(1₃) END PROJECT BEGIN PROJECT

VICINITY MAP

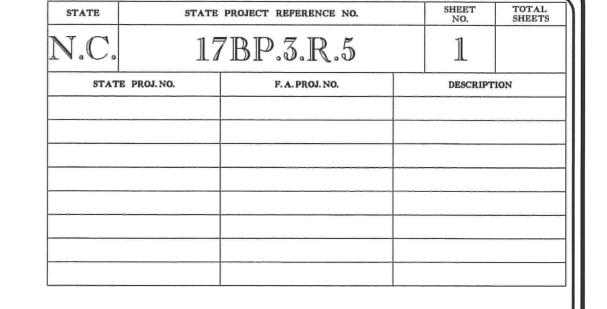
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

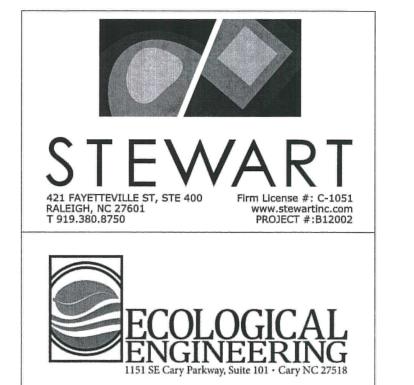
SAMPSON COUNTY

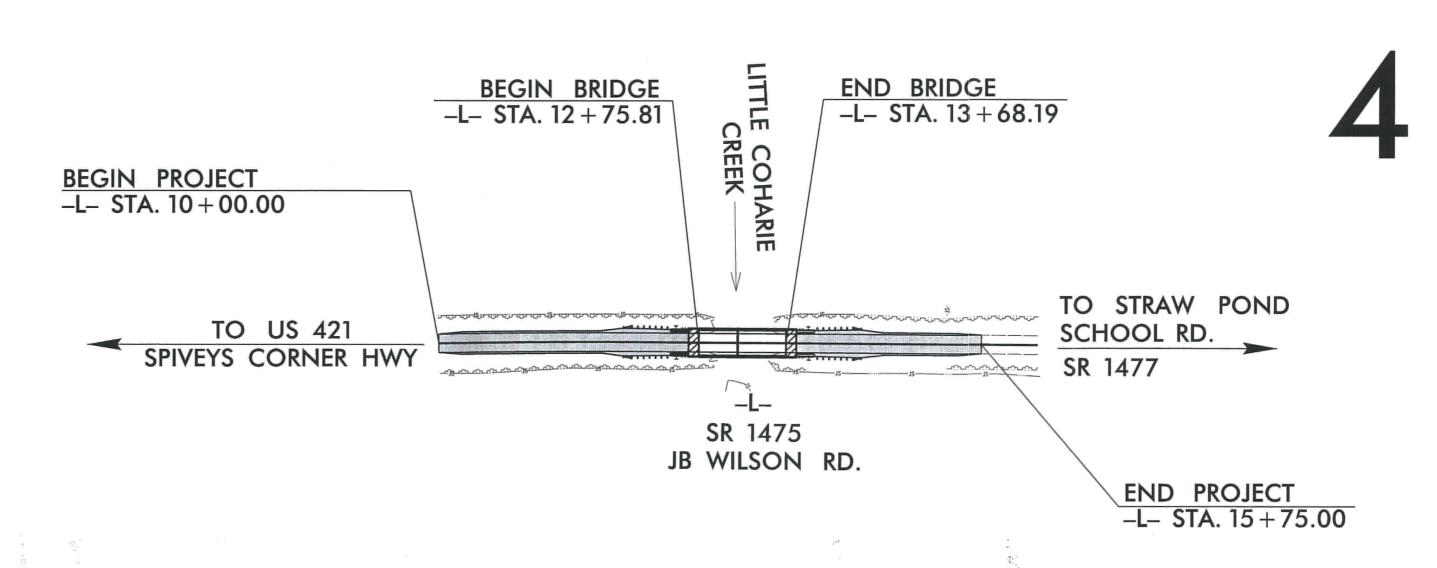
LOCATION: BRIDGE NO. 384 OVER LITTLE COHARIE CREEK ON SR 1475

TYPE OF WORK: GRADING, PAVING, DRAINAGE, AND STRUCTURE









GRAPHIC SCALES **PLANS** PROFILE (HORIZONTAL) PROFILE (VERTICAL)

OFF-SITE DETOUR

See Sheet 1-A For Index of Sheets

See Sheet 1-B For Conventional Symbols

DESIGN DATA

ADT 2009 = 510

ADT 2030 = N/ADHV = NA

D = NA

T = 6% TTST = N/A DUAL = N/A

SUBREGIONAL TIER

V = 55 MPHCLASS = RURAL LOCAL

PROJECT LENGTH

LENGTH ROADWAY PROJECT 17BP.3.R.5 = 0.092 MI LENGTH STRUCTURE PROJECT 17BP.3.R.5 = 0.017 MI TOTAL LENGTH PROJECT 17BP.3.R.5 = 0.109 MI

RIGHT OF WAY DATE: **DECEMBER 4, 2012**

2012 STANDARD SPECIFICATIONS

LETTING DATE: FEBRUARY 6, 2014

For NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STEWART

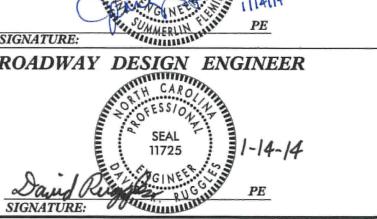
DAVID RUGGLES, PE PROJECT ENGINEER MICHAEL TAYLOR, PE

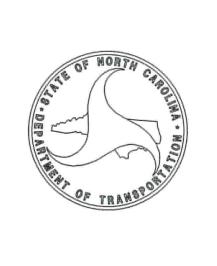
PROJECT DESIGN ENGINEER

AMANDA GLYNN, PE NCDOT CONTACT



SIGNATURE: ROADWAY DESIGN ENGINEER





BRIDGE 810384

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

INDEX OF SHEETS, GENERAL NOTES, AND LIST OF

STANDARD DRAWINGS

PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.5 1–A

R/W SHEET NO.

ROADWAY DESIGN ENGINEER

CARO

SEAL

11725

Firm License No. C-1
421 Fayetteville
Suite
Ralelgh, NC 27
T 919.380.8
www.stewartinc.d

INDEX OF SHEETS

SHEET NUMBER SHEET

1 TITLE SHEET

1-A INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS

-B CONVENTIONAL SYMBOLS

2 PAVEMENT SCHEDULE, TYPICAL SECTIONS, AND WEDGING DETAILS

3 EARTHWORK, DRAINAGE, GUARDRAIL, SHOULDER BERM GUTTER, ASPHALT PAVEMENT REMOVAL, AND ROW DATA SUMMARIES

4 PLAN & PROFILE SHEET

TMP-1 THRU TMP-2 TRANSPORTATION MANAGEMENT PLANS

TMP-3 SIGN DESIGN PLAN

EC-1 THRU EC-4 EROSION CONTROL PLANS

X-1 THRU X-5 CROSS-SECTIONS

S-1 THRU S-17 STRUCTURE PLANS

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch – N. C. Department of Transportation – Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

).NO. TITLE

DIVISION 2 - EARTHWORK 200.03 Method of Clearing - Method II

225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation DIVISION 4 - MAJOR STRUCTURES

422.10 Reinforced Bridge Approach Fills

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

806.01 Concrete Right-of-Way Marker 806.02 Granite Right-of-Way Marker

840.00 Concrete Base Pad for Drainage Structures

840.25 Anchorage for Frames - Brick or Concrete or Precast

840.29 Frames and Narrow Slot Flat Grates

10.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates
10.46 Traffic Bearing Precast Drainage Structure

840.66 Drainage Structure Steps

846.01 Concrete Curb, Gutter and Curb & Gutter

846.04 Drop Inlet Installation in Shoulder Berm Gutter

862.01 Guardrail Placement

362.02 Guardrail Installation

862.03 Structure Anchor Units 876.02 Guide for Rip Rap at Pipe Outlets

Disclaimer: This coordinate list is provided for the convenience of interested contractors and is intended for use during the project bidding process only. Coordinates are localized to this particular project and any conversion to state grid coordinates or other formats will be the resonsibility of the recipient. While every effort has been made to provide up-to-date, accurate information, NCDOT makes on express guarantee as to the validity or potential for revision of this information prior to project letting.

-L- CENTERLINE COORDINATE LIST

Point #	Chain	Station	Northing (Y)	Easting(X)
1	L	11+00.00	519115.9982	2160198.4263
2	L	12+00.00	519100.9924	2160297.2934
3	L	13+00.00	519085.1743	2160396.0344
4	L	14+00.00	519069.3368	2160494.7723
5	L	15+00.00	519053.4993	2160593.5102
6	L	16+00.00	519037.6617	2160692.2481

GENERAL NOTES:

2012 SPECIFICATIONS
EFFECTIVE: 01-17-12
REVISED: 11/01/11

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS, WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

.\Proj\810384_KUY_PSH_UIA.dgn SER:mtaulor

PROJECT REFE	PROJECT REFERENCE NO.						
17BP.3	3.R.5	1B					
R/W	SHEET NO.						

R/W SHEE	T NO.
Firm License No. C 421 Fayettev Suit Raledgh, NC T 919.380 www.stewartin	lle St, e 400 27601 .8750
J L	

Note: Not to Scale

*S.U.E. = Subsurface Utility Engineering

DOLLIND ADJECT	4375	
BOUNDARIES	AND	PROPERTY:

State Line	
County Line	
Township Line	
City Line	
Reservation Line	
Property Line	
Existing Iron Pin	<u></u>
Property Corner	
Property Monument	ECM
Parcel/Sequence Number	
Existing Fence Line	
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	
Existing Endangered Plant Boundary	
Known Soil Contamination: Area or Site	
Potential Soil Contamination: Area or Site —	
BUILDINGS AND OTHER CULT	0 & 0
Gas Pump Vent or U/G Tank Cap Sign	
Well —	-
Small Mine	.,
Foundation —	
Area Outline	<u>- </u>
Cemetery	
School —	
School	
Church — — — — — — — — — — — — — — — — — — —	
Dam —	
HYDROLOGY:	
Stream or Body of Water —	
Hydro, Pool or Reservoir	
Jurisdictional Stream	
Buffer Zone 1	
Buffer Zone 2	— BZ 2 —
Flow Arrow	_
Disappearing Stream —	_>
Spring —	-0
Wetland	−
Proposed Lateral, Tail, Head Ditch ————	
False Sump —	FLOW

CONVENTIONAL PLAN SHEET SYMBOLS

RAILROADS:	
Standard Gauge ————	
RR Signal Milepost ————————————————————————————————————	csx transportation
Switch ————————————————————————————————————	MILEPOST 35
RR Abandoned ————	SWITCH
RR Dismantled	
RIGHT OF WAY:	
Baseline Control Point	•
Existing Right of Way Marker	
Existing Right of Way Line	
Proposed Right of Way Line	
Proposed Right of Way Line with Iron Pin and Cap Marker	$-\frac{R}{W}$
Proposed Right of Way Line with Concrete or Granite Marker	
Existing Control of Access	——————————————————————————————————————
Proposed Control of Access ————	<u> </u>
Existing Easement Line —————	——Е——
Proposed Temporary Construction Easement –	Е
Proposed Temporary Drainage Easement ——	TDE
Proposed Permanent Drainage Easement ——	PDE
Proposed Permanent Drainage / Utility Easement	DUE
Proposed Permanent Utility Easement ———	PUE
Proposed Temporary Utility Easement ———	TUE
Proposed Aerial Utility Easement ————	AUE
Proposed Permanent Easement with Iron Pin and Cap Marker	♦
ROADS AND RELATED FEATURE	
Existing Edge of Pavement	
Existing Curb	
Proposed Slope Stakes Cut	
Proposed Slope Stakes Fill ————	
Proposed Curb Ramp	
Existing Metal Guardrail	
Proposed Guardrail ————————————————————————————————————	
•	
Existing Cable Guiderail	
Proposed Cable Guiderail	
Equality Symbol	
Pavement Removal	
VEGETATION:	
Single Tree	\bigcirc
Single Shrub	\$
Hedge ————	······································
Woods Line	

Orchard Vineyard EXISTING STRUCTURES: MAJOR:		Water Mater Mater Mater Mater Value Water Hy Recorded Designate Above G
Bridge, Tunnel or Box Culvert	CONC	
Bridge Wing Wall, Head Wall and End Wall	-) CONC WW (TV:
MINOR:	,	TV Satell
Head and End Wall	CONC HW	TV Pedes
Pipe Culvert		TV Towe
Footbridge ————————————————————————————————————	>	U/G TV
Drainage Box: Catch Basin, DI or JB	СВ	Recorded
Paved Ditch Gutter		Designate
Storm Sewer Manhole	<u>(S)</u>	Recorded
Storm Sewer —	s	Designate
UTILITIES:		GAS:
POWER:		Gas Valv
Existing Power Pole		Gas Mete
Proposed Power Pole ————		Recorded
Existing Joint Use Pole		Designate
Proposed Joint Use Pole	•	Above G
Power Manhole		
Power Line Tower		SANITARY
Power Transformer ———————————————————————————————————		Sanitary
U/G Power Cable Hand Hole	H _H	Sanitary
H_Frame Pole	••	U/G San
Recorded U/G Power Line	Р ———	Above G
Designated U/G Power Line (S.U.E.*)	P	Recorded Designate
TELEPHONE:		2 331g11a11
Existing Telephone Pole	-—	MISCELLA
Proposed Telephone Pole	-0-	Utility Po
Telephone Manhole		Utility Po
Telephone Booth		Utility Lo
Telephone Pedestal		Utility Tro
Telephone Cell Tower	, I ,	Utility Ur
U/G Telephone Cable Hand Hole ————	H _H	U/G Tan
Recorded U/G Telephone Cable ————	т——т	Undergro
Designated U/G Telephone Cable (S.U.E.*)—	t	A/G Tan
Recorded U/G Telephone Conduit	ТС	Geoenvir
Designated U/G Telephone Conduit (S.U.E.*)		U/G Test
Recorded U/G Fiber Optics Cable ————	T F0	Abandon
Designated U/G Fiber Optics Cable (S.U.E.*)	— — — T FO— — ·	End of In

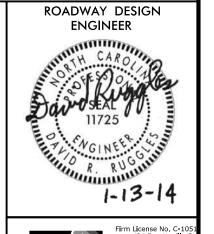
	DIE VVARI
WATER:	·
Water Manhole	
Water Meter	
Water Valve	×
Water Hydrant	
Recorded U/G Water Line	
Designated U/G Water Line (S.	U.E.*)
Above Ground Water Line —	A/G Water
TV:	
TV Satellite Dish	
TV Pedestal ————	
TV Tower	
U/G TV Cable Hand Hole —	——————————————————————————————————————
Recorded U/G TV Cable ——	тү
Designated U/G TV Cable (S.U	J.E.*)
Recorded U/G Fiber Optic Cab	le ти го
Designated U/G Fiber Optic Co	ıble (S.U.E.*)—
GAS:	
Gas Valve	
Gas Meter ———————————————————————————————————	
Recorded U/G Gas Line ——	G
Designated U/G Gas Line (S.U	. E.*) ————————
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	
Sanitary Sewer Cleanout ———	
U/G Sanitary Sewer Line ——	
Above Ground Sanitary Sewer	A/G Sanitary Sewer
Recorded SS Forced Main Line	FSS
Designated SS Forced Main Li	ne (S.U.E.*) — ——————————————————————————————————
MISCELLANEOUS:	
Utility Pole —————	•
Utility Pole with Base ————	
Utility Located Object ————	·
Utility Traffic Signal Box ———	S
Utility Unknown U/G Line —	
U/G Tank; Water, Gas, Oil —	
Underground Storage Tank, Ap	prox. Loc
A/G Tank; Water, Gas, Oil —	
Geoenvironmental Boring ——	
U/G Test Hole (S.U.E.*)	
Abandoned According to Utility	Records — AATUR
End of Information ————	E.O.I.

PROJECT REFERENCE NO. SHEET NO.

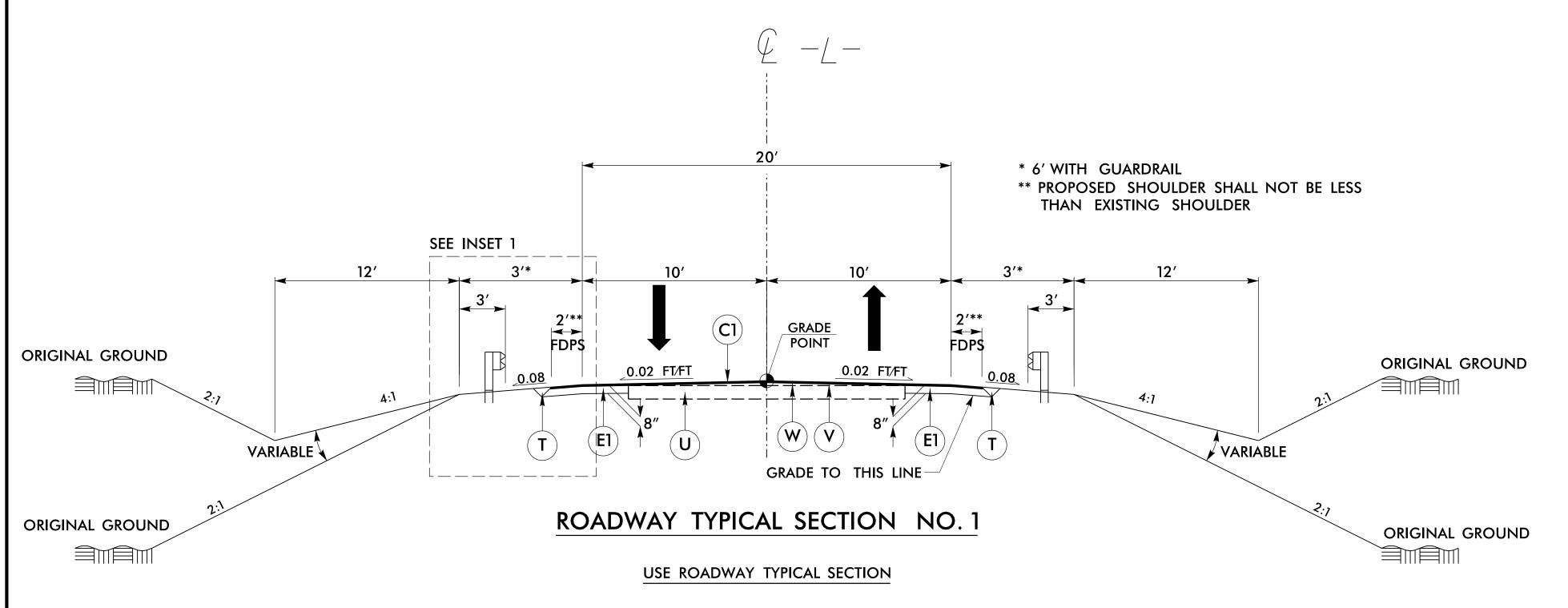
17BP.3.R.5

RW SHEET NO.

SEAL ONLY FOR ROADWAY DESIGN ELEMENTS



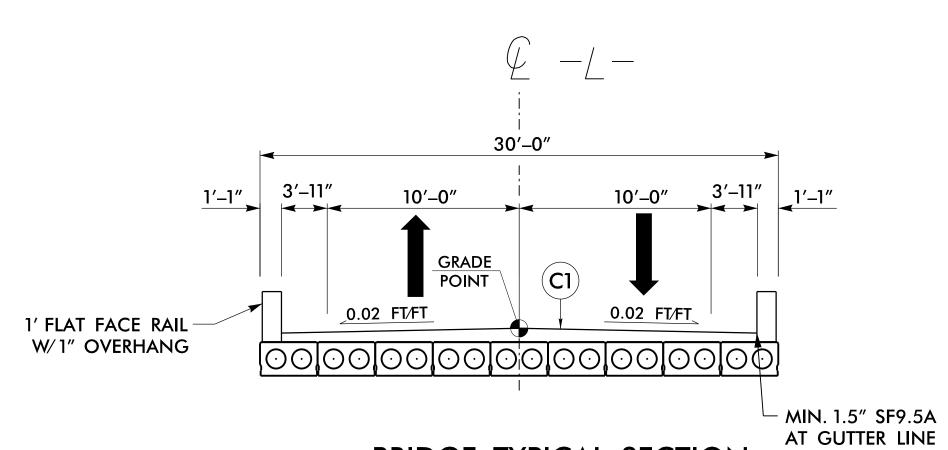




-L- STA. 10+00.00 TO -L- STA. 12+75.81 (BEGIN BRIDGE) -L- STA. 13+68.19 (END BRIDGE) TO -L- STA. 15+75.00

NOTES: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.

SEE STRUCTURE PLANS FOR ASPHALT DEPTHS.

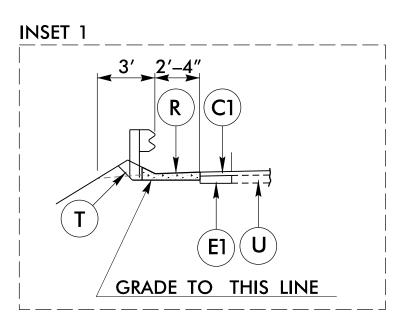


BRIDGE TYPICAL SECTION

USE BRIDGE TYPICAL SECTION

-L- STA. 12+75.81 TO -L- STA. 13+68.19

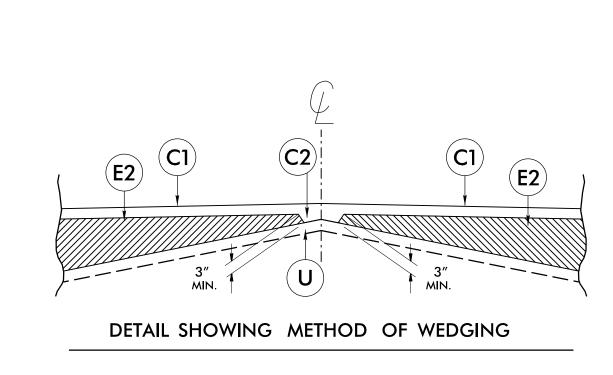
BRIDGE TYPE = CORED SLAB



USE INSET 1

-L- STA. 12 + 45.56 TO -L- STA. 12 + 64.81 (BEGIN APP SLAB) LT & RT -L- STA. 13 + 79.19 (END APP. SLAB) TO -L- STA. 13 + 98.44 LT & RT

	PAVEMENT SCHEDULE
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS
C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT TO EXCEED 1½" IN DEPTH.
E1	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 570 LBS. PER SQ. YD.
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5½" IN DEPTH.
R	SHOULDER BERM GUTTER
Т	EARTH MATERIAL.
U	EXISTING PAVEMENT.
V	MILLING
W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE STANDARD WEDGING DETAIL)



BRIDGE 810384

PROJECT REFERENCE NO. 17BP.3.R.5

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

SUMMARY OF QUANTITIES

STATE OF NORTH CAROLINA **DIVISION OF HIGHWAYS** ROADWAY SUMMARY OF QUANTITIES FOR CONTRACT - _____

WORK ZONE SIGNS (STAT)

Item Number	Sec #	Quantity	Unit	Description	Item Number	Sec #	Quantity	Unit	Description	Item Number	Sec #	Quantity	Unit	Description
000100000-N	800	Lump Sum		MOBILIZATION	441000000-E	1110	94	SF	WORK ZONE SIGNS (BARR)	6093000000-E	1661	0.25	TON	FERT FOR REPAIR SEEDING
000400000-N	801	Lump Sum		CONSTRUCTION SURVEYING	4445000000-E	1145	64	LF	BARRICADES (TYPE III)	6096000000-E	1662	50	LB	SEED FOR SUPP SEEDING
029000000-N	SP	Lump Sum		REINF BRG APPR **********************************	481000000-E	1205	4600	LF	PAINT PVMT MARKINGS 4"	6108000000-E	1665	1.25	TON	FERTILIZER TOPDRESSING
0043000000-N	226	Lump Sum		GRADING	490000000-N	1251	8	EA	PERM RAISED PVMT MARKERS	6117000000-N	1675	4	EA	RESPONSE FOR EROS CONTROL
335200000-E	310	28	LF	15" DRAINAGE PIPE	600000000-E	1605	1100	LF	TEMPORARY SILT FENCE	8035000000-N	402	1	LS	REMV EXIST STR ******** (-L- STA. 13+22.00)
.33000000-E	607	220	SY	INCIDENTAL MILLING	6012000000-E	1610	10	TON	SEDIMENT CONTROL STONE	8112730000-N	450	3	EA	PDA TESTING
489000000-E	610	340	TON	ASP CONC BASE CRS B25.0B	6015000000-E	1615	1.5	ACR	TEMPORARY MULCHING	8121000000-N	412	1	LS	UNCL STR EXCAV STA ***** (-L- STA. 13+22.00)
.525000000-E	610	310	TON	ASP CONC SURF CRS SF9.5A	6018000000-E	1620	100	LB	SEED FOR TEMP SEEDING	8182000000-E	420	49.9	CY	CLASS A CONCRETE (BRIDGE)
L575000000-E	620	36	TON	ASP FOR PLANT MIX	6021000000-E	1620	0.5	TON	FERT FOR TEMP SEEDING	821000000-N	422	1	LS	BRG APPR SLAB ************************************
2286000000-N	840	2	EA	MASNRY DRAINAGE STRUCT	6024000000-E	1622	200	LF	TEMPORARY SLOPE DRAINS	8217000000-E	425	6857	LB	REINF STEEL (BRIDGE)
355000000-N	840	2	EA	FRAME W/GRATE 840.29 STD	6029000000-E	SP	1170	LF	SAFETY FENCE	8364000000-E	450	575	LF	HP12X53 PILES
.556000000-E	846	77	LF	SHOULDER BERM GUTTER	603800000-E	SP	1250	SY	PERM SOIL REINF MAT	8384200000-E	450	490	LF	HP14X73 GALV PILES
3030000000-E	862	50	LF	STL BM GUARDRAIL	6042000000-E	1632	50	LF	1/4" HARDWARE CLOTH	8393000000-N	450	9	EA	PILE REDRIVES
3150000000-N	862	3	EA	ADDIT GUARDRAIL POSTS	6048000000-E	SP	115	SY	FLOAT TURBIDITY CURTAIN	8505000000-E	460	180.5	LF	VERT CONC BARRIER RAIL
3215000000-N	862	4	EA	GR ANCHOR TYPE III	6071012000-E	SP	130	LF	COIR FIBER WATTLE	8608000000-E	876	135	TON	RIP RAP II (2'-0")
270000000-N	862	4	EA	GR ANCHOR TYPE 350	6071020000-E	SP	1		POLYACRYLAMIDE (PAM)	8657000000-N	430	1	LS	ELASTOMERIC BEARINGS
649000000-E	876	2	TON	RIP RAP, CLASS B	608400000-E	1660	3	LB	SEEDING AND MULCHING	8762000000-E	430	900	LF	3'-0"X 1'-9"PRESTR SLABS
656000000-E	876	200	SY	GEOTEXTILE FOR DRAINGE	609000000-E	1661	50	ACR	SEED FOR REPAIR SEEDING					

440000000-E

 COMPUTED BY: BRC
 DATE: 6-26-2012

 CHECKED BY: JCH
 DATE: 6-26-2012

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.5 3A
PROJECT REFERENCE NO. SHEET NO.
DROJECT REFERENCE NO. CHEET NO.

Firm License No. C-1051 421 Fayetteville St, Suite 400 Ralegh, NC 27601 T 919.380.8750 www.stewartlnc.com EWART Firm License No. C-1051 421 Fayetteville St, Suite 400 Ralegh, NC 27601 T 919.380.8750 Www.stewartlnc.com

SUMMARY OF EARTHWORK

IN CUBIC YARDS

STATION STATION		UNCL. EXCAV.	EMBANK. +%	BORROW	WASTE
L STA. 10 + 00.00	–L− STA. 12 + 75.81	8	188	180	
L STA. 13 + 68.19	−L− STA. 15 + 75.00	2	89	87	
PROJECT TO	10	277	267		
EST 5% TO REPLACE TOP			14		
GRAND TO	TAL:	10		281	

RIGHT OF WAY AREA DATA

PARCEL NO.	PROPERTY OWNERS NAMES	TOTAL ACREAGE	AREA TAKEN	AREA REMAINING RT.	AREA REMAINING LT.	CONST. EASE.	PERM. DRAIN. EASE.	TEMP. DRAIN. EASE.
1	BETTY ROSE TART	20.86	_	_	_	0.062	_	-
2	JOSEPH A. WILSON	24.36	-	-	24.36	0.031	327 sf	_
3	GAP PARCEL	_	_	_	_	0.028	349 sf	_

PAVEMENT REMOVAL SUMMARY

SURVEY LINE	STATION	STATION	LOCATION LT/RT/CL	SY
-L-	12 + 64.81	12 + 98.37	CL	81
-L-	13 + 49.87	13 + 79.19	CL	73
			TOTAL:	154

SHOULDER BERM GUTTER SUMMARY

SURVEY LINE	STATION	STATION	LOCATION	LENGTH (FT)
-L-	12 + 45.56	12 + 64.81	LT	19.25
-L-	12 + 45.56	12 + 64.81	RT	19.25
-L-	13 + 79.19	13 + 98.44	LT	19.21
-L-	13 + 79.19	13 + 98.44	RT	19.21
			TOTAL:	76.92

SUB-REGIONAL & REGIONAL LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

STATION	IN (LT,RT, OR CL) STRUCTURE NO.	ATION	LEVATION	EEVATION	(Re	DRAIN CP, CSP, CA	NAGE PIPE AAP, HDPE,	or PVC)		(UN	C.S ILESS NO	S. PIPE TED OTH	HRWISE)			(l	CL/ UNLESS	ASS III R. NOTED	.C. PIPE OTHERW	(ISE)			STD. 838. STD. 838. OR STD. 838. (UNLES NOTED OTHERWI	3.01, 8.11 3.80 SS D	# E AU.02 * AU.02 * AU.02 * AUANTITES * TOTAL L.F. FOR PAY * A' + (1.3 X COL.'B')	│ △	AME, GRATES ND HOOD NDARD 840.03	I. STD. 840.35	I TWO GRATES STD. 840.29	SWC .				ABBREVIATIONS C.B. CATCH BASIN N.D.I. NARROW DROP INLET D.I. DROP INLET G.D.I. GRATED DROP INLET G.D.I. (N.S.) GRATED DROP INLET (NARROW SLOT)
SIZE	-OCATIO	TOP ELEY	NVERT EL	NVERT E	12"	15" 18" 24	4" 30" 36	5" 42"	12" 15	5" 18"	24"	30″	36″	42"	48"	12" 15	5" 18" :	24" 30"	36" 42"	48"	PIPE PIPE	PIPE	CU. YDS	S.	RU 5.0′) B B OR SI			G G.D.	ME WITH	NPE ELBC				J.B. JUNCTION BOX M.H. MANHOLE
THICKNESS OR GAUGE	I FROM TO		_	_					DO NOT USE R .064	.064	t	6/0.	.079	.109	.109						SIDE	24" SIDE DRAIN	R.C.P.	C.S.P.	5.0' THRU 10.0' 10.0' AND ABOVI C.B. STD. 840.01		PE OF GRATE	TRAFFIC BEARIN	G.D.I. (N.S.) FRA/	15" DRAINAGE P				T.B.D.I. TRAFFIC BEARING DROP INLET T.B.J.B. TRAFFIC BEARING JUNCTION BOX REMARKS
13 + 89.00	RT 0401	164.38																						-	1			1	1					
13 + 89.00	RT 0401 0402		161.63	160.50					14	1																								
13 + 89.00	LT 0403	164.38																							1			1	1					
13 + 89.00	LT 0403 0404		161.63	160.00					14	1																								
		<u> </u>	<u> </u>	TOTALS					28	3											+				2			2	2					

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

G = GATING IMPACT ATTENUATOR TYPE 350

NG = NON-GATING IMPACT ATTENUATOR TYPE 350

DEDUCTIONS FOR ANCHOR UNITS

ADDITIONAL GUARDRAIL POSTS = 3

275

50

DEDUCTIONS FOR ANCHOR UNITS

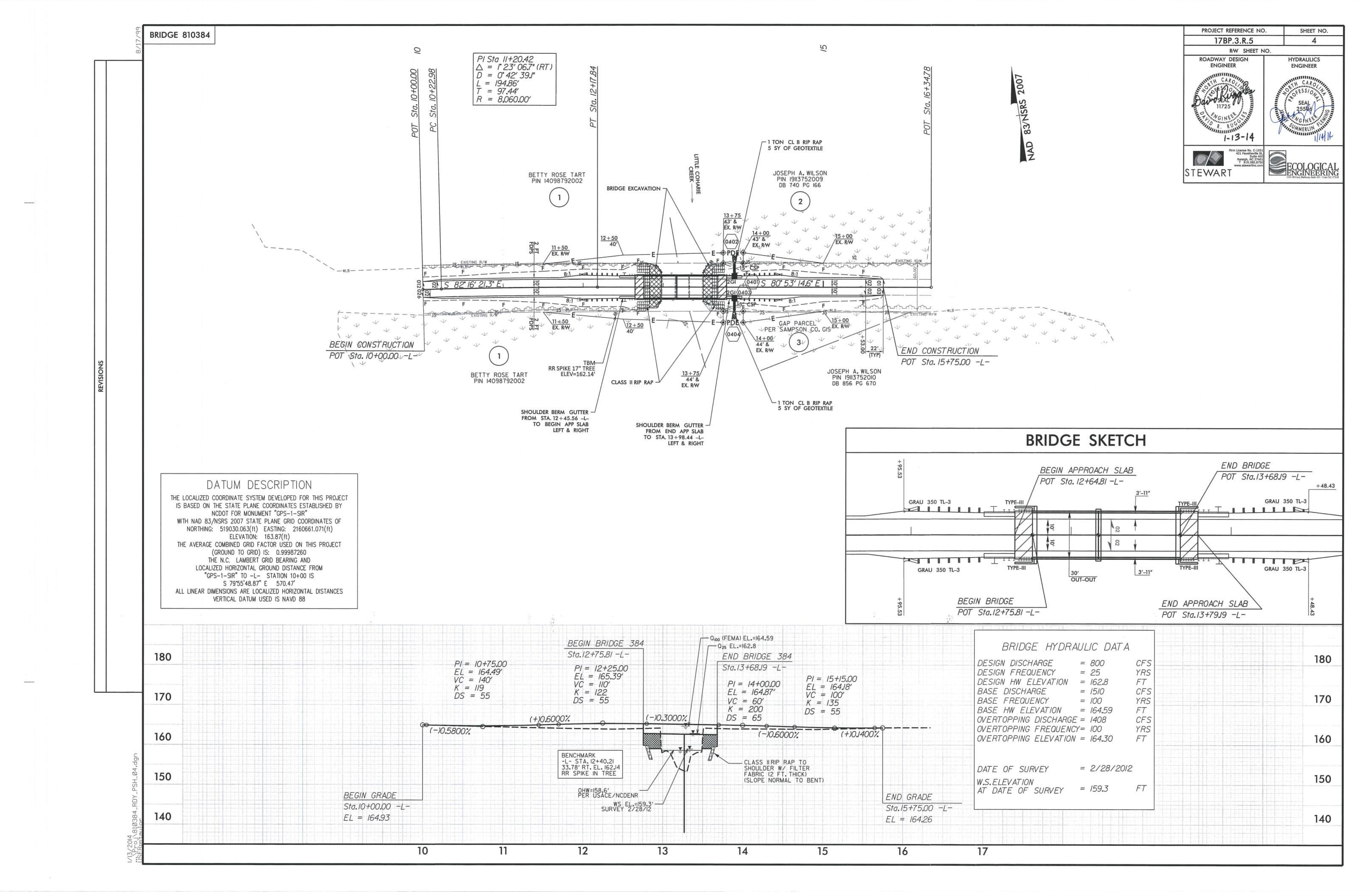
TYPE III @ 18.75' =

GRAU 350 @ 50' = 200

TOTAL = 275

GUARDRAIL SUMMARY

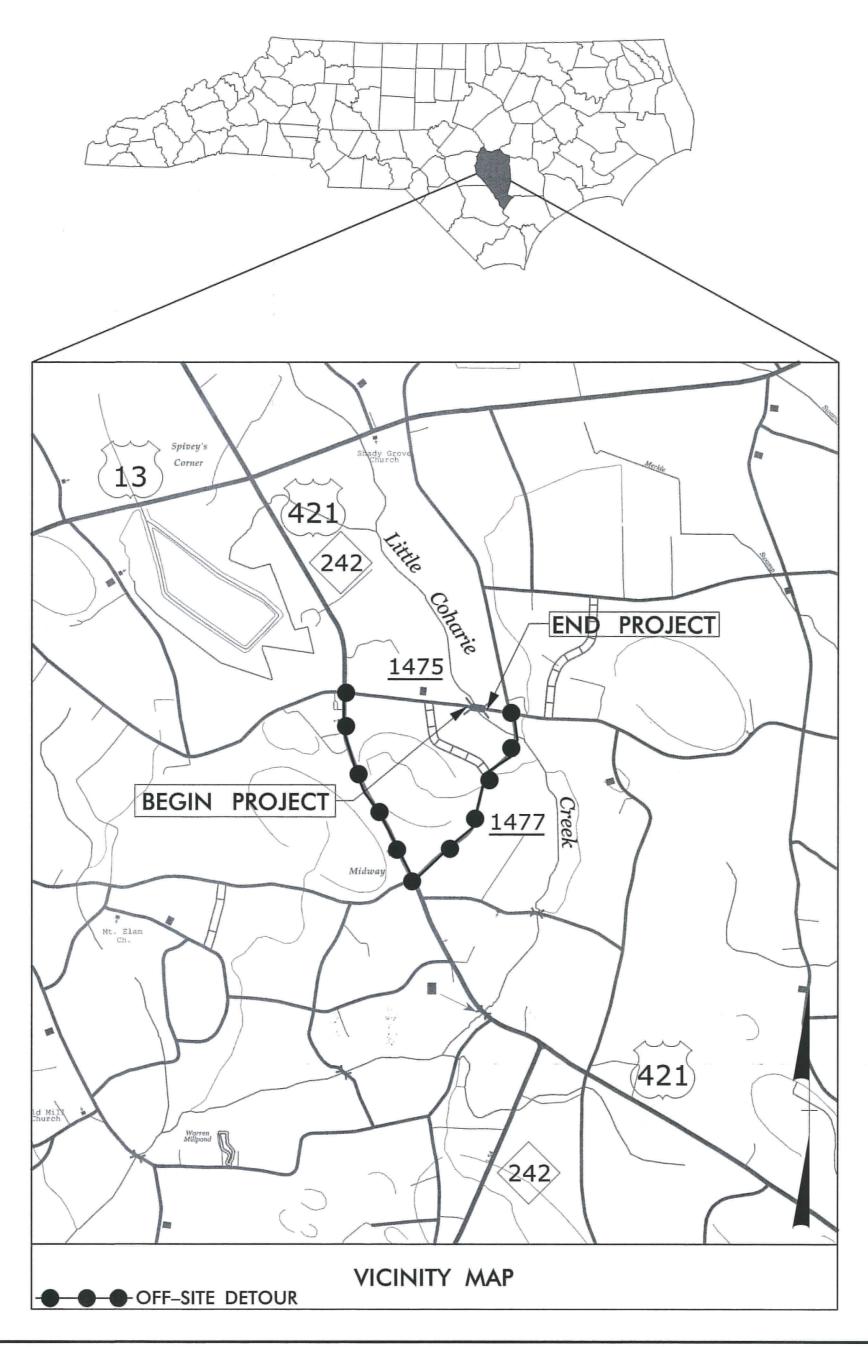
	.,,																										
	SURVEY	DEC STA	5) ID . 674	LOCATION		LENGTH		WARRA	ANT POINT	"N" DIST.	TOTAL	FLARE	LENGTH	٧	W			ANCHORS			A	IMPACT ATTENUATOR TYPE 350	SINGLE FACED	REMOVE EXISTING	REMOVE AND STOCKPILE		
	LINE	BEG. STA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	SHOUL. WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD	XI GRAL 350	III	CAT-1 VI MOD	BIC	AT-1 -	TYPE 350 EA G NO	GUARDRAIL		EXISTING GUARDRAIL	REMARKS	
	-L-	11 + 95.53	BRIDGE (12 + 75.81)	RT	81.25					3′–11″	6′–11″		62.5′		1.25′		1	1									
	-L-	11 + 95.53	BRIDGE (12 + 75.81)	LT	81.25					3′–11″	6′–11″	62.5′		1.25′			1	1									
г Б	-L-	BRIDGE (13 + 68.19)	14 + 48.43	RT	81.25					3′–11″	6′–11″	62.5′		1.25′			1	1									
₩	-L-	BRIDGE (13 + 68.19)	14 + 48.43	LT	81.25					3′–11″	6′–11″		62.5′		1.25′		1	1									
() Ø -																											
SSH	TOTAL				325												4	4									
				TOTAL	225																						



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

TRANSPORTATION MANAGEMENT PLAN

SAMPSON COUNTY





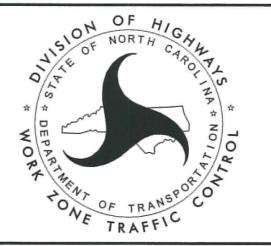
N.C.D.O.T. WORK ZONE TRAFFIC CONTROL

1561 MAIL SERVICE CENTER (MSC) RALEIGH, NC 27699-1561

750 N. GREENFIELD PARKWAY, GARNER, NC 27529 (DELIVERY)

PHONE: (919) 773-2800 FAX: (919) 771-2745

KATHERINE HITE, PE DIVISION TRAFFIC ENGINEER



INDEX OF SHEETS

SHEET NO.

TITLE

TMP-1

TITLE SHEET, VICINITY MAP, INDEX OF SHEETS, AND LIST OF APPLICABLE ROADWAY STANDARD

DRAWINGS AND LEGEND

TMP-1A

TRANSPORTATION OPERATIONS PLAN: (GENERAL NOTES, MANAGEMENT STRATEGIES, AND

3

PHASING)

TMP-2

OFF-SITE DETOUR

TMP-3

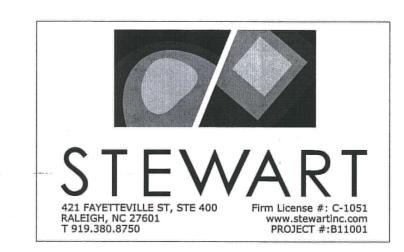
SPECIAL SIGN DESIGN

ROADWAY STD. DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANAUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO. TITLE

1101.03 1101.11 1110.01 1145.01	TEMPORARY ROAD CLOSURES TRAFFIC CONTROL DESIGN TABLES STATIONARY WORK ZONE SIGNS BARRICADES
1205.01 1205.02 1205.12 1250.01 1251.01 1261.01 1261.02 1262.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS PAVEMENT MARKINGS - TWO LANE AND MULTILANE ROADWAYS PAVEMENT MARKINGS - BRIDGES RAISED PAVEMENT MARKERS - INSTALLATION SPACING RAISED PAVEMENT MARKERS - (PERMANENT AND TEMPORARY) GUARDRAIL AND BARRIER DELINEATORS SPACING GUARDRAIL AND BARRIER DELINEATORS TYPE GUARDRAIL END DELINEATION



DAVID RUGGLES, PE TRAFFIC CONTROL PROJECT ENGINEER

MICHAEL TAYLOR, PE TRAFFIC CONTROL DESIGN ENGINEER

APPROVED: SEAL

SEAL

SEAL

SEAL

SEAL

...\TCP\810384_TC_TCP_01_tsh.dgn USER:mtaylor

PROJ. REFERENCE NO.	SHEET NO.
17BP.3.R.5	TMP-1A

GENERAL NOTES / LOCAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

- B) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.
 - PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.
- D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.
 - COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.
- E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

G) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME	MARKINGS	MARKERS
SR 1475 (JB WILSON ROAD)	PATNT	RATSED

H) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

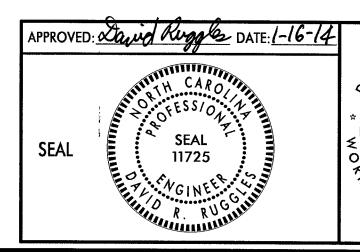
MANAGEMENT STRATEGIES

- CLOSE SR 1475 (JB WILSON ROAD) TO THROUGH TRAFFIC BETWEEN US 421 AND SR 1477.
- DIRECT THROUGH TRAFFIC TO OFF SITE DETOUR.
- MAINTAIN LOCAL TRAFFIC.

PHASING

- STEP 1 INSTALL DETOUR SIGNING AS SHOWN ON SHEET TMP-2 IN ACCORDANCE WITH ROADWAY STANDARD DRAWING 1101.03, SHEET 1 OF 9. SIGNS SHALL BE COVERED IF DETOUR IS NOT OPENED WITHIN 3 DAYS OF SIGN INSTALLATION.
- STEP 2 INSTALL BARRICADES AND CLOSE SR 1475 (JB WILSON ROAD) TO THROUGH TRAFFIC AS SHOWN ON TMP-2 AND IN ACCORDANCE WITH ROADWAY STANDARD DRAWING 1101.03, SHEET 1 OF 9.
- STEP 3 PERFORM THE FOLLOWING WORK WITHIN THE ROAD CLOSURE:
 - REMOVE THE EXISTING STRUCTURE.
 - CONSTRUCT THE PROPOSED STRUCTURE.
 CONSTRUCT THE PROPOSED ROADWAY SECTION -L- UP TO AND INCLUDING THE
 - FINAL LAYER OF SURFACE, THE FINAL PAVEMENT MARKINGS, AND THE FINAL PAVEMENT MARKERS FROM -L- STA. 10+00 TO -L- STA. 15+75.
- STEP 4 REMOVE ALL ROAD CLOSURE SIGNS AND BARRICADES AND OPEN SR 1475 (JB WILSON ROAD) TO THROUGH TRAFFIC.



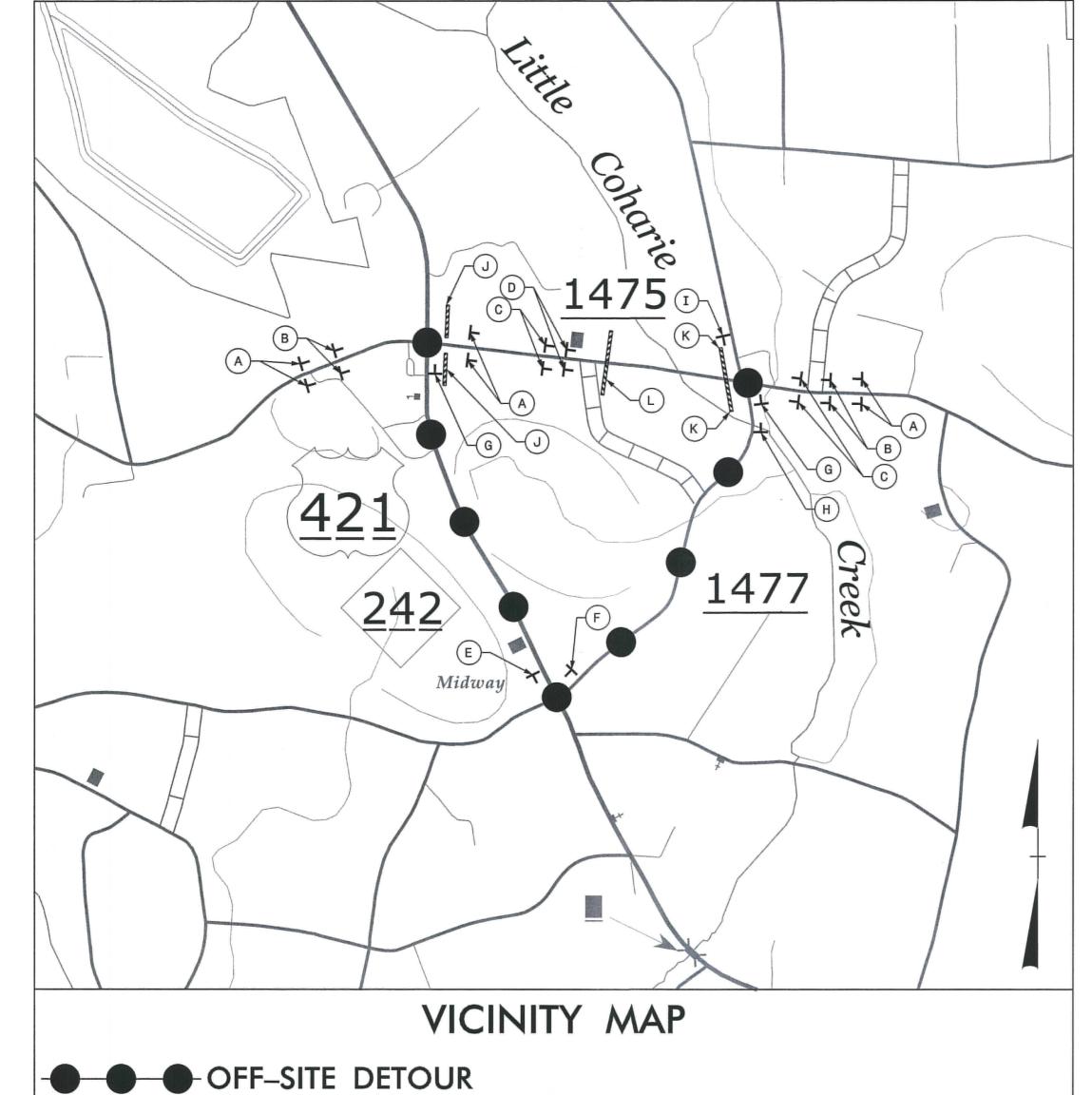


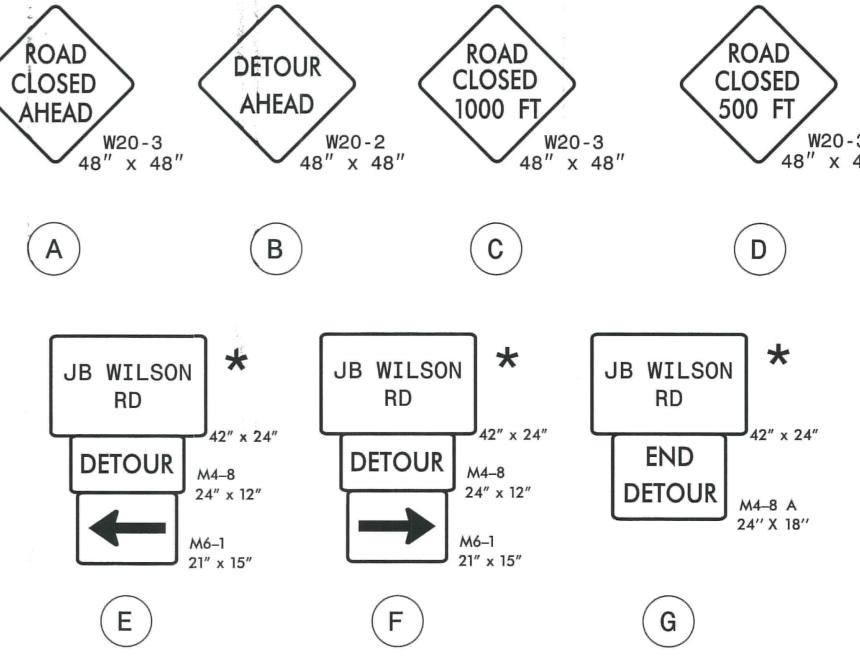


TRANSPORTATION OPERATIONS PLAN

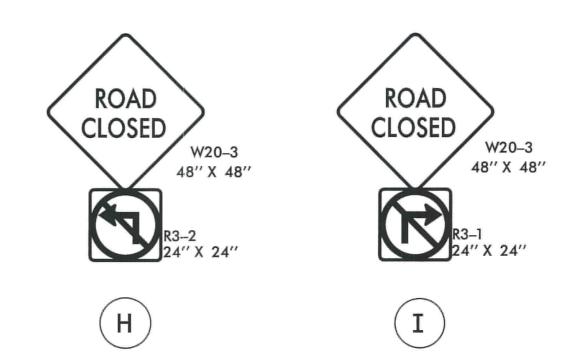
:F\810384_1C_1CP_01A.dgn :m†aylor

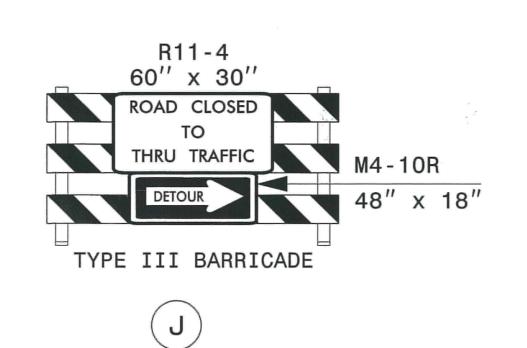


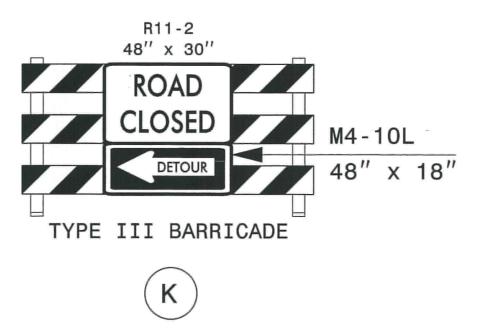


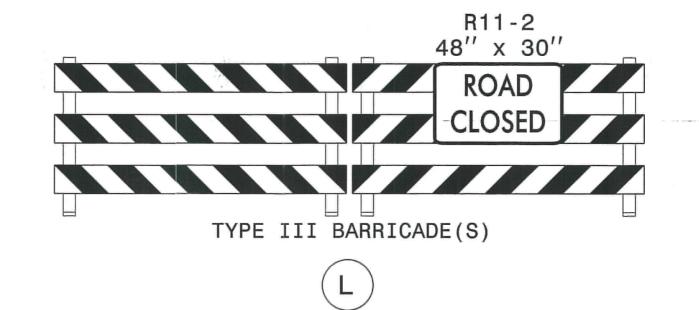


★ SEE SHEET SD-1 FOR SIGN DESIGN

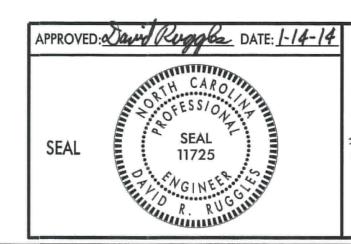


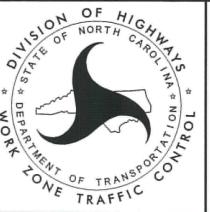












TRANSPORTATION
OPERATIONS
PLAN

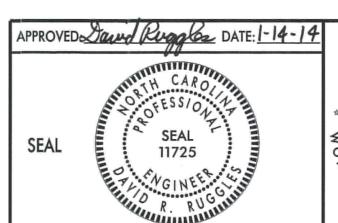
PROJ. REFERENCE NO. SHEET NO. 17BP.3.R.5 TMP-3

BACKG COLOR: Fluorescent Orange SIGN NUMBER: TMP-3 DESIGN BY: MSB CHECKED BY: DATE: May 29, 2013 COPY COLOR: Black TYPE: STATIONARY DIV: 3 PROJECT ID: 17BP.3.R.5 QUANTITY: 4 SYMBOL X Y WID HT SIGN WIDTH: 3'-6" 3'-6" HEIGHT: 2'-0" TOTAL AREA: 7.0 Sq.Ft. **BORDER TYPE: INSET** RECESS: 0.5" WIDTH: 0.5" RADII: 3" WILSON MAT'L: 0.080" (2.0 mm) ALUMINUM NO. Z BARS: LENGTH: USE NOTES: 1,2 Legend and border shall be direct applied black non-reflective sheeting. Background shall be NC GRADE B fluoresent orange retroreflective sheeting. BORDER R=3" TH=0.5" 5.45" 31.1" 5.45" 1N=0.5" Panel Style: Traffic Control.ssi M.U.T.C.D.: 2009 Edition Spacing Factor is 1 unless specified otherwise

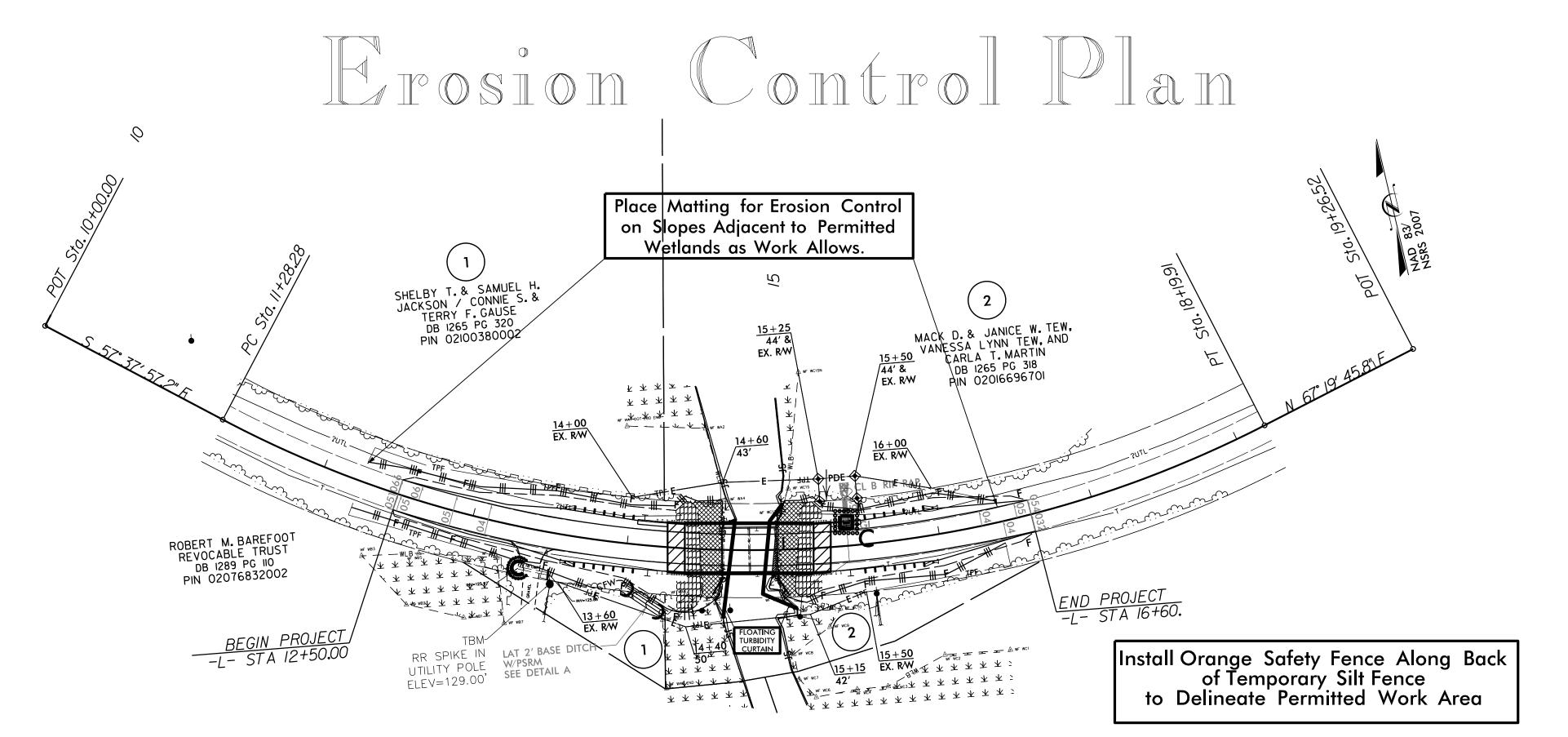
LETTER POSITIONS

						1	Lett	er 1	.oca	tion	s a	re p	anel	edge	e to	low	/er	left	cor	ner					Series Text L
J	В		W	I	L	S	0	N				<u> </u>													C 2
5.5	9.1	11.9	16.9	21.4	23.2	26.2	29.8	33.7			-	-													31
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	-	2 9670 - 26		-					<u>.</u>	<i>3</i> ′				-							# I	19,		_	
													140												









Std.#	<u>Description</u> <u>Symbol</u>
1605.01	Temporary Silt Fence — — — — — — — — — — — — — — — — — — —
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Drains T
1630.02	Silt Basin Type B
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion TD
1630.06	Special Stilling Basin
1632.03	Rock Inlet Sediment Trap Type C
1633.01	Temporary Rock Silt Check Type-A
	Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)
1633.02	Temporary Rock Silt Check Type-B
	Wattle
	Wattle with Polyacrylamide (PAM)
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A

Hang Orange Flagging Along Wattle Barrier Under Bridge to Delineate Permitted Work Area 50 25 0 50 100

	PROJECT REFERENCE NO).	SHEET NO.
) [17BP.3.R.6		EC-01/CONST.04
	R/W SHEET N	10.	
	ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

Jenny Fleming, PE
LEVEL IIIA NAME

3340
LEVEL III CERTIFICATION NO.

ROADSIDE ENVIRONMENTAL UNIT DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS RALEIGH, N.C.

2012 STANDARD SPECIFICATIONS

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY
WITH THE REGULATIONS SET FORTH BY THE
NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011
ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND
NATURAL RESOURCES DIVISION OF WATER QUALITY.

NOTES: ANY DEVIATION FROM OPTIONS GIVEN WILL REQUIRE PRIOR APPROVAL BY ENGINEER.

ADDITIONAL EROSION CONTROL DEVICES MAY NEED TO BE INSTALLED AS DIRECTED BY THE ENGINEER.

2012 STANDARD DRAWINGS

1605.01 Temporary Silt Fence
1606.01 Special Sediment Control Fence
1607.01 Gravel Construction Entrance
1622.01 Temporary Berms and Slope Drains
1630.01 Riser Basin
1630.02 Silt Basin Type B
1630.03 Temporary Silt Ditch
1630.04 Stilling Basin
1630.05 Temporary Diversion

1630.06 Special Stilling Basin

1631.01 Matting Installation

1604.01 Railroad Erosion Control Detail

1632.01 Rock Inlet Sediment Trap Type A
1632.02 Rock Inlet Sediment Trap Type B
1632.03 Rock Inlet Sediment Trap Type C
1633.01 Temporary Rock Silt Check Type A
1633.02 Temporary Rock Silt Check Type B
1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle

1645.01 Temporary Stream Crossing

 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.3.R.5
 EC-02

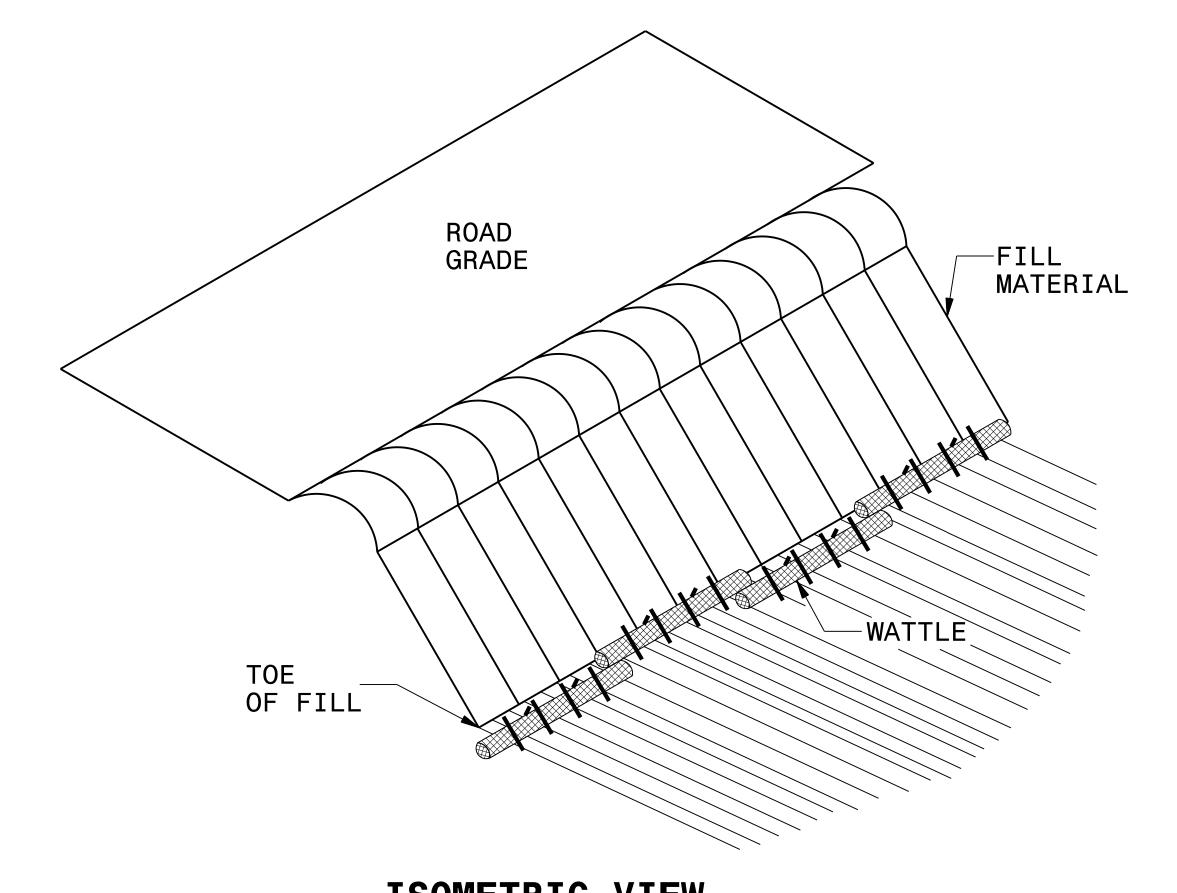
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

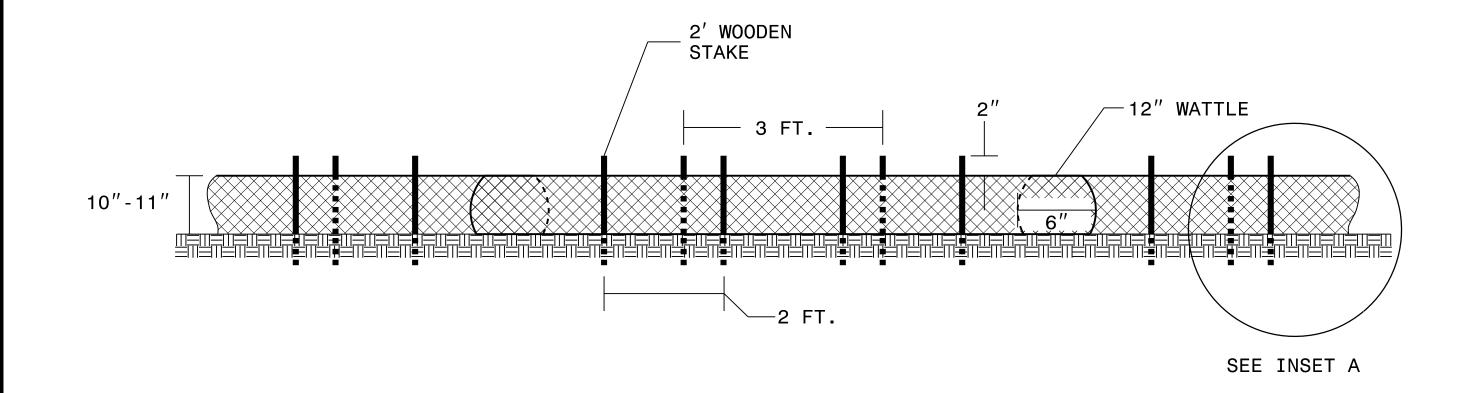
SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1,14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

COIR FIBER WATTLE BARRIER DETAIL

PROJECT REFERENCE NO	SHEET NO.	
17BP.3.R.5		EC-03
R/W SHEET N	10.	
ROADWAY DESIGN ENGINEER	<u></u>	HYDRAULICS ENGINEER



ISOMETRIC VIEW



FRONT VIEW

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

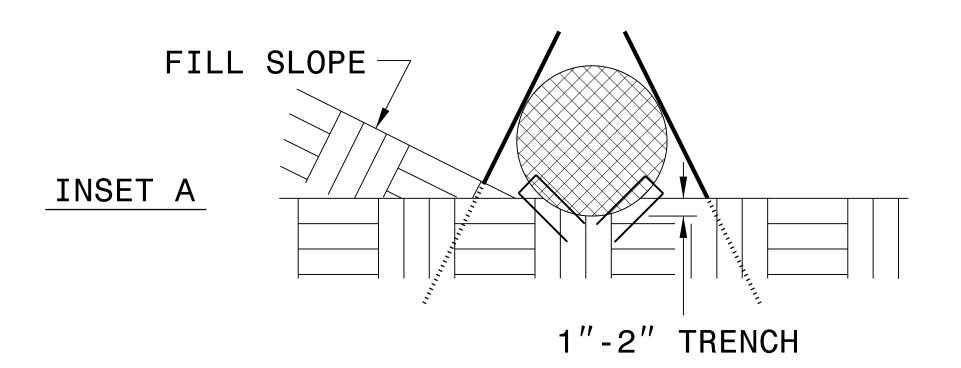
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

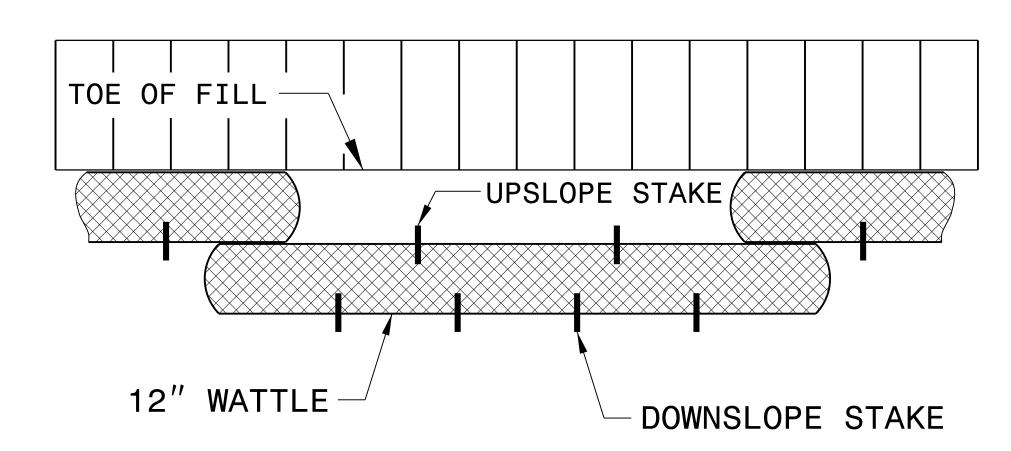
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 20 FT.

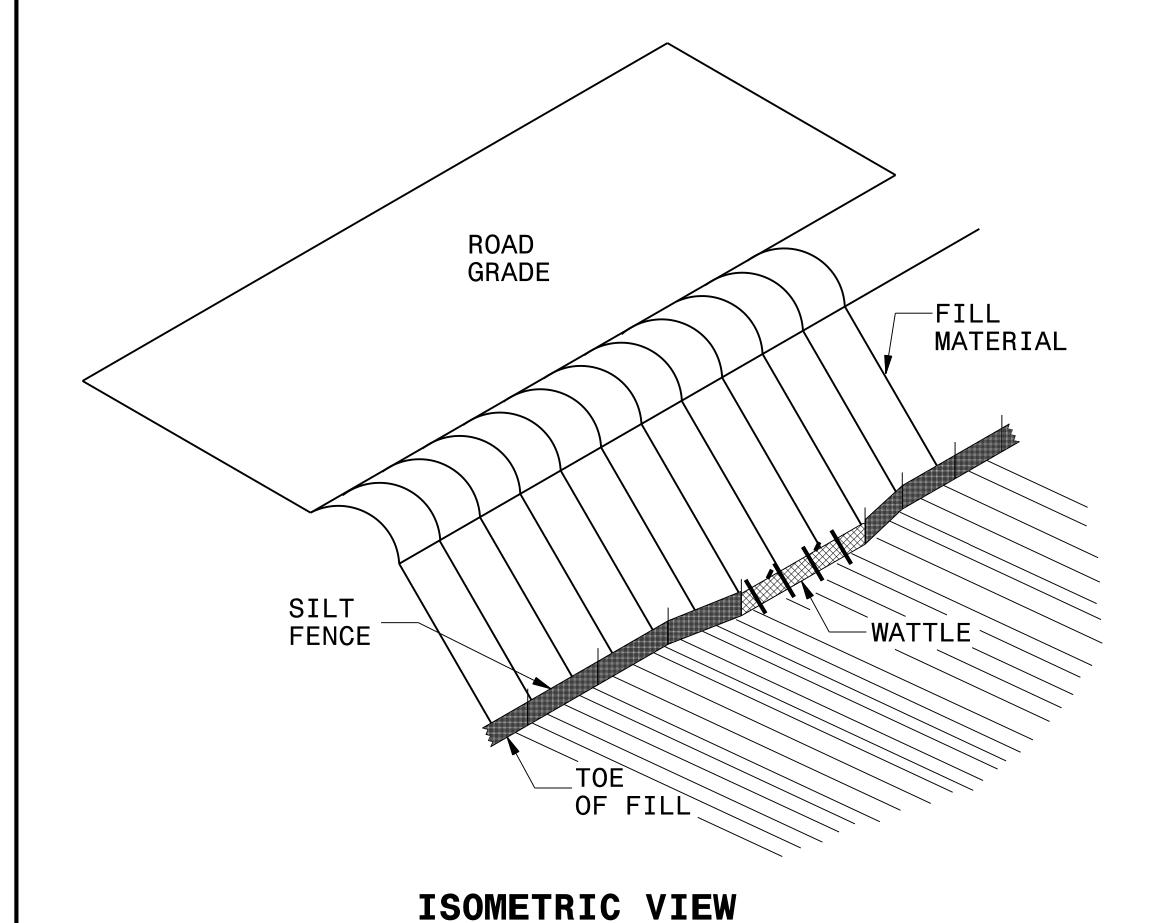


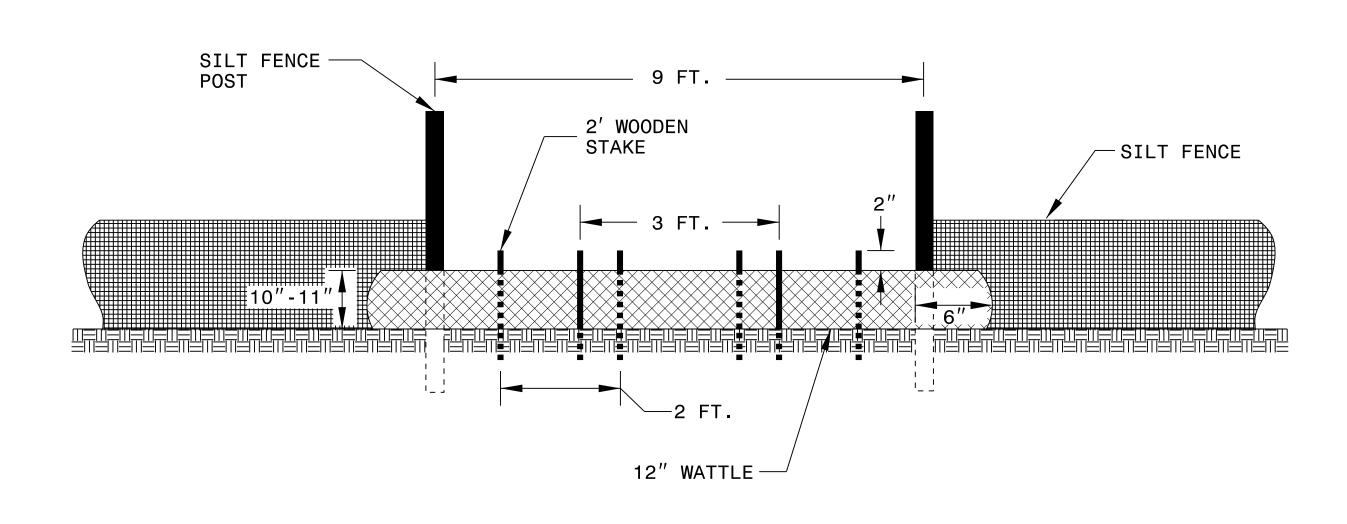


TOP VIEW

SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO	SHEET NO.	
17BP.3.R.5		EC-04
R/W SHEET N	10.	
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER





VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

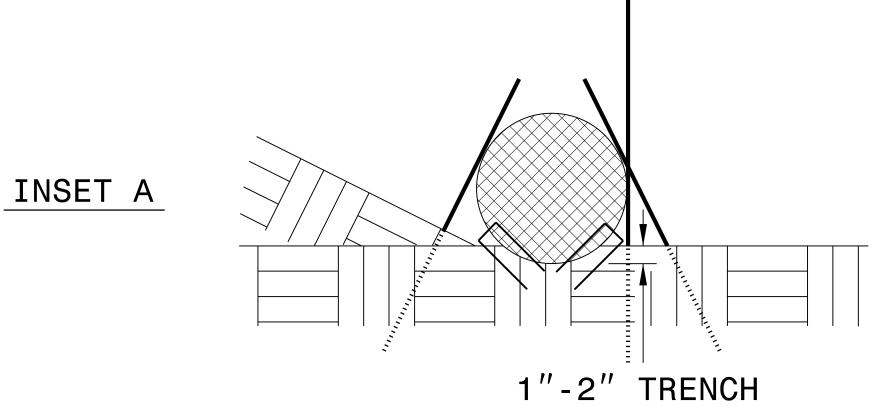
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

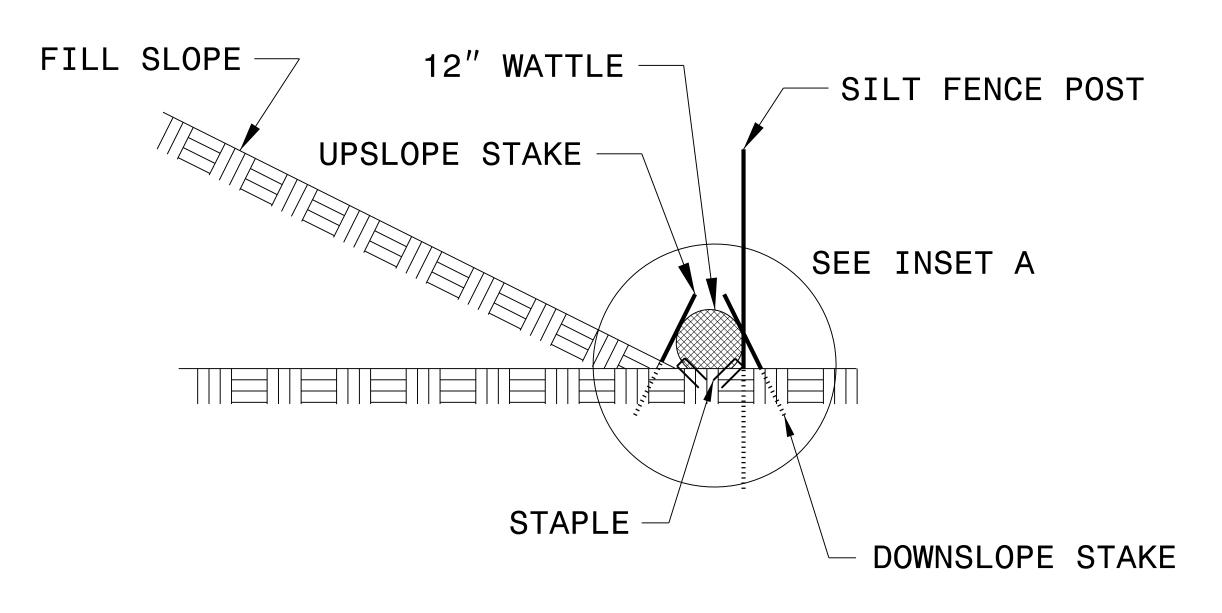
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

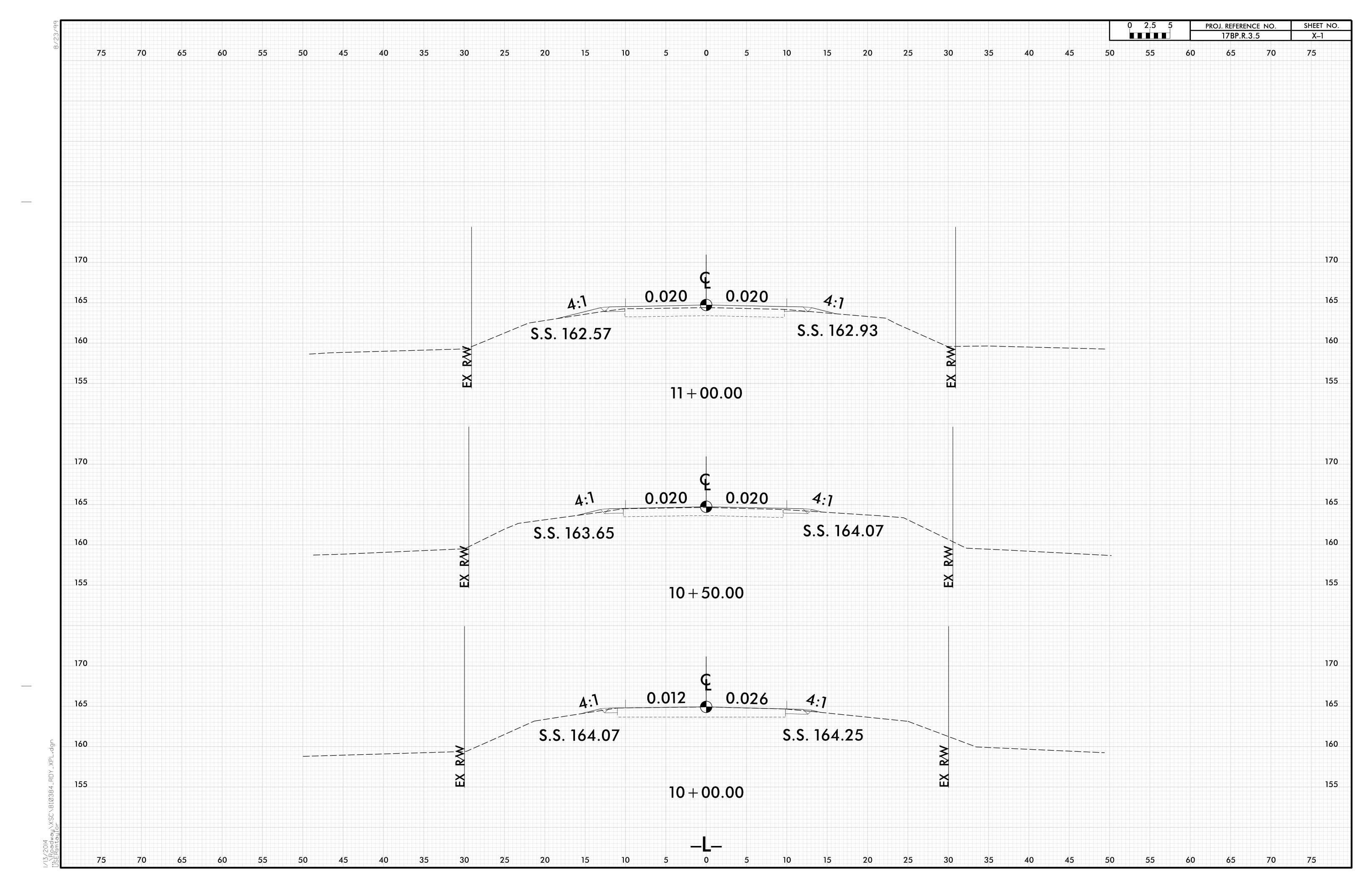
WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

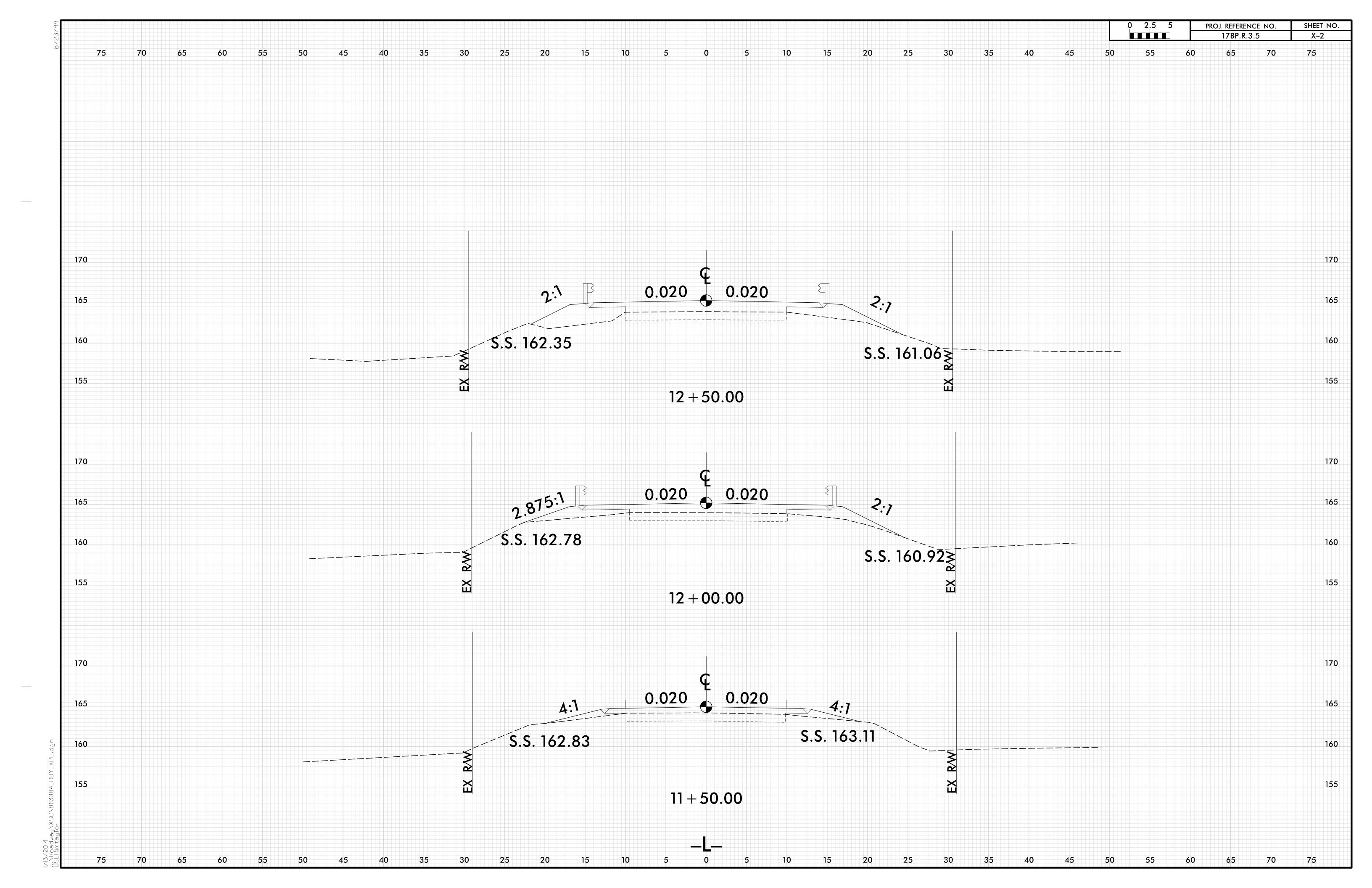
INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

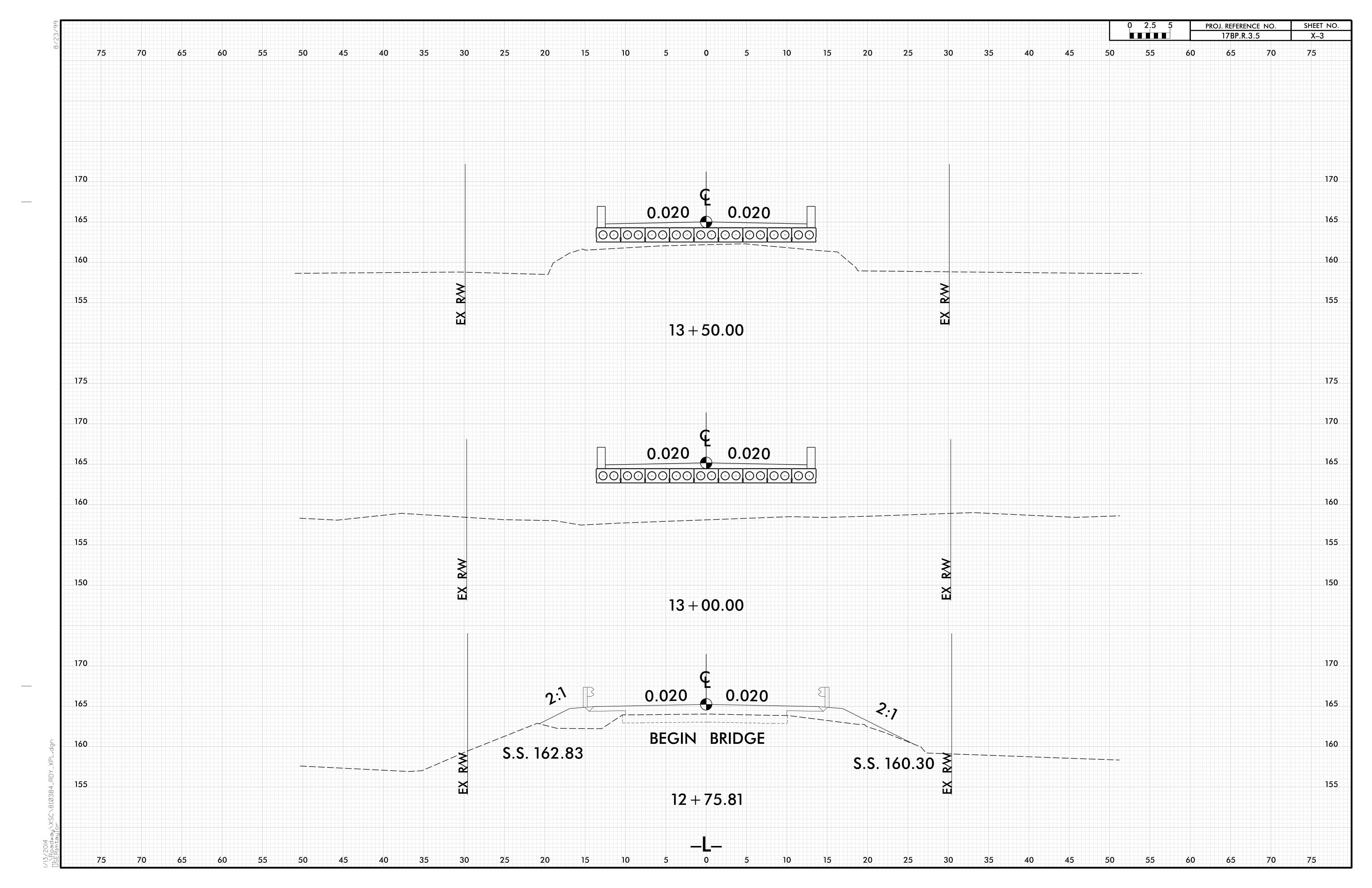


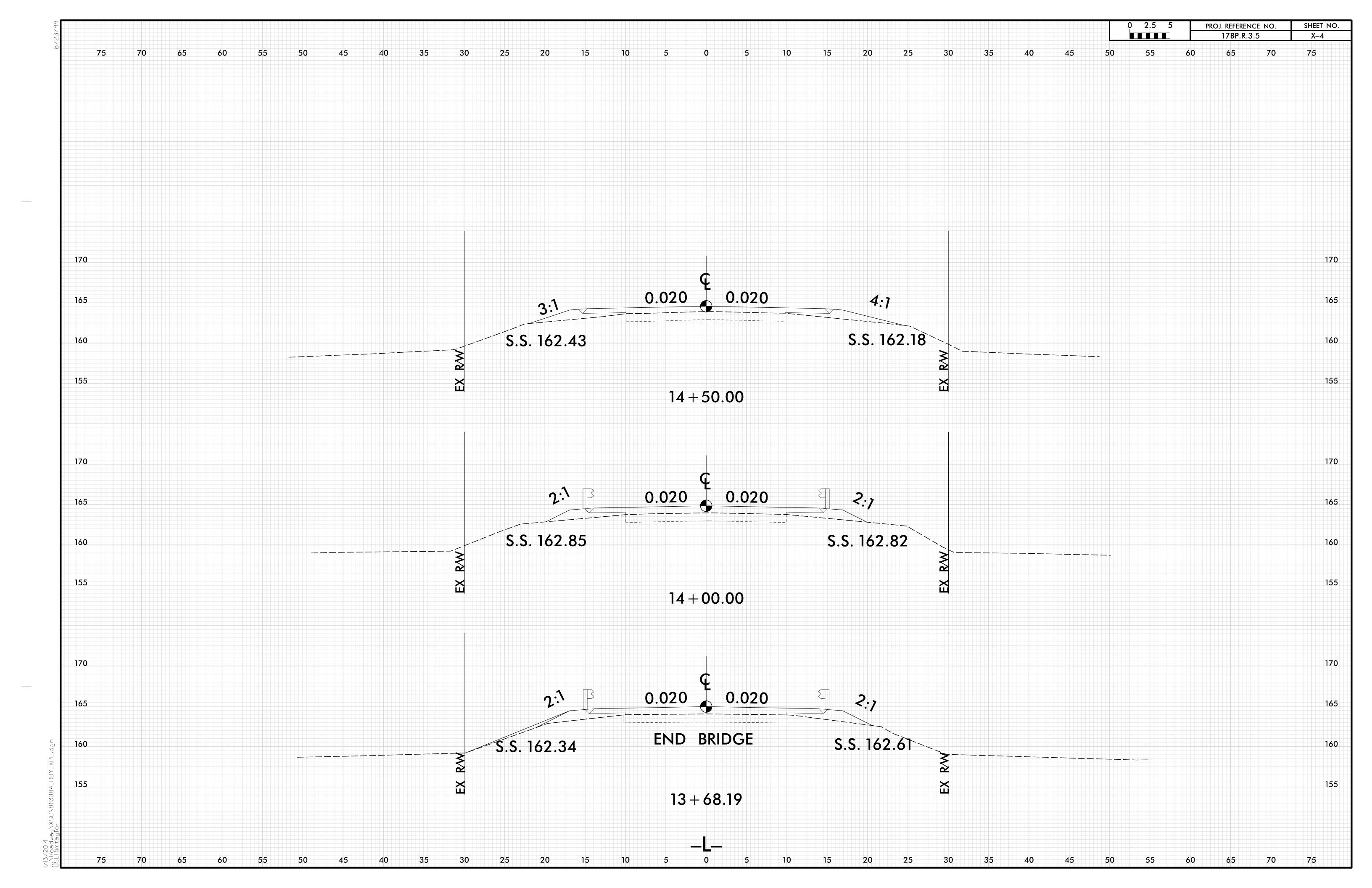


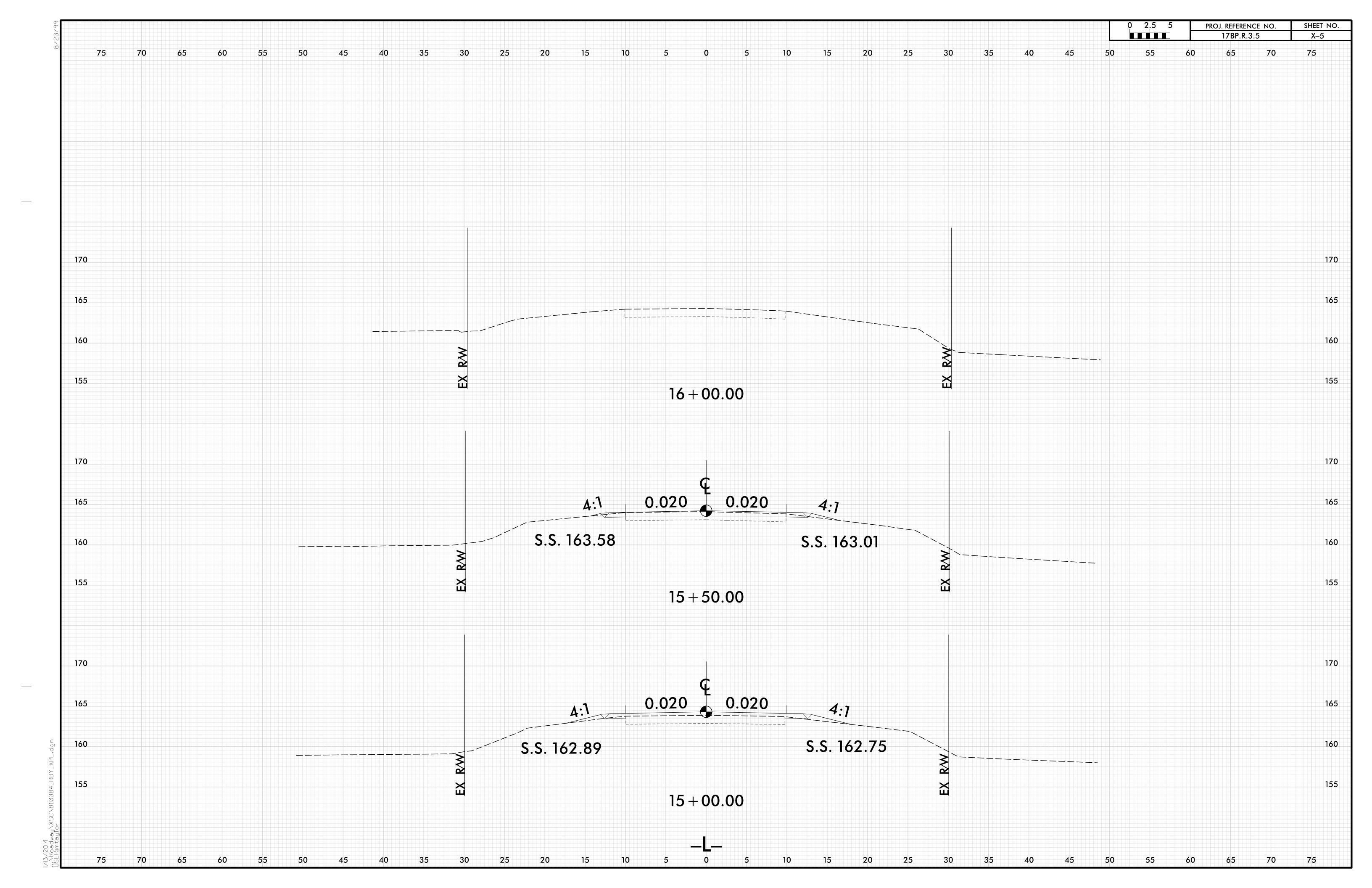
SIDE VIEW

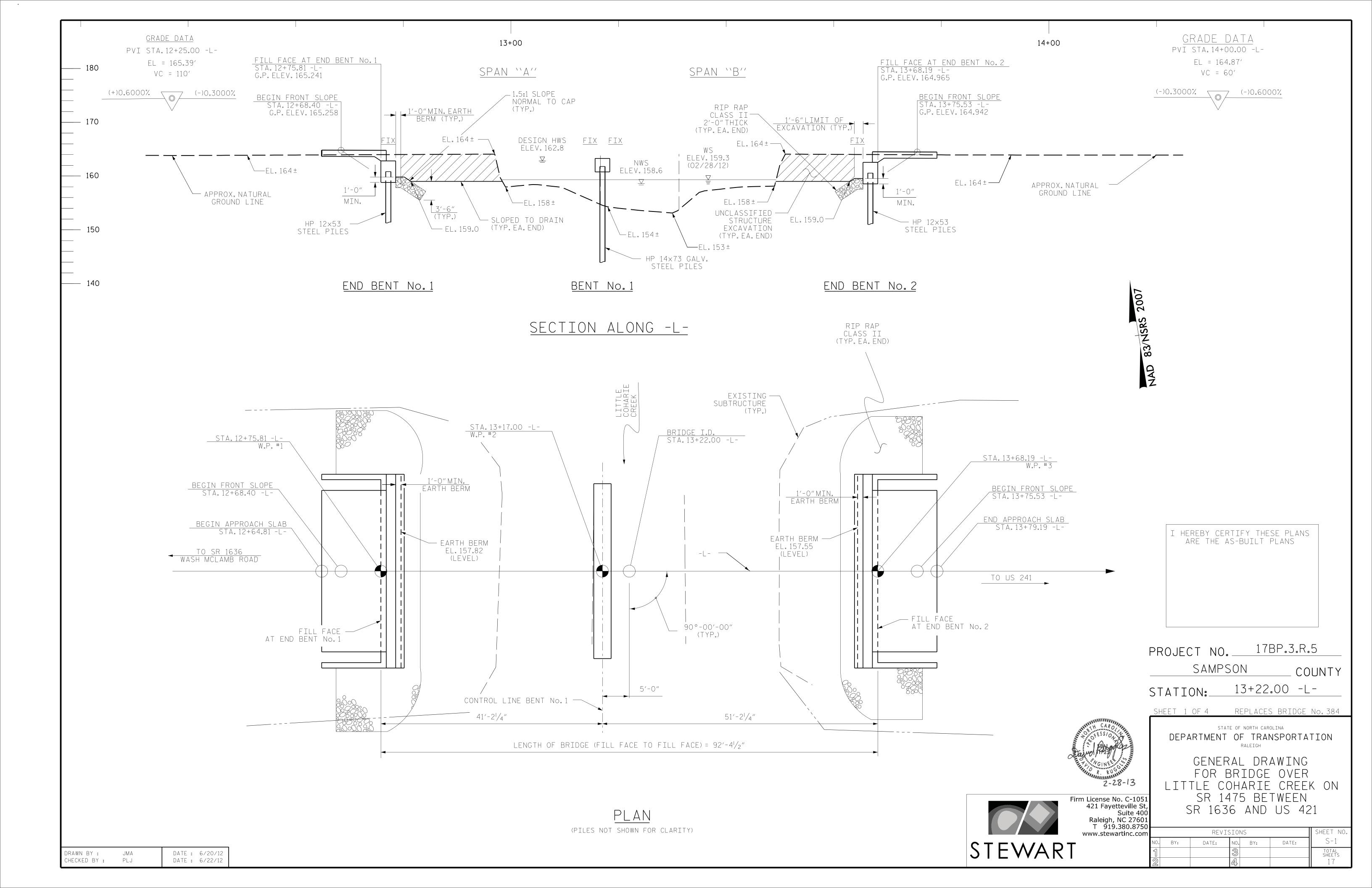












GENERAL NOTES:

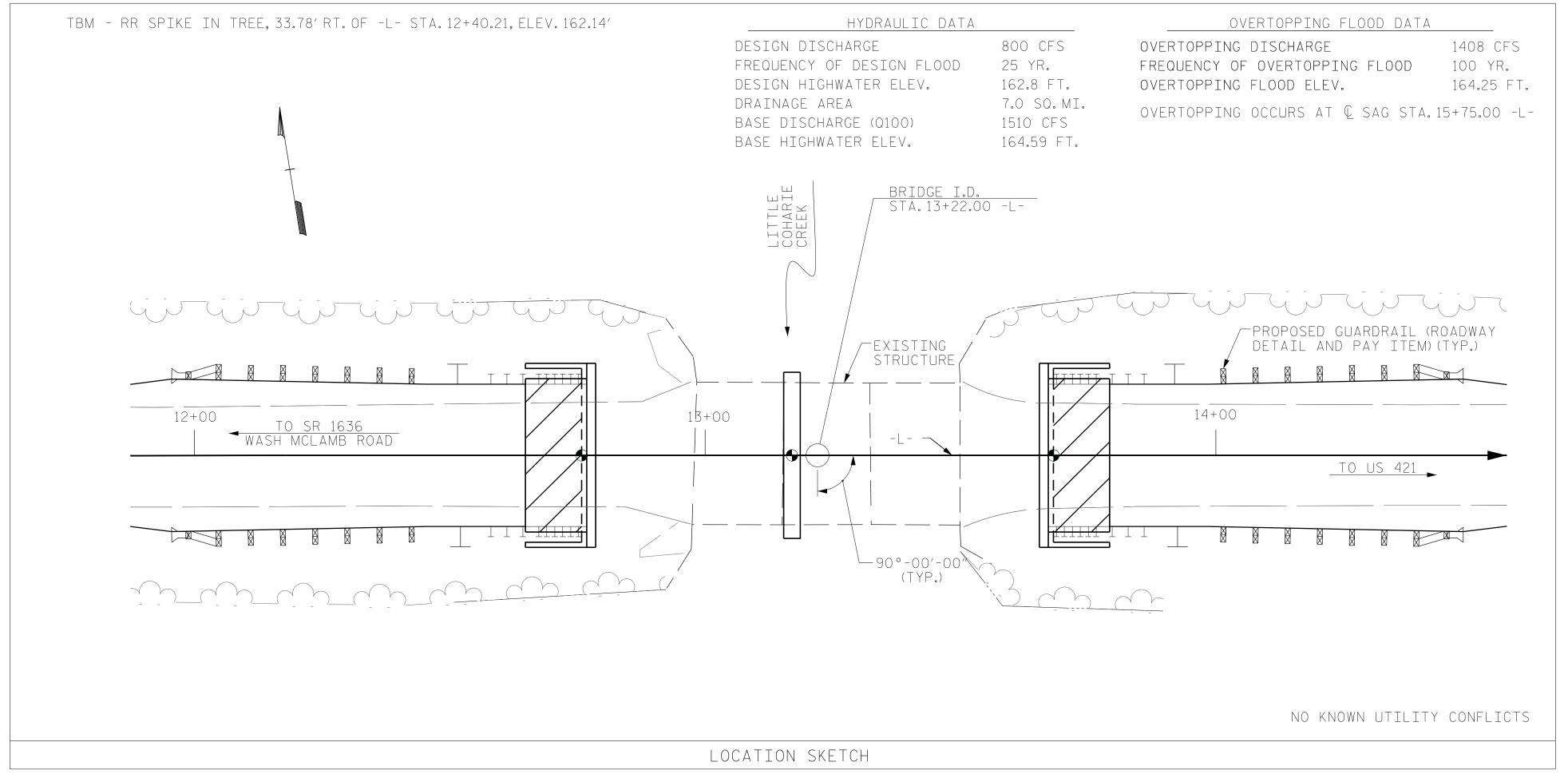
- 1. ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.
- 2. THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- 3. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES", MAY 2001.
- 4. REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.
- 5. THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 20 FT. EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

6. FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

7. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

- 8. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- 9. FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- 10. FOR ALL OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.
- 11. FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.
- 12. THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.
- 13. ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

	REMOVAL OF EXISTING STRUCTURE AT STATION 13+22.00 -L-	PDA TESTING	UNCLASSIFIED STRUCTURE EXCAVATION AT STATION 13+22.00 -L-	CLASS A CONCRETE	BRIDGE APPROACH SLABS AT STATION 13+22.00 -L-	REINFORCING STEEL		P 12X53 STEEL PILES	ŀ	HP 14×73 GALV. STEEL PILES	PILE REDRIVES	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	ELASTOMERIC PF BEARINGS	'-0" x 1'-9" RESTRESSED CONCRETE DRED SLABS
	LUMP SUM	EACH	LUMP SUM	СҮ	LUMP SUM	LBS	No.	LF	No.	LF	EACH	LF	TON	LUMP SUM No	LF
SUPERSTRUCTURE	LUMP SUM				LUMP SUM				_ _			180.50		LUMP SUM 20	900
END BENT No. 1		1	LUMP SUM	20.0		2449	5	275	_		3		72		
BENT No. 1		1		9.9		1959			- 7	490	3				
END BENT No. 2		1	LUMP SUM	20.0		2449	5	300	_		3		63		
TOTAL	LUMP SUM	3	LUMP SUM	49.9	LUMP SUM	6857	10	575	7	490	9	180.50	135	LUMP SUM 20	900



FOUNDATION NOTES:

- 1. PILES AT END BENT No.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 60 TONS PER PILE.
- 2. PILES AT END BENT No. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 70 TONS PER PILE.
- 3. PILES AT BENT No.1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 65 TONS PER PILE.
- 4. DRIVE PILES AT END BENT No.1 TO A REQUIRED DRIVING RESISTANCE OF 100 TONS PER PILE.
- 5. DRIVE PILES AT END BENT No. 2 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS PER PILE.
- 6. DRIVE PILES AT BENT No.1 TO A REQUIRED DRIVING RESISTANCE OF 115 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG AND SCOUR.
- 7. INSTALL PILES AT BENT No.1 TO A TIP ELEVATION NO HIGHER THAN 131.0 FT.
- 8. THE SCOUR CRITICAL ELEVATION FOR BENT No.1 IS ELEVATION 144.5 FT. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.
- 9. TESTING THE FIRST PRODUCTION PILE WITH THE PDA DURING DRIVING, RESTRIKING OR REDRIVING IS REQUIRED AT END BENTS No. 1 AND 2 AND BENT No. 1. FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.
- 10. PILE RESTRIKES ARE RECOMMENDED.
- 11. PILE SPLICES ANTICIPATED.

PROJECT NO. 17BP.3.R.5

SAMPSON COUNTY

STATION: 13+22.00 -L-

SHEET 2 OF 4

CAROLLING OF ESSION AND THE STATE OF ESSION AND THE STATE OF THE STATE

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING FOR BRIDGE OVER LITTLE COHARIE CREEK ON SR 1475 BETWEEN SR 1636 AND US 421

REVISIONS

BY: DATE: NO. BY: DATE: S-2

TOTAL SHEETS
17

Firm License No. C-1051
421 Fayetteville St,
Suite 400
Raleigh, NC 27601
T 919.380.8750
www.stewartinc.com

DRAWN BY: PLJ DATE: 2/26/13 CHECKED BY: DRR DATE: 2/27/13

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS SERVICE III LIMIT STATE STRENGTH I LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) IVELOAD RATING RATING GIRDER DIS DIS 1.32 1.95 1.319 1.75 0.278 1.76 40′ EL 19.5 0.549 40′ EL 0.80 0.278 1.55 40′ 19.5 HL-93(Inv) N/A 0.549 1.709 1.35 EL 40′ 1.95 HL-93(0pr) N/A 0.278 2.28 40′ 19.5 1.71 EL N/A DESIGN LOAD 19.5 0.549 1.54 0.80 0.278 1.94 36.000 1.540 55.449 2.21 40′ EL 40′ EL 1.95 40′ 19.5 HS-20(Inv) EL RATING 19.5 HS-20(0pr) 36.000 1.997 40′ EL 0.549 40′ EL 1.95 N/A 71.878 1.35 0.278 2.86 13.500 0.278 19.5 0.549 4.13 40′ 1.95 0.80 0.278 3.61 19.5 SNSH 3.606 5.1 40′ EL EL 40′ EL 20.000 40′ EL 15.6 0.549 3.07 40′ EL 1.95 0.80 SNGARBS2 2.964 0.278 4.19 0.278 2.96 40′ 19.5 2.906 0.549 2.91 1.95 0.80 0.278 2.92 SNAGRIS2 22.000 63.929 0.278 40′ EL 15.6 40′ EL 40′ 15.6 4.09 2.55 19.5 0.80 27.250 40′ EL 0.549 2.07 40′ EL 1.95 1.80 40′ 19.5 SNCOTTS3 1.803 0.278 0.278 1.95 34.925 1.623 2.29 19.5 0.549 1.82 0.80 0.278 1.62 SNAGGRS4 56.667 0.278 40′ EL 40′ EL 40′ 19.5 35.550 1.578 2.23 19.5 0.549 0.80 0.278 1.58 SNS5A 0.278 40′ EL 1.9 40′ EL 1.95 40′ 19.5 39.950 59.992 0.278 2.12 40′ EL 19.5 0.549 1.77 40′ EL 1.95 0.80 0.278 1.50 40′ 19.5 SNS6A 19.5 1.81 19.5 42.000 1.432 40′ EL 40′ EL 1.95 40′ SNS7B 0.278 2.02 0.549 0.80 0.278 1.43 LEGAL LOAD 33.000 60.976 2.61 19.5 0.549 2.08 1.95 0.80 0.278 1.85 19.5 TNAGRIT3 0.278 40′ EL 40′ EL 40′ 1.848 EL RATING 33.075 19.5 0.549 1.98 0.80 0.278 1.87 40′ EL 40′ EL 1.95 40′ TNT4A 1.872 61.901 0.278 2.65 19.5 41.600 1.587 2.24 19.5 0.549 1.94 1.95 0.80 0.278 1.59 19.5 TNT6A 0.278 40′ EL 40′ EL 40′ 42.000 40′ EL 19.5 1.79 40′ EL 1.95 40′ TNT7A 1.627 68.354 0.278 2.3 0.549 0.80 0.278 1.63 19.5 EL 2.35 19.5 0.549 1.72 1.95 0.80 0.278 42.000 40′ EL 40′ EL 1.66 40′ 19.5 TNT7B 1.664 69.888 0.278 EL

1.65

1.455 | 65.486 | 1.4 | 0.278 | 2.06 | 40′ | EL | 19.5 | 0.549 | 1.56 | 40′ | EL | 1.95 | 0.80 | 0.278 | 1.46 | 40′ | EL | 19.5

0.549

0.549

40′

40′

EL

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2

Z., 3

4.

19.5

0.278

0.278

1.62

1.50

40′

0.80

0.80

1.95

1.95

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.3.R.5

SAMPSON COUNTY

STATION: 13+22.00 -L-

SHEET 3 OF 4

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STEWART

Suite 400 Raleigh, NC 27601 T 919.380.8750 www.stewartinc.com STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

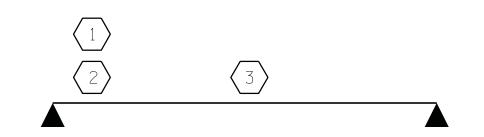
RALEIGH

STANDARD

LRFR SUMMARY FOR 40'CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	S-3
		3			TOTAL SHEETS
					1 7



0.278

69.61

2.28

40′

40′

EL

15.6

LRFR SUMMARY

FOR SPAN "A"

ASSEMBLED BY: JMA DATE: 6/20/12 CHECKED BY: PLJ DATE: 6/22/12

DRAWN BY: CVC 6/IO CHECKED BY: DNS 6/IO

TNAGRIT4

TNAGT5A

TNAGT5B

43.000

45.000

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS SERVICE III LIMIT STATE STRENGTH I LIMIT STATE MOMENT SHEAR MOMENT DISTRIBUTION FACTORS (DF) IVELOAD RATING RATING GIRDER DIS DIS 0.531 1.39 1.394 1.75 0.276 1.57 50′ EL 24.5 50′ EL 2.45 0.80 0.276 1.44 50′ 24.5 HL-93(Inv) N/A 0.531 1.807 1.35 50′ EL 24.5 50′ 2.45 HL-93(0pr) N/A 0.276 2.03 1.81 EL N/A DESIGN LOAD 0.531 1.67 50′ 0.80 0.276 1.79 36.000 1.95 50′ EL 24.5 EL 50′ 24.5 HS-20(Inv) 2.45 EL RATING 2.52 0.531 2.16 2.45 HS-20(0pr) 36.000 50′ EL 24.5 50′ EL N/A 2.161 77.787 1.35 0.276 13.500 0.276 4.95 24.5 0.531 50′ 2.45 0.80 0.276 24.5 SNSH 3.635 49.079 50′ EL 4.7 EL 3.64 50′ EL 20.000 57.42 50′ EL 24.5 0.531 3.42 50′ EL 2.45 0.80 0.276 2.87 50′ 24.5 SNGARBS2 2.871 0.276 3.91 2.778 3.78 19.6 0.531 3.21 50′ 2.45 0.80 0.276 2.78 SNAGRIS2 22.000 0.276 50′ EL EL 50′ 24.5 0.531 2.36 0.80 27.250 50′ EL 24.5 50′ EL 2.45 50′ 24.5 SNCOTTS3 1.814 49.418 0.276 2.47 0.276 1.81 34.925 1.577 55.063 2.15 24.5 0.531 2.01 50′ 2.45 0.80 0.276 1.58 SNAGGRS4 0.276 50′ EL EL 50′ 24.5 35.550 1.537 0.531 2.07 50′ 0.80 0.276 1.54 SNS5A 54.657 0.276 2.09 50′ EL 24.5 EL 2.45 50′ 24.5 39.950 57.43 0.276 50′ EL 24.5 0.531 1.91 50′ EL 2.45 0.80 0.276 1.44 50′ 24.5 SNS6A 1.438 1.96 0.531 1.91 1.37 42.000 1.370 57.54 50′ EL 24.5 50′ EL 2.45 50′ 24.5 SNS7B 0.276 1.87 0.80 0.276 EL LEGAL LOAD 33.000 58.118 24.5 0.531 2.25 50′ 2.45 0.80 0.276 TNAGRIT3 1.761 0.276 50′ EL EL 1.76 50′ 24.5 2.4 EL RATING 33.075 1.777 0.531 2.17 50′ 2.45 0.80 0.276 1.78 50′ EL 24.5 50′ 24.5 TNT4A 0.276 2.42 EL 41.600 1.480 61.558 0.276 2.01 24.5 0.531 2.08 50′ 2.45 0.80 0.276 1.48 24.5 TNT6A 50′ EL EL 50′

50′

50′

50′

50′

2.05

2.13

2.02

EL

EL

EL

24.5

24.5

24.5

24.5

0.531

0.531

0.531

0.531

45.000 **3** | 1.360 | 61.206 | 1.4 | 0.276 | 1.85 | 50′ | EL | 24.5 | 0.531 | 1.68 | 50′ | EL | 2.45 | 0.80 | 0.276 | **1.36** | 50′ | EL |

1.94

1.84

1.77

50′

50′

50′

50′

EL

EL

EL

2.45

2.45

2.45

2.45

0.80

0.80

0.80

0.80

0.276

0.276

0.276

0.276

1.50

1.57

1.49

1.39

50′

50′

50′

EL

EL

24.5

24.5

24.5

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{ extsf{DC}}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2

Z. 3

4.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

GIRDER LOCATION

I - INTERIOR GIRDER EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.3.R.5

SAMPSON COUNTY

STATION: 13+22.00 -L-

SHEET 4 OF 4

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STEWART

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

LRFR SUMMARY FOR 50'CORED SLAB UNIT 90° SKEW

<u>1</u> <u>2</u> <u>3</u>

0.276

0.276

0.276

LRFR SUMMARY
FOR SPAN "B"

ASSEMBLED BY: JMA DATE: 6/20/12 CHECKED BY: PLJ DATE: 6/22/12

DRAWN BY: CVC 6/IO CHECKED BY: DNS 6/IO

42.000

42.000

43.000

45.000

TNT7A

TNT7B

TNAGRIT4

TNAGT5A

TNAGT5B

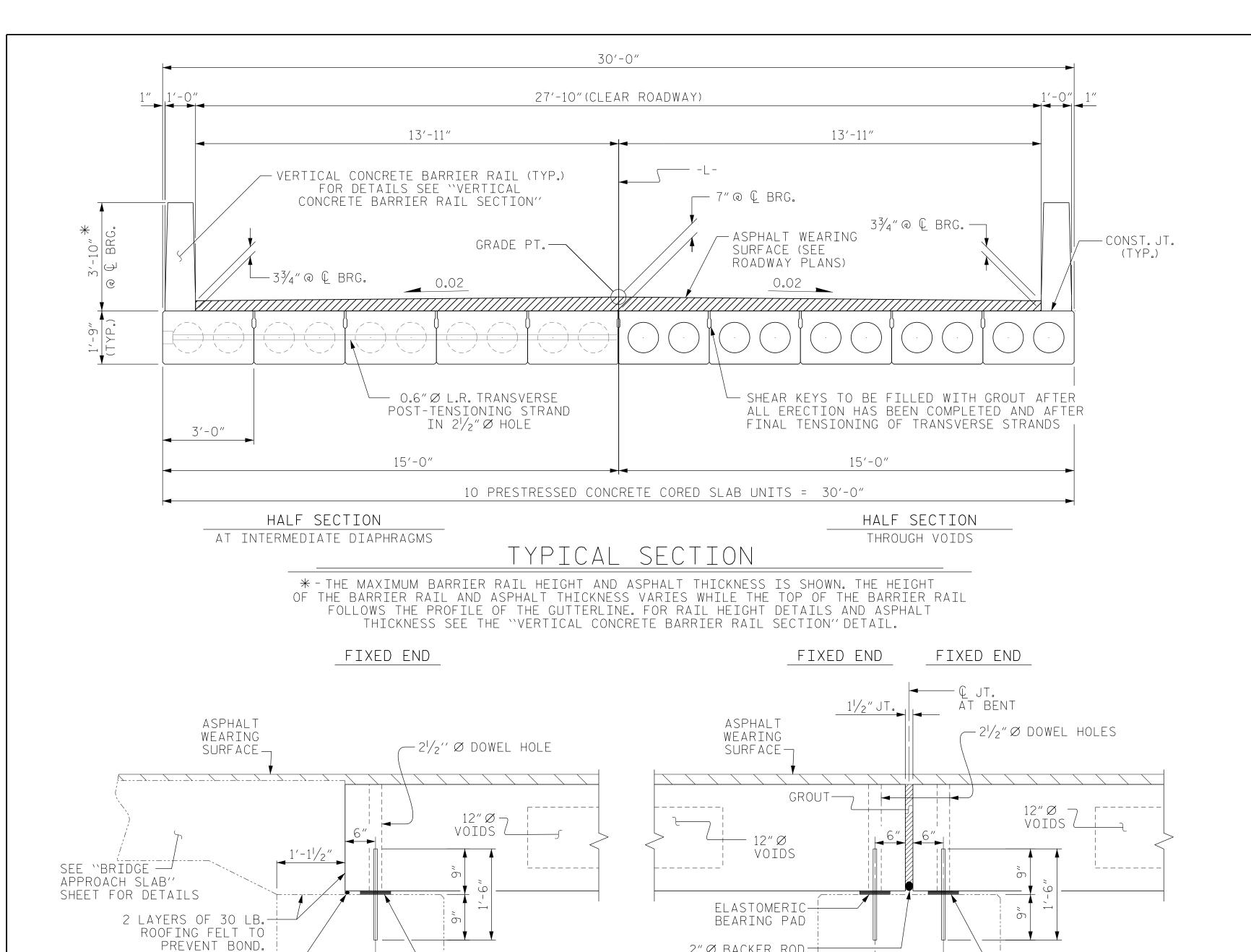
1.502

1.566

1.486

63.087

65.773



- ELASTOMERIC BEARING PAD

SHEETS FOR DETAILS

SECTION B-B

/— ⁵/₈′′ X 5′′ X 5′′ ₽

---FILL RECESS
,,,WITH GROUT, APPLY

STRAND VISE

51/4''D 1/4''EPOXY COATING

SEE "END BENT"

AT END BENT

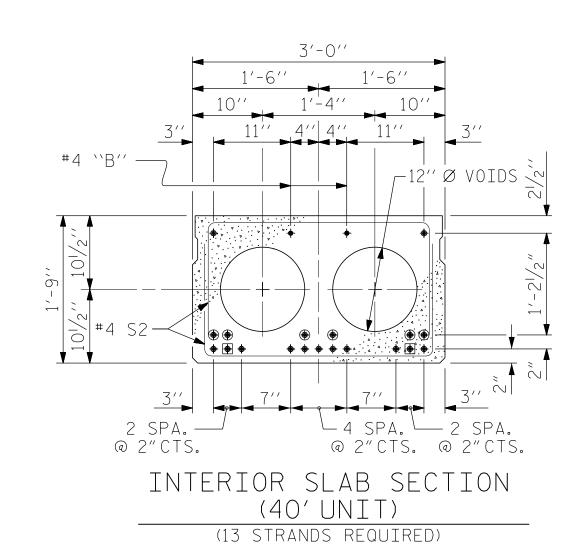
© 0.6" Ø L.R. TRANSVERSE POST-TENSIONING STRAND SHEATHED WITH A NON-CORROSIVE PIPE.

OUTSIDE FACE

OF EXTERIOR CORED SLAB

GROUTED RECESS AT END OF

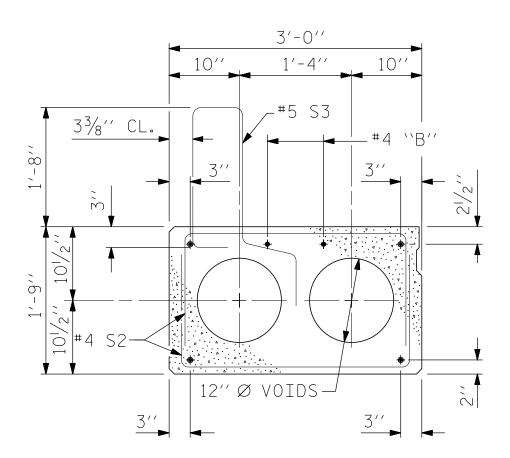
POST-TENSIONED STRAND OF CORED SLABS



r12''Ø VOIDS ≧ 4 SPA. 2 SPA. @ 2"CTS. @ 2"CTS. 2 SPA. — J @ 2"CTS.

> INTERIOR SLAB SECTION (50' UNIT) (19 STRANDS REQUIRED)

0.6'' Ø LOW RELAXATION STRAND LAYOUT



EXT. SLAB SECTION

(FOR PRESTRESSED STRAND LAYOUT, SEE INTERIOR SLAB SECTION.)

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-0" FROM END OF CORED SLAB UNIT SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

PROJECT NO. ____17BP.3.R.5

SAMPSON COUNTY

13+22.00 -L-

SHEET 1 OF 5

Suite 400

DEPARTMENT OF TRANSPORTATION STANDARD

STATE OF NORTH CAROLINA

CORED SLAB UNIT 90° SKEW

SHEET NO REVISIONS S-5 NO. BY: DATE: DATE: TOTAL SHEETS

SECTION AT BENT

-ELASTOMERIC

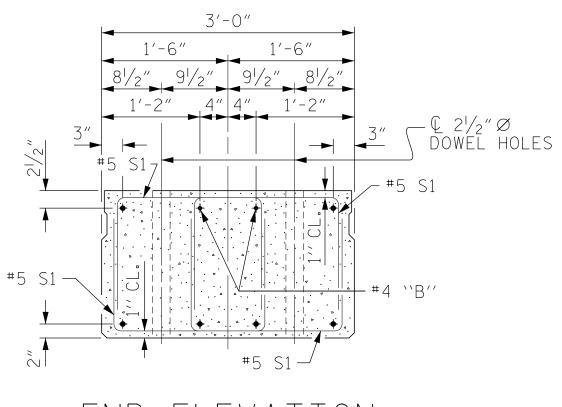
BEARING PAD

FOR DETAILS

SEE "BENT" SHEETS

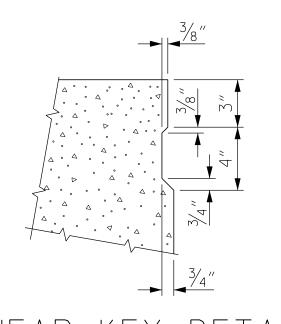
2"∅ BACKER ROD

Q BEARING & #6 DOWELS



END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.



NOTE: OMIT SHEAR KEY ON OUTSIDE FACE OF EXTERIOR CORED SLABS.

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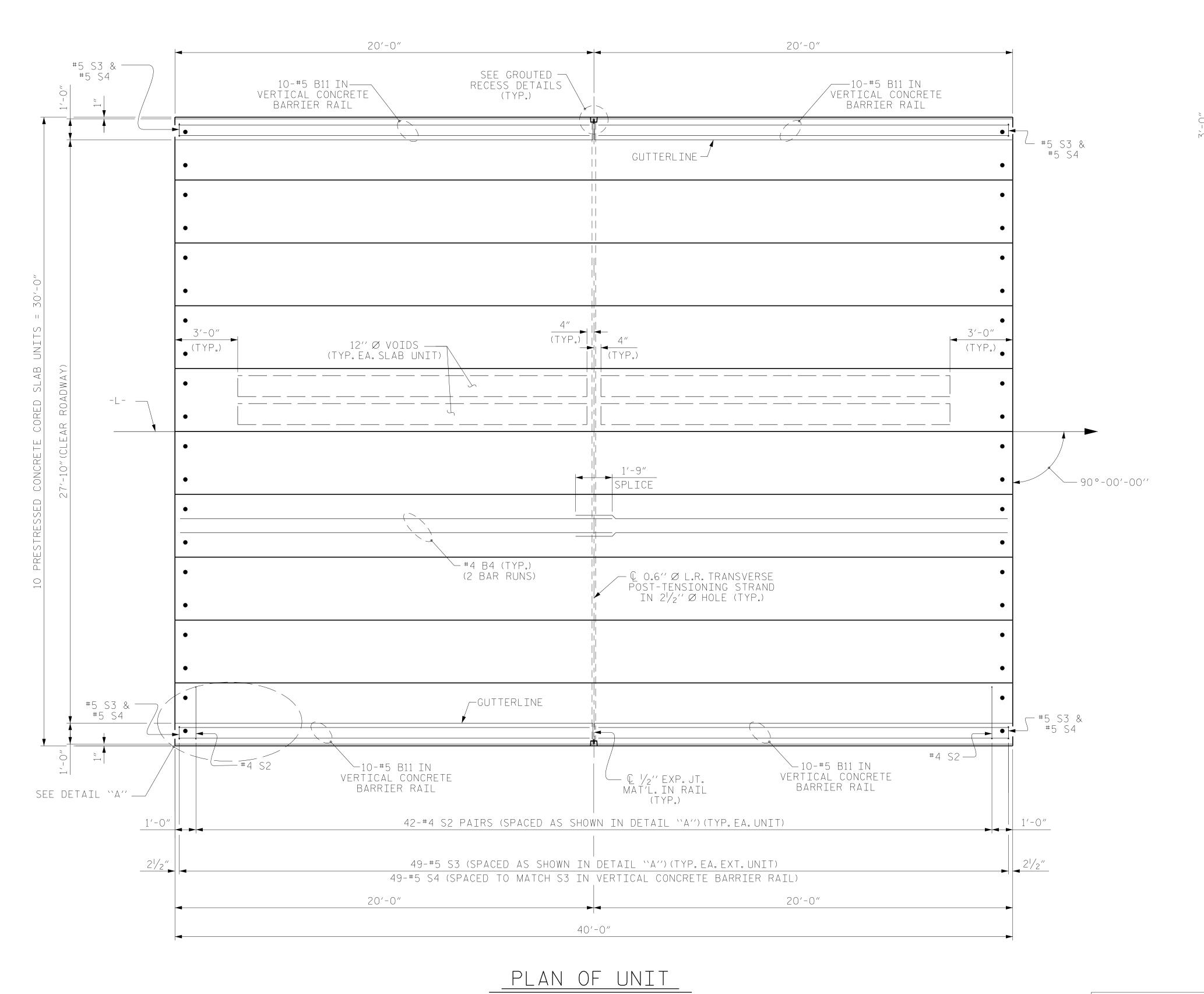
DATE: 6/20/12 ASSEMBLED BY: DATE: 6/22/12 CHECKED BY : PLJ MAA/AAC REV. 12/II DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09

ELEVATION VIEW

11/2" Ø BACKER ROD -

Q BEARING & #6 DOWELS

-HOLE FOR TRANSVERSE STRAND



ASSEMBLED BY :

CHECKED BY :

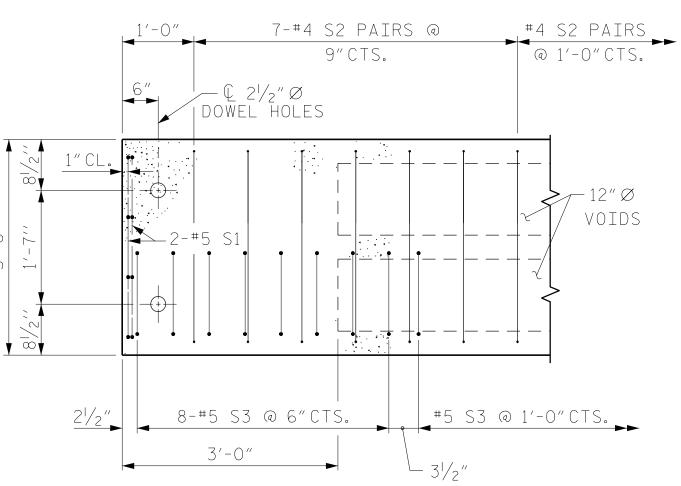
JMA

PLJ

DRAWN BY: DGE 3/09 REV. 12/5/II MAA/AAC CHECKED BY: BCH 3/09

DATE: 6/20/12

DATE: 6/22/12



NOTE: EXTERIOR UNIT SHOWN - INTERIOR UNIT SIMILAR EXCEPT OMIT #5 S3 BARS.

DETAIL \\A''

PROJECT NO. 17BP.3.R.5

SAMPSON COUNTY

STATION: 13+22.00 -L-

SHEET 2 OF 5

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

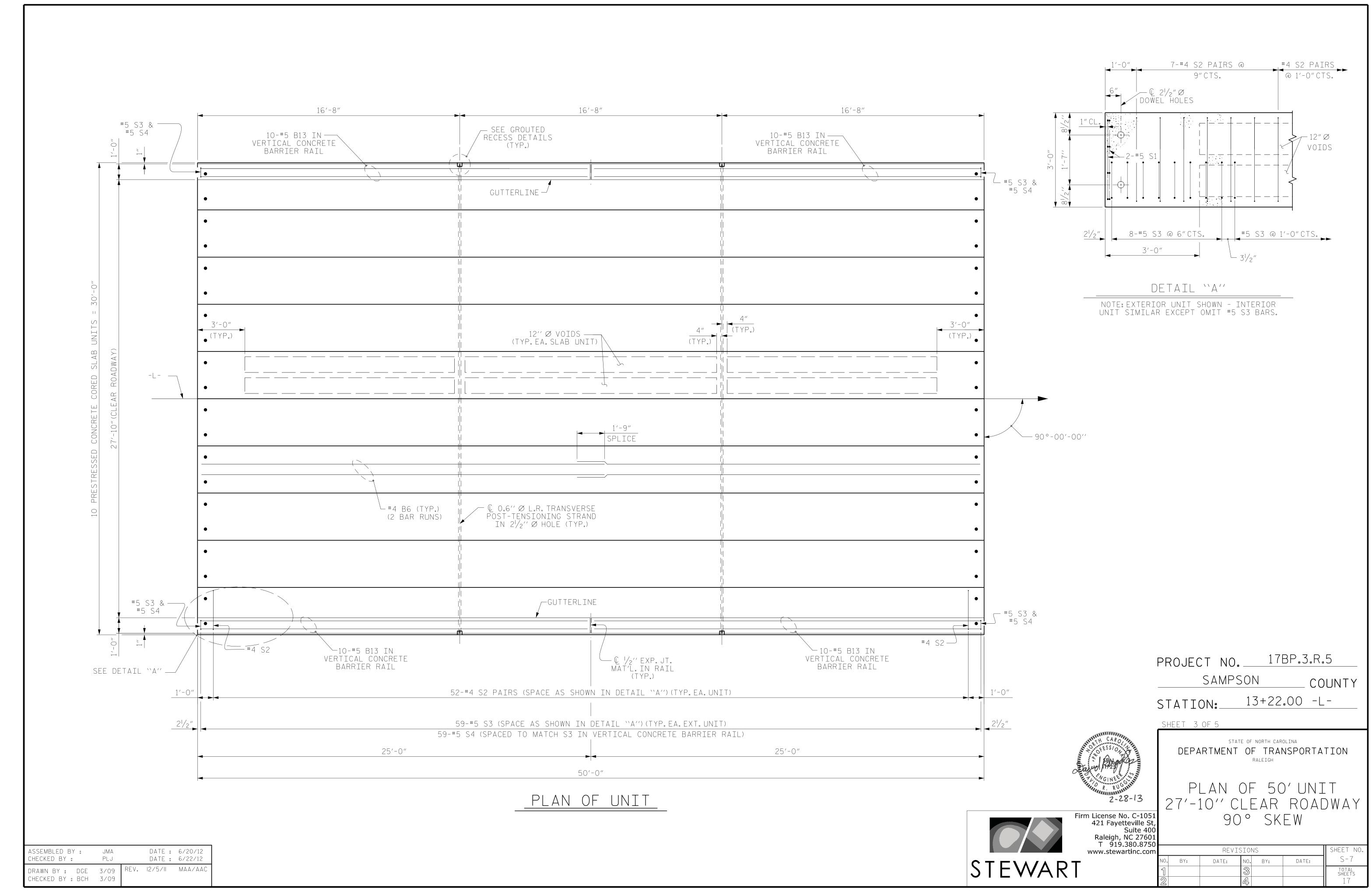
PLAN OF 40'UNIT 27'-10'' CLEAR ROADWAY 90° SKEW



SHEET NO

S-6

TOTAL SHEETS



BILL OF MATERIAL FOR ONE 40' CORED SLAB UNIT EXTERIOR UNIT | INTERIOR UNIT BAR | NUMBER | SIZE | TYPE | LENGTH | WEIGHT LENGTH | WEIGHT #4 STR 20'-9" 55 20′-9″ 55 #5 4'-3" #4 299 5′-4″ 299 84 5′-4″ ***** S3 49 #5 6'-2" 315 REINFORCING STEEL 389 389 LBS. * EPOXY COATED REINFORCING STEEL 6500 P.S.I. CONCRETE CU. YDS. 5.8 5.8 0.6″∅ L.R. STRANDS 13 13 No.

	BILL OF MATERIAL FOR ONE 50'CORED SLAB UNIT						
				EXTERI	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
В6	4	#4	STR	25′-9″	69	25′-9″	69
S1	8	#5	3	4'-3"	35	4'-3"	35
S2	104	#4	3	5'-4"	371	5'-4"	371
* S3	59	#5	1	6'-2"	379		
	ORCING S		LBS	<u> </u> 	475		475
REIN	(Y COATE Iforcing	STEEL	LBS.		379		
6500 F	P.S.I. CO	NCRETE	CU. YDS) ₀	7.1	7.1	
0.6" Ø	L.R. STR	ANDS	No) .	19		19

2"CL. MIN.

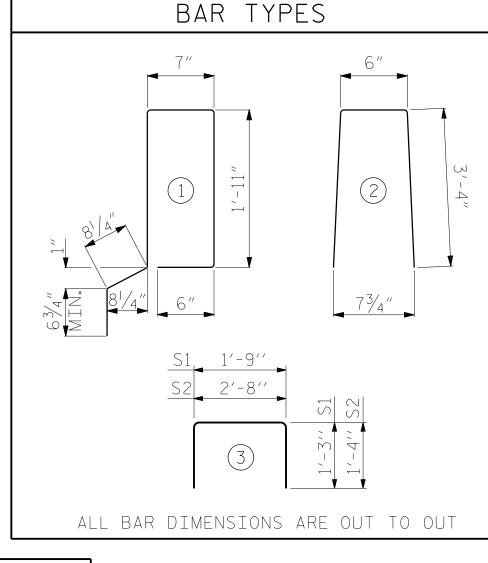
BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL						
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	40' UNIT					
* B11	40	40	#5	STR	19'-7"	817
* S4	98	98	#5	2	7'-2"	733
* EPOX	Y COATED REINFORCING STEEL			LBS.		1550
CLASS AA CONCRETE CU.YDS.						10.5
TOTAL VERTICAL CONCRETE BARRIER RAIL LN.FT.						80.25

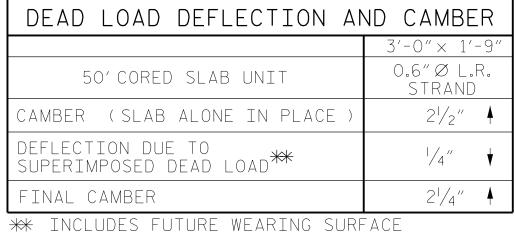
BILL OF MATERIAL FOR VERTICAL CONCRETE BARRIER RAIL							
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT	
	50' UNIT						
 ₩ B13	40	40	#5	STR	24'-7"	1026	
* S4	118	118	#5	2	7'-2"	882	
*EPOXY COATED REINFORCING STEEL LBS.						1908	
CLASS	CLASS AA CONCRETE CU.YDS. 13.1						
TOTAL	OTAL VERTICAL CONCRETE BARRIER RAIL LN. FT. 100.25						

DEAD LOAD DEFLECTION AN	ND CAMBER
	$3'-0'' \times 1'-9''$
40' CORED SLAB UNIT	0.6″∅ L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	11/4″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	1/8″ ▼
FINAL CAMBER	1 1/8″ ♦

GROUT-

ELEVATION AT EXPANSION JOINTS





0.6" Ø L.R.

0.217

58,600

43,950

GRADE 270 STRANDS

SQUARE INCHES) Ultimate strengt

LBS.PER STRAND Applied prestres

LBS. PER STRAND

51/2"	© BEARING PAD
2'-6"	L 1" Ø HOLES BEARING PAD - TYPE I -
	FIXED END

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

PRESTRESSED CONCRETE CORED SLABS.

THE $2^{1}\!/_{2}{''}\varnothing$ dowel holes at fixed ends of slab sections shall be filled with non-shrink grout.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M

BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

(TYPE I - 40 REQ'D)

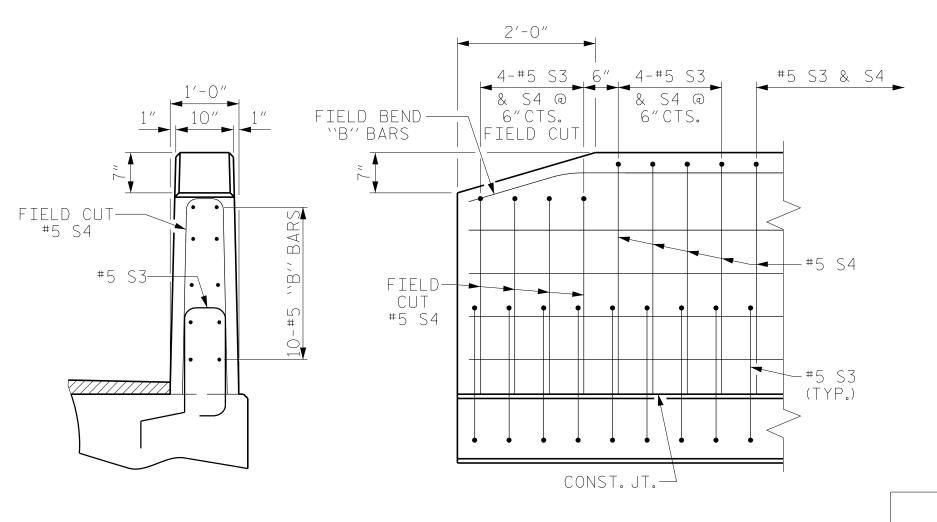
GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
27'-10"CLEAR ROADWAY	ASPHALT OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
	NORMAL CROWN SECTION	
40'UNITS	25/8″	3'-87/8"
50'UNITS	11/2"	3'-73/4"

IALT 3le)	<u></u>	#5 \$4	•	
3'-10" EE 'GUTTERLINE ASPHALT S & RAIL HEIGHT'' TABLE)	10-#5 "B" BARS 61/2" 8" 10"	2" (TYP.) "9-," "9-," 23/8" CL.	21/2" 21/2" 21/2" SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)	SECTION T-T AT OPEN JOINT AT BENT (THIS IS TO BE USED WHERE FOAM JOINT IS NOT USED)
VARIES (SEE THICKNESS	61/2"	33/8"	Û ½″EXP.JT.MAT'L HI PLACE WITH GALVANIZE (NOTE: OMIT EXP.JT. WHEN SLIP FORM IS -► T	D NAILS. Mat'l.
		VERTICAL DIM. VARIES	Q OPEN JT. IN RAIL @ BENT	CHAMFER 3/4" CHAMFER CHAMFER
		#5 S3 (S	SEE "PLAN OF CO	NST. JT.

VERTICAL CONCRETE BARRIER RAIL SECTION

ASSEMBLED BY : CHECKED BY :	PLJ DRR		2/26/13 2/27/13
DRAWN BY: DGE CHECKED BY: BCH		REV. I2/II	MAA/AAC

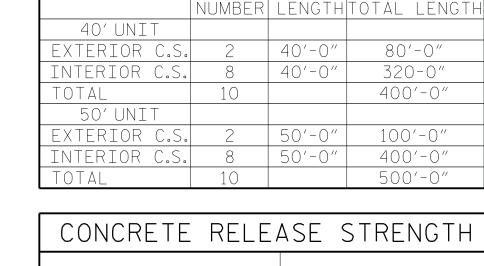
CONST.JT.—



END VIEW

SIDE VIEW

END OF RAIL DETAILS



CORED SLABS REQUIRED

CONCRE	TE RELE	EASE S	TRENGTH
UNIT			PSI
40' UNI	TS		4000
50' UNI	TS		4900

PROJECT NO. 17BP.3.R.5

SAMPSON COUNTY

STATION: 13+22.00 -L-

SHEET 4 OF 5

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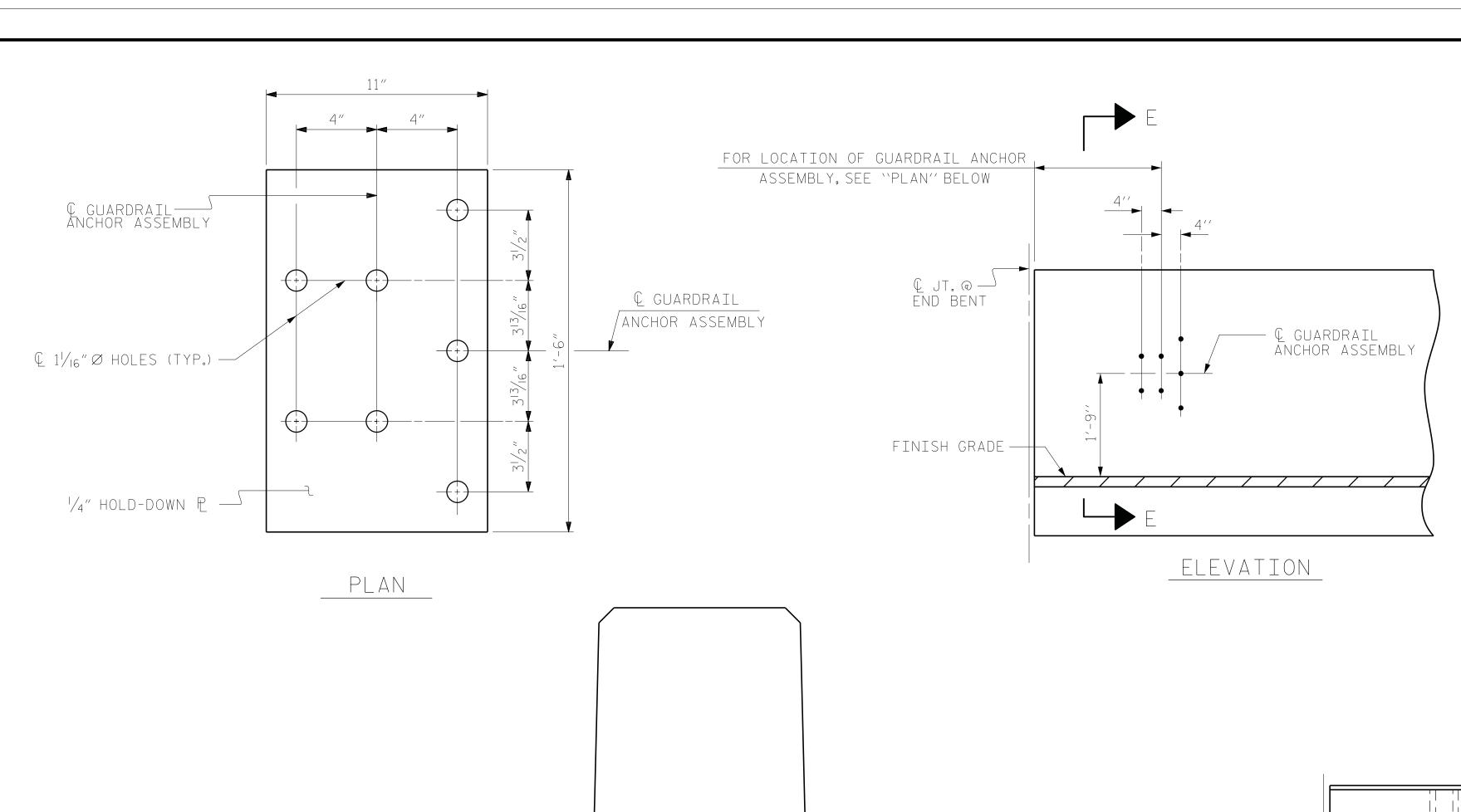
Raleigh, NC 27601 T 919.380.8750 www.stewartinc.com DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD 3'-0'' X 1'-9''

PRESTREŠSÉD CONCRETE CORED SLAB UNIT

		SHEET NO.				
10.	BY:	DATE:	NO.	BY:	DATE:	S-8
1			3			TOTAL SHEETS
2			4			17



· Ç ½″∅ X 1′-2″BOLT WITH ROUND

WASHERS (TYP.)

---- Q GUARDRAIL

ASSEMBLY

ANCHOR

 $-1^{1}/_{4}$ " \varnothing HOLE (TYP.)

SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

Q GUARDRAIL 1'-10" ANCHOR ASSEMBLY Ĺ JT. ⊚ — END BENT € GUARDRAIL
ANCHOR ASSEMBLY

> LOCATION OF ANCHORS FOR GUARDRAIL

PLAN

END BENT No.1 SHOWN, END BENT No.2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $1/4^{\prime\prime}$ HOLD DOWN PLATE AND 7 - $1/8^{\prime\prime}$ Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8'' Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

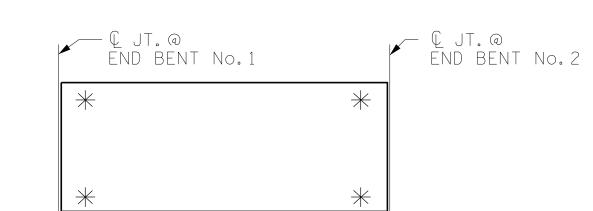
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " \varnothing HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.3.R.5

SAMPSON COUNTY

STATION: 13+22.00 -L-

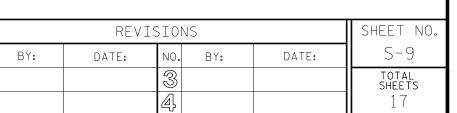
SHEET 5 OF 5



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

GUARDRAIL ANCHORAGE BARRIER RAIL

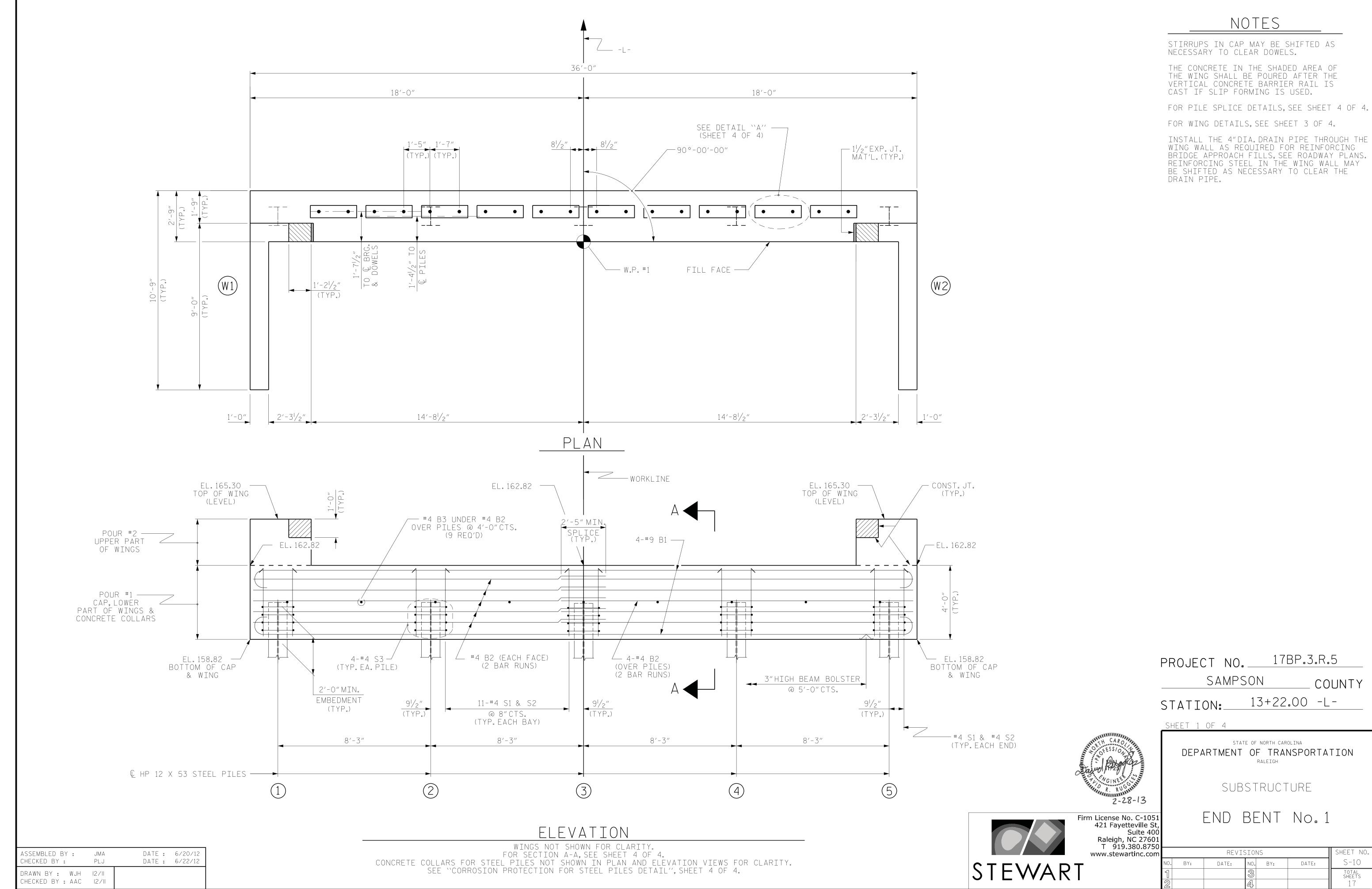


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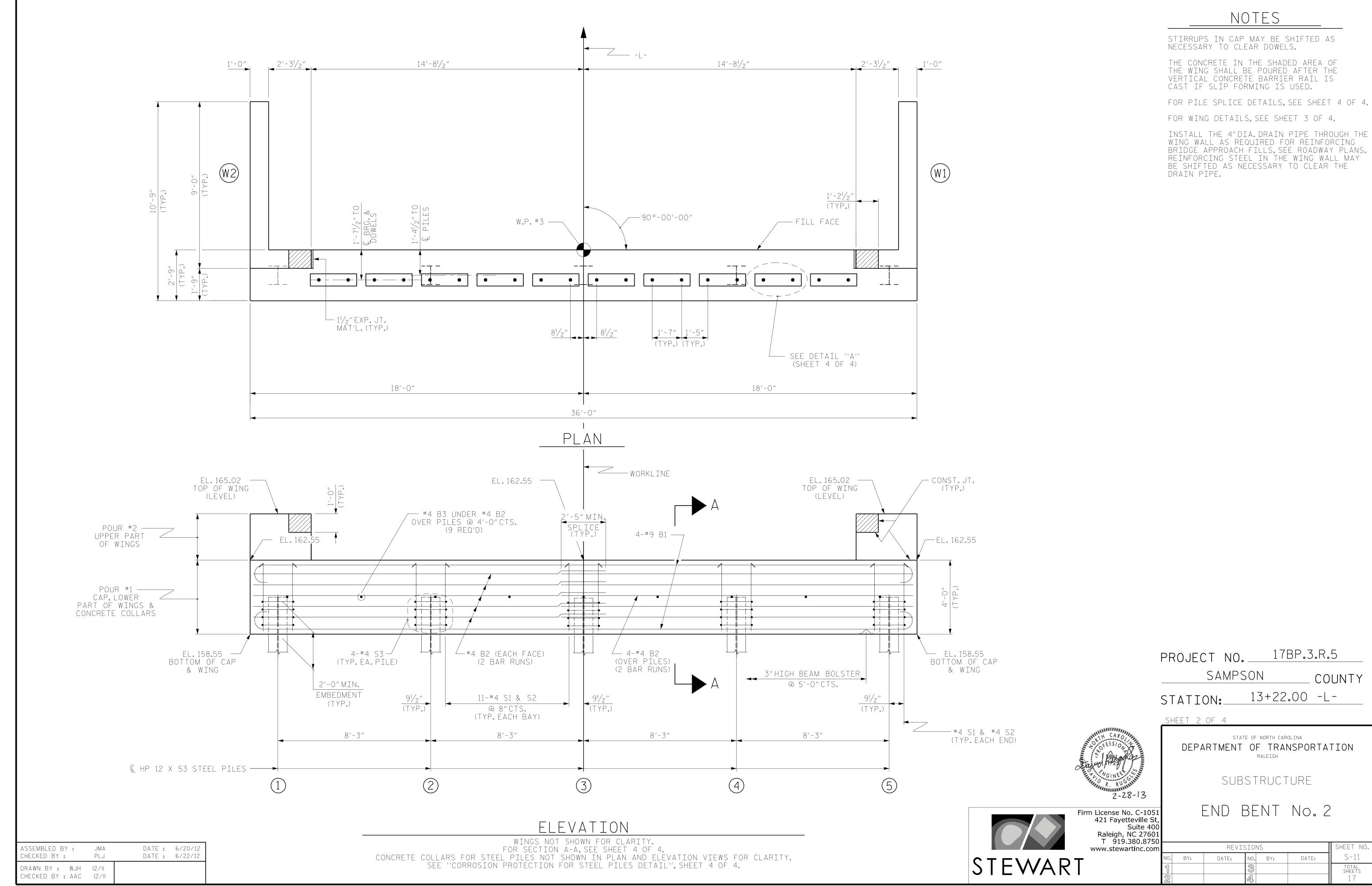
DATE: 6/20/12 SSEMBLED BY: JMA DATE: 6/22/12 CHECKED BY : DDED 5/6/10 DRAWN BY: MAA 5/10 REV. IO/I/II MAA/GM CHECKED BY: GM 5/10

1/4" HOLD-DOWN P

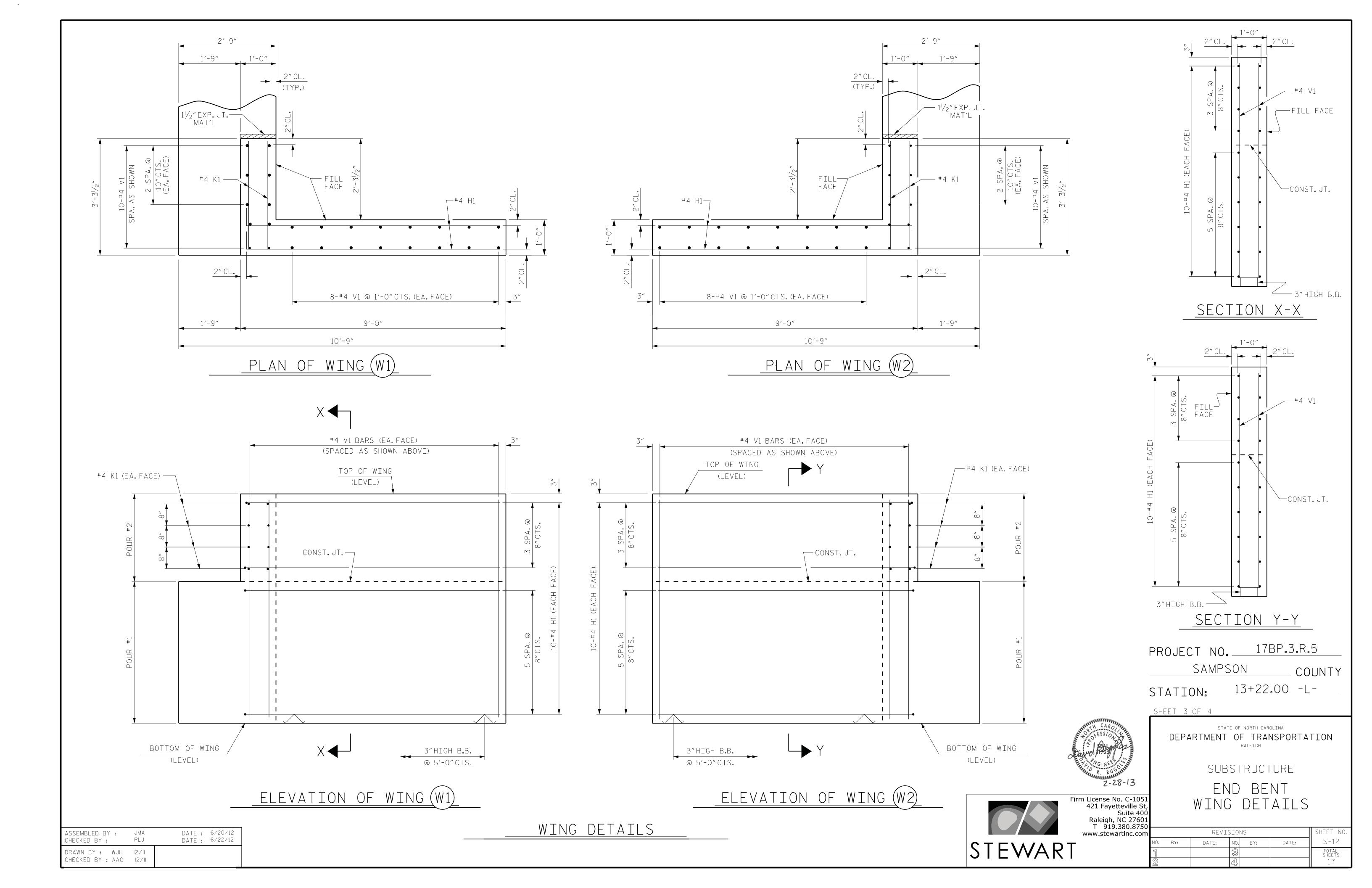
STD. NO. GRA3 (SHT 1)

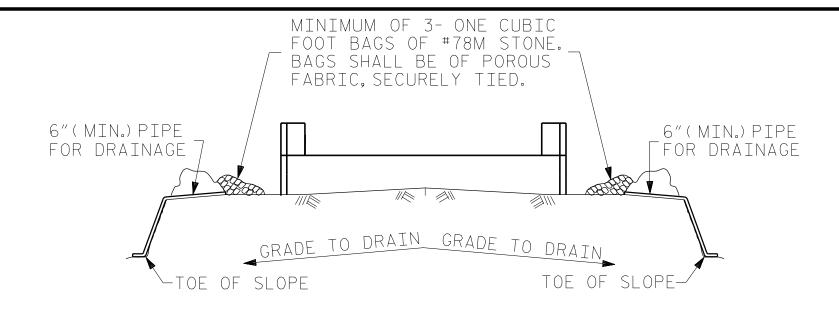


SHEET NO S-10 TOTAL SHEETS



	SHEET NO.						
BY:	DATE:	DATE:	S-11				
		3			TOTAL SHEETS		
		4			17		



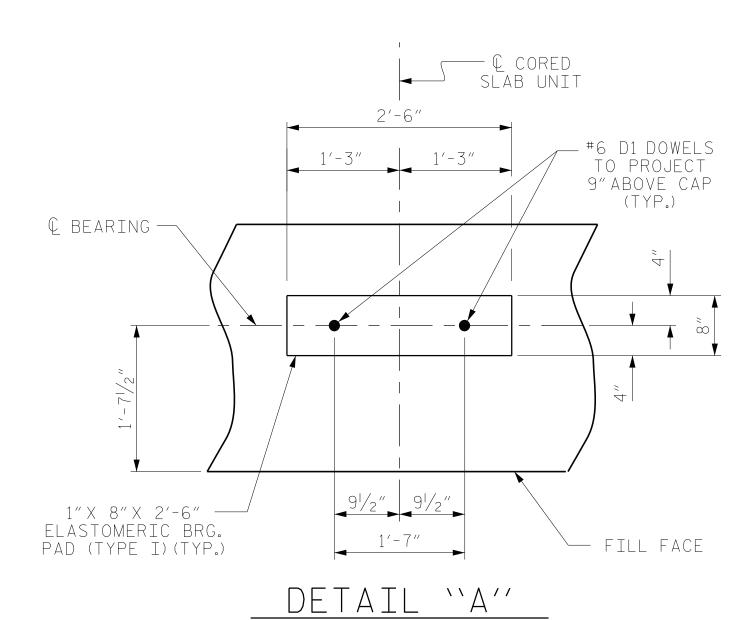


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

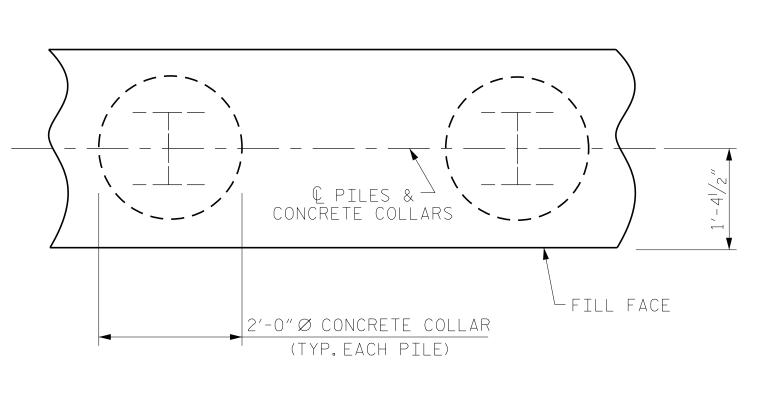
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

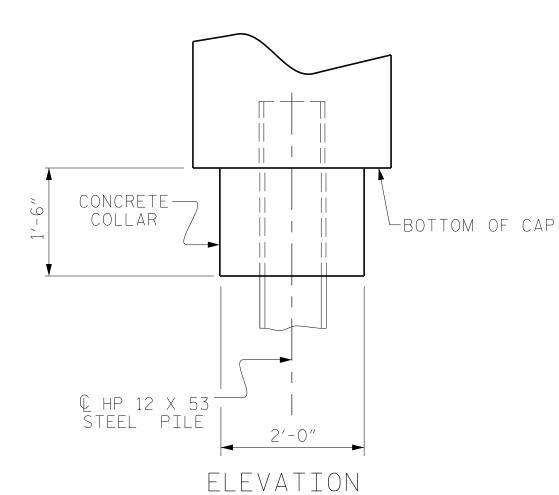
TEMPORARY DRAINAGE AT END BENT



(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

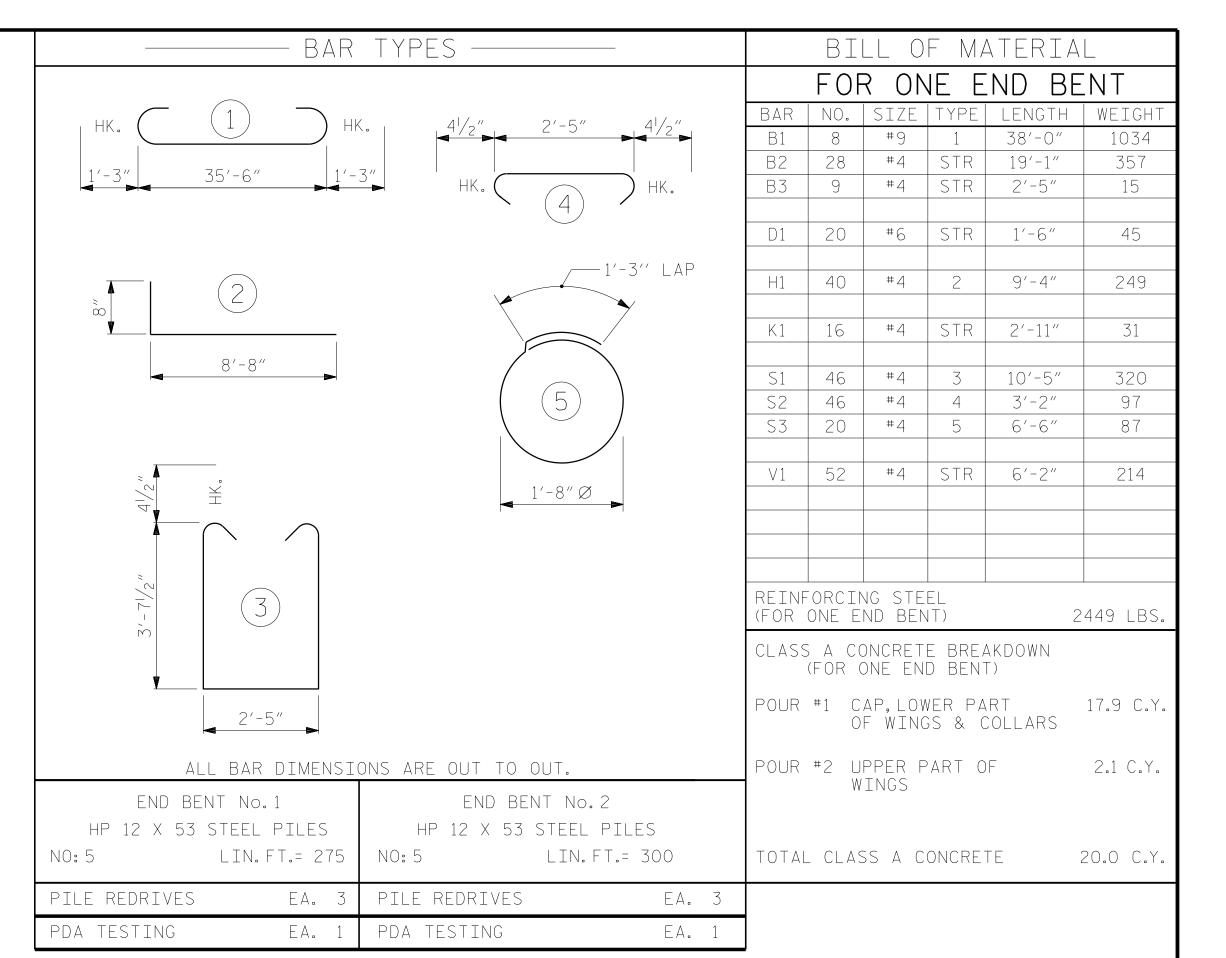


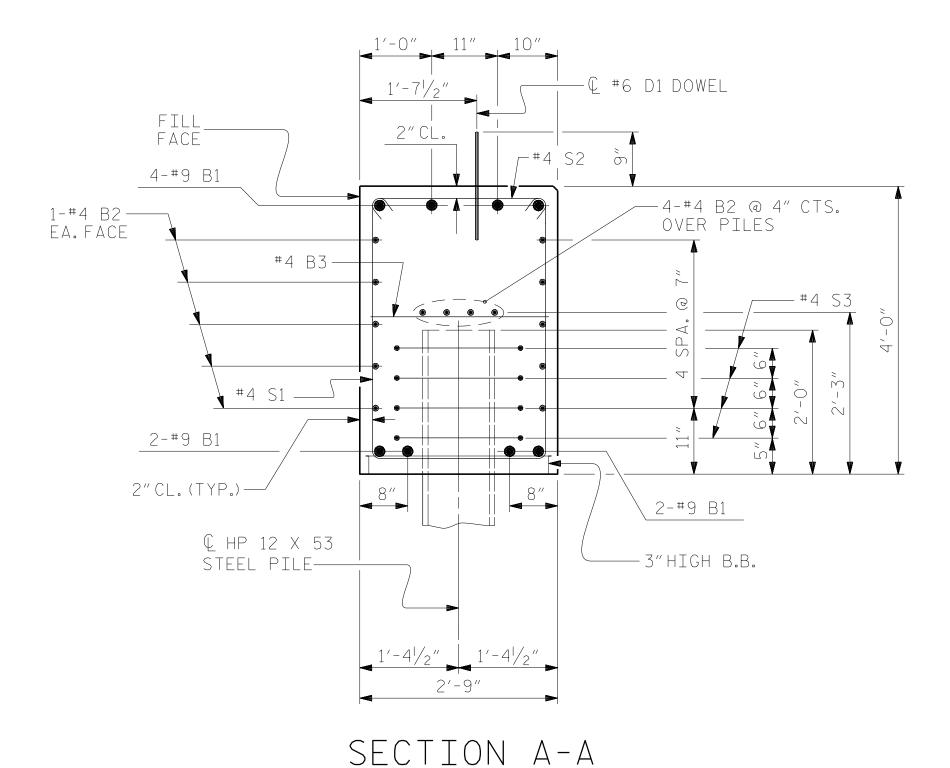
PLAN



BACK GOUGE DETAIL B PILE HORIZONTAL OR VERTICAL $0'' T0 \frac{1}{8}'$ DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS





(CONCRETE COLLAR NOT SHOWN FOR CLARITY. SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL.")

SAMPSON STATION:_

13+22.00 -L-

PROJECT NO. 17BP.3.R.5

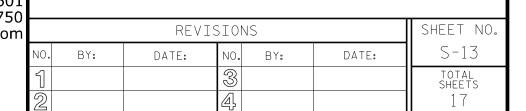
SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

COUNTY

SUBSTRUCTURE

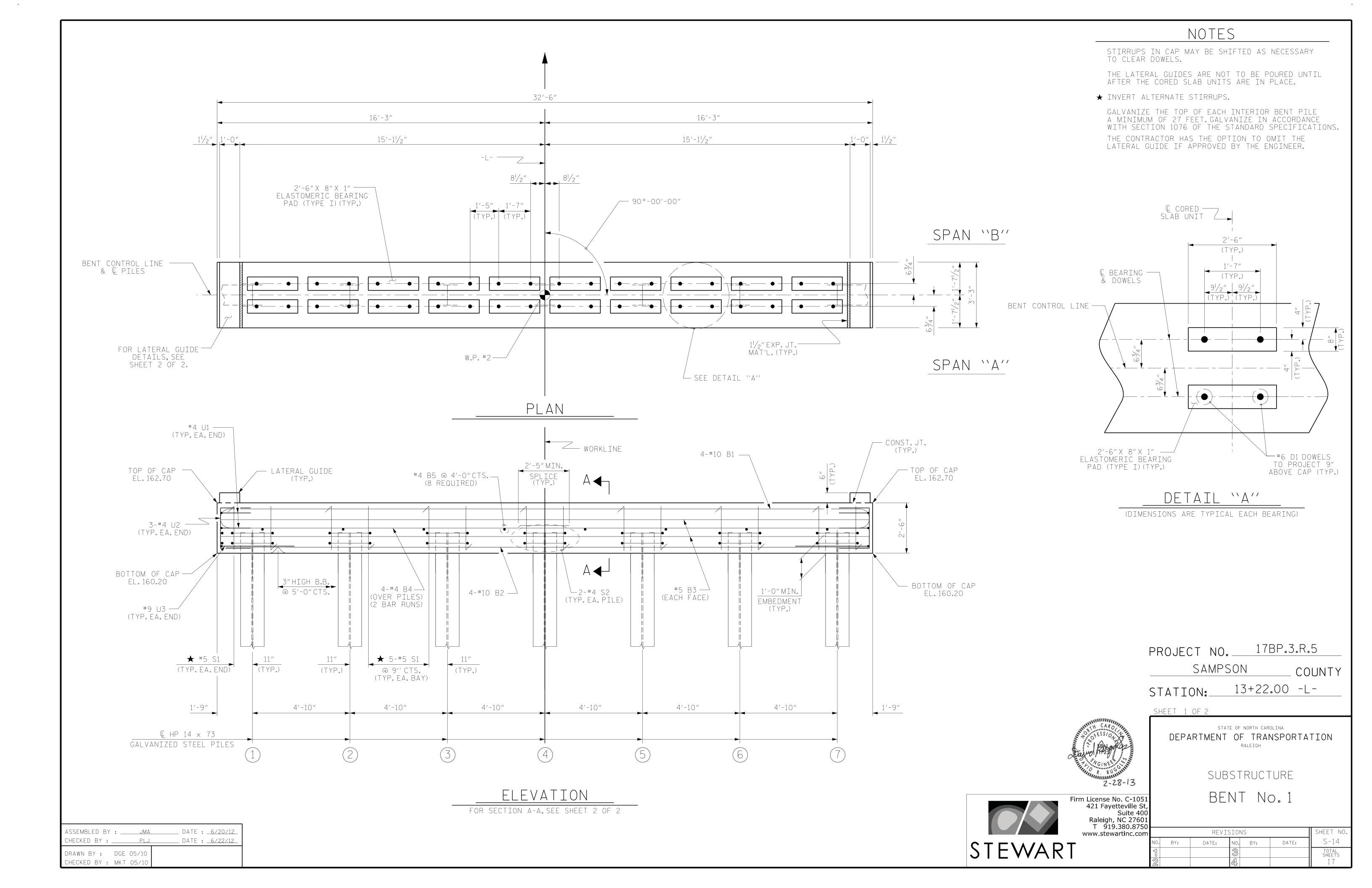
BENT No.1 & 2 DETAILS

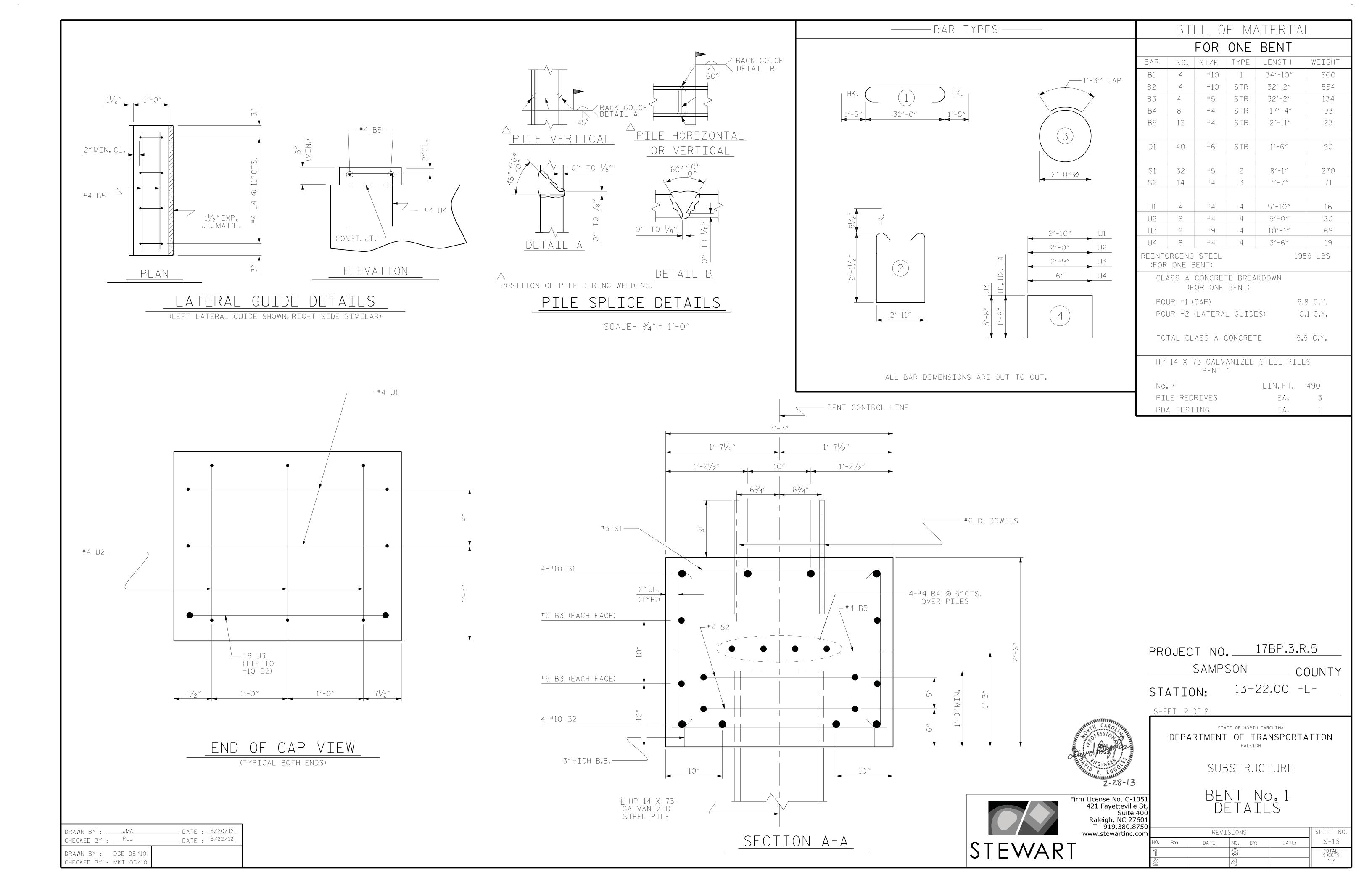


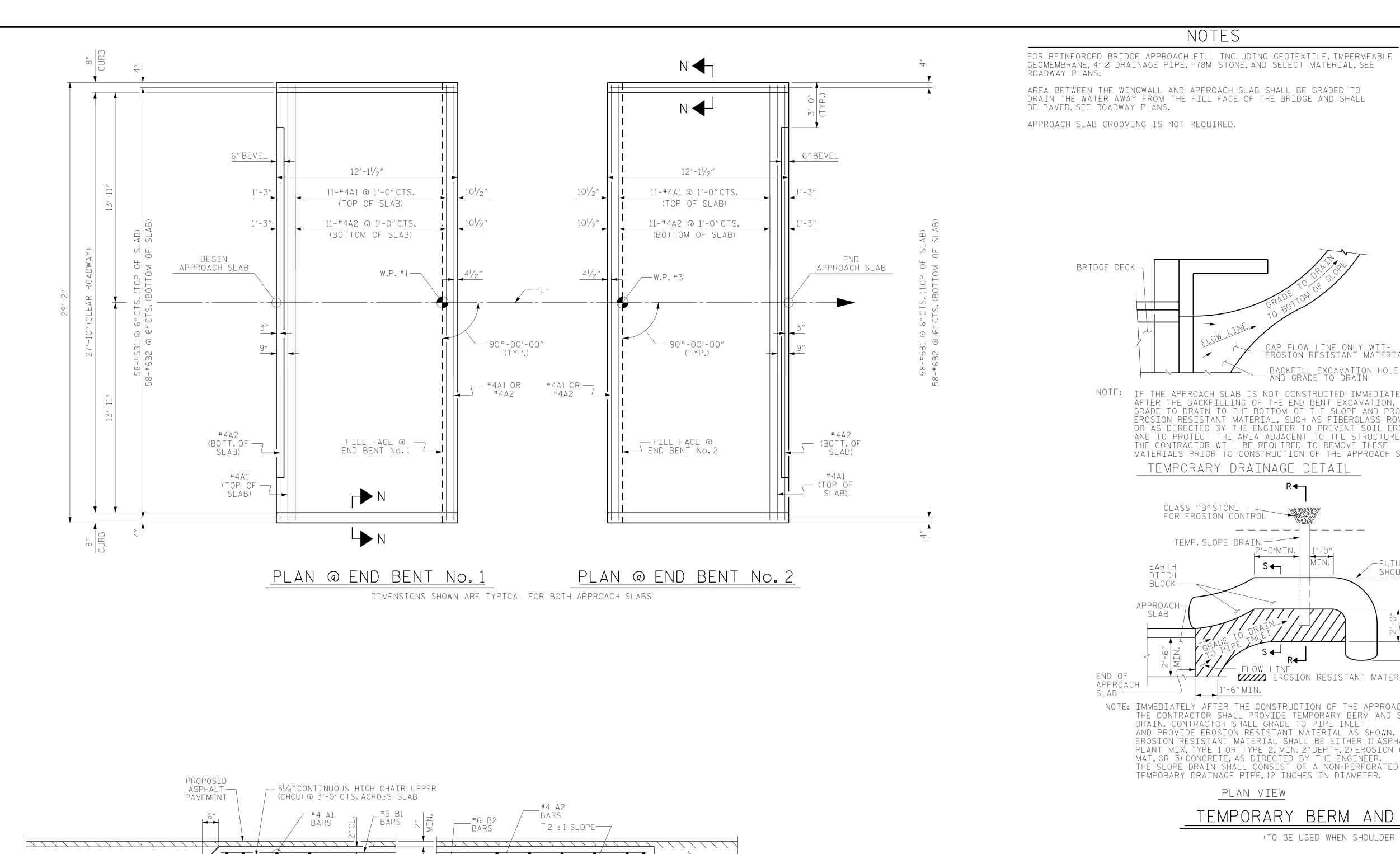
(END BENT No.1 SHOWN, END BENT No.2 SIMILAR BY ROTATION)

ASSEMBLED BY: JMA DATE: 6/20/12 CHECKED BY : DATE: 6/22/12 DRAWN BY: WJH 12/II CHECKED BY : AAC 12/11

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△APPROVED WIRE BAR SUPPORTS @ 3′-0″CTS. ₹

LIMITS OF REINFORCED BRIDGE
-APPROACH FILL (ROADWAY PAY —

SELECT MATERIAL

SECTION THRU SLAB

GEOTEXTILE—

#78M STONE—

4"Ø PERFORATED —

SCHEDULE 40 PVC PIPE

ITEM, SEE NOTES)

- ROADWAY

DATE: 2/26/13

DATE: 2/27/13

PLJ

DRR

DRAWN BY : SHS/MAA 5-09 REV. 12-11 MAA/AAC

ASSEMBLED BY :

CHECKED BY: BCH 5-09

HECKED BY:

[†] normal to end bent

- CORED SLAB

 $-1\frac{1}{2}$ " BACKER ROD

V— IMPERMEABLE GEOMEMBRANE

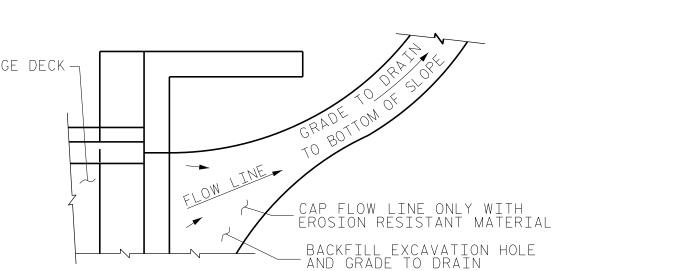
— 2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND

NOTES

FOR REINFORCED BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, IMPERMEABLE GEOMEMBRANE, 4" Ø DRAINAGE PIPE, #78M STONE, AND SELECT MATERIAL, SEE

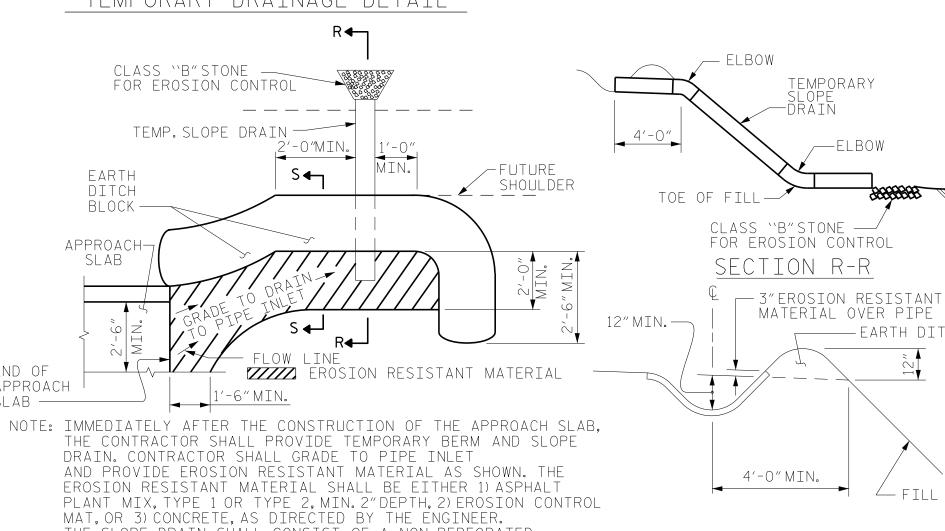
AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

TEMPORARY DRAINAGE DETAIL



PLAN VIEW

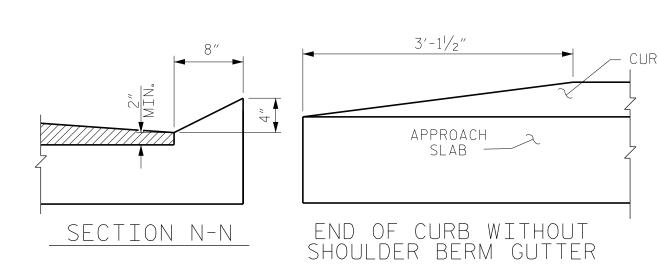
TEMPORARY BERM AND SLOPE DRAIN DETAILS

Suite 400

Raleigh, NC 27601 T 919.380.8750

www.stewartinc.com

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)



CURB DETAILS

SPL	ICE LE	NGTHS
BAR SIZE	EPOXY COATED	UNCOATED
#4	2'-0"	1'-9"
#5	2'-6"	2'-2"
#6	3′-10″	2'-7"



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STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SECTION S-S

PROJECT NO. 17BP.3.R.5

SAMPSON

STATION:

STANDARD

13+22.00 -L-

BILL OF MATERIAL

APPROACH SLAB AT EB #1

BAR | NO. | SIZE | TYPE | LENGTH | WEIGH

APPROACH SLAB AT EB #2

BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT

1016

1266

1016

1266

LBS.

LBS.

C.Y.

LBS.

LBS.

C.Y.

EARTH DITCH BLOCK

COUNTY

* A1 | 13 | #4 | STR | 28'-10"

A2 | 13 | #4 | STR | 28'-10"

*B1 58 #5 STR 11'-2"

REINFORCING STEEL

REINFORCING STEEL

CLASS AA CONCRETE

REINFORCING STEEL

REINFORCING STEEL

CLASS AA CONCRETE

* EPOXY COATED

* EPOXY COATED

B2 | 58 | #6 | STR | 11'-8"

* A1 | 13 | #4 | STR | 28'-10" A2 | 13 | #4 | STR | 28'-10"

*B1 | 58 | #5 | STR | 11'-2" B2 | 58 | #6 | STR | 11'-8"

BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT

90° SKEW

	SHEET NO.					
NO.	BY:	DATE:	NO.	BY:	DATE:	S-16
1			3			TOTAL SHEETS
9			/AL			17

STEWART

STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS - - - - - - - - - - - - - A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SO. TN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ----- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER - - - -375 LBS. PER SQ. IN. EQUIVALENT FLUID PRESSURE OF EARTH - - - - -30 LBS. PER CU. FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLAB; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4"WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12"INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS.
SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN

ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN, AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" SHEAR STUDS FOR THE 3/4" STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" STUDS FOR 4 - 3/4" STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" STUDS ALONG THE BEAM AS SHOWN FOR 3/4" STUDS BASED ON THE RATIO OF 3 - 7/8" STUDS FOR 4 - 3/4" STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY, AT HIS OPTION.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS, THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE".

ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL RAPD POSTS NOT BE ACCEPTED CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

17BP.3.R.5 PROJECT NO. __ SAMPSON COUNTY 13+22.00 -L-STATION:_



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Suite 40 Raleigh, NC 2760 T 919.380.875 BY:

STANDARD NOTES

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

REVISIONS SHEET NO S-17 DATE: NO. BY: DATE: TOTAL SHEETS 17

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