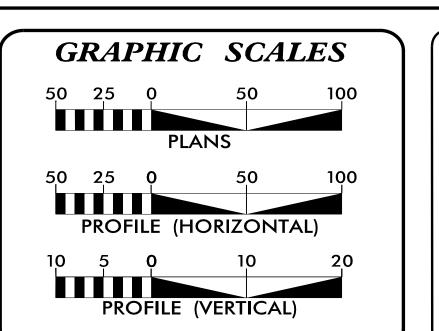
UO-1

SHEET NO.

NOTE:

ALL UTILITY WORK SHOWN ON THIS SHEET WILL BE DONE BY OTHERS. NO PAYMENT WILL BE MADE TO THE CONTRACTOR FOR UTILITY WORK SHOWN ON THIS SHEET.





# INDEX OF SHEETS

SHEET NO.: **DESCRIPTION:** TITLE SHEET **UO**–2 UBO PLAN SHEET

#### UTILITY OWNERS WITH CONFLICTS

(A) TELEPHONE – TRI COUNTY BROADBAND (B) TELEPHONE – CENTURYLINK

PREPARED IN THE OFFICE OF:



M A Engineering
Consultants, Inc.

598 East Chatham Street - Suite 137
Cary, NC 27511
Phone: 919.297.0220 Fax: 919.297.0221
NC License: F-0160

WEBB WHITE UTILITY PROJECT MANAGER DWAYNE SMITH PROJECT UTILITY COORDINATOR



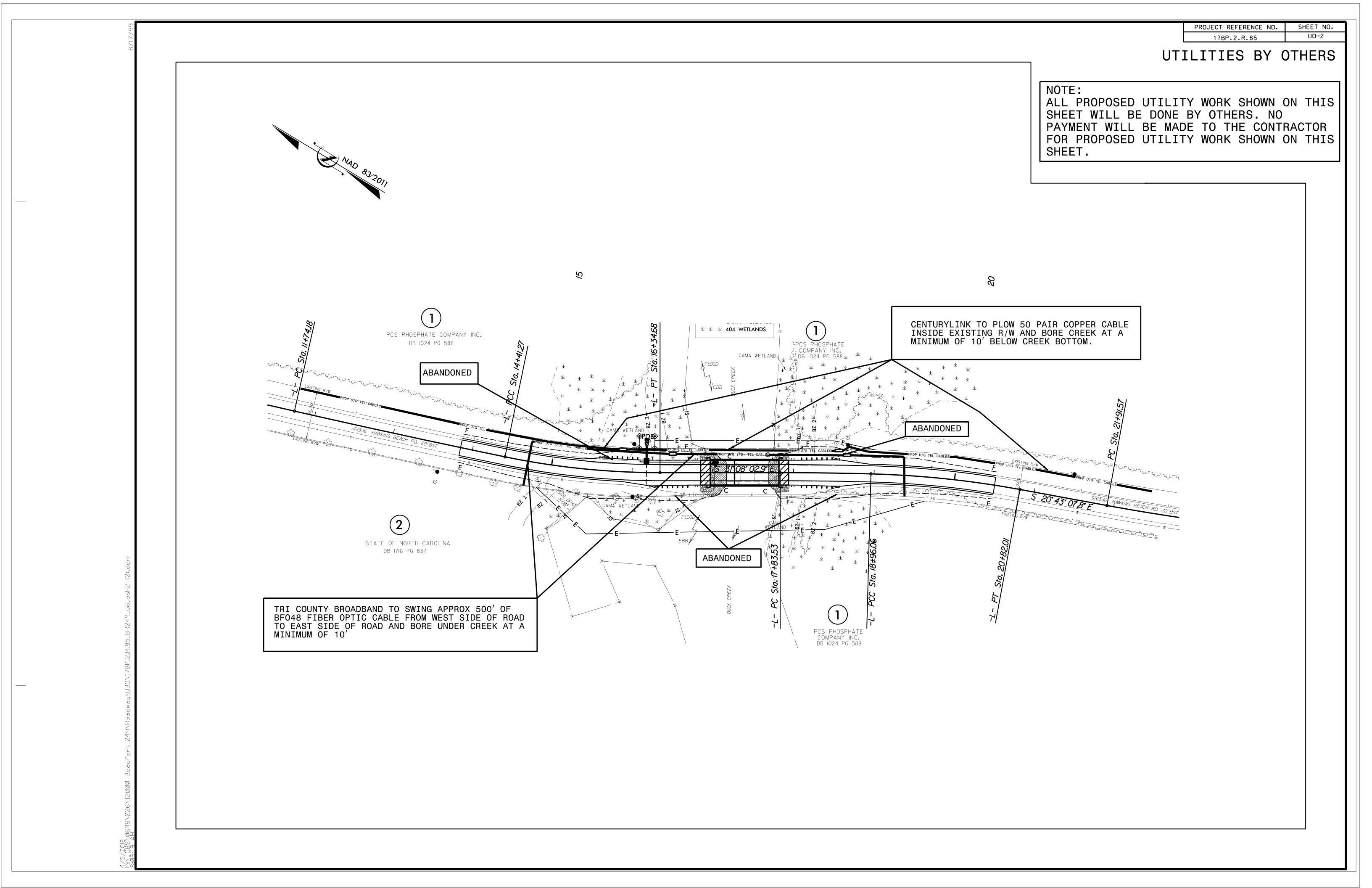
**DIVISION OF HIGHWAYS DIVISION** 2

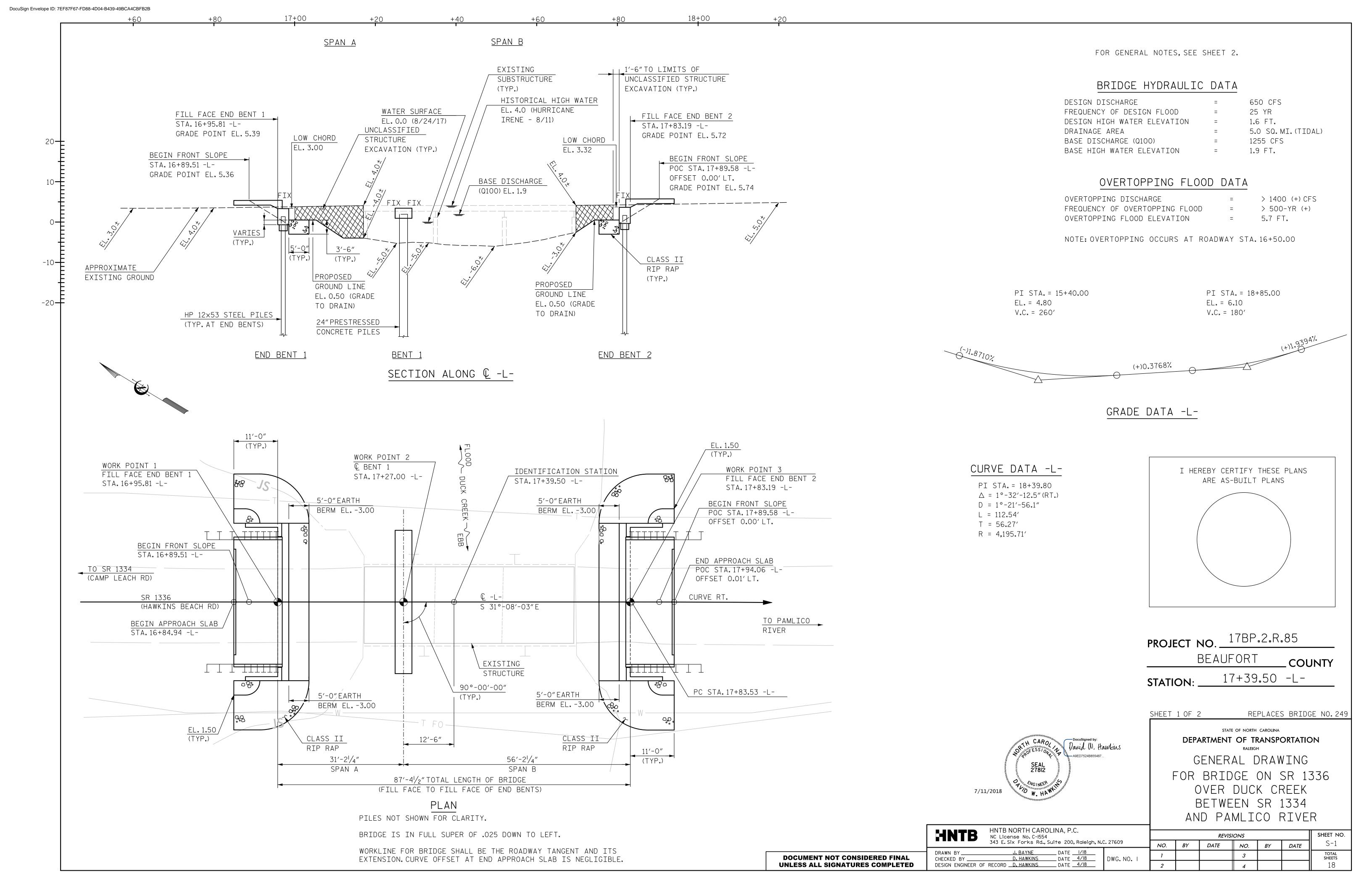
DIV ADDRESS 1037 W.H. SMITH BLVD PO BOX 1587 GREENVILLE NC 27835

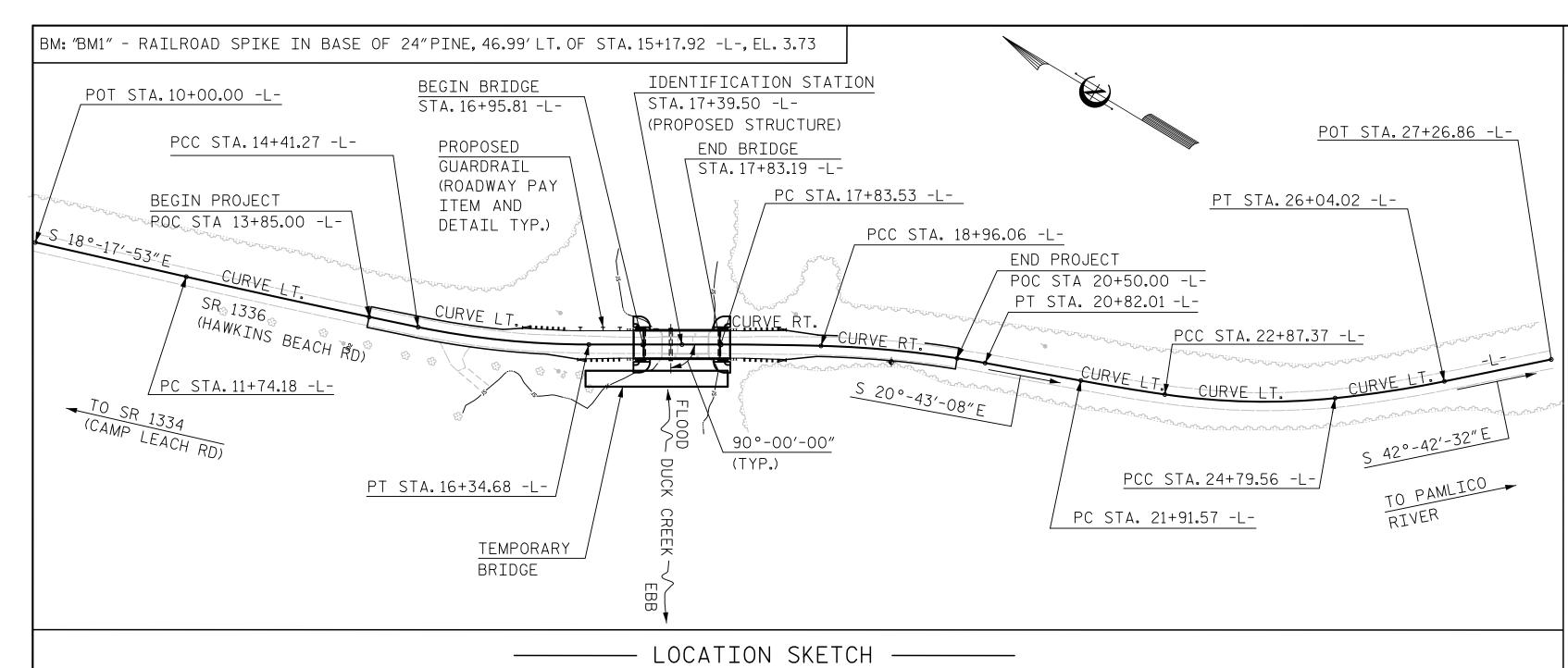
HEATHER LANE, P.E.

**DIVISION** 2

PROJECT DEVELOPMENT UNIT DIVISION BRIDGE PROGRAM MANAGER







FOUNDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO.1 AND END BENT NO.2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 51 TONS PER PILE AND 71 TONS PER PILE. RESPECTIVELY.

DRIVE PILES AT END BENT NO.1 AND END BENT NO.2 TO A REQUIRED DRIVING RESISTANCE OF 85 TONS PER PILE AND 120 TONS PER PILE, RESPECTIVELY.

PILES AT BENT NO.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 100 TONS PER PILE.

DRIVE PILES AT BENT NO.1 TO A REQUIRED DRIVING RESISTANCE OF 185 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG OR SCOUR.

INSTALL PILES AT BENT NO.1 TO A TIP ELEVATION NO HIGHER THAN -40.0 FT.

THE SCOUR CRITICAL ELEVATION FOR BENT NO.1 IS ELEVATION -12.0 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT

BENT NO.1. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS.

								TOTA	L BILL OF	MATERIAL	-							
	CONSTRUCTION, MAINTENANCE, & REMOVAL OF TEMP. STRUCTURE AT STATION 17+39.50	REMOVAL OF EXISTING STRUCTURE AT STATION	ASSESSMENT	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 17+39.50 -L-	CLASS AA CONCRETE	BRIDGE APPROACH SLABS AT STATION 17+39.50 -L-	EPOXY COATED REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR 24" PRESTRESSED CONCRETE PILES	PILE DRIVING EQUIPMENT SETUP FOR HP 12×53 STEEL PILES	24" PRESTRESSED CONCRETE PILES	HP 12x53 STEEL PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-0"x1'-9" PRESTRESSED CONCRETE CORED SLABS
	LUMP SUM	LUMP SUM	LUMP SUM	EACH	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	EACH	EACH	NO. LIN.FT.	NO. LIN.FT.	EACH	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO. LIN.FT.
SUPERSTRUCTURE		LUMP SUM					LUMP SUM							170.50			LUMP SUM	22 935
END BENT 1					LUMP SUM	21.6		2,594		7		7 420	4		105	100		
BENT 1						12.7		2,415	7		7 595		4					
END BENT 2					LUMP SUM	21.6		2,594		7		7 455	4		105	105		
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	1	LUMP SUM	55.9	LUMP SUM	7,603	7	14	7 595	14 875	12	170.50	210	205	LUMP SUM	22 935

### GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

THIS BRIDGE SHALL BE CONSTRUCTED USING TOP-DOWN CONSTRUCTION METHODS. THE USE OF A TEMPORARY CAUSEWAY OR WORK BRIDGE IS NOT PERMITTED. CONTRACTOR SHALL NOT PLACE A CRANE ON SPAN B.

- FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 17+39.50 -L-.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 19.5 FT. ON EACH SIDE OF CENTERLINE BRIDGE AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY STRUCTURE AT STATION 17+39.50 -L-FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY STRUCTURE, SEE SPECIAL PROVISIONS.

THE EXISTING THREE SPAN STRUCTURE WITH SPAN LENGTHS OF 17'-10", 17'-1" AND 17'-9" WITH TIMBER FLOOR AND 19 LINES OF 6×12 TIMBER JOISTS AT SPANS 1 AND 3 AND 20 LINES OF 6×12 TIMBER JOISTS AT SPAN 2 WITH A 20.1' OUT TO OUT DECK WIDTH ON STEEL CAP AND TIMBER PILE END BENTS WITH ONE STEEL CAP AND TIMBER PILE INTERIOR BENT AND ONE STEEL CAP AND REINFORCED CONCRETE SUB CAP ON TIMBER PILE INTERIOR BENT SHALL BE REMOVED. IN ADDITION, ANY PILES REMAINING FROM PREVIOUS BRIDGE CONSTRUCTION OR MAINTENANCE OPERATIONS SHALL BE REMOVED AND INCLUDED IN THE LUMP SUM PAY ITEM FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 17+39.50 -L-"

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE CONTAINS THE NECESSARY CORROSION PROTECTION REQUIRED FOR A CORROSIVE SITE.

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE BENT CAPS, AND END BENT CAPS AND SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL BAR SUPPORTS USED IN THE BARRIER RAIL, BENT CAPS, AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE CONCRETE IN THE BENT CAPS AND PILES IN END BENT NO.1, BENT NO.1 AND END BENT NO.2 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

ALL METALLIZED SURFACES SHALL RECEIVE A SEAL COATING AS SPECIFIED IN TABLE 2 OF THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM. FOR THERMAL SPRAYED COATINGS, SEE SPECIAL PROVISIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES."

FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

PROJECT NO. 17BP.2.R.85

BEAUFORT COUNTY

STATION: 17+39.50 -L-

SHEET 2 OF 2

Docusigned by:

David W. Hawkins

7/11/2018

The state of the state of

DEPARTMENT OF TRANSPORTATION
RALEIGH
GENERAL DRAWING

STATE OF NORTH CAROLINA

GENERAL DRAWING
FOR BRIDGE ON SR 1336
OVER DUCK CREEK
BETWEEN SR 1334
AND PAMLICO RIVER

HNTB NORTH CAROLINA, P.C.

NC License No. C-I554
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DRAWN BY
CHECKED BY
DESIGN ENGINEER OF RECORD D. HAWKINS
DATE 4/18

DWG. NO. 2

REVISIONS

NO. BY
DATE NO.

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

#### LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS STRENGTH I LIMIT STATE SERVICE III LIMIT STATE MOMENT SHEAR MOMENT LIVELOAD FACTORS LIVELOAD FACTORS TING CONTI DIST, LEFT SPAN DISTI FACT IST ACT SIM . A DI: LEF SP 1.037 1.75 0.283 1.83 30′ EL 14.5 0.574 1.04 30′ EL 1.45 0.283 1.58 30′ HL-93(Inv)N/A 0.80 EL 14.5 1.35 2.38 14.5 0.574 1.34 1.45 1.344 0.283 30′ EL 30′ HL-93(0pr) N/A EL N/A \_\_\_ --\_\_\_ DESIGN LOAD 42.587 36.000 1.183 2.53 0.574 1.18 1.45 0.80 0.283 2.20 HS-20(Inv) 1.75 0.283 30′ EL 11.6 30′ EL 30′ EL 11.6 RATING 36.000 1.533 55.205 3.28 30′ EL 0.574 1.53 30′ EL 1.45 HS-20(0pr) 1.35 0.283 11.6 N/A \_\_\_ ----\_\_\_ 13.500 39.081 5.18 30′ EL 14.5 0.574 30′ EL 1.45 0.283 3.56 30′ SNSH 2.895 1.4 0.283 2.89 0.80 EL 14.5 0.80 0.283 20.000 2.240 44.792 0.283 4.53 30′ EL 11.6 0.574 30′ EL 1.45 SNGARBS2 2.24 3.15 30′ EL 11.6 22.000 2.157 11.6 2.16 1.45 0.80 0.283 3.20 SNAGRIS2 47.463 0.283 4.6 30′ EL 0.574 30′ EL 30′ 11.6 EL 1.462 39.849 14.5 0.574 1.46 0.80 0.283 1.79 SNCOTTS3 27.250 0.283 2.6 30′ EL 30′ EL 1.45 30′ EL 14.5 SNAGGRS4 34.925 1.346 46.999 0.283 2.5 30′ EL 14.5 0.574 1.35 30′ EL 1.45 0.80 0.283 1.72 30′ 14.5 1.4 EL 0.80 0.283 35.550 50.733 30′ EL 14.5 0.574 1.43 30′ EL 1.45 SNS5A 1.427 0.283 2.42 1.67 30′ EL 14.5 53.59 2.29 0.80 0.283 1.58 SNS6A 39.950 1.341 0.283 30′ EL 14.5 0.574 1.34 30′ EL 1.45 30′ EL 14.5 42.000 1.369 57.505 0.283 2.23 0.574 1.37 0.80 0.283 1.53 SNS7B 30′ EL 14.5 30′ EL 1.45 30′ 14.5 EL LEGAL LOAD 0.80 0.283 TNAGRIT3 33.000 1.593 52.58 1.4 0.283 2.97 30′ EL 14.5 0.574 1.59 30′ EL 1.45 2.04 30′ EL 14.5 RATING TNT4A 33.075 30′ EL 14.5 0.574 30′ EL 1.45 1.483 49.043 0.283 2.82 1.48 0.80 0.283 1.94 30′ EL 14.5 0.283 41.600 1.433 59.622 0.283 2.56 30′ EL 14.5 0.574 1.43 30′ EL 1.45 0.80 30′ 14.5 TNT6A 1.76 EL 42.000 1.363 57.264 2.64 14.5 0.574 1.36 0.80 0.283 1.82 30′ EL 30′ EL 1.45 30′ 14.5 TNT7A 0.283 EL 55.915 14.5 1.33 0.80 0.283 TNT7B 42.000 1.331 0.283 2.49 30′ EL 0.574 30′ EL 1.45 1.72 30′ 14.5 1.4 EL 55.356 TNAGRIT4 43.000 1.287 0.283 2.58 30′ EL 14.5 0.574 1.29 30′ EL 1.45 0.80 0.283 1.78 30′ EL 14.5 TNAGT5A 45.000 1.381 62.151 2.5 30′ EL 14.5 0.574 1.38 30′ EL 1.45 0.80 0.283 1.72 30′ 14.5 0.283 EL

30′

EL

0.283

2.41

LRFR SUMMARY

FOR SPAN 'A'

1.21

0.574

30′

1.45

0.80

0.283

1.66

30′

11.6

EL

1.212

TNAGT5B

DATE: 3/18

DATE: 4/18

ASSEMBLED BY : M. WRIGHT

CHECKED BY: D. HAWKINS

DRAWN BY: CVC 6/10

CHECKED BY : DNS 6/10

45.000

54.54

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

#### COMMENTS:

### (#) CONTROLLING LOAD RATING

- $\langle 1 \rangle$  DESIGN LOAD RATING (HL-93)
- $\langle 2 \rangle$  DESIGN LOAD RATING (HS-20)
- $\langle 3 \rangle$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

### GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

PROJECT NO. \_\_\_17BP.2.R.85

BEAUFORT COUNTY

17+39.50 -L-

7/11/2018

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

(NON-INTERSTATE TRAFFIC)

HNTB NORTH CAROLINA, P.C.

NC License No. C-1554

343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DATE 3/18
DATE 4/18
DATE 4/18

CHECKED BY \_

DESIGN ENGINEER OF RECORD D. HAWKINS

**REVISIONS** BY DATE NO. BY DATE NO. DWG. NO. 3

**DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED** 

STD. NO. 21LRFR1\_90S\_30L

SHEET NO.

S-3

		LOAD AN	D RE	SIST	ANCE	E FAC	CTOR	RAT	ING	(LRF	ED) S	UMMA	RY F	OR F	PRES	TRES	SSED	CONO	CRETI	E GI	RDER	2S		
										STRE	ENGTH	I LIN	MIT S	TATE				SE	RVICE	III	LIMIT	STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (f+)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.055		1.75	0.275	1.23	55′	EL	27	0.523	1.23	55′	EL	5.4	0.80	0.275	1.05	55′	EL	27	
DESIGN		HL-93(0pr)	N/A		1.591		1.35	0.275	1.59	55′	EL	27	0.523	1.59	55′	EL	5.4	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.322	47.585	1.75	0.275	1.54	55′	EL	27	0.523	1.47	55′	EL	5.4	0.80	0.275	1.32	55′	EL	27	
		HS-20(0pr)	36.000		1.9	68.396	1.35	0.275	1.99	55′	EL	27	0.523	1.9	55′	EL	5.4	N/A						
		SNSH	13.500		2.776	37.476	1.4	0.275	4.04	55′	EL	27	0.523	4.17	55′	EL	5.4	0.80	0.275	2.78	55′	EL	27	
		SNGARBS2	20.000		2.155	43.095	1.4	0.275	3.14	55′	EL	27	0.523	3.02	55′	EL	5.4	0.80	0.275	2.15	55′	EL ———	27	
	_	SNAGRIS2	22.000		2.079	+	1.4	0.275	3.03	55′	EL 	27	0.523		55′	EL 	5.4	0.80	0.275		55′	EL 	27	
	>	SNCOTTS3	27.250		1.384	37.708	1.4	0.275	2.01	55′	EL	27	0.523	2.09	55′	EL	5.4	0.80	0.275	1.38	55′	EL ———	27	
	S	SNAGGRS4	34.925		1.189	41.527	1.4	0.275	1.73	55′	EL	27	0.523	1.77	55′	EL	5.4	0.80	0.275	1.19	55′	EL	27	
		SNS5A	35.550		1.16	41.255	1.4	0.275	1.69	55′	EL	27	0.523	1.82	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
		SNS6A	39.950		1.079	43.102	1.4	0.275	1.57	55′	EL	27	0.523	1.68	55′	EL	5.4	0.80	0.275	1.08	55′	EL EL	27	
LEGAL		SNS7B	42.000		1.028	43.175	1.4	0.275	1.5	55′ ———	EL	27	0.523	1.67	55′ ———— 55′	EL	5.4	0.80	0.275	1.03	55′ 55′	EL	27	
RATING	_	TNAGRIT3	33.000		1.32	43.556	1.4	0.275	1.92	55′ ———	EL	27	0.523	1.98	  55′	EL	5.4	0.80	0.275	1.32		EL	27	
		TNT4A TNT6A	33.075 41.600		1.33	43.979	1.4	0.275 0.275	1.94	55′ ——55′	EL EL	27	0.523 0.523	1.91 1.83	55′	EL EL	5.4 5.4	0.80	0.275	1.33 1.10	55′ 55′	EL EL	27	
	_  -	TNT7A	42.000		1.101	45.811 46.804	1.4 1.4	0.275	1.6 1.62	55′	EL	27	0.523	1.71	55′	EL	5.4	0.80	0.275	1.11	55′	EL	27	
	SIL	TNT7B	42.000		1.114	48.848	1.4	0.275	1.69	55′	EL	27	0.523	1.62	55′	EL	5.4	0.80	0.275	1.16	55′	EL	27	
	-	TNAGRIT4	43.000		1.101	47.33	1.4	0.275	1.6	55′	EL	27	0.523	1.56	55′	EL	5.4	0.80	0.275	1.10	55′	EL	27	
	-	TNAGT5A	45.000		1.031	46.405	1.4	0.275	1.5	55′	EL	27	0.523	1.58	55′	EL	5.4	0.80	0.275	1.03	55′	EL	27	
		TNAGT5B	45.000		1.013	45.582	1.4	0.275	1.47	55′	EL	27	0.523	1.48	55′	EL	5.4	0.80	0.275	1.01	55′	EL	27	

LRFR SUMMARY

FOR SPAN 'B'

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.5
FACTORS	SERVICE III	1.00	1.0

### NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

### COMMENTS:

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\langle 3 \rangle$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

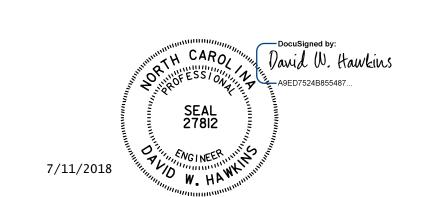
EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.2.R.85

BEAUFORT COUNTY

17+39.50 -L-



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

LRFR SUMMARY FOR 55' CORED SLAB UNIT 90° SKEW (NON-INTERSTATE TRAFFIC)

CHECKED BY \_\_\_\_\_\_ D. HAWKINS
DESIGN ENGINEER OF RECORD D. HAWKINS

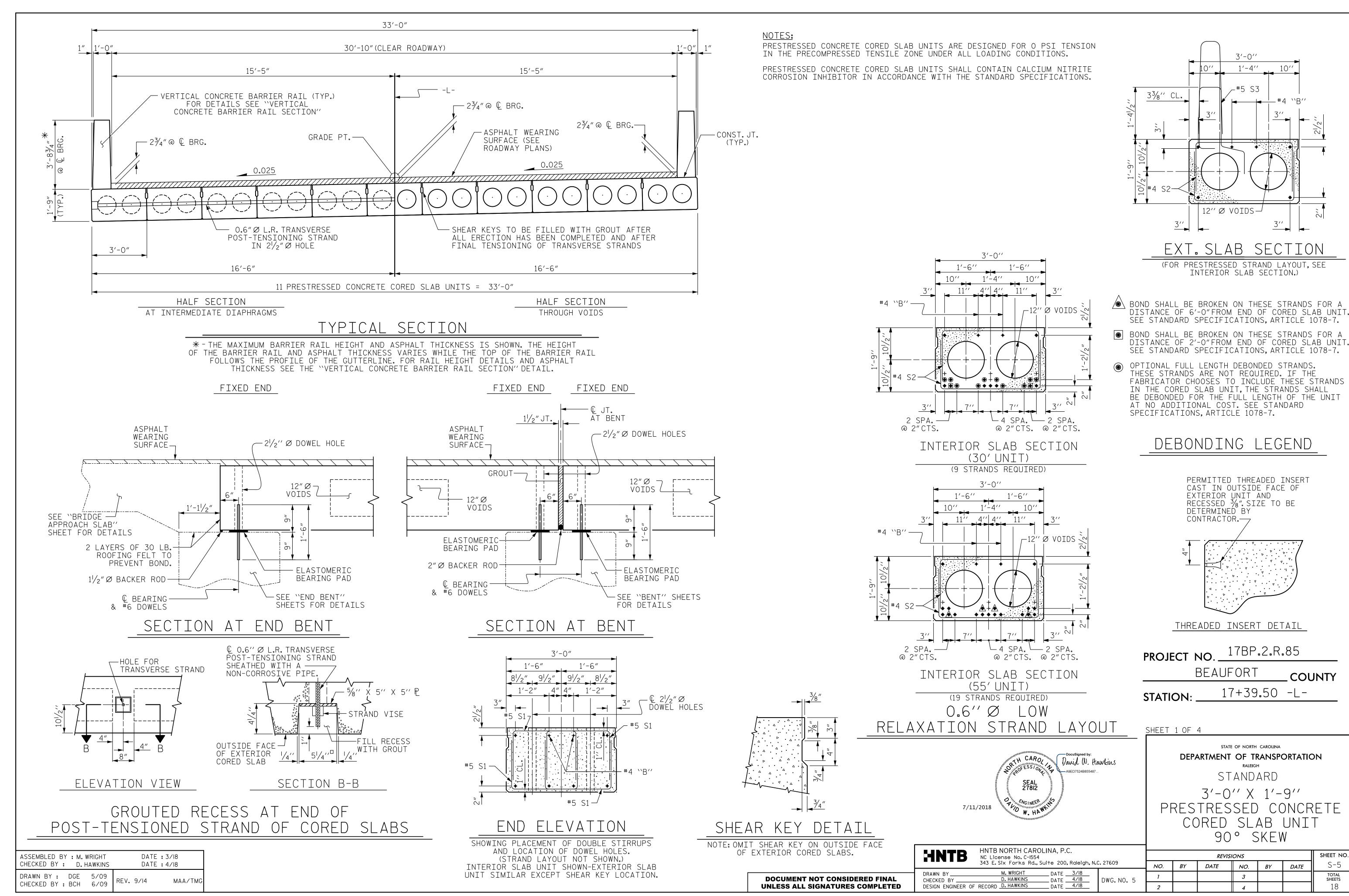
HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DATE 3/18
DATE 4/18
DATE 4/18 DWG. NO. 4

SHEET NO. **REVISIONS** S-4 BY DATE NO. BY DATE

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

STD. NO. 21LRFR1\_90S\_55L

ASSEMBLED BY : M. WRIGHT CHECKED BY : D. HAWKINS DATE: 3/18 DATE: 4/18 DRAWN BY: CVC 6/10 CHECKED BY: DNS 6/10



STD. NO. 21" PCS2\_33\_90S

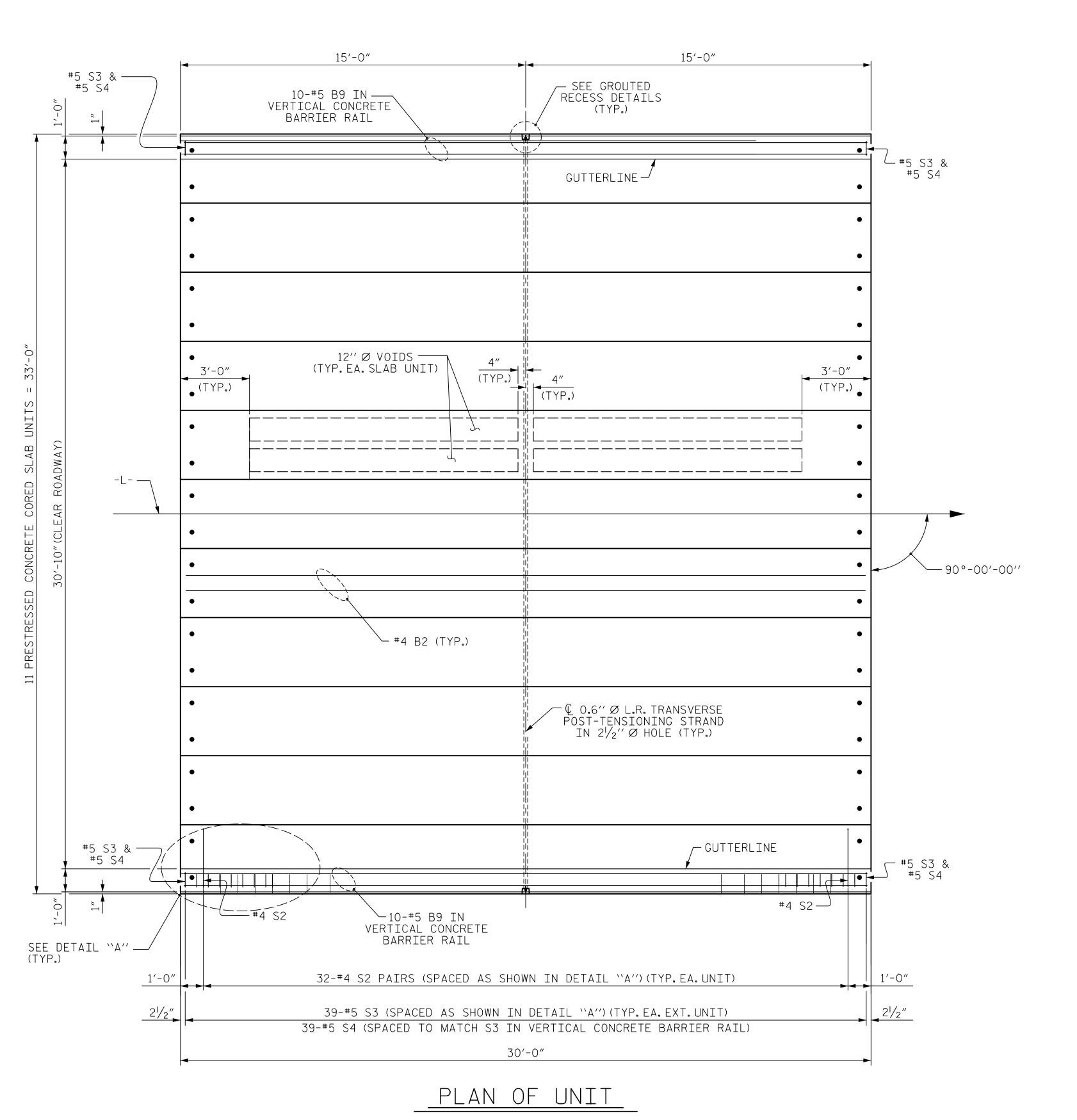
DATE: 3/18

DATE: 4/18

REV. 12/5/II MAA/AAC REV. 8/I4 MAA/TMG

ASSEMBLED BY: M. WRIGHT CHECKED BY: D. HAWKINS

DRAWN BY: DGE 3/09 CHECKED BY: BCH 3/09



(TYPICAL EACH END OF UNIT)
NOTE: EXTERIOR UNIT SHOWN - INTERIOR
UNIT SIMILAR EXCEPT OMIT #5 S3 BARS.

DETAIL "A'

PROJECT NO. 17BP.2.R.85

BEAUFORT COUNTY

**STATION**: \_\_\_\_\_17+39.50 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

PLAN OF 30'UNIT

30'-10'' CLEAR ROADWAY

HNTB NORTH CAROLINA, P.C.

NC License No. C-1554
343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609

DRAWN BY M. WRIGHT DATE 3/18
CHECKED BY D. HAWKINS DATE 4/18
DESIGN ENGINEER OF RECORD D. HAWKINS DATE 4/18

DWG. NO. 6

7/11/2018

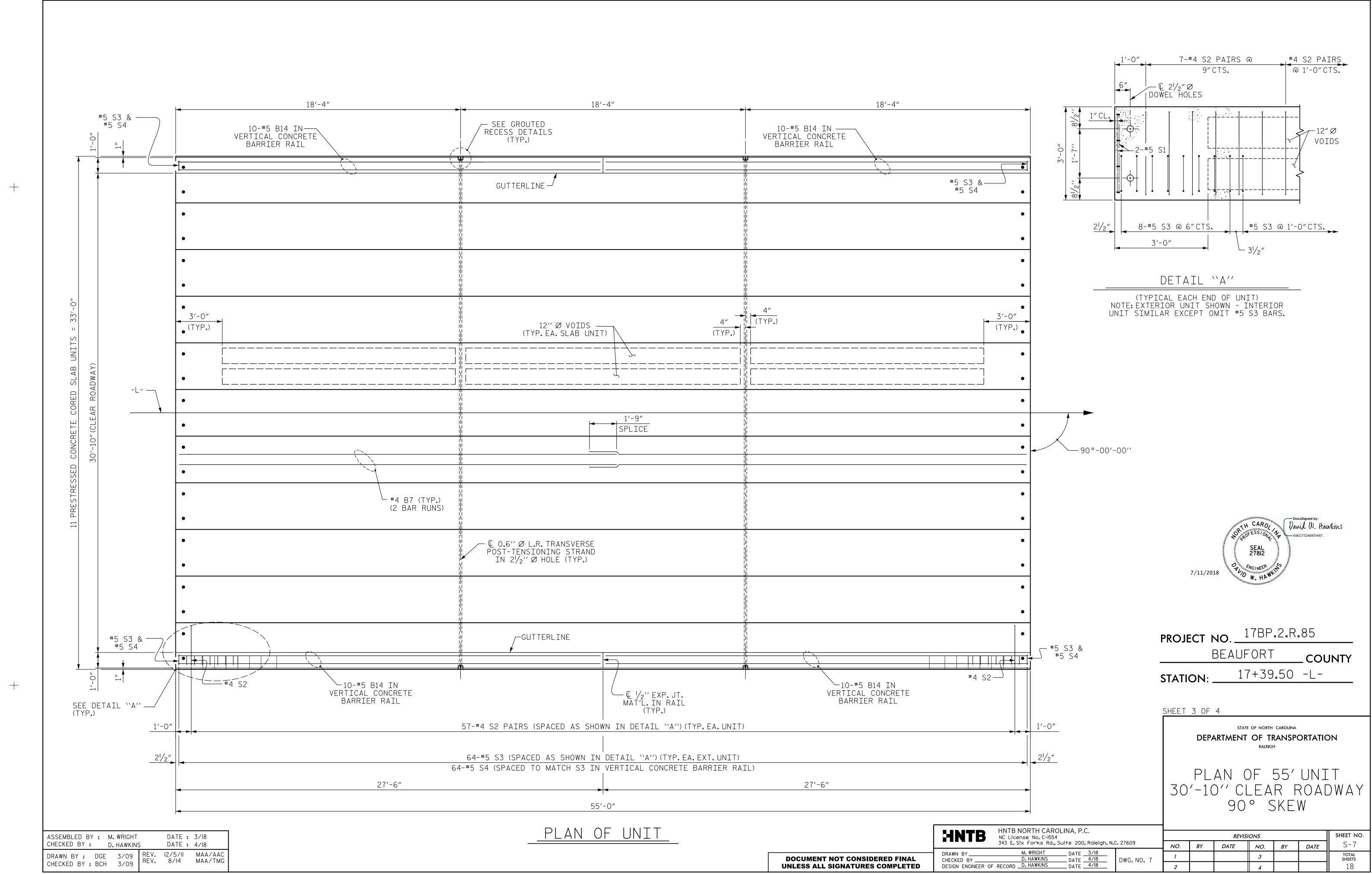
DOCUMENT NOT CONSIDERED FINAL

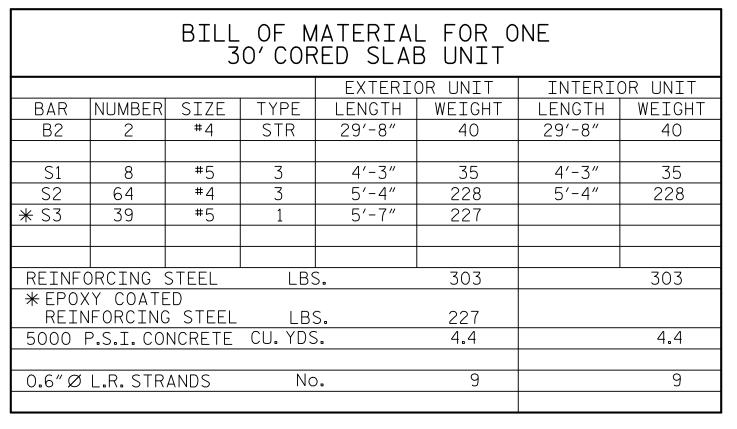
UNLESS ALL SIGNATURES COMPLETED

 REVISIONS
 SHEET NO.

 NO.
 BY
 DATE
 NO.
 BY
 DATE
 S - 6

 1
 3
 TOTAL SHEETS
 18





BILL OF MATERIAL FOR ONE

55' CORED SLAB UNIT

28′-3″

4′-3″

5′-4″

5′-7″

BAR | NUMBER | SIZE | TYPE |

4

114

64

REINFORCING STEEL

0.6" Ø L.R. STRANDS

ASPHALT ('' TABLE)

3'-8¾" "CUTTERLINE / RAIL HEIGHT'

VARIES (THICKNE)

ASSEMBLED BY : M. WRIGHT

REINFORCING STEEL

6500 P.S.I. CONCRETE CU. YDS.

\* EPOXY COATED

\* S3

#4

#5

#4

#5

STR

No.

2"CL.MIN.

1'-0"

10"

**→•** '•|

I FNGTH | WFTGHT

EXTERIOR UNIT | INTERIOR UNIT

75

35

406

373

516

7.8

19

— #5 S4

(TYP.)

-#5 S3

I FNGTH | WFTGHT

75

35

406

516

7.8

19

GROUT-

3,

SECTION T-1

AT OPEN JOINT AT BENT

(THIS IS TO BE USED WHERE FOAM JOINT IS NOT USED)

€ OPEN JT. IN →

RAIL @ BENT

-- #5 S3 (SEE ''PLAN OF

UNIT" FOR SPACING)

VERTICAL CONCRETE BARRIER RAIL SECTION

© 1/2″EXP. JT. MAT'L HELD IN PLACE WITH GALVANIZED NAILS.

(NOTE: OMIT EXP. JT. MAT'L.

WHEN SLIP FORM IS USED)

CHAMFER

ELEVATION AT EXPANSION JOINTS

28′-3″

4′-3″

5′-4″

GRADE 270 ST	RANDS
	0.6″Ø L.R
AREA (SQUARE INCHES) ULTIMATE STRENGTH	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

NUMBER LENGTH TOTAL LENGTH  30' UNIT  EXTERIOR C.S. 2 30'-0" 60'-0"	CORED	SLABS	S REQ	UIRED
33 3.72 1		NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR C.S. 2 30'-0" 60'-0"	30'UNIT			
	EXTERIOR C.S.	2	30'-0"	60'-0"
INTERIOR C.S. 9 30'-0" 270'-0"	INTERIOR C.S.	9	30'-0"	270'-0"
TOTAL 11 330'-0"	TOTAL	11		330′-0″

]	CORED	SLABS	S REQ	UIRED
		NUMBER	LENGTH	TOTAL LENGTH
	55' UNIT			
	EXTERIOR C.S.	2	55′-0″	110'-0"
	INTERIOR C.S.	9	55′-0″	495′-0″
	TOTAL	11		605′-0″

-¢ bearing pad

−¢ 1″Ø HOLES

-BEARING PAD

- TYPE I -

FIXED END

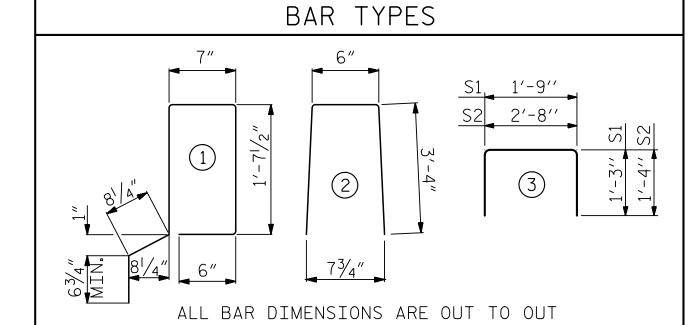
(TYPE I - 44 REQ'D)

ELASTOMERIC

BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL

BE 50 DUROMETER HARDNESS.



ΒI	ILL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	RIER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIG
	30' UNIT					
<b>∗</b> B9	20	20	#5	STR	29'-7"	61
<u>* S4</u>	78	78	#5	2	7′-2″	58
<b>★</b> EP0X	L Y COATED REINFORCING STEEL			LBS.		120
CLASS	AA CONCRETE			CU.YDS.	,	7.
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN. FT.		60.2

BIL	L OF MATERIAL FOR VERTIC	CAL CONCR	ETE I	BARR	IER RA	\IL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	55' UNIT					
<b></b> ₩ B14	40	40	#5	STR	27'-1"	1130
<del>*</del> S4	128	128	#5	2	7′-2″	957
₩ EPOX	Y COATED REINFORCING STEEL			LBS.		2087
CLASS	AA CONCRETE			CU.YDS.	ı	14.1
						110.25

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0" × 1'-9"
30'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1/4″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	1/8″ ♦
FINAL CAMBER	1/8″ ♠
** INCLUDES FUTURE WEARING SURF	ACE

GUTT

END VIEW

**2**<sup>1</sup>/<sub>2</sub>"

SECTION S-S

AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY

WHEN SLIP FORM IS USED)

CHAMFER

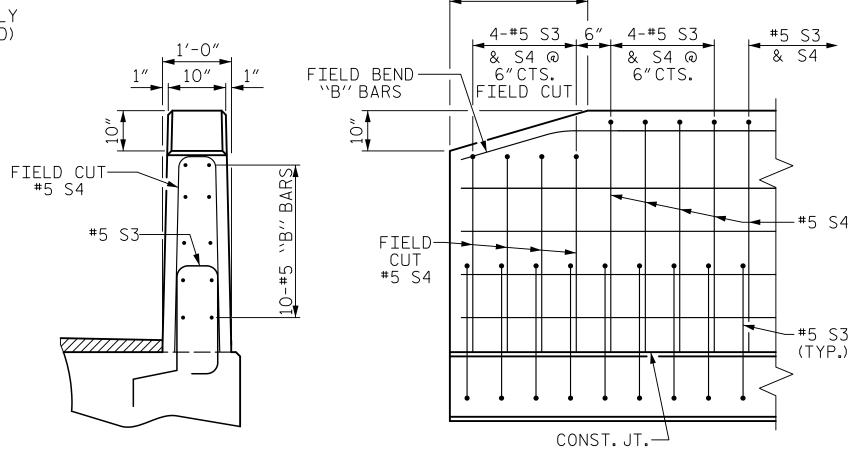
DEAD LOAD DEFLECTION AT	ND CAMBER
	3'-0" × 1'-9"
55'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	11/2″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	3/8″ ♦
FINAL CAMBER	11/8″ ♦

** INCLUDES FUTURE WEARING SURFACE
------------------------------------

UTTERLINE ASPHALT THICKNESS & RAIL HEIGHT		
	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
30'UNITS	25/8″	3′-85⁄8″
55'UNITS	15/8″	3′-75/8″

2'-0"

SIDE VIEW



CONCRETE RELEASE STRENGTH UNIT PSI 30'UNITS 4000 4900 55' UNITS

7/11/2018

David W. Hawkins

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR

NOTES

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE

TENSIONING OF THE STRANDS.

PRESTRESSED CONCRETE CORED SLABS.

SPECIFICATIONS.

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST. AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS,  $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-0" CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

PRESTRESSED CONCRETE CORED SLAB UNITS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE CORED SLAB UNITS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

> BEAUFORT COUNTY 17+39.50 -L-STATION:

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0'' X 1'-9'' PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

END OF RAIL DETAILS

**DOCUMENT NOT CONSIDERED FINAL** CHECKED BY . **UNLESS ALL SIGNATURES COMPLETED** 

HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 \_\_\_ DATE <u>3/18</u> \_\_\_ DATE <u>4/18</u> D. HAWKINS DWG. NO. 8 DESIGN ENGINEER OF RECORD D. HAWKINS \_\_ DATE <u>4/18</u>

TH CARO!

SHEET NO. **REVISIONS** S-8 DATE NO. BY DATE NO. BY

CHECKED BY: D. HAWKINS DATE : 4/18 DRAWN BY: DGE 5/09 MAA/TMG REV. II/I4 CHECKED BY: BCH 6/09

CONST.JT.—

DATE : 3/18

STD. NO. 21" PCS3\_33\_90S

ASSEMBLED BY : M. WRIGHT

DRAWN BY: MAA 5/10

CHECKED BY : GM 5/10

CHECKED BY: D. HAWKINS

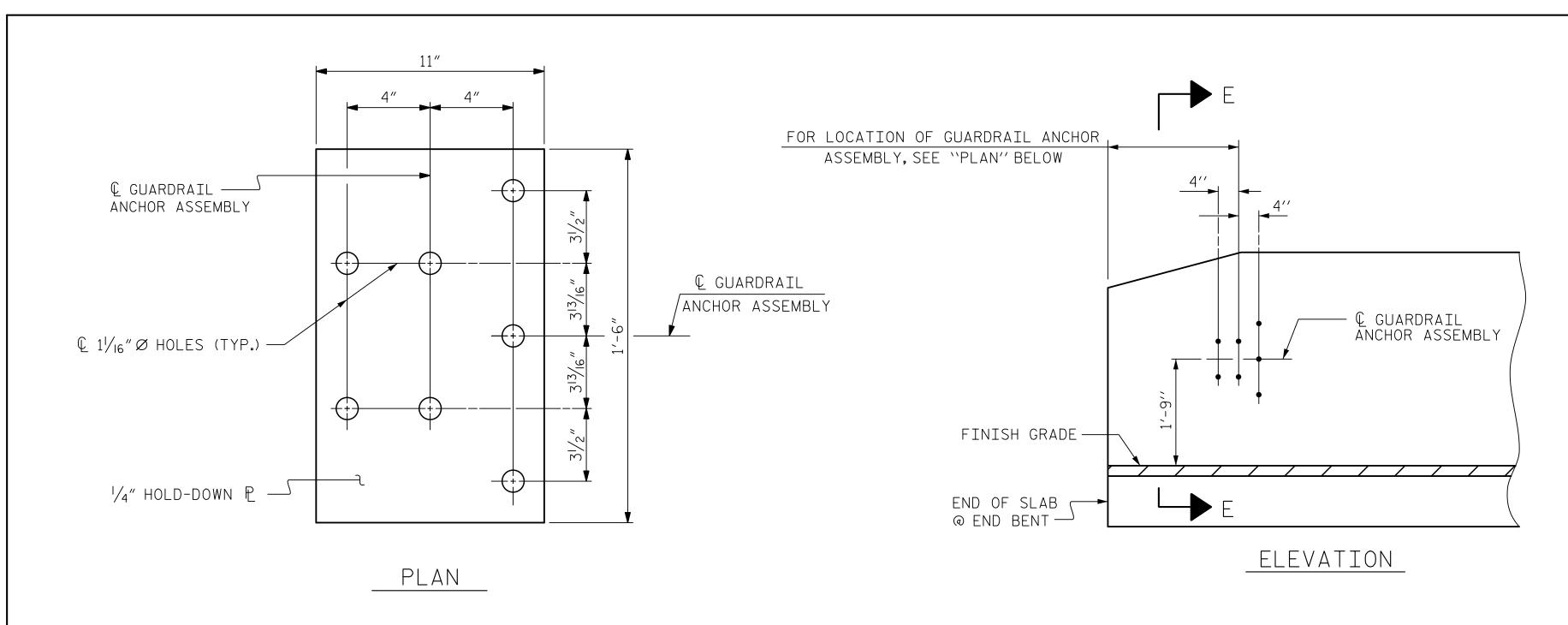
DATE: 3/18

DATE: 4/18

MAA/GM

MAA/TMG

MAA/THC



### NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A  $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 -  $\frac{7}{8}$ "  $\varnothing$  BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

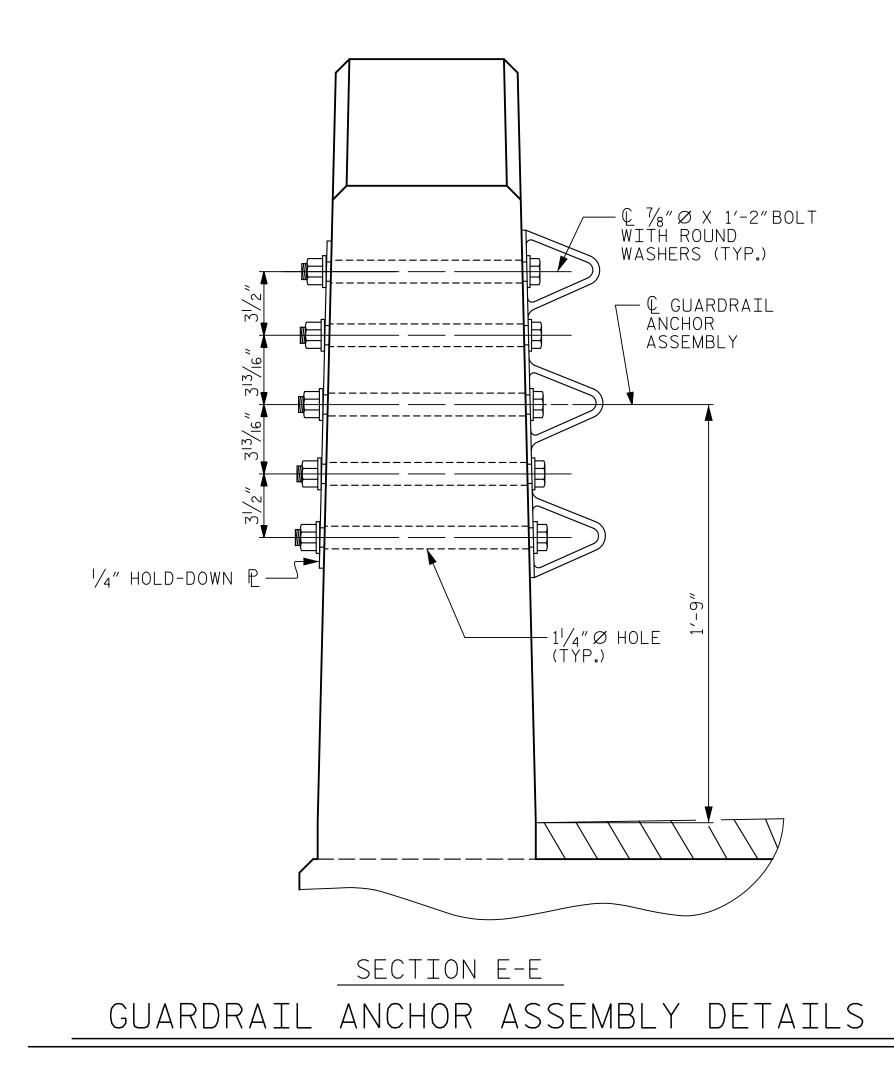
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

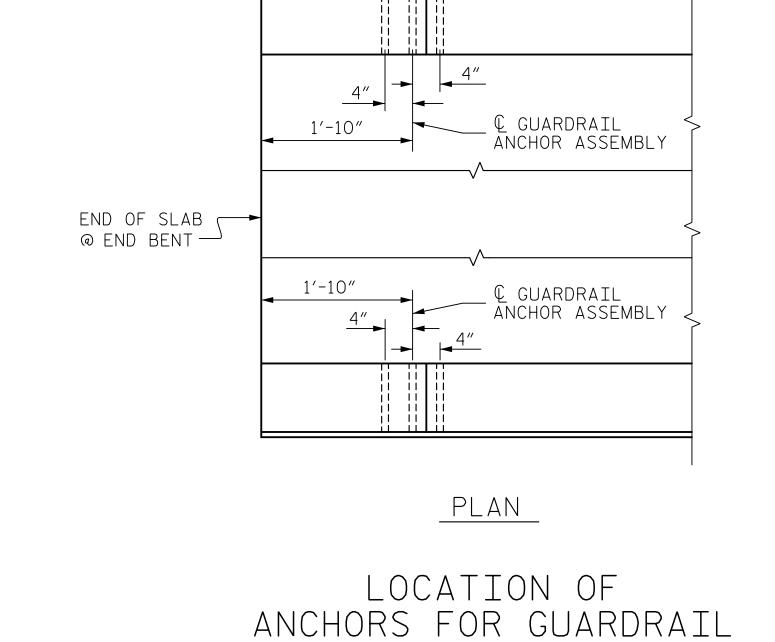
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1  $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.







### SKETCH SHOWING POINTS OF ATTACHMENT

\* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. \_\_\_\_17BP.2.R.85 BEAUFORT COUNTY

17+39.50 -L-

7/11/2018

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD GUARDRAIL ANCHORAGE

DETAILS FOR VERTICAL CONCRETE BARRIER RAIL

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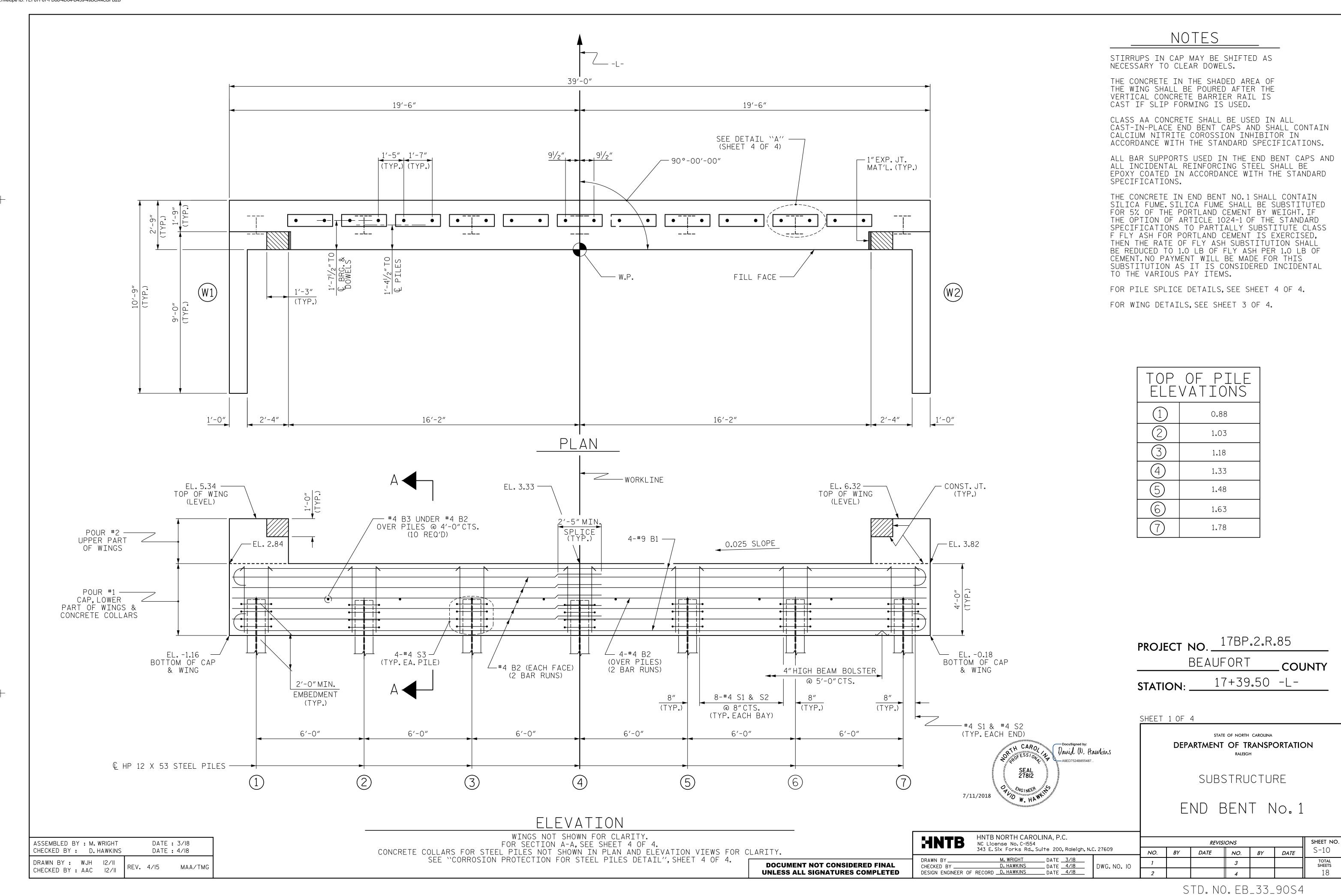
**REVISIONS** SHEET NO. S-9 BY DATE NO. BY DATE DWG. NO. 9

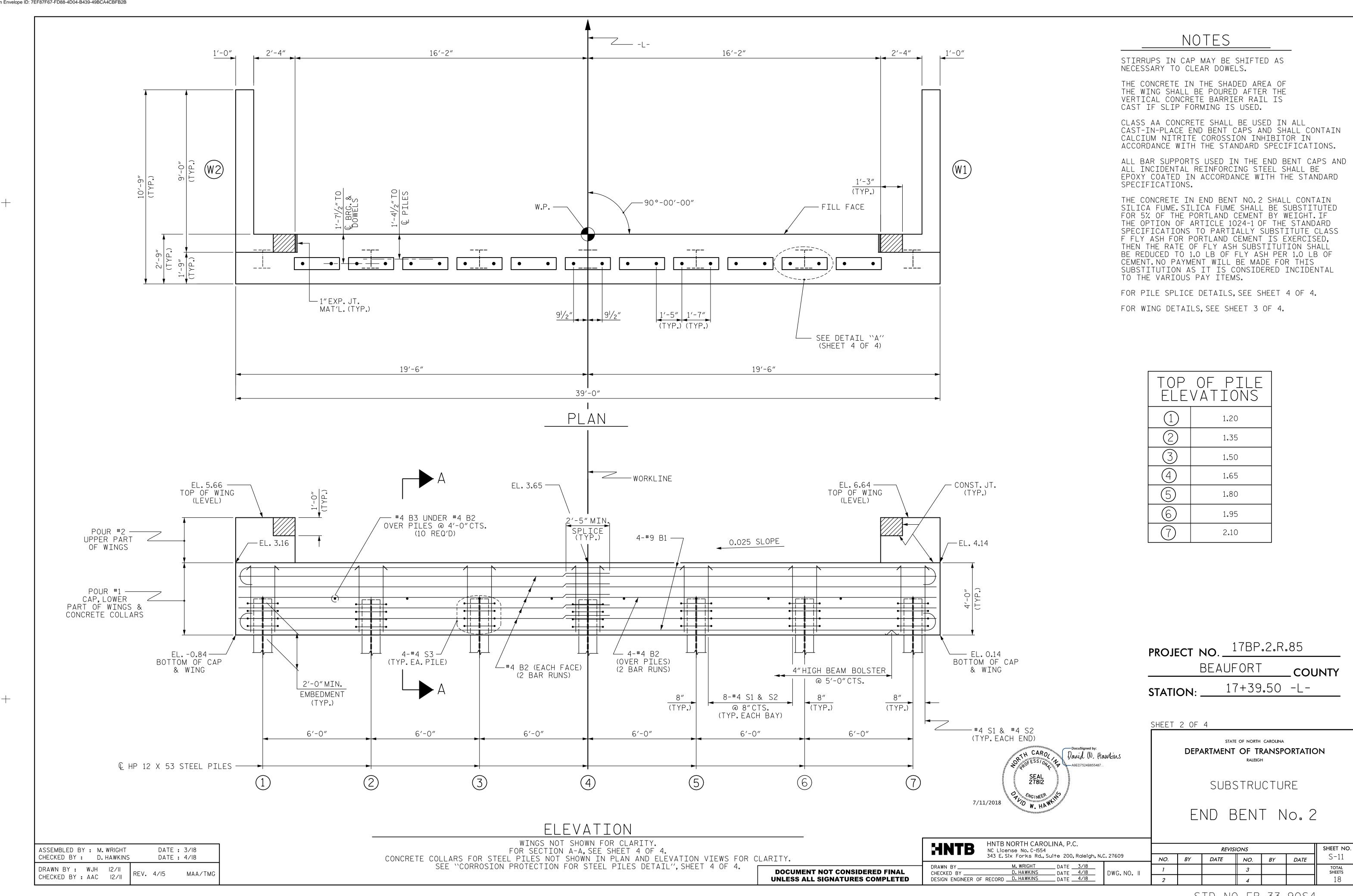
**DOCUMENT NOT CONSIDERED FINAL** UNLESS ALL SIGNATURES COMPLETED

END BENT #1 SHOWN, END BENT #2 SIMILAR.

DATE 3/18
DATE 4/18
DATE 4/18 CHECKED BY \_ DESIGN ENGINEER OF RECORD D. HAWKINS

STD. NO. GRA3 (SHT 1)





ASSEMBLED BY : M. WRIGHT

DRAWN BY: WJH 12/II CHECKED BY: AAC 12/II

CHECKED BY: D. HAWKINS

DATE : 3/18

DATE : 4/18

MAA/TMG

NO. BY DATE

**REVISIONS** 

DATE

NO.

DWG. NO. 12

DATE 3/18
DATE 4/18
DATE 4/18

D. HAWKINS

DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED

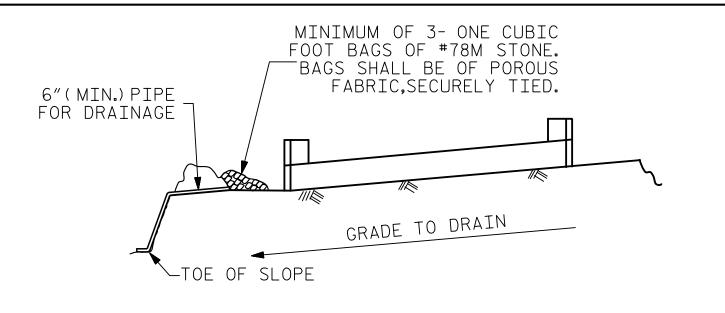
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DESIGN ENGINEER OF RECORD D. HAWKINS

BY

SHEET NO.

total sheets 18

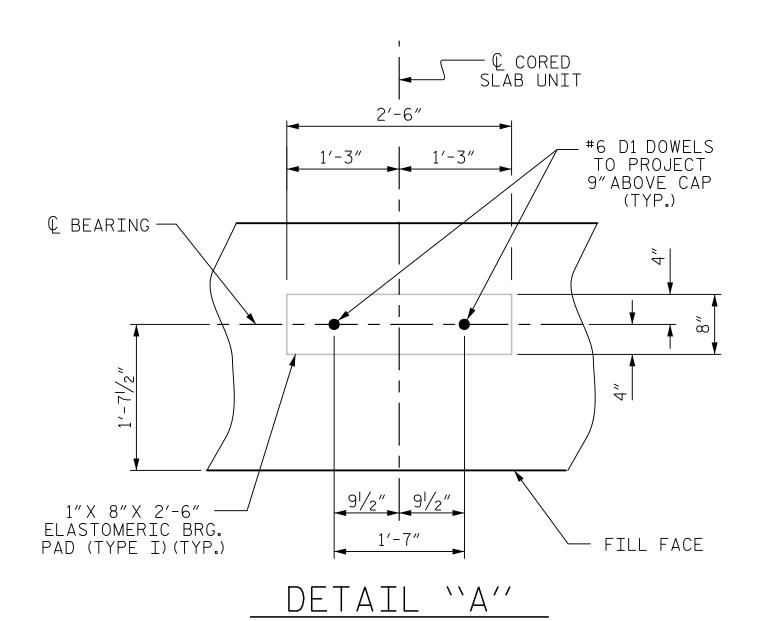


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

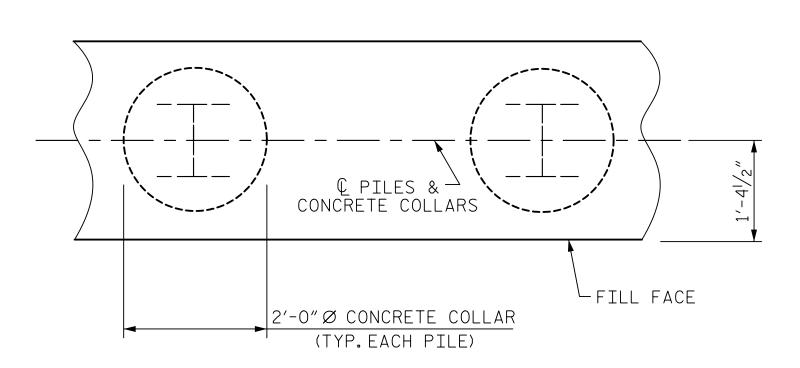
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

### TEMPORARY DRAINAGE AT END BENT



(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

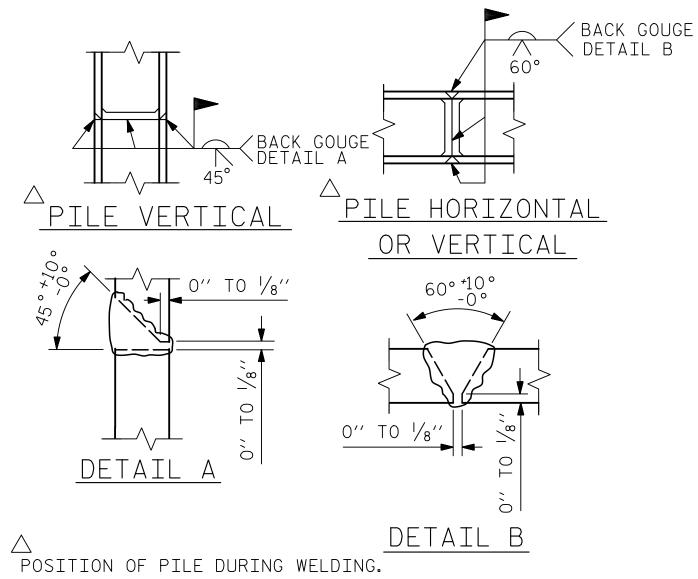


PLAN

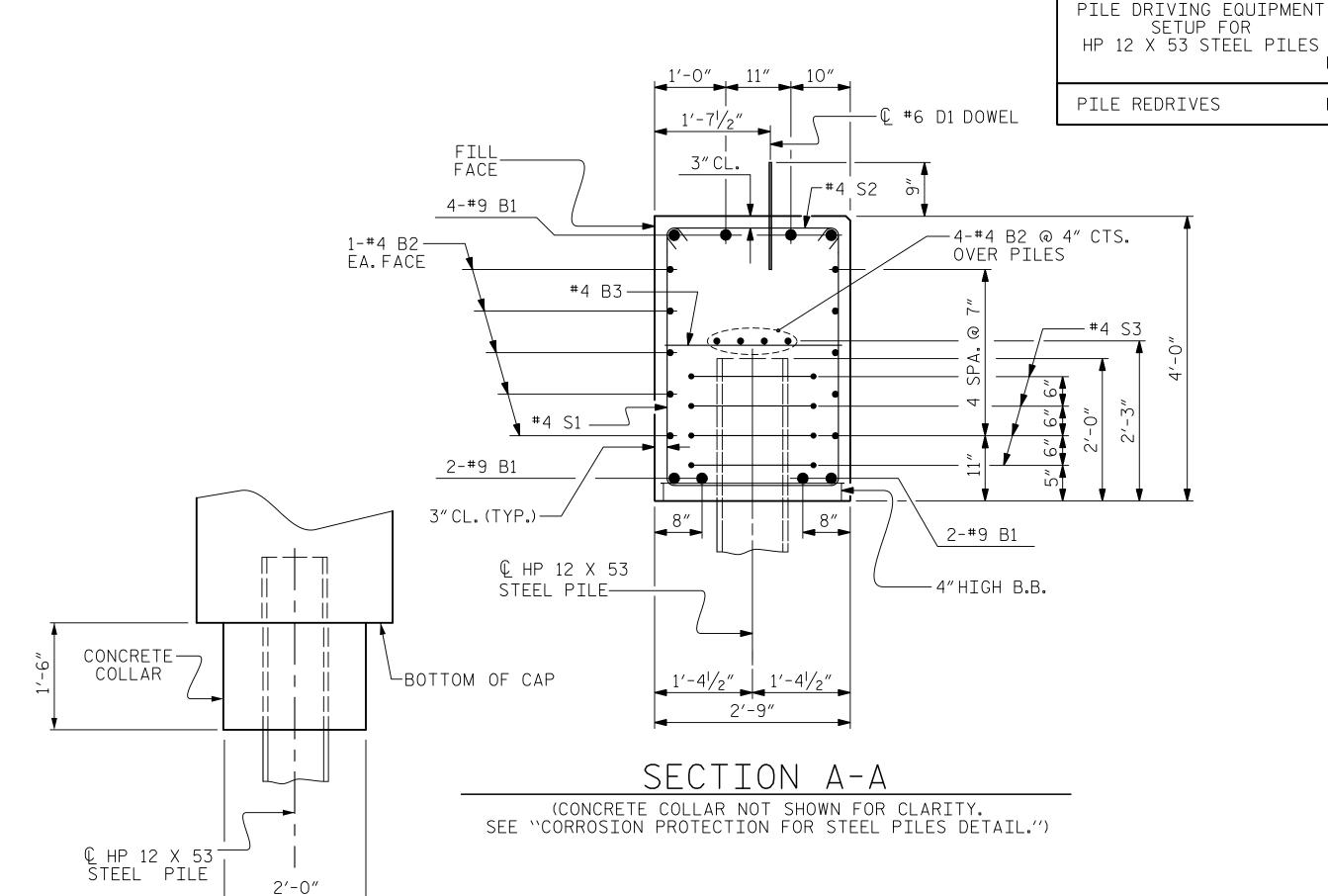
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

ELEVATION

DATE:3/18 ASSEMBLED BY : M. WRIGHT CHECKED BY: D. HAWKINS DATE:4/18 DRAWN BY: WJH 12/II MAA/THC REV. 4/17 CHECKED BY : AAC | 12/11



PILE SPLICE DETAILS



BAR TYPES BILL OF MATERIAL FOR ONE END BENT BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 2′-3″ #9 40'-10" **∗** B1 \*B2 | 28 | #4 | STR | 20'-6" 38'-4" \*B3 | 10 | #4 | STR | 2′-3″ #6 | STR | \* D1 | 22 | 1′-6″ \* H1 | 40 | #4 | 2 | 9′-2″ \* K1 | 16 | #4 | STR | 2′-9″ 8'-6" \* S1 | 50 | #4 | 3 | 9'-11" (5)\*S2 | 50 | #4 | 4 3′-0″ 6'-6" \*S3 | 28 | #4 | 5 \* V1 | 52 | #4 | STR | 6'-0" 1′-8″Ø \* EPOXY COATED REINFORCING STEEL (FOR ONE END BENT) 2,594 LBS. CLASS AA CONCRETE BREAKDOWN (FOR ONE END BENT) POUR #1 CAP, LOWER PART 2'-3" OF WINGS & COLLARS POUR #2 UPPER PART OF ALL BAR DIMENSIONS ARE OUT TO OUT. WINGS END BENT No.1 END BENT No. 2

HP 12 X 53 STEEL PILES

PILE DRIVING EQUIPMENT

SETUP FOR

HP 12 X 53 STEEL PILES

PILE REDRIVES

LIN. FT.= 455

NO: 7

NO: 4

NO: 7

HP 12 X 53 STEEL PILES

NO: 7

**DOCUMENT NOT CONSIDERED FINAL** 

**UNLESS ALL SIGNATURES COMPLETED** 

LIN. FT.= 420

NO: 7

NO: 4

**PROJECT NO**. \_\_\_\_17BP.2.R.85 BEAUFORT COUNTY 17+39.50 -L-

TOTAL CLASS AA CONCRETE

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

1,111

383

15

50

245

29

331

100

122

208

19.5 C.Y.

2.1 C.Y.

21.6 C.Y.

SUBSTRUCTURE

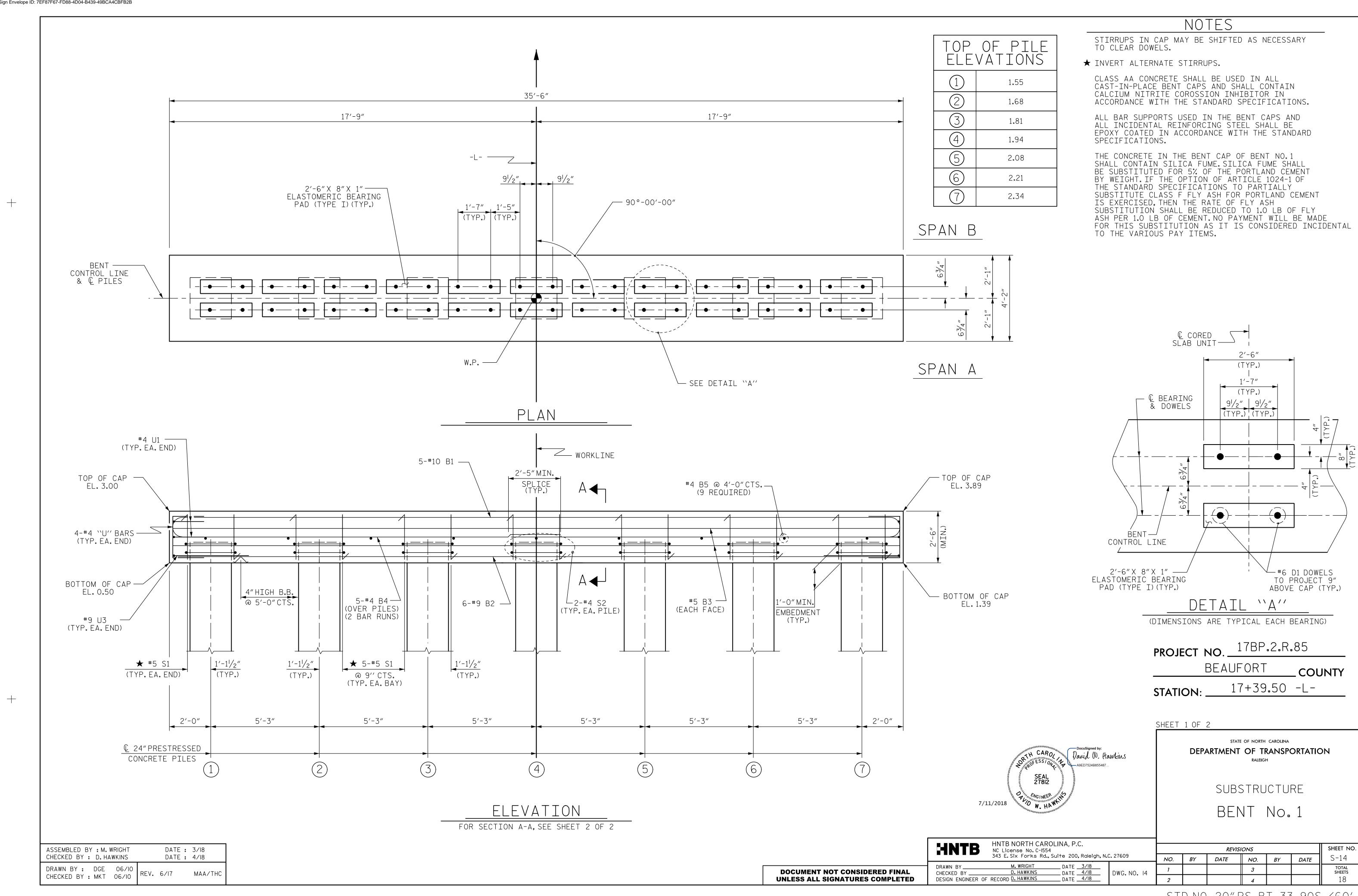
END BENT No.1 & 2 DETAILS

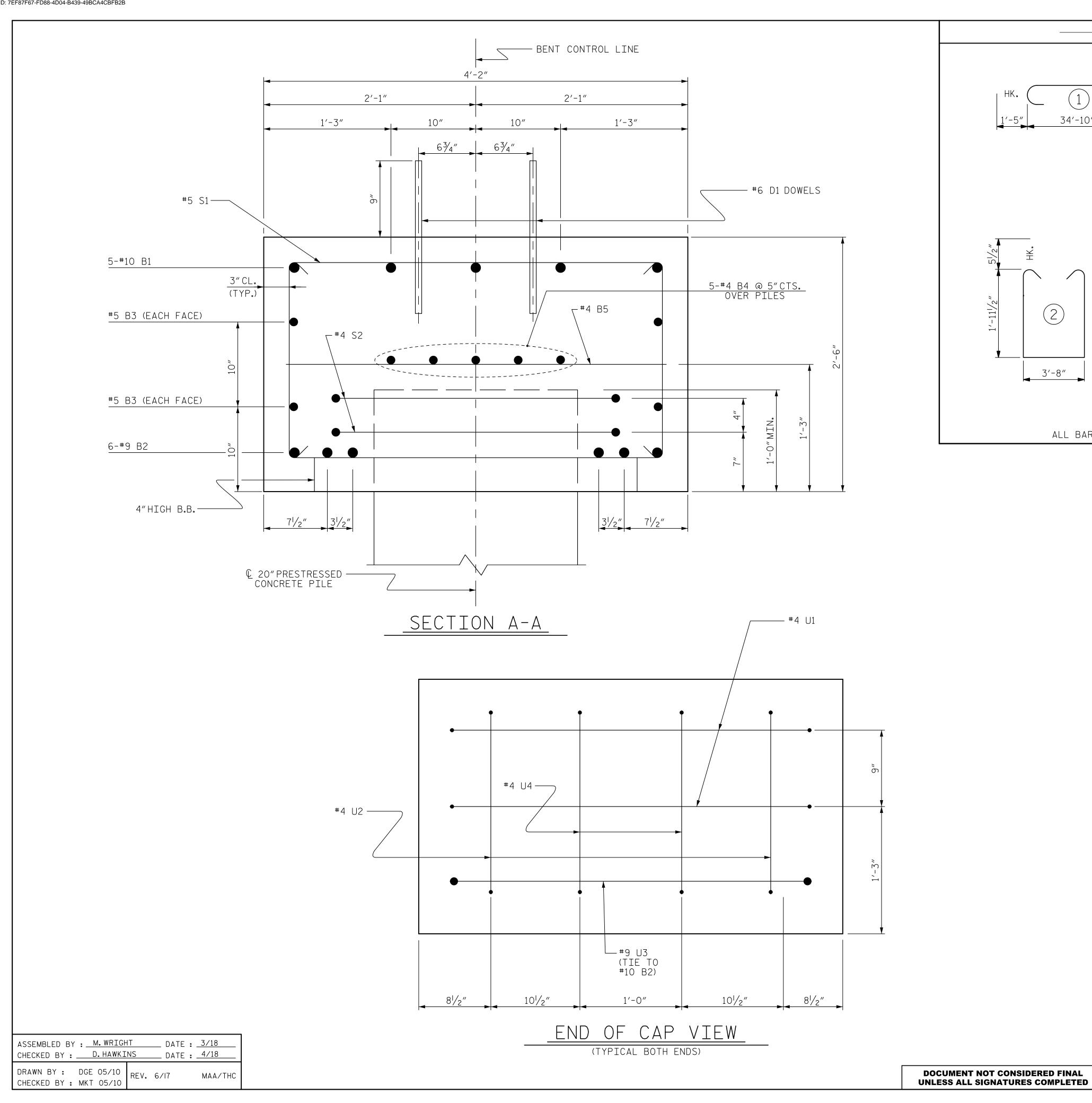
HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 \_\_\_\_ DATE <u>3/18</u> \_\_\_\_ DATE <u>4/18</u> D. HAWKINS CHECKED BY \_ DWG. NO. 13 DESIGN ENGINEER OF RECORD D. HAWKINS \_\_\_ DATE <u>4/18</u>

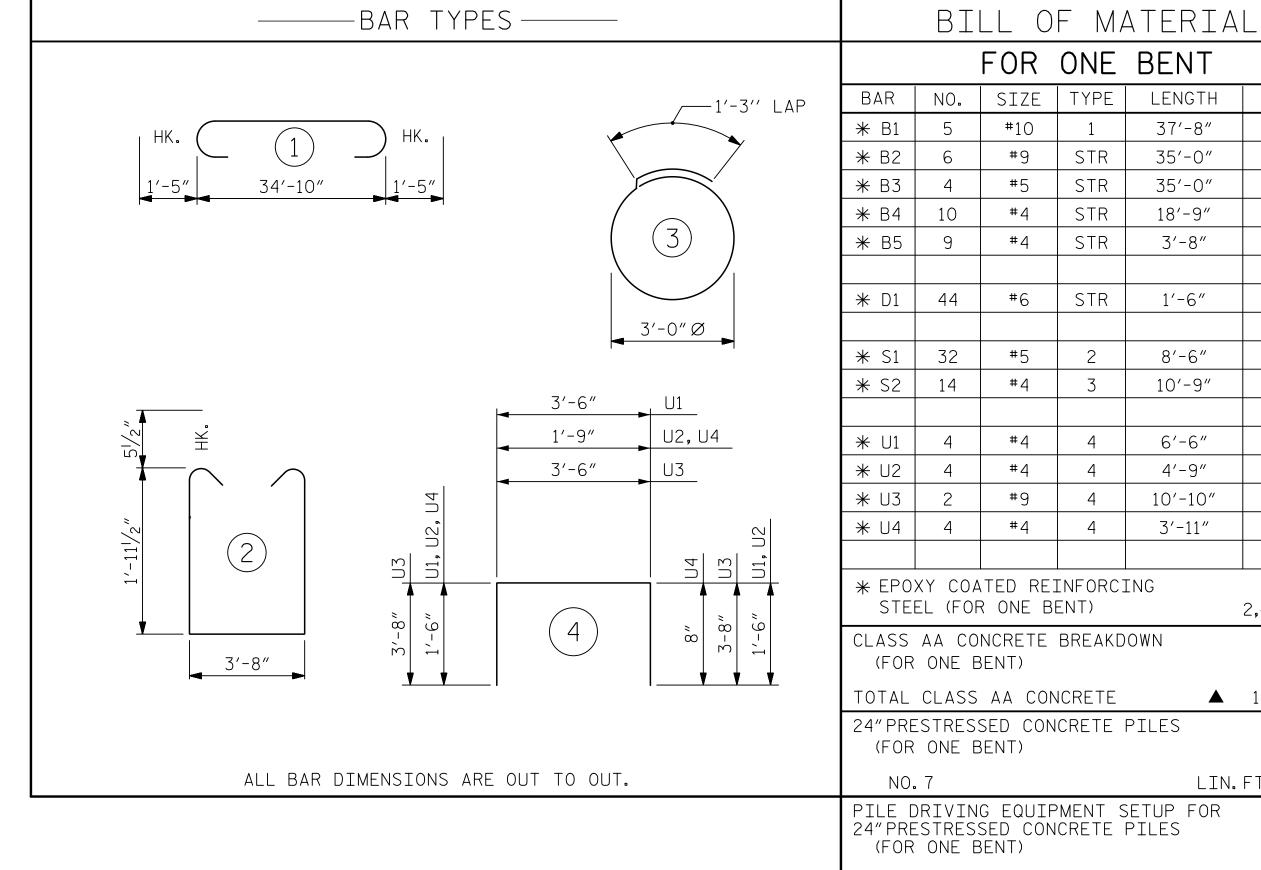
7/11/2018

SHEET NO. **REVISIONS** S-13 DATE NO. BY DATE NO. BY TOTAL SHEETS

STD. NO. EB\_33\_90S4







▲ CONCRETE DISPLACED BY THE 24"PRESTRESSED CONCRETE PILES HAS BEEN DEDUCTED FROM THE CONCRETE QUANTITY.

WEIGHT

810

714

146

125

22

99

284

101

17

13

74

10

2,415 LBS

▲ 12.7 C.Y.

LIN.FT. 595

NO. 7

NO. 4

37′-8″

35′-0″

35′-0″

18′-9″

3′-8″

1′-6″

8'-6"

10′-9″

6′-6″

4′-9″

10'-10"

3′-11″

4

PROJECT NO. 17BP.2.R.85 BEAUFORT COUNTY <u>17+39</u>.50 -L-

SHEET 2 OF 2

PILE REDRIVES

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION SUBSTRUCTURE

BENT No. 1

HNTB NORTH CAROLINA, P.C. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 DATE 3/18
DATE 4/18
DATE 4/18 M. WRIGHT
D. HAWKINS CHECKED BY \_\_\_\_ DWG. NO. 15 DESIGN ENGINEER OF RECORD D. HAWKINS

SHEET NO. **REVISIONS** NO. BY DATE BY DATE NO. TOTAL SHEETS

24′′ 🗆

EQUAL SPA.

TYPICAL SECTION

3" CL. TO

WIRE SPIRAL

DATE : 3/18

DATE: 4/18

REV. IO/I/II

REV. 12/14 REV. 12/17

MAA/GM

MAA/TMG

MAA/THC

ELEVATION

PRESTRESS

3" CL. TO
WIRE SPIRAL

W4.0 COLD DRAWN —

STEEL WIRE SPIRAL

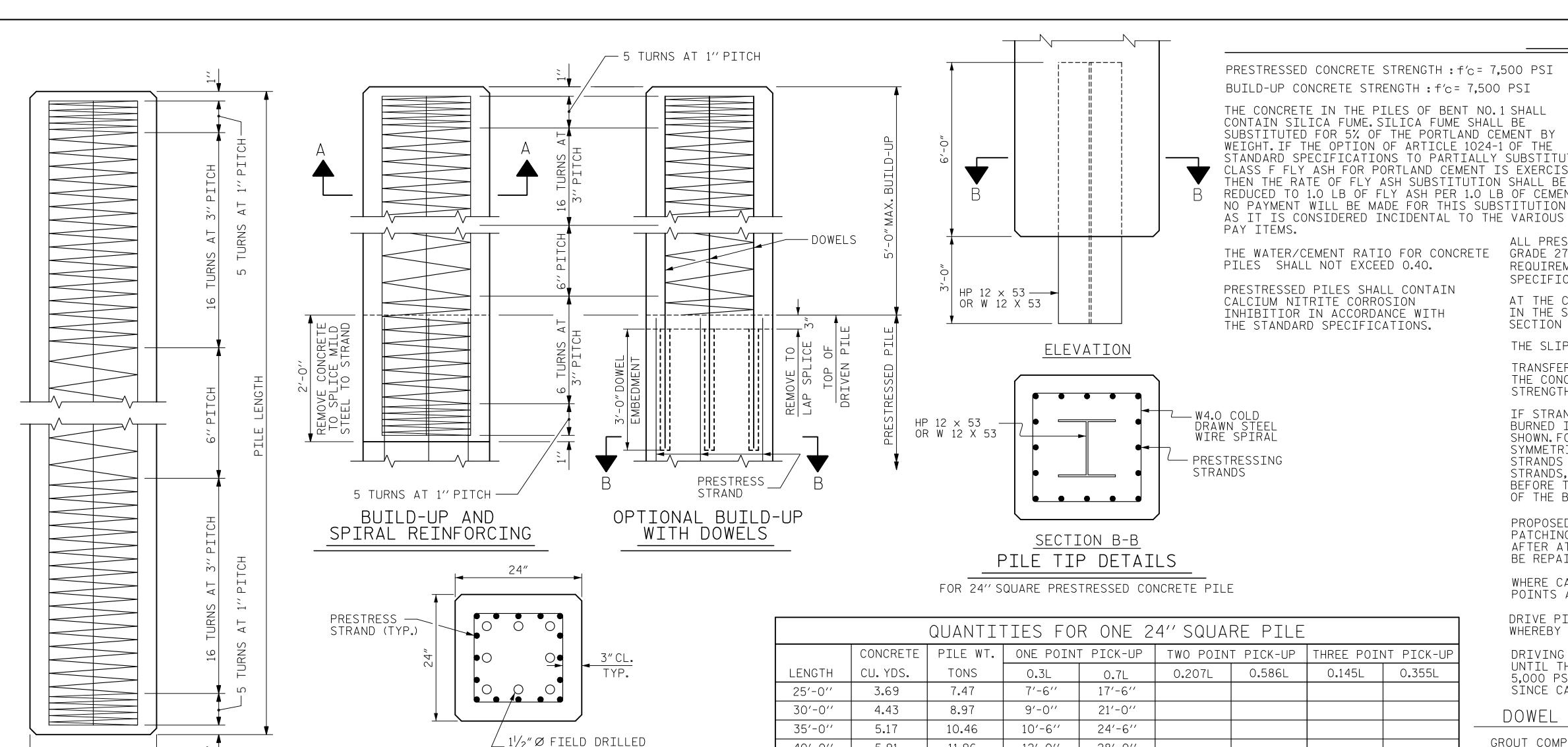
ASSEMBLED BY : M. WRIGHT

DRAWN BY: WJH 1/89

CHECKED BY: CRK 3/89

CHECKED BY: D. HAWKINS

STRANDS -



HOLE (TYP.) W/ #9 DOWEL.

1" TYP.

BARS-

3" CL. TO

SECTION A-A

WIRE SPIRAL

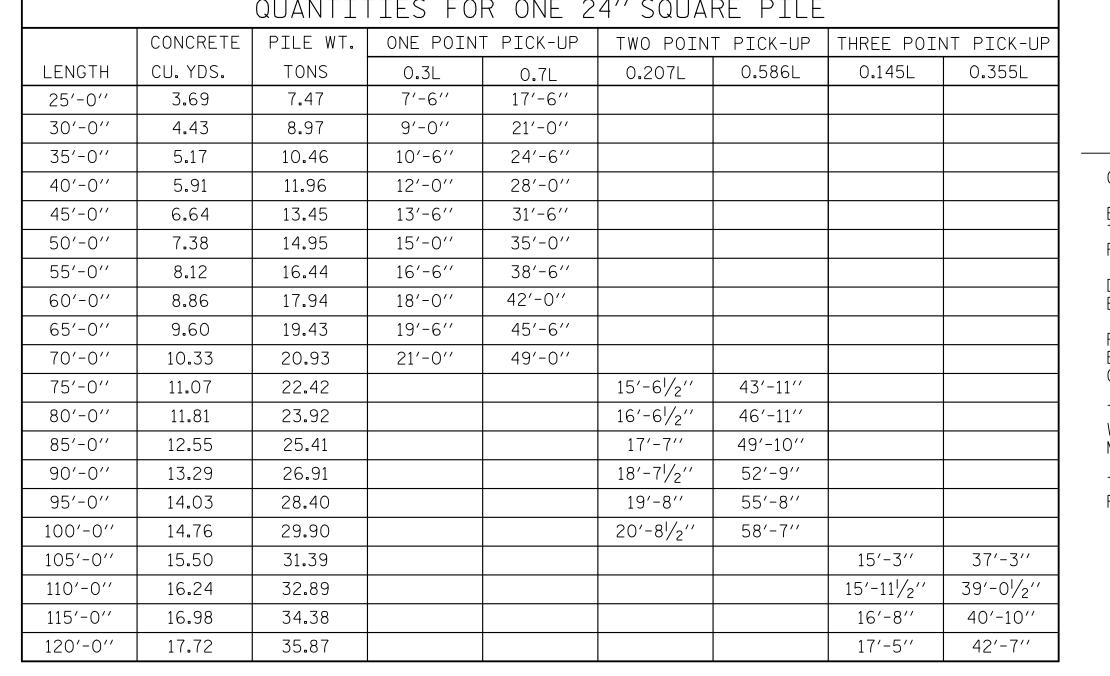
SECTION "B-B"

(AT THE CONTRACTOR'S OPTION, PILE

BUILD-UP MAY BE CONSTRUCTED WITH DOWELS.)

W4.0 COLD DRAWN

STEEL WIRE SPIRAL —



NOTES

PRESTRESSED CONCRETE STRENGTH : f'c = 7,500 PSI STRAND DATA:

THE CONCRETE IN THE PILES OF BENT NO.1 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB OF CEMENT.

APPLIED ULTIMATE SIZE GRADE AREA PRESTRESS STRENGTH FORCE 30,980# 41,300# 270 L.R. 0.153 PER STRAND PER STRAND 58,600# 43,940# 270 L.R. PER STRAND PER STRAND

THE WATER/CEMENT RATIO FOR CONCRETE PILES SHALL NOT EXCEED 0.40.

PRESTRESSED PILES SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITIOR IN ACCORDANCE WITH

REQUIREMENTS SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

GRADE 270 STRANDS CONFORMING TO AASHTO M203. STRAND SAMPLING

AT THE CONTRACTOR'S OPTION,  $\frac{1}{2}$ " OR 0.6" STRANDS MAY BE USED IN THE STRAND CONFIGURATION SHOWN IN THE TYPICAL SECTION DETAIL. MIXING OF STRAND SIZE IS NOT ALLOWED.

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION

THE SLIP-FORM METHOD OF CASTING PILES WILL NOT BE PERMITTED.

TRANSFER THE LOAD FROM THE ANCHORAGES TO THE PILE AFTER THE CONCRETE HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI.

IF STRAND STRESS IS RELIEVED BY BURNING, THE STRANDS SHALL BE BURNED IN OPPOSITE PAIRS AS INDICATED IN THE TYPICAL PATTERN SHOWN. FOR ANY NUMBER OF STRANDS, BURN IN OPPOSITE PAIRS AND SYMMETRICALLY ABOUT BOTH THE VERTICAL AND HORIZONTAL AXES, STRANDS 1-1 SHALL BE BURNED BEFORE 2-2, ETC, NOT MORE THAN 4 STRANDS, SAY 5-5 AND 6-6, MAY BE BURNED AT ANY ONE SECTION BEFORE THESE SAME PAIRS OF STRANDS ARE BURNED AT BOTH ENDS OF THE BED AND BETWEEN EACH PAIR OF PILES IN THE BED.

PROPOSED DEVICES FOR LIFTING PILES, RECESS DETAILS, AND PATCHING MATERIAL SHALL BE DETAILED IN SHOP DRAWINGS. AFTER ATTACHMENTS HAVE BEEN REMOVED, OPENINGS SHALL BE REPAIRED SUCH THAT THE APPEARANCE OF THE PILE IS UNIFORM.

WHERE CAST-IN-PLACE LIFTING DEVICES ARE NOT USED, PICK-UP POINTS ARE TO BE INDICATED WITH A 2" WIDE BLACK MARK.

DRIVE PILES USING A METHOD APPROVED BY THE ENGINEER. WHEREBY THE HEAD OF THE PILE IS NOT DAMAGED.

DRIVING OF THE BUILT-UP PILE WILL NOT BE PERMITTED UNTIL THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF 5,000 PSI AND UNTIL A PERIOD OF SEVEN DAYS HAS ELAPSED SINCE CASTING OF THE BUILD-UP.

### DOWEL INSTALLATION FOR OPTIONAL BUILD-UP

GROUT COMPRESSIVE STRENGTH: f'c= 5,000 PSI

BEFORE DRILLING DOWEL HOLES, REMOVE THE UPPER 3"OF CONCRETE FROM THE TOP OF THE PILE WITHOUT DAMAGE TO THE REINFORCING STEEL. THE REMOVAL PLANE SHOULD BE NORMAL TO THE EDGE OF THE PILE.

DOWEL HOLES SHALL BE POSITIONED TO MAINTAIN  $\frac{1}{2}$  CLEAR TO ALL EXISTING PRESTRESSING STRANDS IN THE CONCRETÉ PILE.

FIELD DRILLED HOLES SHALL BE CLEAN AND FREE OF ANY OBSTRUCTIONS BEFORE GROUTING OF DOWELS. DOWEL BARS SHALL BE INSTALLED AND GROUTED WITH AN APPROVED NON-SHRINK GROUT.

THE SPIRAL REINFORCING IN ALL BUILD-UPS SHALL BE W4.0 COLD DRAWN WIRE WHICH SHALL BE SECURED TO THE LONGITUDINAL REINFORCEMENT TO MAINTAIN PITCH.

THE SPIRAL REINFORCING IN THE BUILD-UP AND THE PRESTRESSED CONCRETE PILE SHALL BE SPLICED BY OVERLAPPING A MIN. OF ONE TURN.

> **PROJECT NO**. \_\_\_\_17BP.2.R.85 BEAUFORT COUNTY 17+39.50 -L-

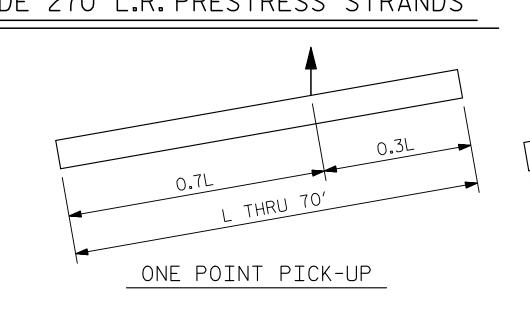
 $\frac{1}{2}$ " OR 0.6" Ø GRADE 270 L.R. PRESTRESS STRANDS

. . . . .

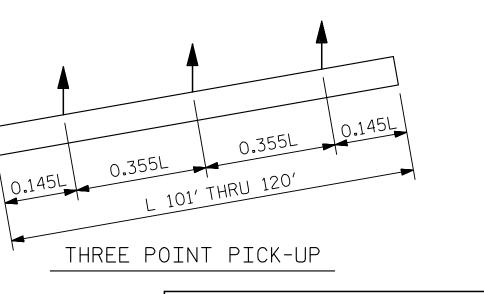
4 1 5

TYPICAL PATTERN

FOR BURNING STRANDS



TWO POINT PICK-UP PICK-UP POINTS



**DOCUMENT NOT CONSIDERED FINAL** 

**UNLESS ALL SIGNATURES COMPLETED** 

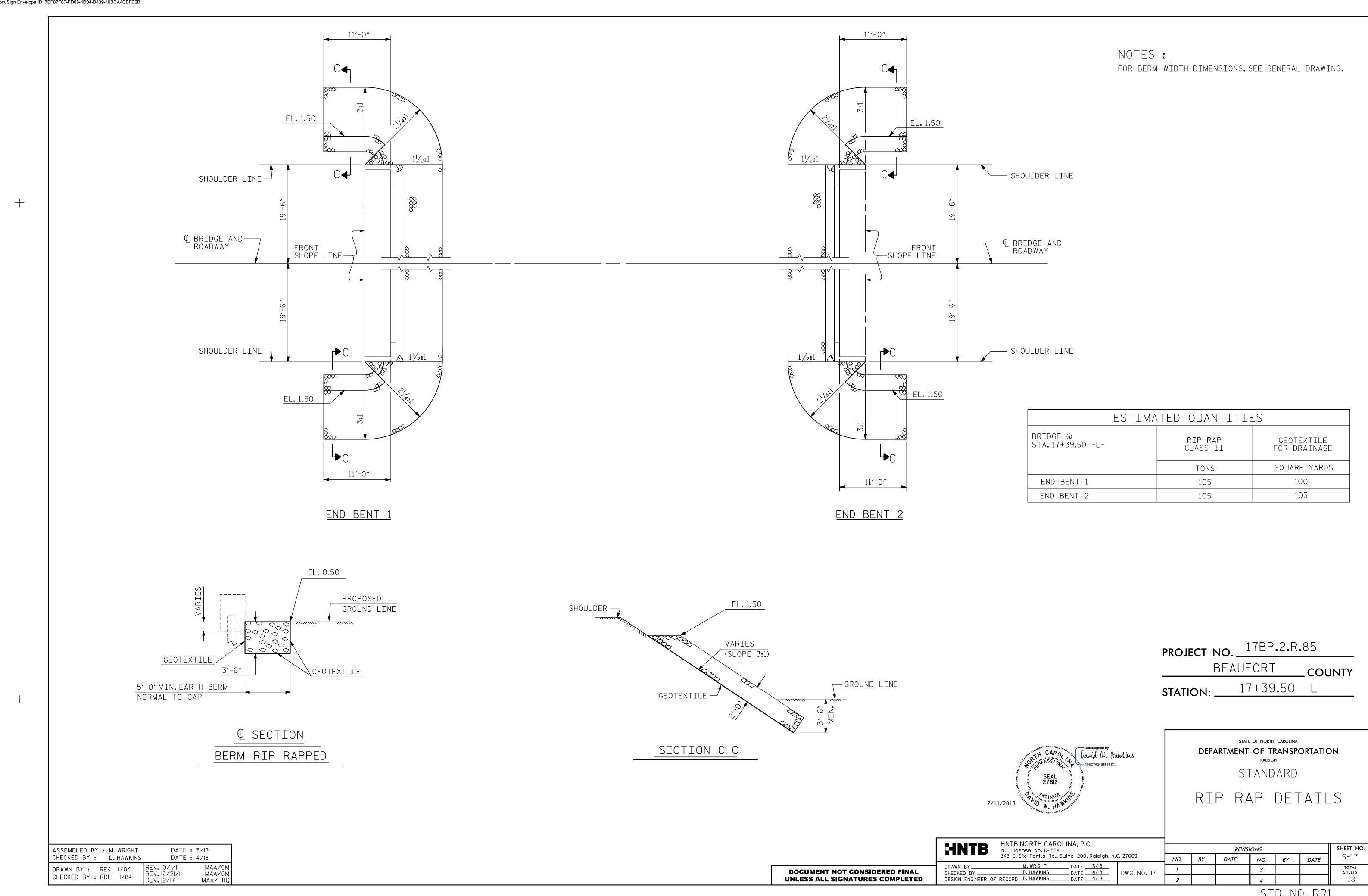
7/11/2018

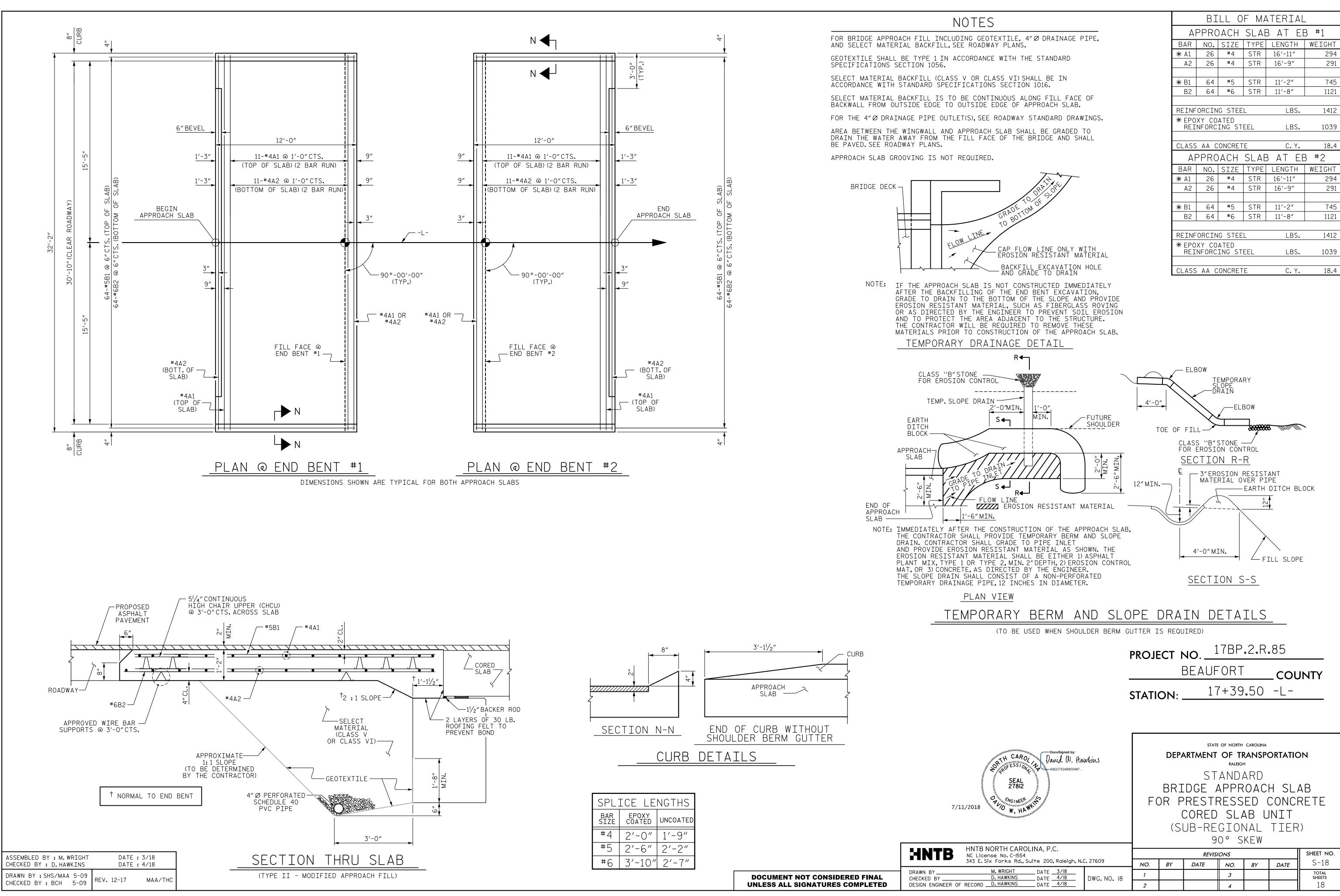
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

24" PRESTRESSED CONCRETE PILE

HNTB NORTH CAROLINA, P.C. SHEET NO. NC License No. C-1554 343 E. Six Forks Rd., Suite 200, Raleigh, N.C. 27609 **REVISIONS** S-16 DATE NO. BY DATE NO. BY \_\_\_ DATE <u>3/18</u> \_\_\_ DATE <u>4/18</u> M. WRIGHT D. HAWKINS DWG. NO. 16 CHECKED BY . DESIGN ENGINEER OF RECORD D. HAWKINS \_\_\_ DATE <u>4/18</u>





## STANDARD NOTES

#### DESIGN DATA:

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# <u>ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:</u>

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE  $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE  $\frac{7}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF  $3-\frac{7}{8}$ " Ø STUDS FOR  $4-\frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF  $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR  $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF  $3-\frac{7}{8}$ " Ø STUDS FOR  $4-\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST \$\frac{1}{16}\textit{"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY /16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH