

PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.80 /A

DOCUMENT NOT CONSIDERED FINAL

**UNLESS ALL SIGNATURES COMPLETED** 

ROADWAY DESIGN
ENGINEER

H CARO

4/14/2022

S E A L

Docusigned (b) 3/3400

David Z. Keiser

Serad good England

CDM Smith Inc.
5400 Glenwood Avenue
Suite 400
Raleigh, NC 27612-3228
NC COA No. F-1255

GENERAL NOTES:

2018 SPECIFICATIONS
EFFECTIVE: 01-16-2018
REVISED:

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

### CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY MODIFIED METHOD III.

### SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

### SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

### SUBSURFACE DRAINS:

SUBSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD, NO. 815.02 AT LOCATIONS DIRECTED BY THE ENGINEER.

### GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

### END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

### UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE

BRUNSWICK ELECTRIC MEMBERSHIP — POWER DISTRIBUTION
BRUNSWICK COUNTY PUBLIC WORKS — WATER DISTRIBUTION
ATLANTIC TELEPHONE — TELECOMMUNICATION

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS, EXCEPT AS SHOWN ON THE PLANS.

### RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

INDEX OF SHEETS

SHEET NUMBER SHEET

1 TITLE SHEET

1A INDEX OF SHEETS, GENERAL NOTES AND STANDARD DRAWINGS

1B CONVENTIONAL SYMBOLS

2A-1 THRU 2A-2 PAVEMENT SCHEDULE AND TYPICAL SECTIONS

2C-1 ROADWAY DETAILS

2G-1 THRU 2G-2 GEOTEXTILE DETAILS

3B-1 ROADWAY SUMMARY

3D-1 DRAINAGE SUMMARY

3G-1 GEOTECHNICAL SUMMARY

4 PLAN AND PROFILE SHEET

RWO1 THRU RWO4-REV RIGHT OF WAY PLANS

TMP-1 THRU TMP-2 TRANSPORTATION MANAGEMENT PLANS

PMP-1 THRU PMP-2 PAVEMENT MARKING PLANS

EC-1 THRU EC-5 EROSION CONTROL PLANS

U0-1 THRU U0-2 UTILITIES BY OTHERS PLANS

UC-1 THRU UC-4 UTILITY CONSTRUCTION PLANS

X-1 THRU X-9 CROSS-SECTION SHEET INDEX AND CROSS-SECTIONS

S-1 THRU S-26 STRUCTURE PLANS

EFF. 01-16-2018

### 2018 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2018 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE DIVISION 2 - EARTHWORK

200.03 Method of Clearing - Modified Method III (Use Detail in Lieu of Standard) 225.02 Guide for Grading Subgrade - Secondary and Local 225.04 Method of Obtaining Superelevation - Two Lane Pavement

275.01 Rock Plating (Use Detail in Lieu of Standard)

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation 310.10 Driveway Pipe Construction

DIVISION 4 - MAJOR STRUCTURES

422.02 Bridge Approach Fills - Type II Modified Approach Fill

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

876.02 Guide for Rip Rap at Pipe Outlets

560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

Subsurface Drain
Concrete Base Pad for Drainage Structures
Brick Grated Drop Inlet Type 'B' - 12" thru 36" Pipe
Frames and Narrow Slot Flat Grates
Concrete Curb, Gutter and Curb & Gutter
Drop Inlet Installation in Shoulder Berm Gutter
Guardrail Placement
Guardrail Installation
Structure Anchor Units

NRADAM SINGALAMATAN SINGALAMATA STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

PROJECT REFERENCE NO. 17BP**.**3.R**.**80

# CONVENTIONAL PLAN SHEET SYMBOLS

BOUNDARIES AND PROPERT	<b>Y</b> :	RAILROADS:	<b>1</b> L/
State Line		Standard Gauge	
County Line		RR Signal Milepost	CSX TRANSPORTATION  O
Township Line		Switch —	MILEPOST 35
City Line		RR Abandoned	<i>SWITCH</i>
Reservation Line		RR Dismantled	
Property Line			NTDOI
Existing Iron Pin (EIP)		RIGHT OF WAY & PROJECT CO.	NIROL:
Computed Property Corner		Primary Horiz Control Point	
Existing Concrete Monument (ECM)		Primary Horiz and Vert Control Point	
Parcel/Sequence Number —		Secondary Horiz and Vert Control Point ——	
Existing Fence Line		Vertical Benchmark	^
Proposed Woven Wire Fence		Existing Right of Way Monument	
Proposed Chain Link Fence		Proposed Right of Way Monument ————— (Rebar and Cap)	
Proposed Barbed Wire Fence		Proposed Right of Way Monument	
•		(Concrete)  Existing Permanent Easement Monument	<u></u>
Proposed Wetland Boundary		Existing Permanent Easement Monument —— Proposed Permanent Easement Monument ——	$\langle \cdot \rangle$
Existing Endangered Animal Boundary —		(Rebar and Cap)	<b>\Delta</b>
Existing Endangered Plant Boundary ——		Existing C/A Monument ————	$\triangle$
Existing Historic Property Boundary		Proposed C/A Monument (Rebar and Cap) —	
		Proposed C/A Monument (Concrete) ———	
Known Contamination Area: Soil		Existing Right of Way Line	
Potential Contamination Area: Soil		Proposed Right of Way Line ————	
Known Contamination Area: Water		Existing Control of Access Line ————	<u>(Ĉ)</u>
Potential Contamination Area: Water		Proposed Control of Access Line ———	<del></del>
Contaminated Site: Known or Potential		Proposed ROW and CA Line ————	RW
BUILDINGS AND OTHER CU	LTURE:	Existing Easement Line —————	E
Gas Pump Vent or U/G Tank Cap	O	Proposed Temporary Construction Easement—	E
Sign —	<u> </u>	Proposed Temporary Drainage Easement——	TDE
Well —	O	Proposed Permanent Drainage Easement ——	PDE
Small Mine	<del></del>	Proposed Permanent Drainage/Utility Easement	DUE
Foundation —		Proposed Permanent Utility Easement ———	PUE
Area Outline		Proposed Temporary Utility Easement ———	TUE
Cemetery		Proposed Aerial Utility Easement ————	AUE
Building —		ROADS AND RELATED FEATURE	<b>'S:</b>
School		Existing Edge of Pavement	
Church		Existing Curb	
Dam —		Proposed Slope Stakes Cut	
HYDROLOGY:		Proposed Slope Stakes Fill	
Stream or Body of Water —		Proposed Curb Ramp	CR
Hydro, Pool or Reservoir —		Existing Metal Guardrail	
Jurisdictional Stream		Proposed Guardrail	
Buffer Zone 1		Existing Cable Guiderail	
Buffer Zone 2	BZ 2	Proposed Cable Guiderail	
Flow Arrow		Equality Symbol	•
Disappearing Stream —	<u> </u>		
Spring —		Pavement Removal	
Wetland	<u> </u>	VEGETATION:	
Proposed Lateral, Tail, Head Ditch		Single Tree	£
False Sump —	← FLOW	Single Shrub	\$

Hedge -

		W
Woods Line		\
Orchard		\
Vineyard —	- Vineyard	\
EXISTING STRUCTURES:		\ (
MAJOR:		l
Bridge, Tunnel or Box Culvert	CONC	l
Bridge Wing Wall, Head Wall and End Wall	- ) CONC WW (	l
MINOR:		
Head and End Wall		<i>-</i>
Pipe Culvert		T\
Footbridge ————————————————————————————————————		7
Drainage Box: Catch Basin, DI or JB		Į
Paved Ditch Gutter		
Storm Sewer Manhole		(
Storm Sewer	s	
UTILITIES:		
* SUE - Subsurface Utility Engineering		
LOS – Level of Service – A,B,C or D POWER:	(Accuracy)	
Existing Power Pole	_	l
Proposed Power Pole		Į
Existing Joint Use Pole		G
Proposed Joint Use Pole		(
Power Manhole		
Power Line Tower		
Power Transformer		
U/G Power Cable Hand Hole		
H–Frame Pole		4
U/G Power Line Test Hole (SUE – LOS A)*		S
U/G Power Line (SUE – LOS B)*		•
U/G Power Line (SUE – LOS C)*		,
U/G Power Line (SUE – LOS D)*	P	ļ
TELEPHONE:		4
Existing Telephone Pole		•
Proposed Telephone Pole		•
Telephone Manhole	<del>-</del>	•
Telephone Pedestal	-	•
Telephone Cell Tower	- <b>,</b>	M
U/G Telephone Cable Hand Hole	- H <sub>H</sub>	Į
U/G Telephone Test Hole (SUE – LOS A)* —		Į
U/G Telephone Cable (SUE – LOS B)*		ι
U/G Telephone Cable (SUE – LOS C)*	- — — T — — —	ι
U/G Telephone Cable (SUE – LOS D)*	- т	ι
U/G Telephone Conduit (SUE – LOS B)*	- — — — TC— — —	ι
U/G Telephone Conduit (SUE – LOS C)*	- — — TC— — —	ι
U/G Telephone Conduit (SUE – LOS D)*	- тс	
U/G Fiber Optics Cable (SUE – LOS B)*	- — — — T FO— — ·	(
U/G Fiber Optics Cable (SUE – LOS C)*		
U/G Fiber Optics Cable (SUE – LOS D)*		É

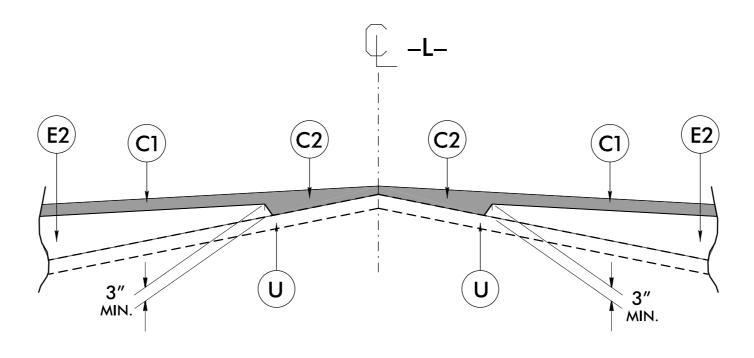
WATER:	
Water Manhole	W
Water Meter	
Water Valve	$\otimes$
Water Hydrant	e <sup>©</sup>
U/G Water Line Test Hole (SUE – LOS A)*	<b>&amp;</b>
U/G Water Line (SUE – LOS B)*	
U/G Water Line (SUE – LOS C)*	
U/G Water Line (SUE – LOS D)*	
Above Ground Water Line	
TV:	
TV Pedestal	C
TV Tower —	$\bigotimes$
U/G TV Cable Hand Hole	H <sub>H</sub>
U/G TV Test Hole (SUE – LOS A)*	•
U/G TV Cable (SUE – LOS B)*	
U/G TV Cable (SUE – LOS C)*	тv
U/G TV Cable (SUE – LOS D)*	Tv
U/G Fiber Optic Cable (SUE – LOS B)* ——	TV FO
U/G Fiber Optic Cable (SUE – LOS C)* ——	—— — TV FO— ——
U/G Fiber Optic Cable (SUE – LOS D)*	
GAS:	
Gas Valve	$\Diamond$
Gas Meter ———————————————————————————————————	$\Diamond$
U/G Gas Line Test Hole (SUE – LOS A)* —	
U/G Gas Line (SUE – LOS B)*	
U/G Gas Line (SUE – LOS C)*	
U/G Gas Line (SUE – LOS D)*	
Above Ground Gas Line	A/G Gas
SANITARY SEWER:	
Sanitary Sewer Manhole	
Sanitary Sewer Cleanout ————————————————————————————————————	<b>(±)</b>
U/G Sanitary Sewer Line ——————	
Above Ground Sanitary Sewer ————	
SS Force Main Line Test Hole (SUE – LOS A)*	
SS Force Main Line (SUE – LOS B)*	
SS Force Main Line (SUE – LOS C)*	
SS Force Main Line (SUE – LOS D)*	FSS———
MISCELLANEOUS:  Utility Pole ————————————————————————————————————	_
Utility Pole with Base ————————————————————————————————————	
	<u> </u>
Utility Unknown U.G. Line (SUE LOS B)*	S
Utility Unknown U/G Line (SUE - LOS B)*	
U/G Tank; Water, Gas, Oil ———————————————————————————————————	
Underground Storage Tank, Approx. Loc. ——	(UST)
A/G Tank; Water, Gas, Oil ———————————————————————————————————	
Geoenvironmental Boring ————————————————————————————————————	AATUR
End of Information	E.O.I.
	L. <b>V</b> .I.

	PAVEMENT SCHEDULE  (FINAL PAVEMENT DESIGN)
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS.
C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 1" IN DEPTH OR GREATER THAN 1.5" IN DEPTH.
E1	PROP. APPROX. 5.5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 627 LBS. PER SQ. YD.
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0C AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5.5" IN DEPTH.
J1	PROP. 8" AGGREGATE BASE COURSE
R1	SHOULDER BERM GUTTER
Т	EARTH MATERIAL.
U	EXISTING PAVEMENT.
V	INCIDENTAL MILLING
W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE -L- WEDGING DETAIL)

NOTE: ALL PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.

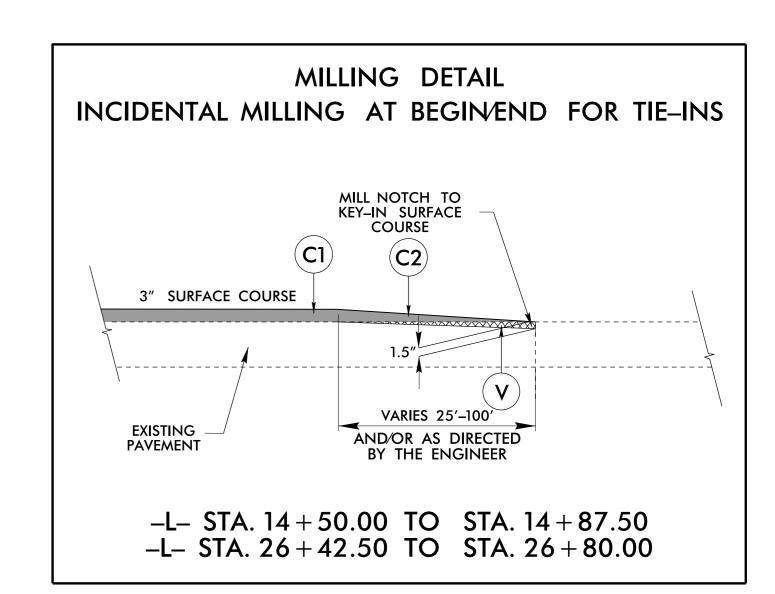
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17BP.3.R.80		2A-I
ROADWAY DESIGN ENGINEER  H CARO  SE AL  033400  Doewslaged by:  David D Zoska Shiring	P/	AVEMENT DESIGN ENGINEER
CDM Smith Inc. 5400 Glenwood Avenue Suite 400 Raleigh, NC 27612-3228 NC COA No. F-1255		

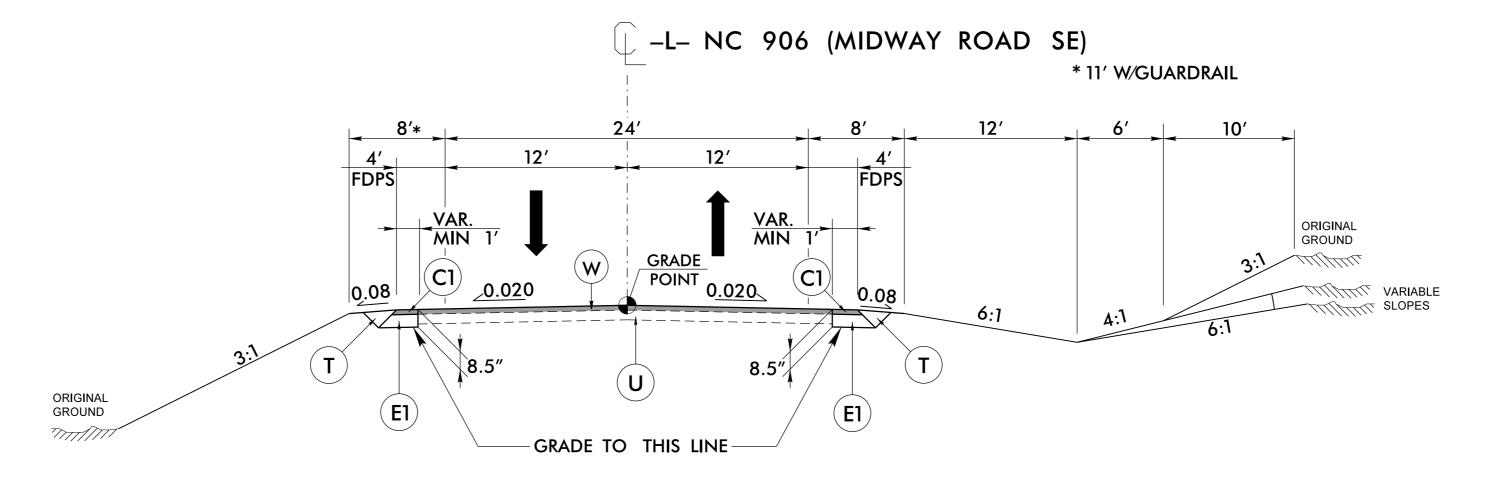
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Detail Showing Method Of Wedging

\_L\_





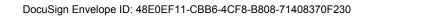
### TYPICAL SECTION NO. 1

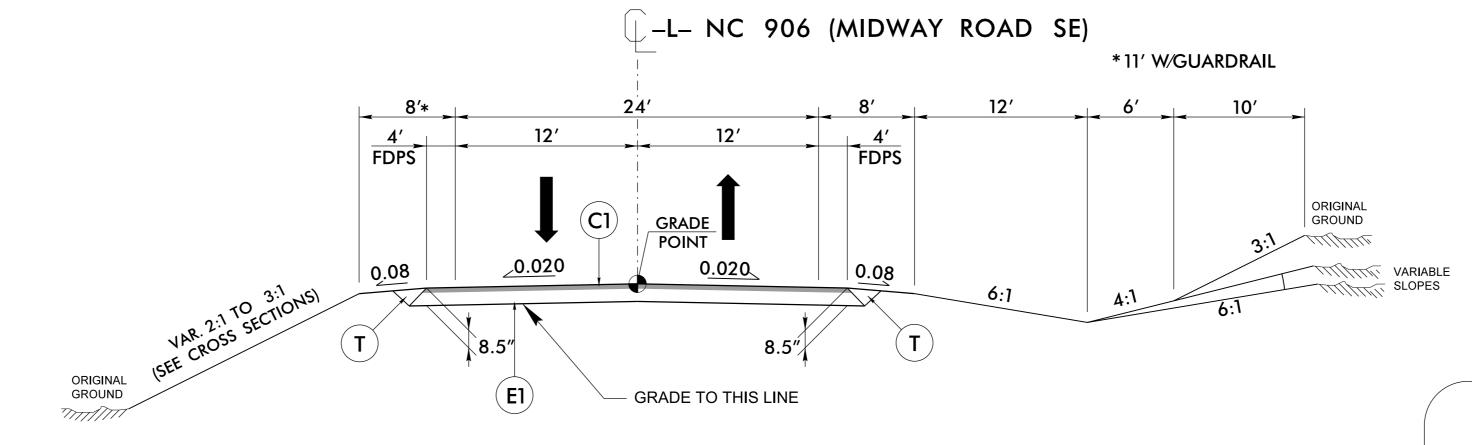
USE TYPICAL SECTION NO. 1

TA. 14 + 50.00 TO STA. 16 + 12.19

-L- STA. 14 + 50.00 TO STA. 16 + 12.10 -L- STA. 24 + 60.93 TO STA. 26 + 80.00

garianga kacamatanga kabingan



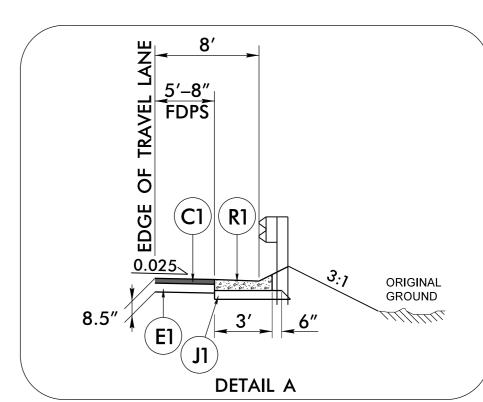


NOTE: 2:1 SLOPES USED WITH ROCK PLATING

### TYPICAL SECTION NO. 2

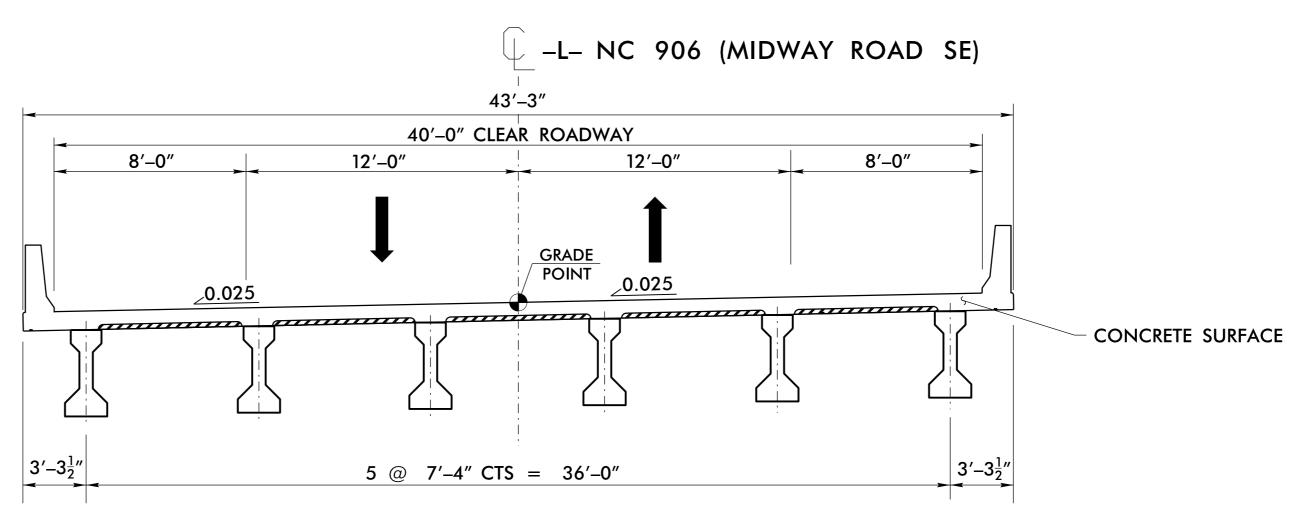
USE TYPICAL SECTION NO. 2

-L- STA. 16+12.10 TO STA. 19+35.50 (BEGIN BRIDGE) -L- STA. 21+00.50 (END BRIDGE) TO STA 24+60.93



USE DETAIL A IN CONJUNCTION WITH TYPICAL SECTION NO. 2

-L- STA. 18+00.00 TO STA. 19+11.00 (LT) -L- STA. 21+24.00 TO STA. 22+02.00 (LT)



### TYPICAL SECTION NO. 3

USE BRIDGE TYPICAL SECTION NO. 3

-L- STA. 19 + 35.50 (BEGIN BRIDGE) TO STA. 21 + 00.50 (END BRIDGE)

PROJECT REFERENCE NO.

17BP.3.R.80

ROADWAY DESIGN
ENGINEER

PAVEMENT DESIGN
ENGINEER

SE AL

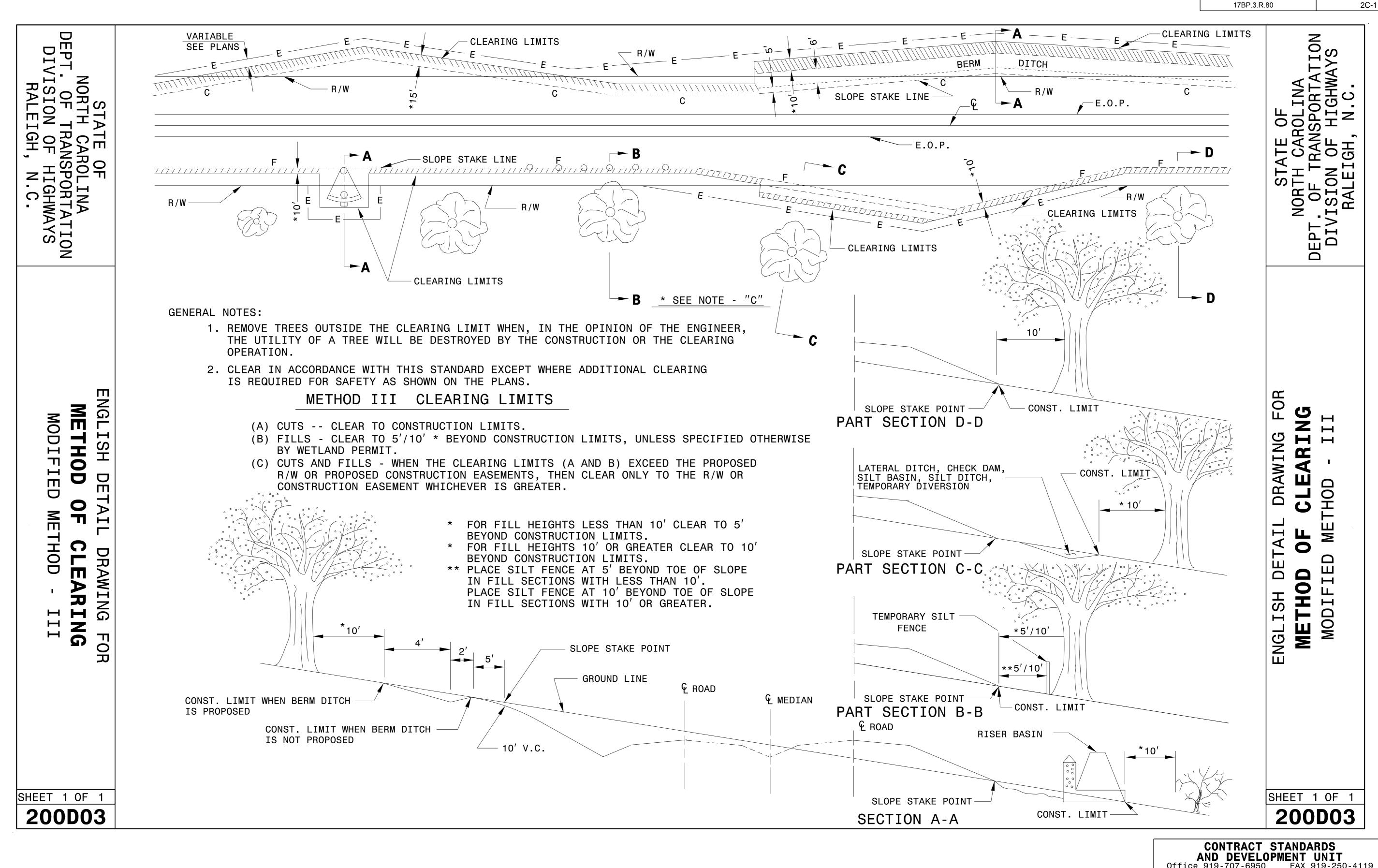
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PA	VEMENT SCHEDULE (FINAL PAVEMENT DESIGN)							
C1	3" S9.5B							
C2	VAR. S9.5B							
E1	5.5" B25.0C							
E2	VAR. B25.0C							
J1	8" ABC							
R1	SBG							
Т	EARTH MATERIAL							
U	EXIST. PAVEMENT							
>	INCIDENTAL MILLING							
W	WEDGING							
	MENT EDGESLOPES 1:1 SS NOTED OTHERWISE							

oj/B5311\_Rdy\_typ.dgn

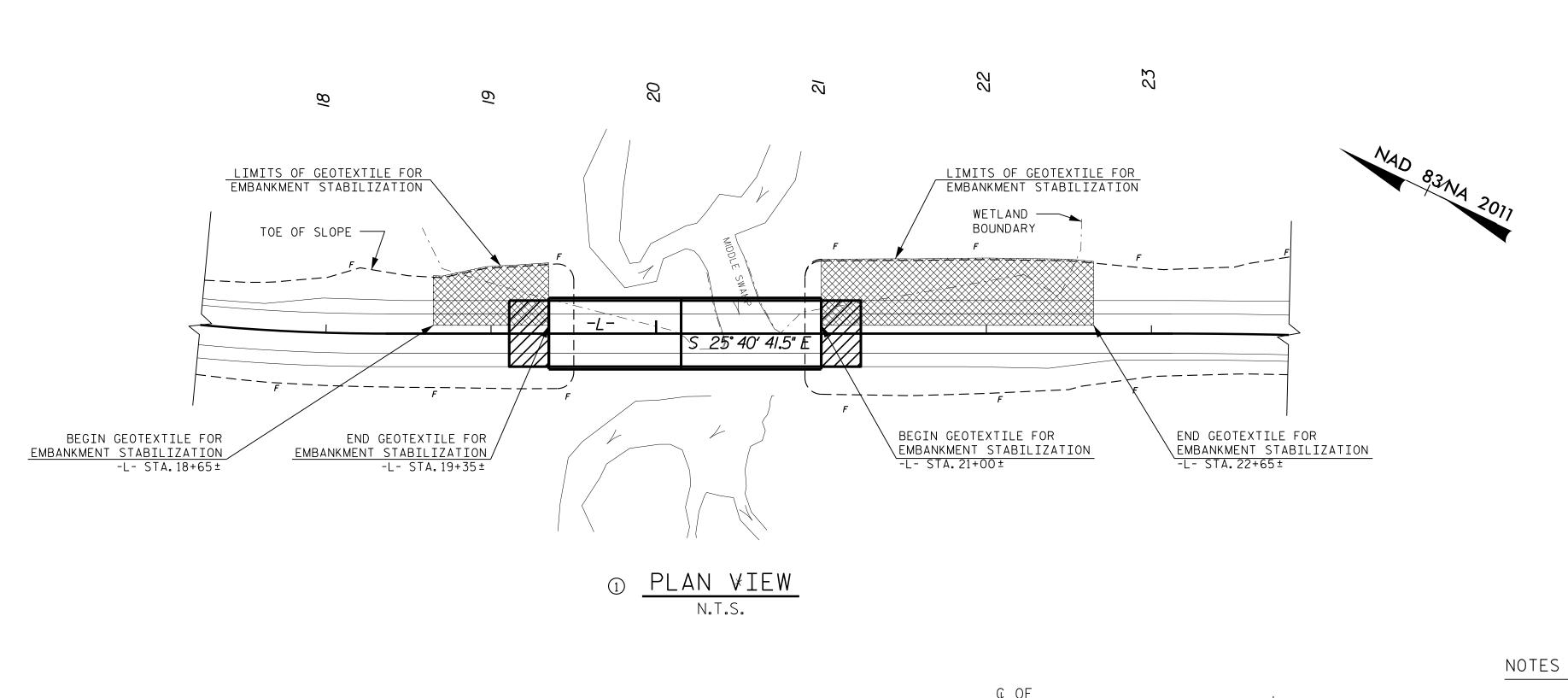


CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

PROJECT REFERENCE NO.

# SEE TITLE BLOCK

RIGINAL BY:	T.S.S.	DATE: _	FEB.2000	
ODIFIED BY:	K.A.K.	DATE: _	AUG.2016	
HECKED BY:		DATE:		
ILE SPEC.: kkemp	f/english/02	00d301.dgn		
				•



FILL

① TYPICAL CROSS SECTION

N.T.S.

- 1. DO NOT GRUB, ONLY CLEAR THE AREA WITHIN THE LIMITS OF THE GEOTEXTILE FOR EMBANKMENT STABILIZATION.
- 2. PLACE GEOTEXTILE FOR EMBANKMENT STABILIZATION PERPENDICULAR TO EMBANKMENT CENTERLINE ON THE EXISTING GROUND AS SHOWN IN THE PLAN OR AS DIRECTED BY THE ENGINEER.
- 3. PLACE THE GEOTEXTILE WITHOUT ANY WRINKLES OR CREASES.
- 4. PLACE 3 FT. OF SELECT GRANULAR MATERIAL ON THE GEOTEXTILE FOR EMBANKMENT STABILIZATION.
- 5. NO SEAMS OR JOINTS ARE ALLOWED IN THE MACHINE DIRECTION OF GEOTEXTILE.
- 6. THE TERMS ROLL AND MACHINE DIRECTION ARE USED INTERCHANGEABLY.
- 7. ALL JOINTS IN THE CROSS MACHINE DIRECTION MUST BE OVERLAPPED A MINMUM OF 18 INCHES.
- 8. FOR GEOTEXTILE FOR EMBANKMENT STABILIZATION, SEE GEOTEXTILE FOR EMBANKMENT STABILIZATION SPECIAL PROVISION.
- 1 9. FOR ROCK PLATING, SEE ROADWAY PLAN SHEET 3G-1.

QUANTITIES

GEOTEXTILE FOR EMBANKMENT STABILIZATION SELECT GRANULAR MATERIAL

1,200 SY# 1,050 CY

PROJECT REFERENCE NO. SHEET NO.

2G-1

**ENGINEER** 

17BP.3.R.80 (B-5311)

10/27/2023

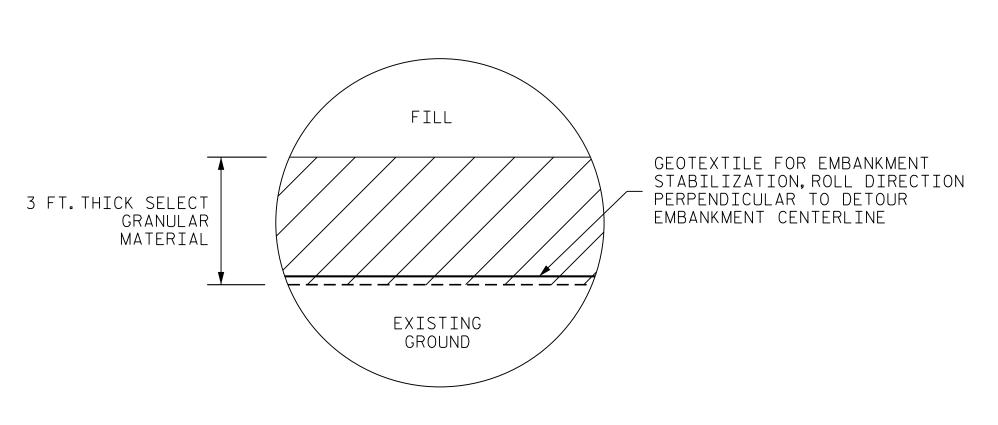
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GEOTECHNICAL ENGINEER

032171

Try of fan

# GEOTEXTILE FOR EMBANKMENT STABILIZATION ESTIMATED QUANTITY DOES NOT INCLUDE OVERLAPS OR WASTE.



SEE GEOTEXTILE -

DETAILS

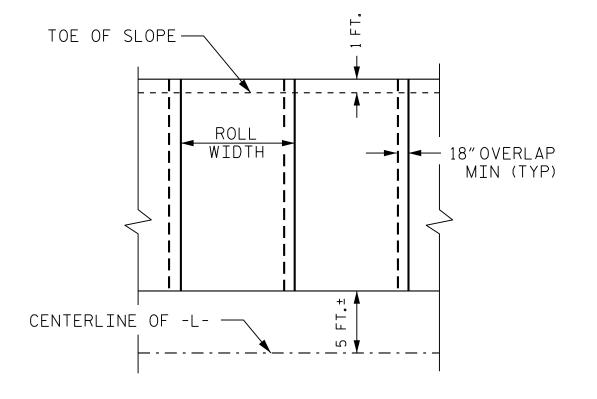
GEOTEXTILE DETAILS

N.T.S.

ROCK PLATING

(SEE NOTE 9)

TOE OF SLOPE ---



5 FT.±

— EXISTING GROUND

GEOTEXTILE OVERLAP DETAIL

(PLAN VIEW, N.T.S.)

DEPARTA
DEPARTA
DIV

NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

### GEOTEXTILE FOR EMBANKMENT STABILIZATION DETAILS

 REVISIONS

 NO.
 BY
 DATE
 NO.
 BY
 DATE

 1
 J. PARK
 10 / 2021 3
 ...
 ...

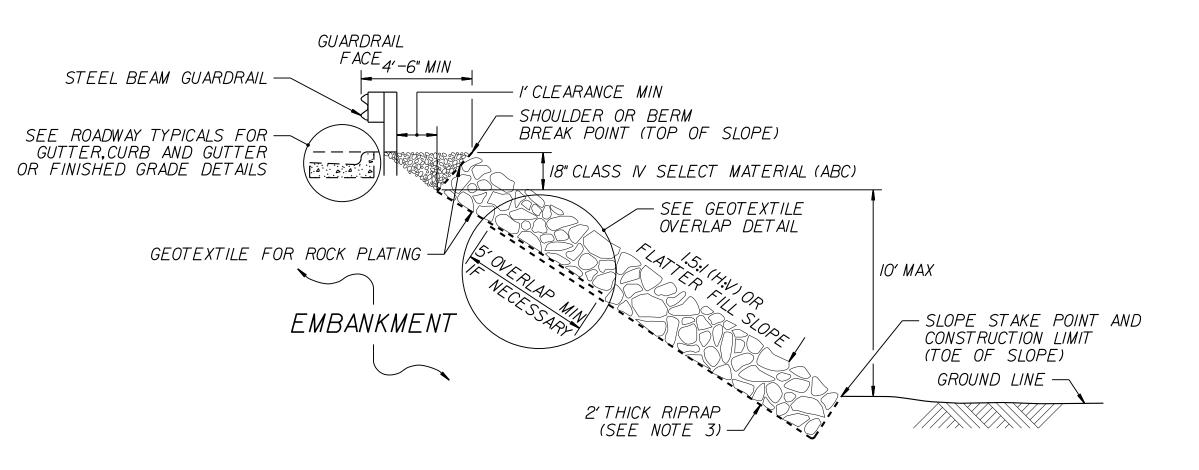
 2
 4
 ...
 ...

PREPARED BY: J. PARK

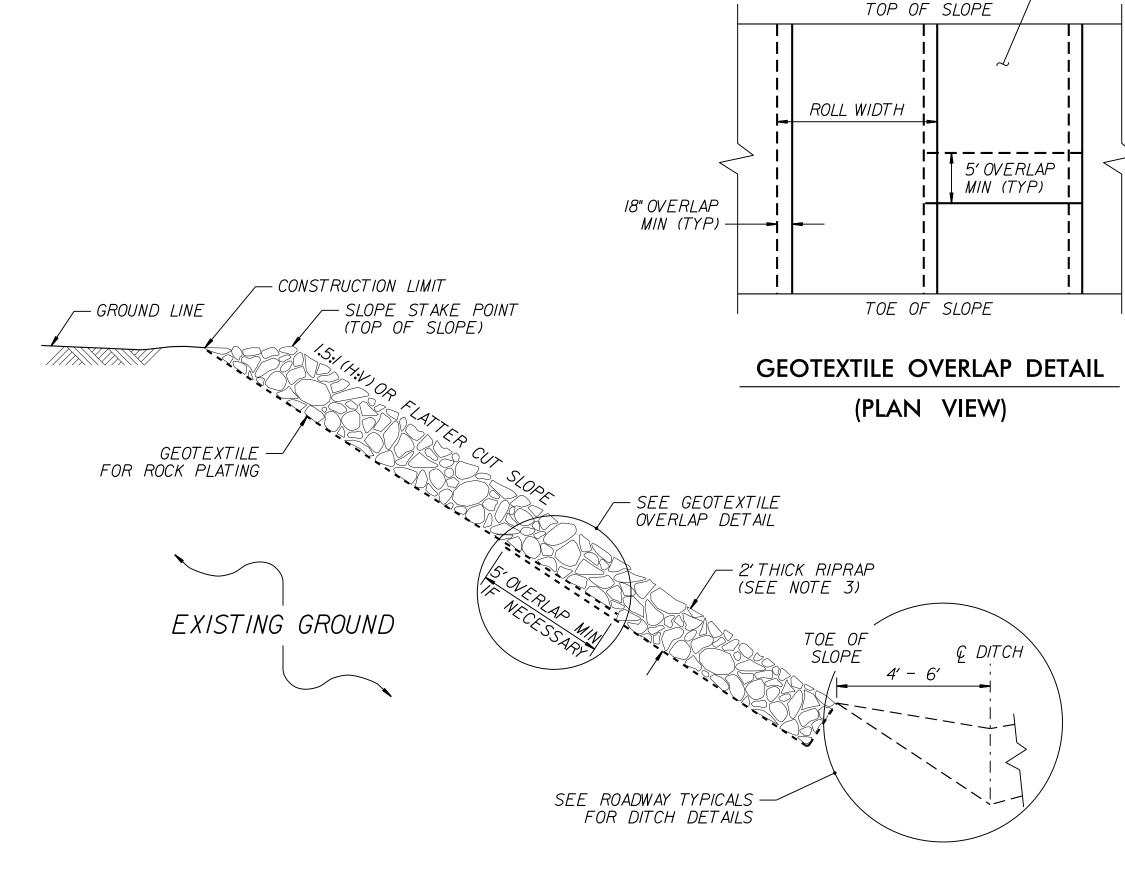
DATE: 8 / 2018

REVIEWED BY: J. BATTS

DATE: 8 / 2018



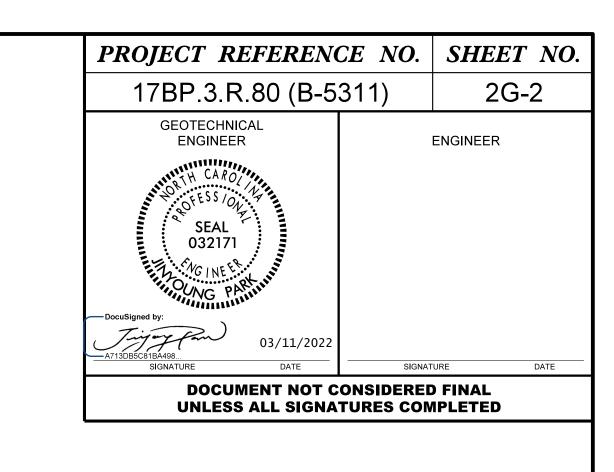
### ROCK PLATING DETAIL NO. 1 - TYPICAL SECTION

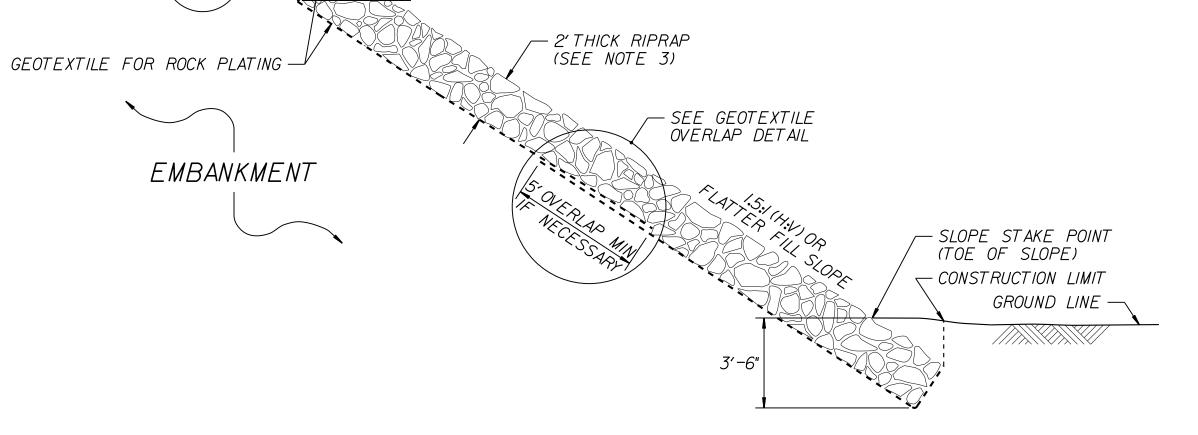


### ROCK PLATING DETAIL NO. 3 - TYPICAL SECTION

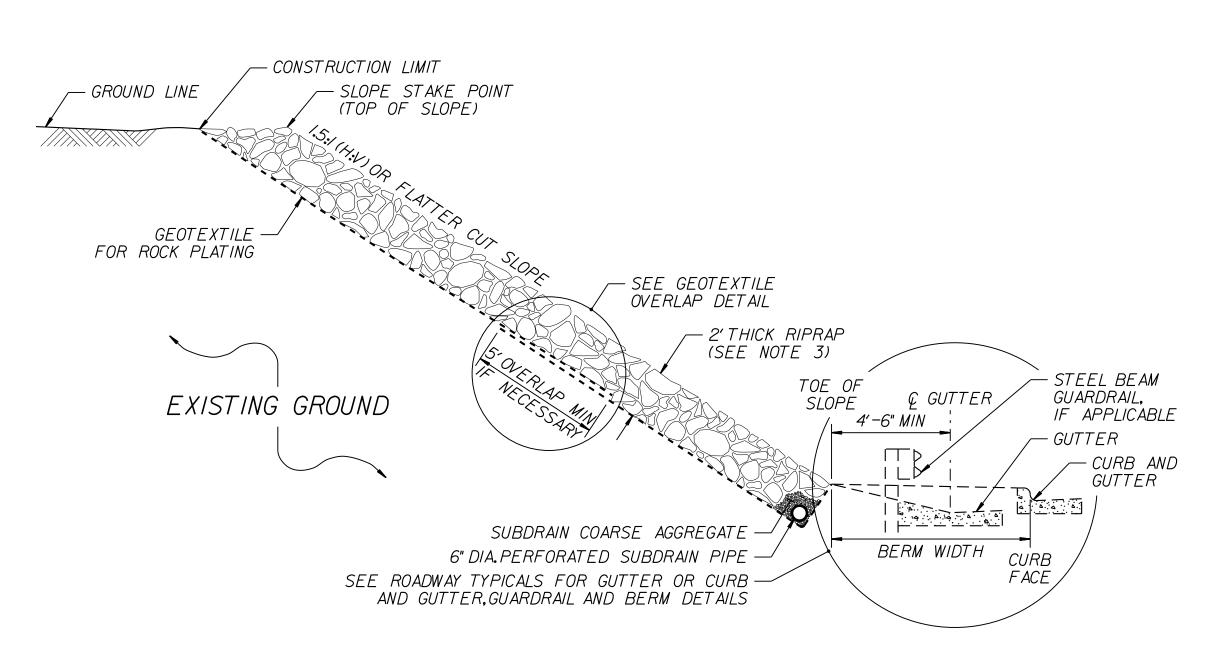
### NOTES:

- I. SEE ROADWAY PLANS AND SUMMARY SHEETS FOR ROCK PLATING LOCATIONS.
- 2. FOR ROCK PLATING, SEE ROCK PLATING (SPECIAL) PROVISION.
- 3. USE CLASS 1,2 OR B RIPRAP UNLESS REQUIRED OTHERWISE IN THE ROADWAY SUMMARY SHEETS.

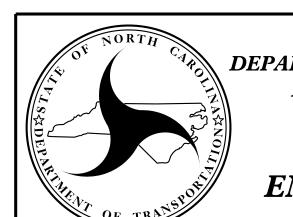




### ROCK PLATING DETAIL NO. 2 - TYPICAL SECTION



### ROCK PLATING DETAIL NO. 4 - TYPICAL SECTION



GUARDRAIL

STEEL BEAM GUARDRAIL

SEE ROADWAY TYPICALS FOR -GUTTER,CURB AND GUTTER OR FINISHED GRADE DETAILS

GEOTEXTILE FOR

ROCK PLATING (TYP)

FACE 4'-6" MIN

'I'CLEARANCE MIN

-SHOULDER OR BERM

BREAK POINT (TOP OF SLOPE)

18" CLASS IV SELECT MATERIAL (ABC)

NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS

GEOTECHNICAL ENGINEERING UNIT

# MODIFIED STANDARD ROCK PLATING DETAILS

REVISIONS									
NO.	BY	DATE	NO.	BY	DATE				
1			3						
2			4						

PREPARED BY: JINYOUNG PARK DATE: 03/2022
REVIEWED BY: JAMEY BATTS DATE: 03/2022

G = GATING IMPACT ATTENUATOR TYPE 350

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

 PROJECT REFERENCE NO.
 SHEET NO.

 17BP.3.R.80
 3B-1

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

### GUARDRAIL SUMMARY

ΕΥ	BEG. STA.	END STA.	LOCATION		LENGTH		WARRA	NT POINT	"N" DIST.	TOTAL	FLARE	FLARE LENGTH		W	ANCHORS			IMPACT ATTENUATOR TYPE 350	IMPACT ATTENUATOR SINGLE REMO TYPE 350 FACED EXISTII GUARDRAIL GUARD		REMOVE AND STOCKPILE	DELLA DICE	
NE	BEG. STA.	END SIA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	SHOUL. WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	TYPE B-77 G	EU –3			EA G NG		GUARDRAIL	EXISTING GUARDRAIL	REMARKS
									1														
_	16 + 35.00	19 + 35.00	RT	300.00			19 + 35.00		8.00	11.00	50.00		1.00		1								
L-	17 + 97.50	19 + 35.00	LT	137.50				19 + 35.00	8.00	11.00		50.00		1.00	1								
_	21+00.00	22+37.50	RT	137.50				21+00.00	8.00	11.00		50.00		1.00	1								
-L-	21+00.00	24+00.00	LT	300.00			21+00.00		8.00	11.00	50.00		1.00		1								
-L-	17 + 47.30	22+03.71	RT			1															460.00		
-L-	19 + 17.16	23+13.56	LT																		400.00		
			SUBTOTALS	875.00											4						860.00		
			TYPE B-77, 4@22.875'	<del>-</del> 91.50																			
			GREU TL-3, 4@50.00'	-200.00																			
			PROJECT TOTALS	583.50											4	4					860.00		
			SAY	587.50		1			1						4	4					860.00		

### SUMMARY OF EARTHWORK (CY)

LOCATION	UNCLASSIFIED EXCAVATION	UNDERCUT	EMBT + %	BORROW	WASTE
-L- 14 + 50.00 - 19 + 35.50	201		3,081	2,880	
-L- 21+00.50 - 26+80.00	72		4,330	4,258	
SUBTOTAL	273		7,411	7,138	
TOTAL	273		7,411	7,138	
PROJECT TOTAL	273		7,411	7,138	
EST. 5% TO REPLACE TOP SOIL ON BORROW PIT				357	
GRAND TOTAL	273		7,411	7,495	
SAY	300			7,500	

UNDERCUT (CONTINGENCY) = 300 CY
SELECT GRANULAR MATERIAL, CLASS III (CONTINGENCY) = 300 CY

NOTE: Earthwork quantities are calculated by the Roadway Design Unit.
These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

Note: Approximate quantities only. Unclassified Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading."

### SUMMARY OF PAVEMENT REMOVAL (SY)

LINE	STATION – STATION	LOCATION	REMOVAL (SY)
-L-	STA. 14+50 TO STA. 14+65	LT	0.86
-L-	STA. 14+50 TO STA. 14+63	RT	0.85
-L-	STA. 15 + 51 TO STA. 16 + 08	RT	8.36
-L-	STA. 16+12 TO STA. 20+19	CL	1,159.39
-L-	STA. 20+75 TO STA. 24+61	CL	1,090.89
-L-	STA. 25+66 TO STA. 25+98	RT	1.54
-L-	STA. 26+56 TO STA. 26+80	RT	1.20
-L-	STA. 26+67 TO STA. 26+80	LT	0.76
GRAND TOTAL			2,263
SAY			2,270

### SUMMARY OF SHOULDER BERM GUTTER (LF)

LINE	STATION – STATION	LOCATION	LENGTH (LF)
-L-	STA. 18 + 00 +/- TO STA. 19 + 11 +/-	LT	111
-L-	STA. 21+24 +/- TO STA. 22+02 +/-	LT	78
GRAND TOTAL			189
SAY			190

COMPUTED BY:	PH	DATE:	04/07/2022
CHECKED BY:		DATE:	

### NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS**

PROJECT NO. 3D-1 17BP.3.R.80

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout.

	See "S	tandard Speci	ification	ns For R	Roads and Struct	ures, S	Section 300-5".	,, oject	COM	LIST OF		PES.	EN	NDW	'ALI	LS.	ETC	. <i>(F</i> (	OR PIPI	ES 4	18 IN	<i>ICH</i>	ES a	& U1	NDE.	(R)										
LINE & STATION	FFSET	STRUCTURE NUMBER		HOO IS		C. S	S. PIPE			R. C. PIPE CLASS III			NDWALLS	.01 OR STD. 838.11 NOTED OTHERWISE) RCED ENDWALLS	NOED ENDWALES	SKAINAGE STRUCTURE	QUANTITIOR DRAIN STRUCTUF  NOTE: TOTAL LIN. FOR PAY QUANTITY SHALL BE A + (1.3 X E	ES AGE RES	FRAME, GRATES, AND HOOD STD. 840.03	CONCRETE TRANSITIONAL	D. 840.04 OR STD. 840.05 PROACH D.I. STD. 840.13		10.17 OR STD. 840.26 10.18 OR STD. 840.27	ME WITH GRATE STD. 840.20	ME W/ 2 GRATES STD. 840.20 IE W/ GRATE STD. 840.22 IE W/ 2 GRATES STD. 840.22	E W/ GRATE STD. 840.24 E W/ 2 GRATES STD. 840.24 NE W/ GRATE STD. 840.29	ME W/ 2 GRATES STD. 840.29 0.30 DRIVEWAY STD. 840.30	D. 840.32 S AND FRAMES STD. 840.33	0.52,	ER STD. 840.54 B. TO J.B. B. TO D.I.	I. TO J.B. 3. TO D.I.	MO	NO.	RS CL. "B" STD. 840.72	ICK PIPE PLUG STD. 840.71	ABBREVIATIONS  C.A.A. CORRUGATED ALUMINIUM ALLOY  C.B. CATCH BASIN  C.S. CORRUGATED STEEL  D.I. DROP INLET  G.D.I. GRATED DROP INLET  H.D.P.E. HIGH DENSITY POLYETHYLENE  J.B. JUNCTION BOX  M.H. MANHOLE
SIZE THICKNESS OR GAUGE	0	FROM TO TO TO TOP ELEVATION	4 INVERT ELEVATION	HINVERT ELEVATION	.064 .064 .064 .064 .079		2 48 54 60 66 72 78 84	12 15	5 18	24 30 36 42 48 54	60 66	72 78		C STD. 838	Y C	MASONRY MASONR	5. 1	11 ABOVE E.B. STD. 840.01 OR STD	GRATE TYPE	D.I. STD. 852.04 OR STE	OPEN THROAT C.B. ST CONCRETE BRIDGE AF	D.I. STD. 840.14 OR STE D.I. FRAME AND GRATE	G.D.I. TYPE "A" STD. 84 G.D.I. TYPE "B" STD. 84	G.D.I. TYPE "D" STD. 84 G.D.I. (W.S. FLAT) FRAI	G.D.I. (W.S. SAG) FRAN G.D.I. (W.S. SAG) FRAN G.D.I. (W.S. SAG) FRAN	G.D.I. (N.S. SAG) FRAM G.D.I. (N.S. SAG) FRAM G.D.I. (N.S. FLAT) FRAM	G.D.I. (N.S. FLAT) FRAN DRIVEWAY D.I. STD. 84 FRAME W/ GRATE FOR	J.B. STD. 840.31 OR STD. ANGLED VANE GRATES /	,   -,	M.H. FRAME AND COVER (CONVERT EXISTING C.B. CONVERT EXISTING C.B.	CONVERT EXISTING D. CONVERT EXISTING J.E	ADJUST D.I.		S FLOWABLE FILL CONCRETE COLLAF	CONCRETE AND BR	N.S. NARROW SLOT P.V.C. POLYVINYL CHLORIDE R.C. REINFORCED CONCRETE T.B.D.I. TRAFFIC BEARING DROP INLET T.B.J.B. TRAFFIC BEARING JUNCTION BOX W.S. WIDE SLOT  REMARKS
L 18+25 L 19+04	19 L <sup>-</sup>	T 0402 25.6 0402 0401 T 0403 27.2	21.6	20.2	12			76								1	1						1				1						2			
L 21+31	19 L	0404 0405 T 0404 0406 0405 0406	23.2 24.1 23.8	23.8	20			64	4							1	1						1				1						2			
L 21+95 L 16+12 L 16+12	19 L <sup>-</sup> 39 R 32 R	T 0407							28					+	+		1	+					1				1								22	
															+																					
															+																					
															+																					
				ET TOTALS					0 28							Δ	4						4				4						4		22	

COMPUTED BY: Tyler Bottoms DATE: 8/24/21 .

CHECKED BY: Jinyoung Park DATE: 9/17/21 (12-17-19)

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# PROJECT NO. SHEET NO. 17BP.3.R.80 (B-5311) 3G-1

### SUMMARY OF SUBSURFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
	CONTIN	IGENCY		SD	200
				TOTAL LF:	200

\*UD = Underdrain \*BD = Blind Drain

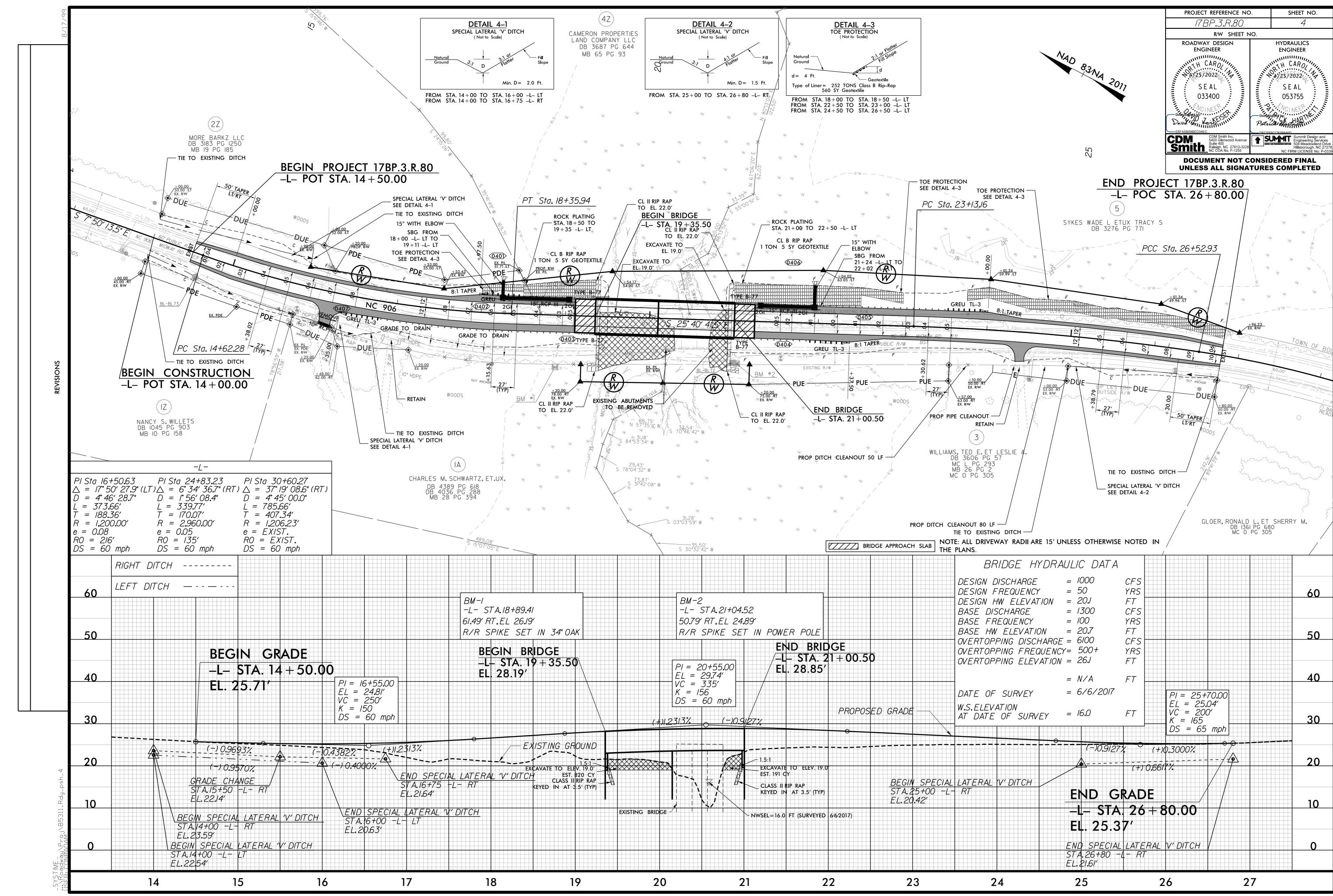
\*SD = Subsurface Drain

REVISED BY: <u>Jinyoung Park</u> DATE: <u>9/17/21</u>

### SUMMARY OF ROCK PLATING

LINE	Beginning Slope (H:V)	Approx. Station	Ending Slope (H:V)	Approx. Station	Location LT/RT	Rock Plating Detail No. 1/2/3/4	Riprap Class* 1/2/B	Rock Plating SY
-L-	2.5:1	18+25 ±	2:1	19+15 ±	LT	2	2	150
-L-	2:1	21+15 ±	2.5:1	22+75 ±	LT	2	2	400
							TOTAL SY:	550

\*Use Class 1, 2 or B riprap if riprap class is not shown for rock plating location.



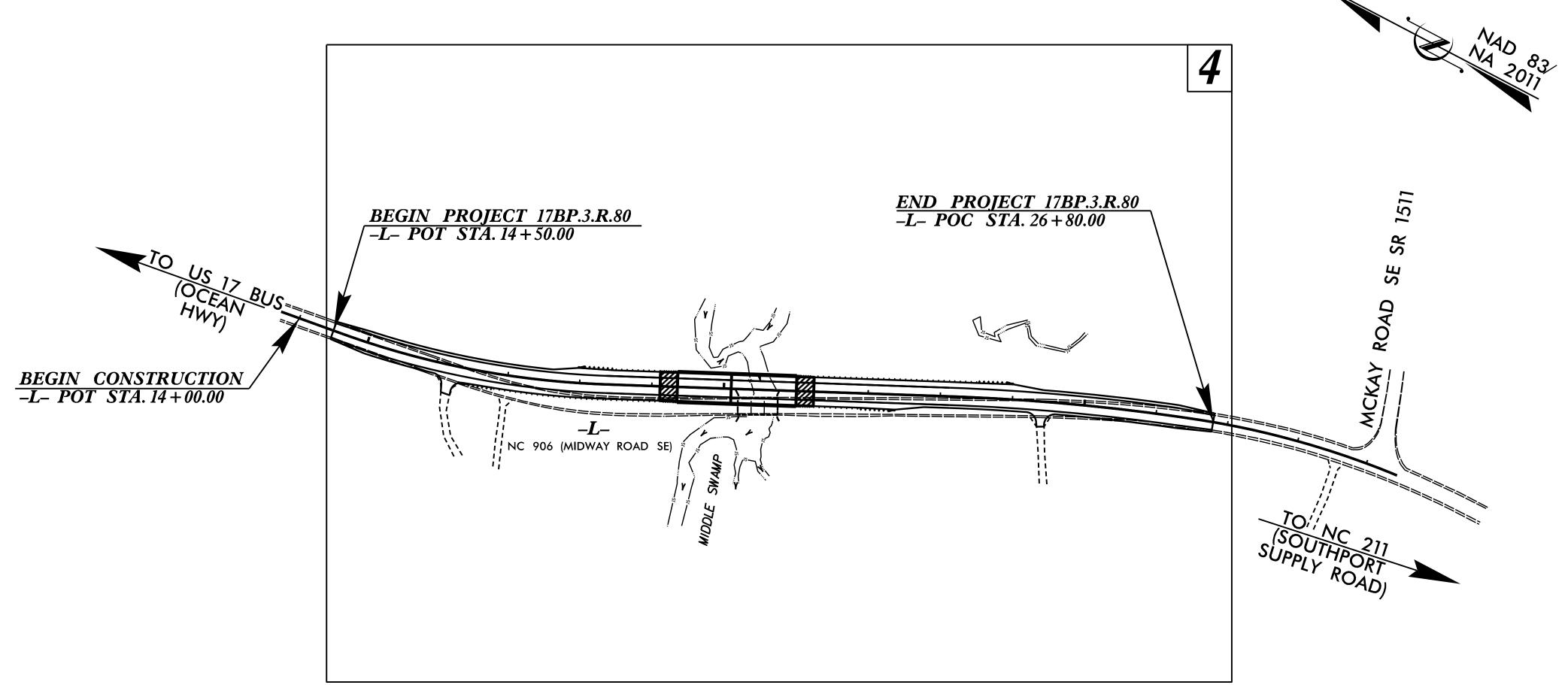
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

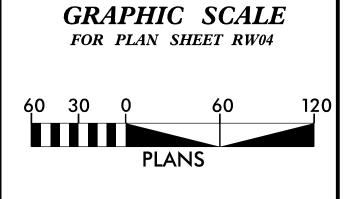
N.C. |RW01| 5 17BP.3.R.80

SURVEY CONTROL, EXISTING CENTERLINES, RIGHT OF WAY, EASEMENTS AND PROPERTY TIES

# BRUNSWICK COUNTY

LOCATION: BRIDGE NO. 104 OVER MIDDLE SWAMP ON NC 906 (MIDWAY ROAD SE)





### **DATUM DESCRIPTION**

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCGS FOR MONUMENT "GPS2" WITH NAD 83/NSRS 2011 STATE PLANE GRID COORDINATES OF NORTHING: 112241.097(ft) EASTING: 2256300.363(ft) ELEVATION: 29.02(ft) THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 1.000092895110 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM "GPS2" TO -L- STATION 10+00.00 IS S 01-02'23.7" E 222.25(ft)

ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES

VERTICAL DATUM USED IS NAVD 88

Prepared in the Office of:

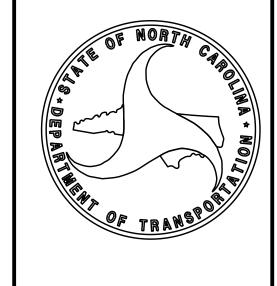
LOCATION AND SURVEYS UNIT DIVISION 3 5310 BARBADOS BLVD, SUITE 102 CASTLE HAYNE, NORTH CAROLINA 28429

2018 STANDARD SPECIFICATIONS

RIGHT OF WAY DATE: **NOVEMBER** 26, 2021

LETTING DATE: AUGUST 18, 2022 **SURVEYOR** 

PROFESSIONAL LAND



### SURVEY CONTROL SHEET

### W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

BL				
POINT	DESC.	NORTH	EAST	ELEVATION
BL71	TRV CAP & REBAR	110189.5491	2256913.9370	27.56
BL72	TRV CAP & REBAR	110945.2166	2256543.9380	22.54
BL73	TRV CAP & REBAR	111557.1623	2256340.5050	24.40
B53112	GPS CAP & REBAR	112241.0970	2256300.3630	29.02
B53111	GPS CAP & REBAR	113387.6921	2256099.5340	34.85

BM1 ELEVATION = 26.19 N 111130 E 2256443 RR SPIKE IN 34"OAK B5311-1 ELEVATION = 24.89 BM2 N 110940 E 2256545

BL-73

RR SPIKE IN POWER POLE

B53II-2

my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

### Class of survey: AA

Type of GPS field procedure: RTN Dates of survey: May 2016 Datum/Epoch: NAVD 88 Published/Fixed-control use: N/A for RTN Localized around: GPS2 Northing: 112241.097 Easting: 2256300.363 Combined grid factor: 1.000092895110

Geoid model: 12A Units: US Survey feet

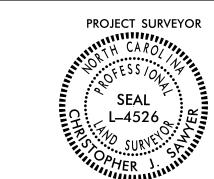
I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed May 2016, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC

This 16th day of March, 2022.

56.1600 as applicable.

Christopher Sawyer Professional Land Surveyor L-4526

I, Christopher J. Sawyer, PLS, certify that the Project Control was verified under



PROJECT REFERENCE NO.

17BP.3.R.80

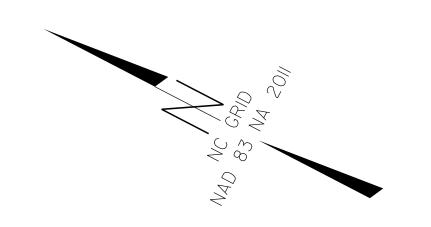
Location and Surveys

DIVISION 3 LOCATION AND SURVEY'S UNIT 5310 BARBADOS BLVD, SUITE 102 CASTLE HAYNE, NORTH CAROLINA 28429

SHEET NO.

RW02C-1

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



EY8				
POINT	N	E	BEARING	DIST
POT	11Ø177.Ø28	2256363.985		
LINE			S 69°43′16.3" E	544.56
POT	109988.290	2256874.791		

13	0.000	ן שששיש								
SPIRAL			N 19°48′Ø3.8" E	99.99						Ø2°22′3Ø.Ø"(LT)
SC	1Ø9688.675	2256816.821								
CURVE			N Ø2°12′38.7" W	842.04	40°51′24.9"(LT)	04°45′00.0"	860.15	449.27	1206.23	
CS	110530.085	2256784.339								
SPIRAL			N 25°48′21.5" W	199.94						04°45′00.0"(LT)
ST	110710.085	2256697.3Ø1								
LINE			N 27°23′21.1" W	467.55						
TS	111125.220	2256482.214								
SPIRAL			N 25°12′Ø7.Ø" W	124.93						Ø6°33′45.Ø"(RT)
SC	111238.256	2256429.019								
CURVE			N 19°34′54.8" W	23.71	Ø2°29′22.6"(RT)	10°30′00.0"	23.71	11.86	545.67	
CS	111260.593	2256421.073								
SPIRAL			N 11°20′10.0" W	199.70						10°30′00.0"(RT)
ST	111456.399	2256381.819								
LINE			N Ø7°5Ø′13.5" W	1450.50						
POT	112893.350	2256184.033								
EY9										
POINT	T N	F	T BEARING T	DIST	DELTA			T	R	
POT	110196.858	2256870.139		2101			_			<del> </del>
LINE	1101 )0:030	2230070:137	N 70°53′17.4" E	55.65						<del> </del>
PC	110215.077	2256922.717	11 / 0 33 1/ a L	55.05						<del> </del>
CURVE	110213.077	2230 )22.717	N 67°36′42.Ø" E	374.27	Ø6°33′11.8"(LT)	01°45′00.0"	374.47	187.44	3274.04	<del> </del>
PT	110357,629	2257268,776	11 07 00 12.00 E	J/ 1.2/	20 00 11:0 (E17	21 13 6616	O / 1	10/8/11	0271801	<del> </del>

### NOTES:

N 27° 23′ 21" W 467.55

BL-72

DELTA S

\BM#2

100.00

200.00

125.00

200.00

66.67

133.38

83.39

66.71

41.72

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. BASELINE CONTROL MONUMENTS FROM R-3434 WERE USED WITH NEW LEVELING AND TRAVERSE PERFORMED ON THE NA 2011 DATUM. THE EXISTING ALIGNMENTS FROM R-3434 WERE USED AND TRANSLATED TO THE NA 2011 DATUM. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

-N 70° 53′ 17" E 55.65

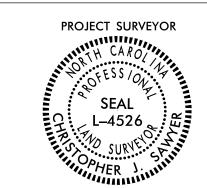
PROJECT REFERENCE NO. 17BP.3.R.80

> DIVISION 3 LOCATION AND SURVEY'S UNIT 5310 BARBADOS BLVD., SUITE 102 CASTLE HAYNE, NORTH CAROLINA 28429

Location and Surveys

SHEET NO.

RW02D-1



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I, Christopher J. Sawyer, PLS, certify that the data compiled came from available surveys/mapping performed by others and provided to me by NCDOT and do not certify to the accuracy or quality of the individual data sources.

This 16th day of March, 2022.

DocuSigned by:
ChristophryLawyer

Professional Land Surveyor L-4526

NORTH STATION EAST POT 2256304.3966 10+00.00 112018.8874 14+62.28 111560.9276 2256367.4314 PT 18+35.94 111204.5768 2256474.7327 PC 23 + 13.16 110774.4866 2256681.5195 PCC 26 + 52 . 93 110460.5012 2256810.8693 PT 34+38.60 109688.6745 2256816.8209

### L(OLD ALIGNMENT)

TYPE	STATION	NORTH	EAST
POT	10+00.00	112018.9396	2256304.7710
PC	15+86.08	111438.4219	2256385.2952
PT	18+48.75	111189.6022	2256465.3Ø15
PC	22+Ø9.26	110870.5686	2256633.1711
PCC	26+64.46	110453.0502	2256813.3886
PT	35+42.94	109594.5943	2256782.948Ø

L(OLD ALIGNMENT) IS FOR REFERENCE ONLY. SOME OF THE R/W WAS PURCHASED UNDER THIS ALIGNMENT BEFORE THE PROJECT WAS REDESIGNED. PLEASE SEE THE MONUMENTS WITH "\*\*".

### NOTES:

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

-NIM4\WORKING\IDM Series Sheets\I/Br.J.R.6M-LS-rwMZd-1.d

# RIGHT OF WAY CONTROL SHEET

17BP.3.R.80	a 10 d	RW03E-1REV
Location	ana	Survevs

PROJECT REFERENCE NO.

DIVISION 3 LOCATION AND SURVEY'S UNIT 5310 BARBADOS BLVD., SUITE 102 CASTLE HAYNE, NORTH CAROLINA 28429



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I, Christopher J. Sawyer, certify that the right of way and permanent easement monumentation for this project shown herein was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:10,000 (Class A). Field work was performed on January 27, 2022, and all coordinates are based on NAD83/2011; That this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 12th day of April, 2022.

Professional Land Surveyor L-4526

ROW MARKER IRON PIN AND CAP-E

ALIGN	STATION	OFFSET	NORTH	<u>EAST</u>	
L	15.90.00	-23.35	111441.1868	2256414.2070	
L	16.70.34	-13.73	111362.4873	2256426.5623	
L	17.53.43	-24.48	111289.1735	2256464.3064	
L	17.61.80	56.24	111251.1553	2256392.5963	
L	17.85.68	57.60	111227.5479	2256400.9951	
L	19.11.60	57.63	111111.4172	2256455.5783	
L	19.20.00	78.00	111095.0202	2256440.8601	
L	19.20.00	57.38	111103.9555	2256459.4444	
L	19.54.17	-54.00	111121.4210	2256574.6262	NOT SET
L	21.20.00	75.00	110916.0717	2256530.2270	
L	21.20.00	51 • 40	110926.2959	2256551.4919	
L	22.04.02	-63.05	110900.1647	2256691.0471	NOT SET
L	24.01.34	-59.99	110718.8261	2256773.3555	
L	26.01.34	-49.96	110526.2684	2256840.4818	NOT SET
L	26.96.23	-30.00	110428.1321	2256852.9841	

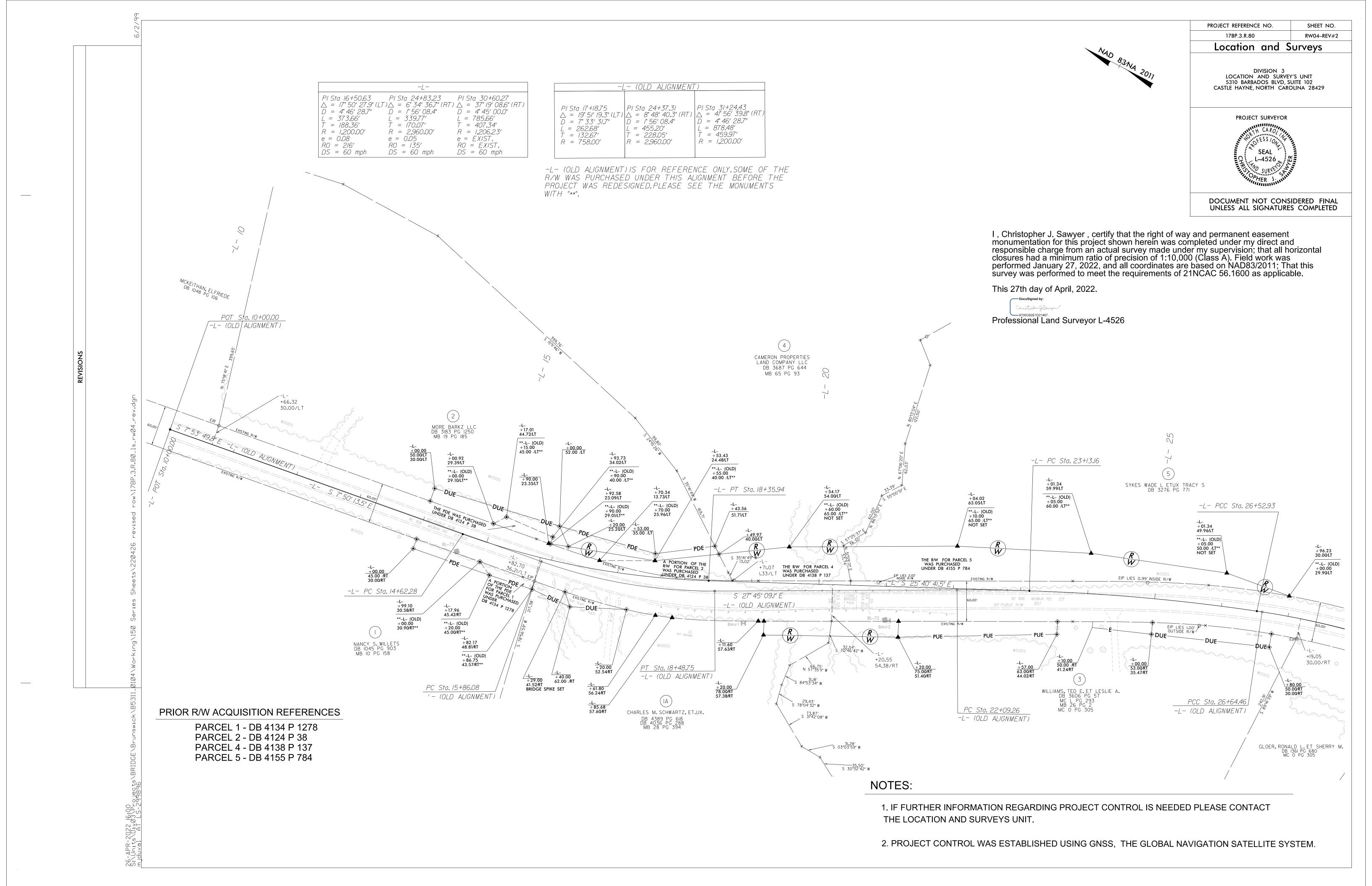
ROW MARKER PERMANENT EASEMENT-E

	I NOW INDI		TINI CHOCMCINI C	=	
AL I GN	STATION	OFFSET	NORTH	EAST	
L	14.00.00	-30.00	111626.7142	2256388.6591	
L	14.00.00	-50.00	111629.4413	2256408.4724	7
L	14.00.00	45.00	111616.4874	2256314.3597	7
L	14.00.00	30.00	111618.5328	2256329.2196	7
L	14.99.10	30.58	111519.4356	2256342.8596	7
L	15.00.92	-29.39	111527.6793	2256402.2906	7
L	15.17.01	-44.72	111515.0065	2256420.1029	7
L	15.17.96	45.42	111497.6883	2256331.6434	1
L	15.82.17	48.81	111431.7280	2256342.2319	7
L	15.92.58	-23.09	111438.6709	2256414.5585	7
L	15.93.73	-34.02	111440.2372	2256425.4361	1
L	16.00.00	-52,00	111438.8105	2256444.3537	7
L	16.20.00	-25.20	111413.2166	2256423.4252	7
L	16.29.00	41.52	111386.5663	2256361.5941	
L	16.40.00	62.00	111369.8744	2256345.0894	
L	17.20.00	52.54	111293.2382	2256380.2911	
L	17.53.00	-35.00	111293.4597	2256473.9196	
L	18.43.56	-51.71	111220.1192	2256524.6323	
L	18.49.97	-40.00	111209.2674	2256516.8554	
L	23.57.00	63.00	110708.3818	2256643.0465	
L	23.57.00	44.02	110716.3498	2256660.2674	
L	25.00.00	35.47	110590.3333	2256724.2410	
L	25.00.00	53.00	110583.7490	2256707.9944	
L	26.80.00	30.00	110425.6475	2256790.8780	
L	26.80.00	50.00	110419.5284	2256771.8371	٦

BRIDGE SPIKE SET

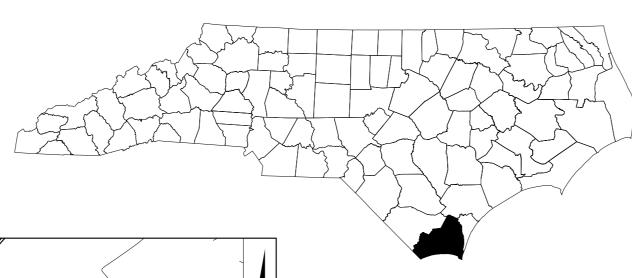
### **NOTES:**

- 1. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.
- 2. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.



# TRANSPORTATION MANAGEMENT PLAN

# BRUNSWICK COUNTY



LOCATION: REPLACE BRIDGE 104 OVER MIDDLE SWAMP ON NC 906 (MIDWAY ROAD SE)

# VICINITY MAP DETOUR ROUTE ● ● ● N.T.S.

### INDEX OF SHEETS

SHEET NO. TITLE

TITLE SHEET, VICINITY MAP AND INDEX OF SHEETS TMP - 1

LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS, AND LEGEND TMP-1A

TRANSPORTATION OPERATIONS PLAN: (MANAGEMENT STRATEGIES, GENERAL NOTES AND LOCAL NOTES) TMP-1B

OFF-SITE DETOUR TMP-2

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



"from the MOUNTAINS to the COAST"

PLANS PREPARED BY:

ADAM CONRAD, P.E.

RYAN DEMUYNCK, E.I.

NCDOT CONTACTS:

KEN THORNEWELL, P.E. PROJECT ENGINEER

MIKE STEELMAN

PROJECT DESIGN ENGINEER





APPROVED: Adam M. Consad *DATE*:\_\_\_\_\_

SEAL



SHEET NO.

TMP-1

PROJ. REFERENCE NO. SHEET NO. 17BP.3.R.80 TMP-1A

### ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

TITLE STD. NO.

1101.03	TEMPORARY ROAD CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1145.01	BARRICADES

### **LEGEND**

### **GENERAL**

DIRECTION OF TRAFFIC FLOW

DIRECTION OF PEDESTRIAN TRAFFIC FLOW

----- EXIST. PVMT.

NORTH ARROW

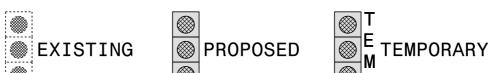
—— PROPOSED PVMT.

TEMP. SHORING (LOCATION PURPOSES ONLY)

### SIGNALS







### PAVEMENT MARKINGS

——EXISTING LINES ——TEMPORARY LINES

### TRAFFIC CONTROL DEVICES

BARRICADE (TYPE III)

DRUM SKINNY DRUM O TUBULAR MARKER

CHANGEABLE MESSAGE SIGN

### TEMPORARY SIGNING

O PORTABLE SIGN

### PAVEMENT MARKERS

CRYSTAL/CRYSTAL

CRYSTAL/RED

APPROVED: Adam M. Consad ODDCOFFA0FD94AD...

ROADWAY STANDARD [DRAWINGS & LEGEND

CDM Smith Inc. 5400 Glenwood Avenue Suite 400 Raleigh, NC 27612-3228 NC COA No. F-1255

TEMPORARY CRASH CUSHION FLASHING ARROW BOARD

FLAGGER

LAW ENFORCEMENT

TRUCK MOUNTED ATTENUATOR (TMA)

── STATIONARY SIGN

STATIONARY OR PORTABLE SIGN

◆ YELLOW/YELLOW

PAVEMENT MARKING SYMBOLS

PAVEMENT MARKING SYMBOLS

PROJ. REFERENCE NO. SHEET NO. 17BP.3.R.80 TMP-1B

### **MANAGEMENT STRATEGIES**

- 1- CLOSE NC 906 (MIDWAY ROAD SE) TO TRAFFIC AND DETOUR TRAFFIC OFF-SITE.
- 2- LOCAL ACCESS TO ALL RESIDENCES AND BUSINESSES WILL BE MAINTAINED BETWEEN CLOSURE POINTS AT ALL TIMES DURING CONSTRUCTION.

### GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE **ENGINEER.** 

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

### TRAFFIC PATTERN ALTERATIONS

A) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

### SIGNING

- B) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.

D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

### TRAFFIC CONTROL DEVICES

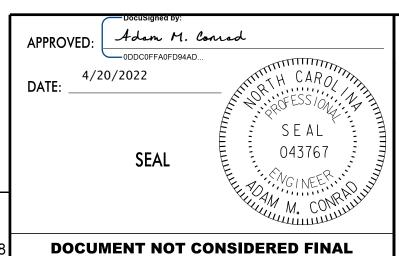
F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

### **PHASING**

- STEP 1: USING RSD 1101.03 (SHEET 1 OF 9), INSTALL DETOUR ROUTE SIGNING TO CLOSE NC 906 (MIDWAY ROAD SE) FROM STA. 14+00+/- TO STA. 26+80+/- -L-.
- STEP 2: AWAY FROM TRAFFIC, COMPLETE CONSTRUCTION OF PROPOSED BRIDGE AND ROADWAY APPROACHES, INCLUDING DRAINAGE, GUARDRAIL, FINAL PAVEMENT MARKINGS AND MARKERS ON PROPOSED -L- FROM STA. 14+00+/- TO STA. 26+80+/-.
- STEP 3: REMOVE TEMPORARY TRAFFIC CONTROL DEVICES AND OPEN -L- TO PROPOSED 2-LANE, 2-WAY TRAFFIC PATTERN.

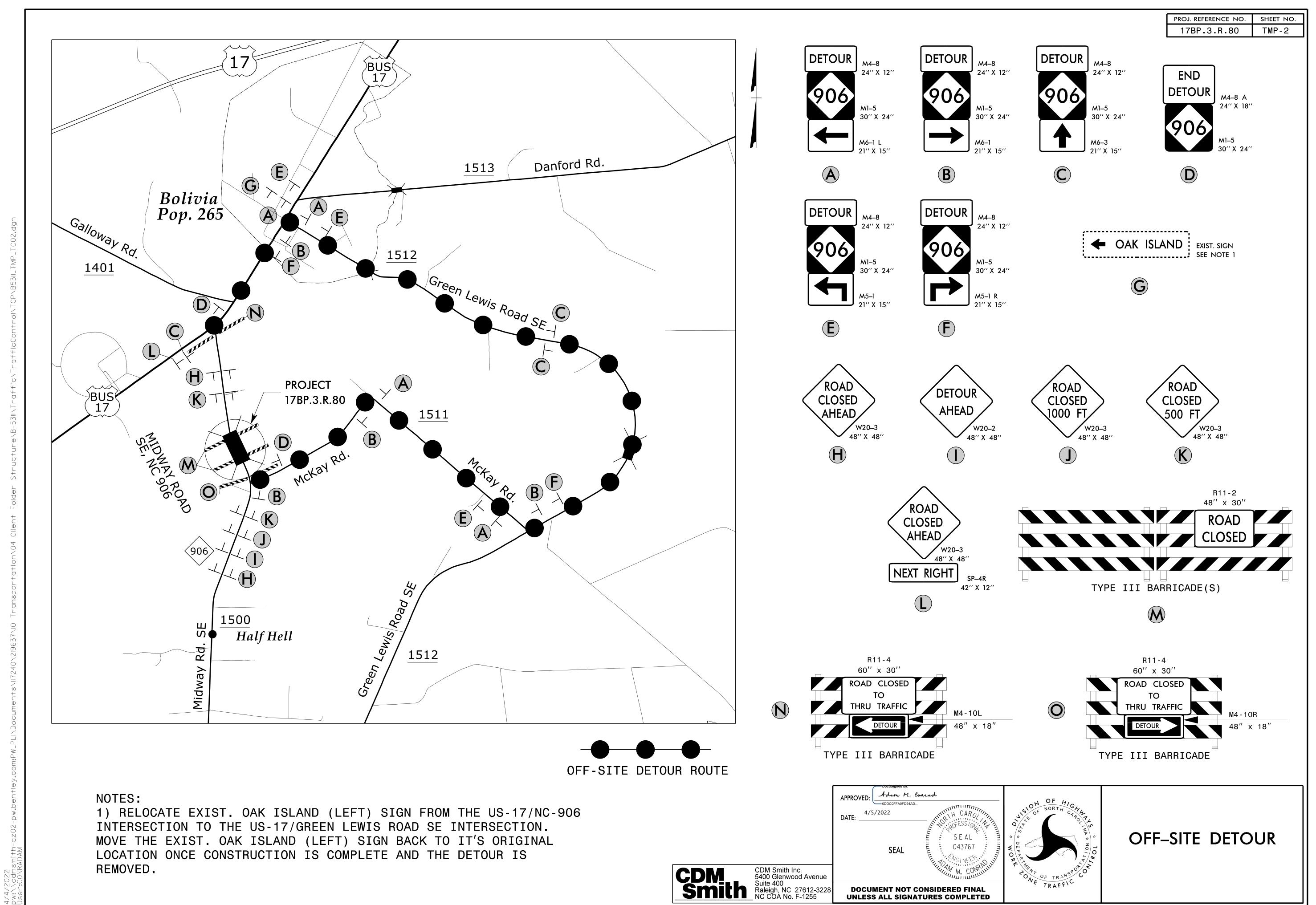
### LOCAL NOTES

1- NOTIFY BRUNSWICK COUNTY EMERGENCY SERVICES AND PUBLIC SCHOOLS AT LEAST ONE MONTH PRIOR TO ROAD CLOSURE.





TRANSPORTATION **OPERATIONS** PLAN



# 0 00 B

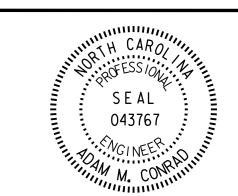
# STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

# PAVEMENT MARKING PLAN BRUNSWICK COUNTY

LOCATION: NC 906 (MIDWAY ROAD SE) OVER MIDDLE SWAMP

SHEET NO. PMP-I17BP.3.R.80

Adam M. Consad



**UNLESS ALL SIGNATURES COMPLETED** 

### ROADWAY STANDARD DRAWING

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" -PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE

STD. NO.	<u>TITLE</u>
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO-LANE AND MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	RAISED PAVEMENT MARKERS - INSTALLATION SPACING
1251.01	RAISED PAVEMENT MARKERS - PERMANENT AND TEMPORARY
1253.01	RAISED PAVEMENT MARKERS - SNOWPLOWABLE
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING
1262.01	GUARDRAIL END DELINEATION

		SUMMARY OF QUANTITIES	$\overline{}$	
ITEM NO	).	ITEM DESCRIPTION	QUANTITY	UNIT
DESC. NO.	SECT.			
4685000000 - E 4847010000 - E 4895000000 - N 4900000000 - N		THERMOPLASTIC PAVEMENT MARKING LINES (4", 90 MILS) POLYUREA PAVEMENT MARKING LINES (4", 20 MILS) NON-CAST IRON SNOWPLOWABLE MARKERS (GENERIC) PERMANENT RAISED PAVEMENT MARKERS	4,260 660 15 2	L.F. L.F. EA.

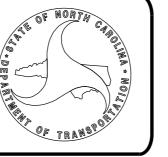
PLAN PREPARED BY: CDM SMITH, INC.

DAVID Z. KEISER, P. E. PROJECT MANAGER

ADAM M. CONRAD, P. E. PROJECT DESIGN ENGINEER

CDM Smith Inc. 5400 Glenwood Avenue Suite 400

PLAN REVIEWED B	Y: N.C.D.O.T. DIVISION 3
JESSI LEONARD, P.E.	DIVISION TRAFFIC ENGINEER
ANTHONY W. LAW	DIVISION CONSTRUCTION ENGINEER



### GENERAL NOTES

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, OR DIRECTED BY THE ENGINEER.

A) INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE AS FOLLOWS:

MARKING ROAD NAME MIDWAY ROAD SE THERMOPLASTIC MIDWAY ROAD SE (BRIDGE) POLYUREA

MARKER SNOWPLOWABLE

PERMANENT RAISED

C) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS.

B) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

D) PASSING ZONES WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.

E) REMOVE ALL SURFACE LAITANCE BY ACCEPTABLE METHODS ON CONCRETE BRIDGE DECKS PRIOR TO PLACING POLYUREA MARKING MATERIAL.

### *INDEX*

SHEET NO.

DESCRIPTION

PMP - 1

PAVEMENT MARKING PLAN TITLE SHEET

PMP-2

PAVEMENT MARKING DETAIL

### **PAVEMENT** MARKING SCHEDULE

SYMBOL

DESCRIPTION

THERMOPLASTIC (4", 90 MILS)

T1 T13

WHITE EDGELINE YELLOW DOUBLE CENTER

POLYUREA (4", 20 MILS)

V1 V13

WHITE EDGELINE YELLOW DOUBLE CENTER

PERMANENT RAISED PAVEMENT MARKERS

NON-CAST IRON SNOWPLOWABLE MARKERS

ME

15

YELLOW & YELLOW

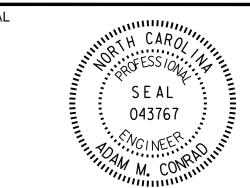
YELLOW & YELLOW

17BP.3.R.80 PMP-2 Adam M. Consad

SHEET NO.

TIP NO.

4/20/2022



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

CDM Smith Inc. 5400 Glenwood Avenue Suite 400 Raleigh, NC 27612-3228 NC COA No. F-1255

—-L- STA. 15+00 +/-(T13)— -L- STA. 14+50 +/-TIE TO EXIST. MARKINGS -L- STA. 26+30 +/- — (T13)— (ME)— -L- STA. 15+00 +/- — -L- STA. 19+35 +/-(V13)— —-L- STA. 21+00 +/--L- STA. 26+30 +/- — -L- STA. 26+80 +/-TIE TO EXIST. MARKINGS

See Sheet I-A For Index of Sheets
See Sheet I-B For Conventional Symbols

Clements
Curve

1401

1401

1513

1512

1509

1509

1500

1500

1500

1500

N.T.S.

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PLAN FOR PROPOSED
HIGHWAY EROSION CONTROL

# BRUNSWICK COUNTY

LOCATION: REPLACE BRIDGE 104 OVER MIDDLE SWAMP ON NC 906 (MIDWAY ROAD SE)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, SIGNING AND STRUCTURE

STATE STATE PROJECT REFERENCE NO.

SHEET NO.

TOTAL SHEETS

F.A. PROJ. NO.

DESCRIPTION

TOTAL SHEETS

DEC-1

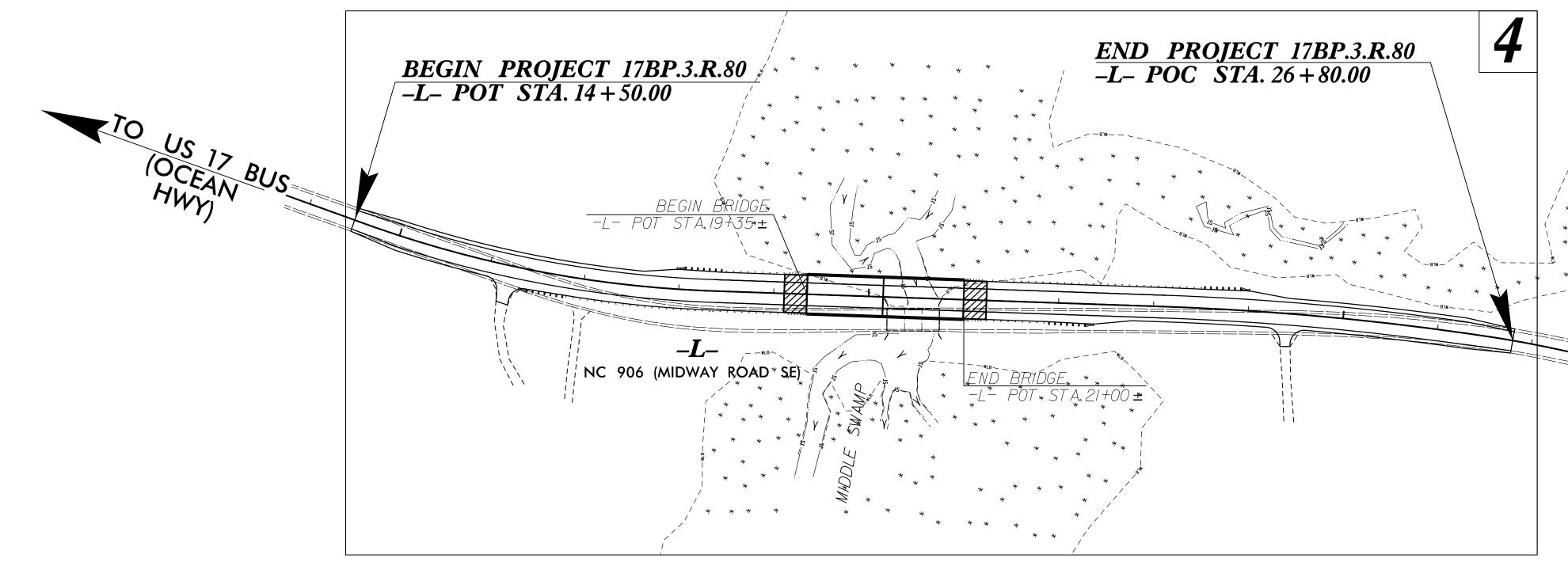
EROSION AND SEDIMENT CONTROL MEASURES

<u>Séd.</u> #	Description Symbol
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Drains
1630.02	Silt Basin Type B
1633.01	Temporary Rock Silt Check Type-A
	Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)
1633.02	Temporary Rock Silt Check Type-B
	Wattle / Coir Fiber Wattleew-
	Wattle / Coir Fiber Wattle with Polyacrylamide (PAM)
1634.01	Temporary Rock Sediment Dam Type-A
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A
1635.02	Rock Pipe Inlet Sediment Trap Type-B
1630.04	Stilling Basin
1630.06	Special Stilling Basin
	Rock Inlet Sediment Trap:
1632.01	Туре А
1632.02	Туре В
1632.03	Туре С
	Skimmer Basin
	Tiered Skimmer Basin
	T folly years

EROSION CONTROL PLANS
FOR CLEARING AND
GRUBBING PHASE OF
CONSTRUCTION.

THIS PROJECT CONTAINS

THE OUSIDE BUFFER, WETLAND, OR WATER BOUNDARY SHALL BE CLEARLY MARKED BY HIGHLY VISIBLE FENCING (ORANGE SAFETY FENCE).



GRAPHIC SCALE



THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE APPLICABLE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE APRIL 1, 2019 AND ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENTAL QUALITY DIVISION OF WATER RESOURCES.



Prepared in the Office of:

### 320 Executive Ct. Hillsborough, NC 27278–8551

Voice: (919)732-3883 Fax: (919)732-6776 www.summitde.net

Designed by:

HE YANG

4408

LEVEL III CERTIFICATION NO.

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"—Roadway Design Unit – N. C. Department of Transportation – Raleigh, N. C., dated January 2018 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans

1604.01 Railroad Erosion Control Detail 1605.01 Temporary Silt Fence 1606.01 Special Sediment Control Fence 1607.01 Gravel Construction Entrance 1622.01 Temporary Berms and Slope Drai

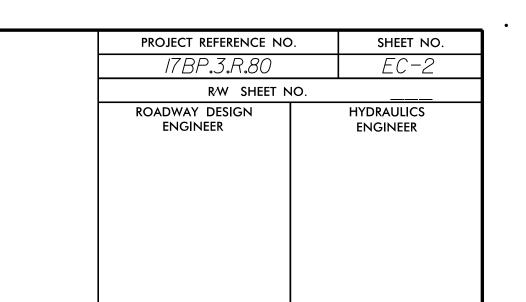
1606.01 Special Sediment Control Fence
1607.01 Gravel Construction Entrance
1622.01 Temporary Berms and Slope Drains
1630.01 Riser Basin
1630.02 Silt Basin Type B

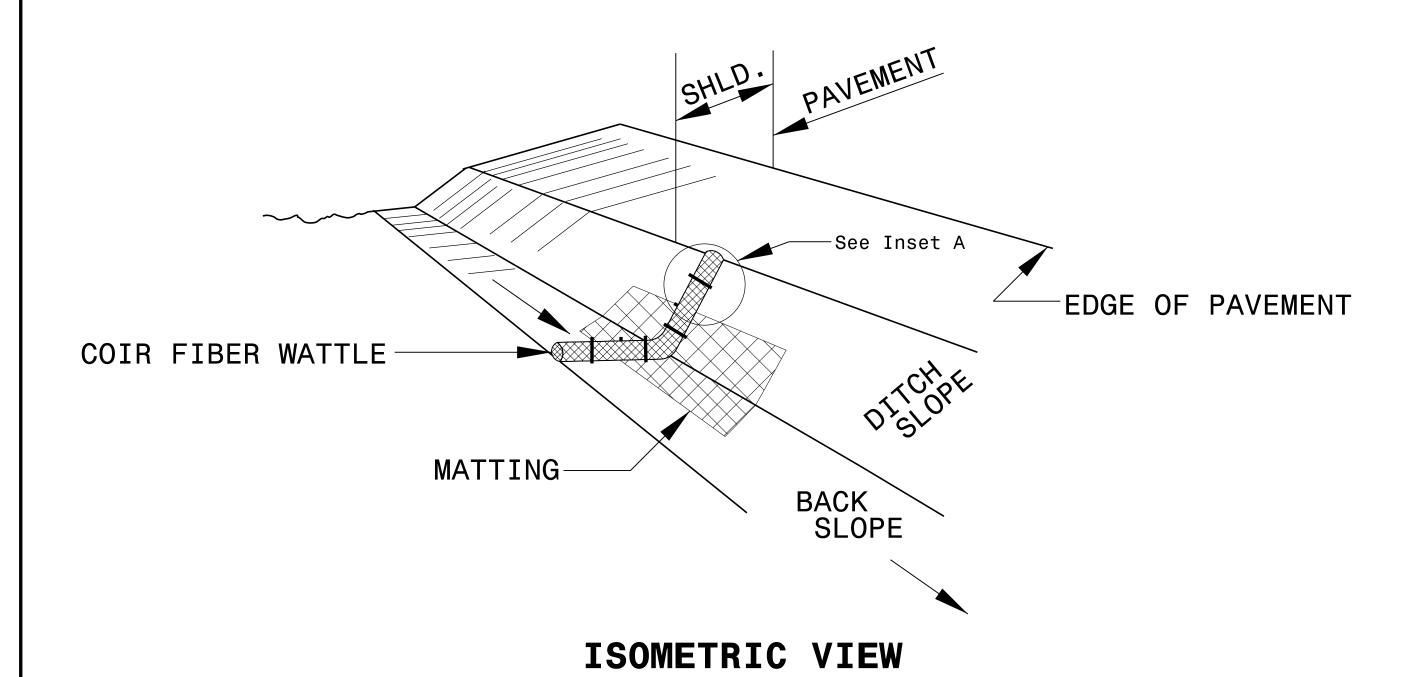
1630.03 Temporary Silt Ditch 1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin 1631.01 Matting Installation 1632.01 Rock Inlet Sediment Trap Type A
1632.02 Rock Inlet Sediment Trap Type B
1632.03 Rock Inlet Sediment Trap Type C
1633.01 Temporary Rock Silt Check Type A
1633.02 Temporary Rock Silt Check Type B
1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle

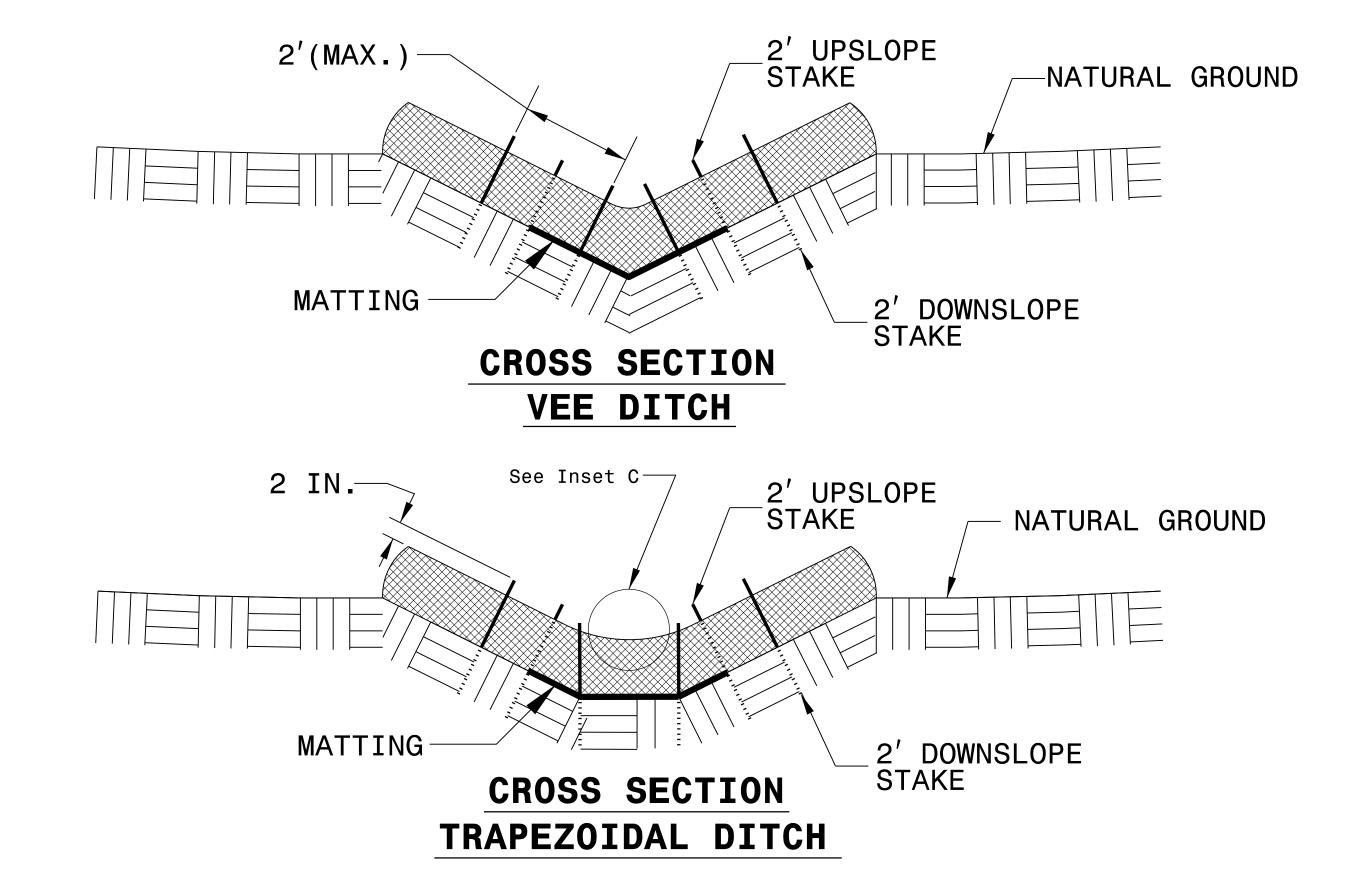
1645.01 Temporary Stream Crossing

JON 29-5311-EC\_dsn\_tsh.dc he.yang

# COIR FIBER WATTLE WITH POLYACRYLAMIDE (PAM) DETAIL







NOTES:

FLOW

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE CDM Smith Inc. 5400 Gillow Suite 400 Suite 400 Suite 400 Raleigh, NC 27612-3228 NC FIRM LICENSE No: P-0

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

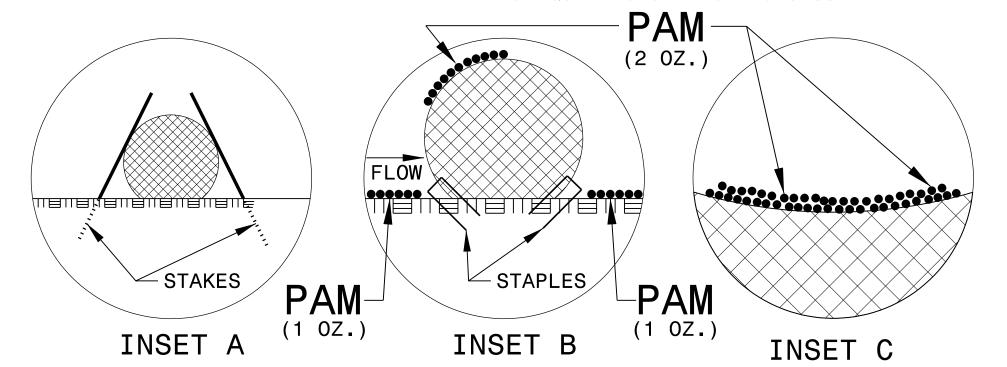
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

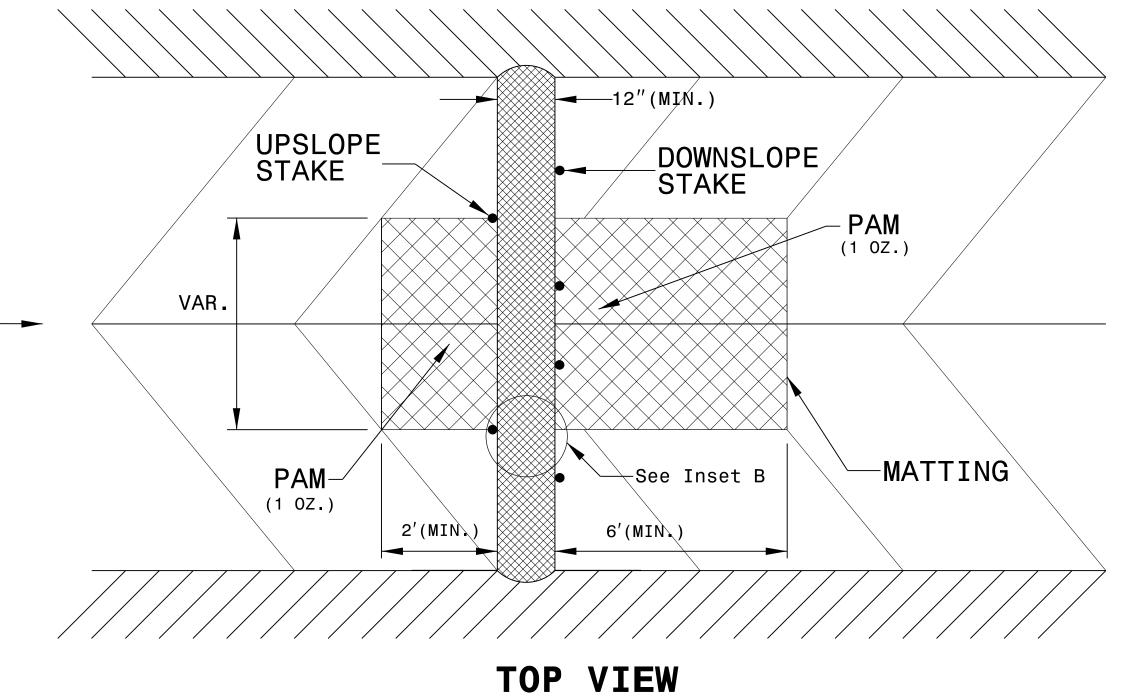
INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH WATTLE.

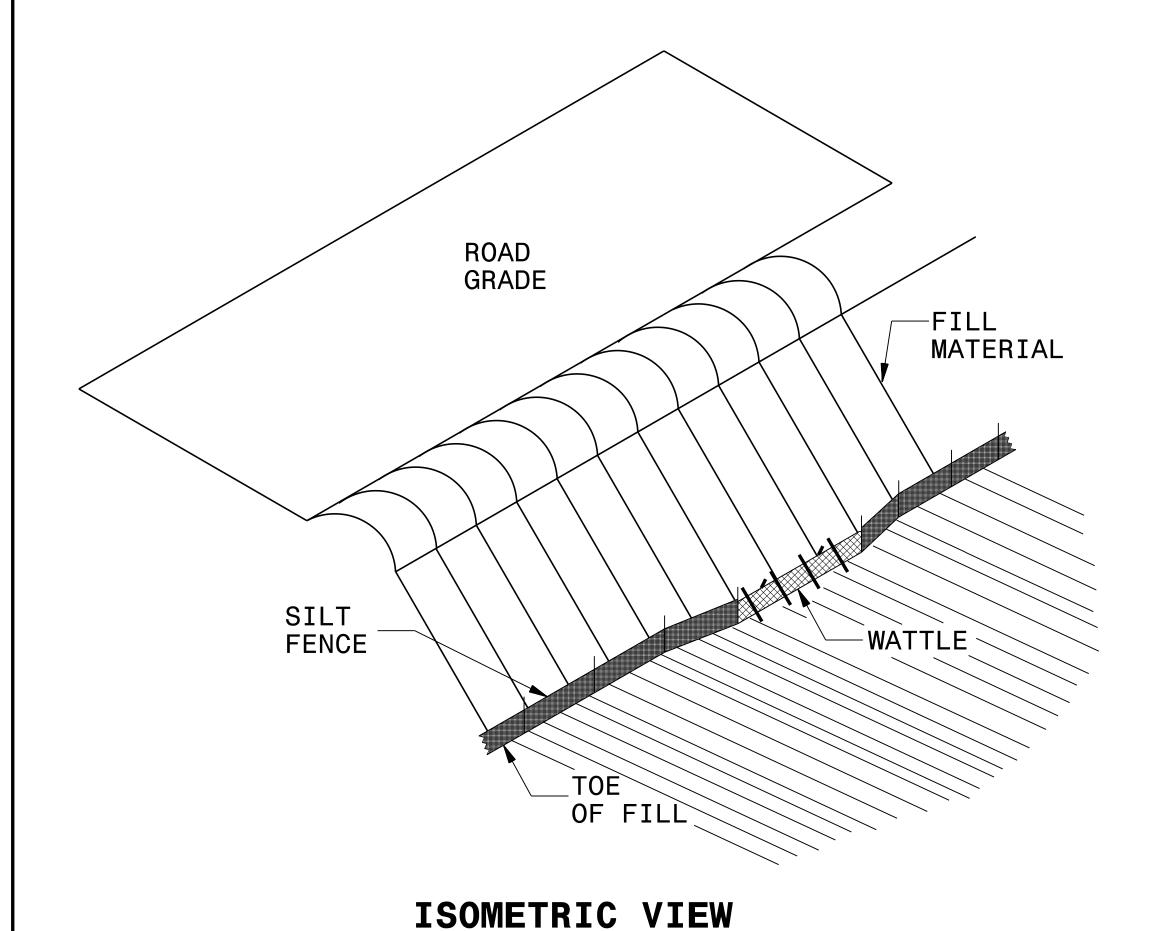
INITIALLY APPLY 2 OUNCES OF ANIONIC OR NEUTRALLY CHARGED PAM OVER WATTLE WHERE WATER WILL FLOW AND 1 OUNCE OF PAM ON MATTING ON EACH SIDE OF WATTLE. REAPPLY PAM AFTER EVERY RAINFALL EVENT THAT IS EQUAL TO OR EXCEEDS 0.50 IN.





# SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO	D. SHEET NO.			
17BP.3.R.80	EC-2A			
R/W SHEET N	NO			
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER			
CDM Smith Inc. 5400 Glenwood Avenue Suite 400 Raleigh, NC 27612-3228 NC COA No. F-1255	SUMMIT Summit Design and Engineering Services 504 Meadowland Drive Hillsborough, NC 27278 NC FIRM LICENSE No: P-0339			



SILT FENCE

9 FT.

2' WOODEN
STAKE

SILT FENCE

4 FT.

2" WATTLE

**VIEW FROM SLOPE** 

### NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

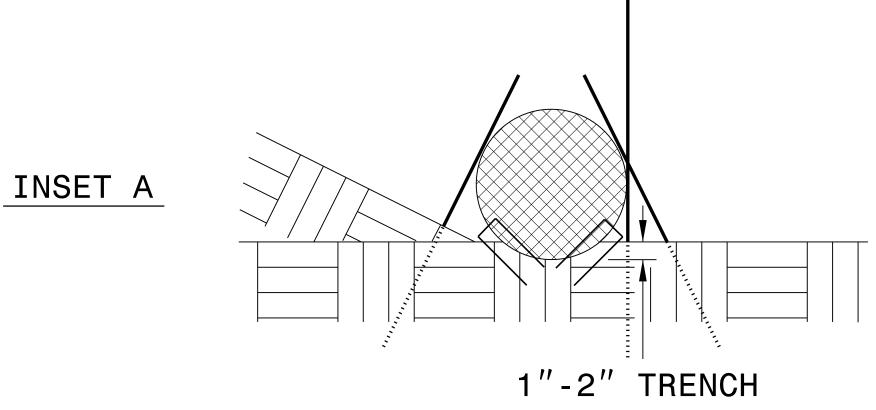
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

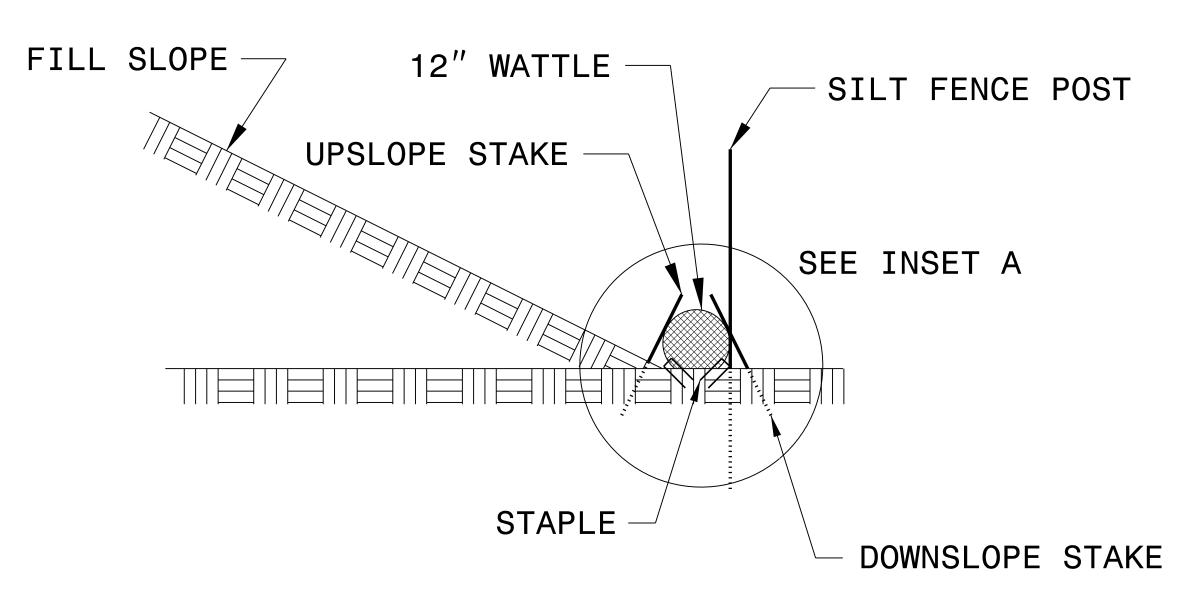
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.





SIDE VIEW

DIVIS	SION	OF	HIC	GHWAYS	
STATE	$\mathbb{OF}$	NOR	TH	CAROLIN	<b>J</b>

PROJECT REFERENCE NO	D. SHEET NO.
17BP.3.R.80	EC-3
	<del></del>
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER
CDM Smith Inc. 5400 Glenwood Avenue Suite 400 Raleigh, NC 27612-3228 NC COA No. F-1255	SUMMIT Summit Design and Engineering Services 504 Meadowland Drive Hillsborough, NC 27278 NC FIRM LICENSE No: P-033

# SOIL STABILIZATION TIMEFRAMES

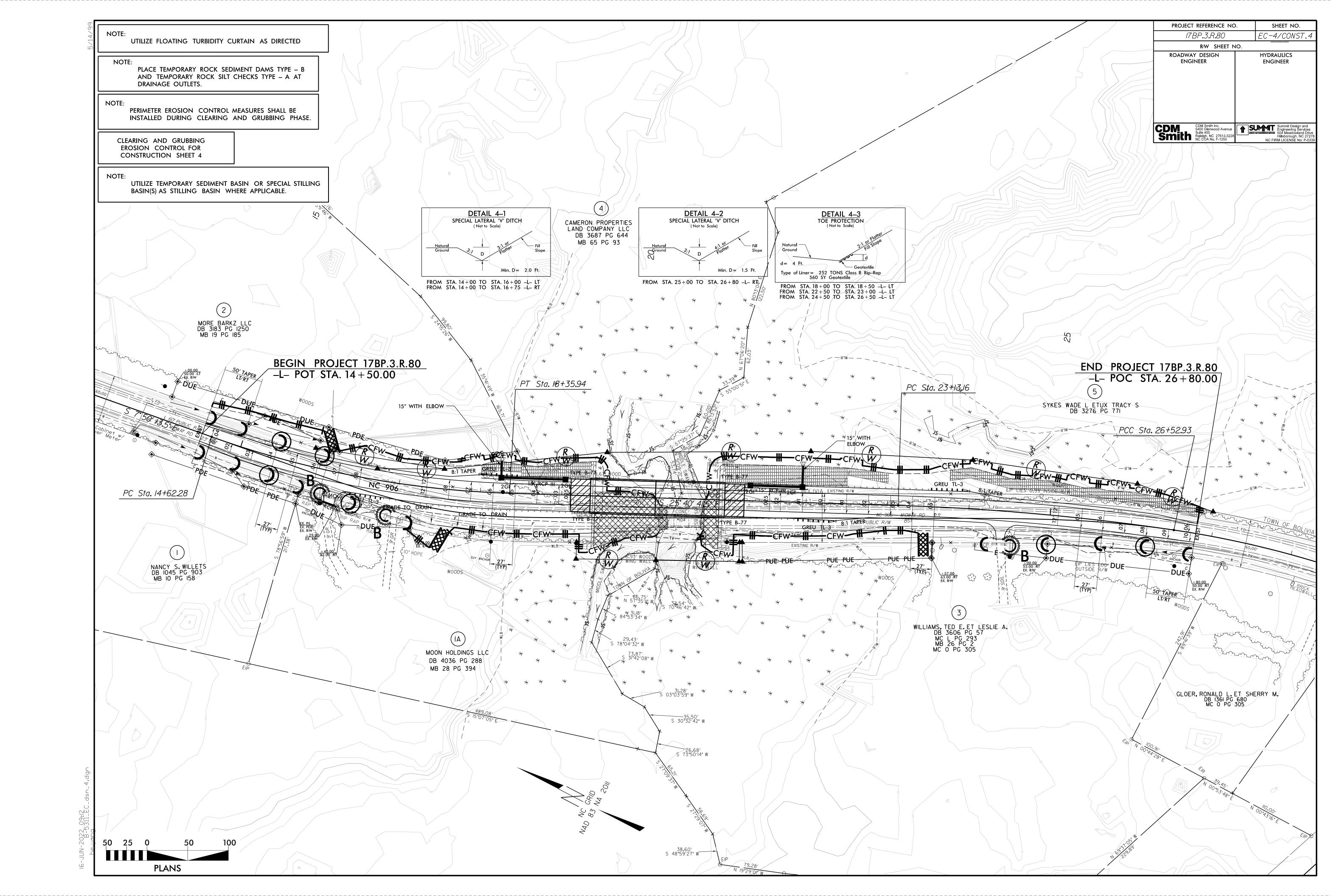
SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

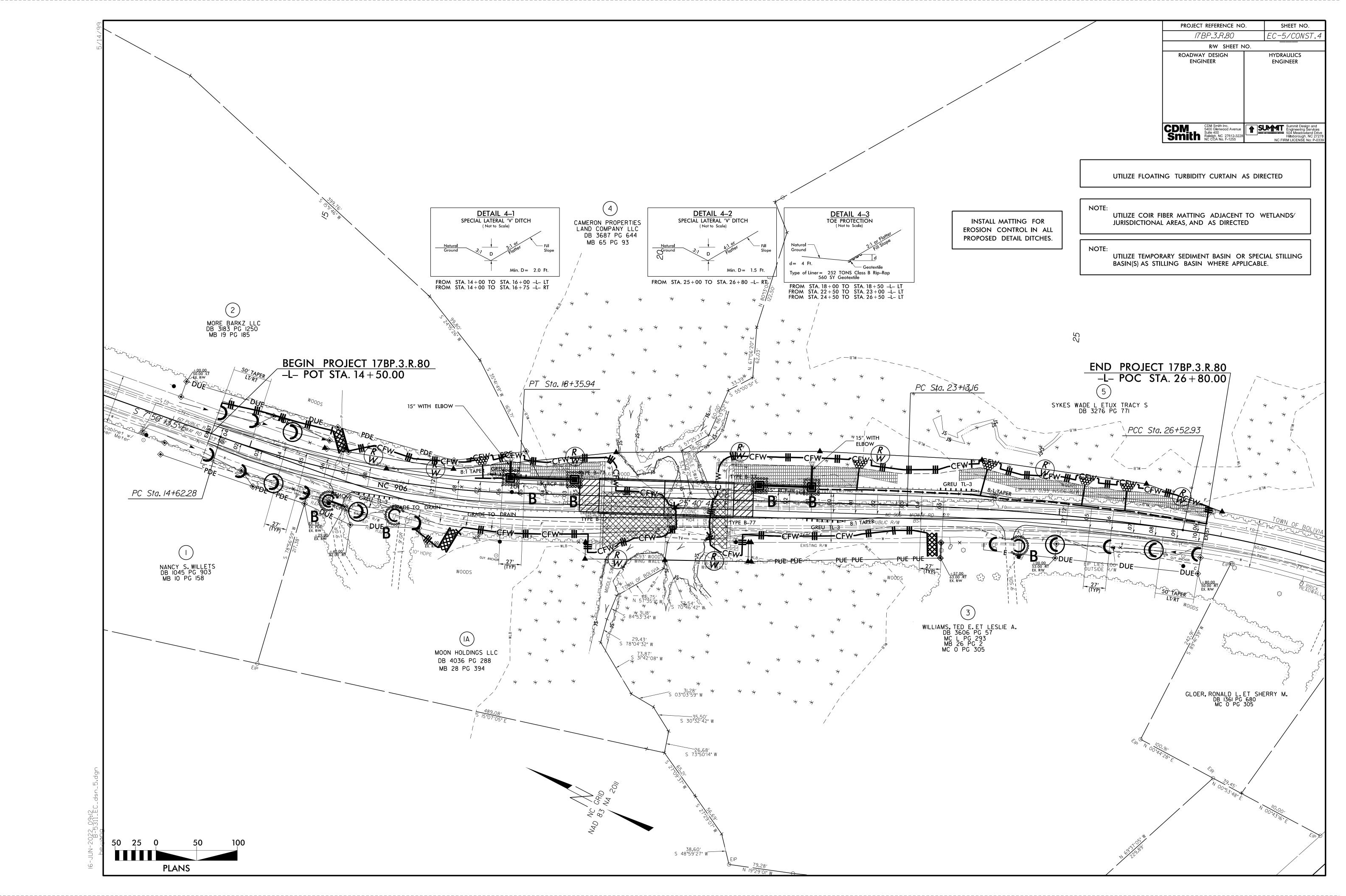
# DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

PROJECT REFERENCE NO	SHEET NO.
17BP.3.R.80	EC-3A
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

# SOIL STABILIZATION SUMMARY SHEET

	COIR FIBER	MATTIN	IG FOR	FILL	<b>SLOPES</b>			COIR F	IBER	MATTING	<b>FOR</b>	CUT D	ITCHES
CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE	(SY)	CONST SHEET NO.	L/N	IE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
4	-   -	19+00	19+35	LT	77		4	- L	. –	14+00	16+00	LT	280
4	- L -	21+00	23+00	LT	487		4	- L	, -	14+00	15+50	R1	210
		FIL	- SLOPE	TOTAL	564		4	-	, -	15+50	16+75	RT	205
							4	-	, -	25+00	26+80	R1	220
										CU	T DITCH	TOTAL	915
										FIL	L SLOPE	TOTAL	564
											SU	OTOTAL	1479
								M	160. M	ATTING TO	BE INS	TALLED	
									AS DIR	ECTED BY	THE ENG	INEER	1570
												TOTAL	3049
												SAY	3100





STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# UTILITIES BY OTHERS PLANS

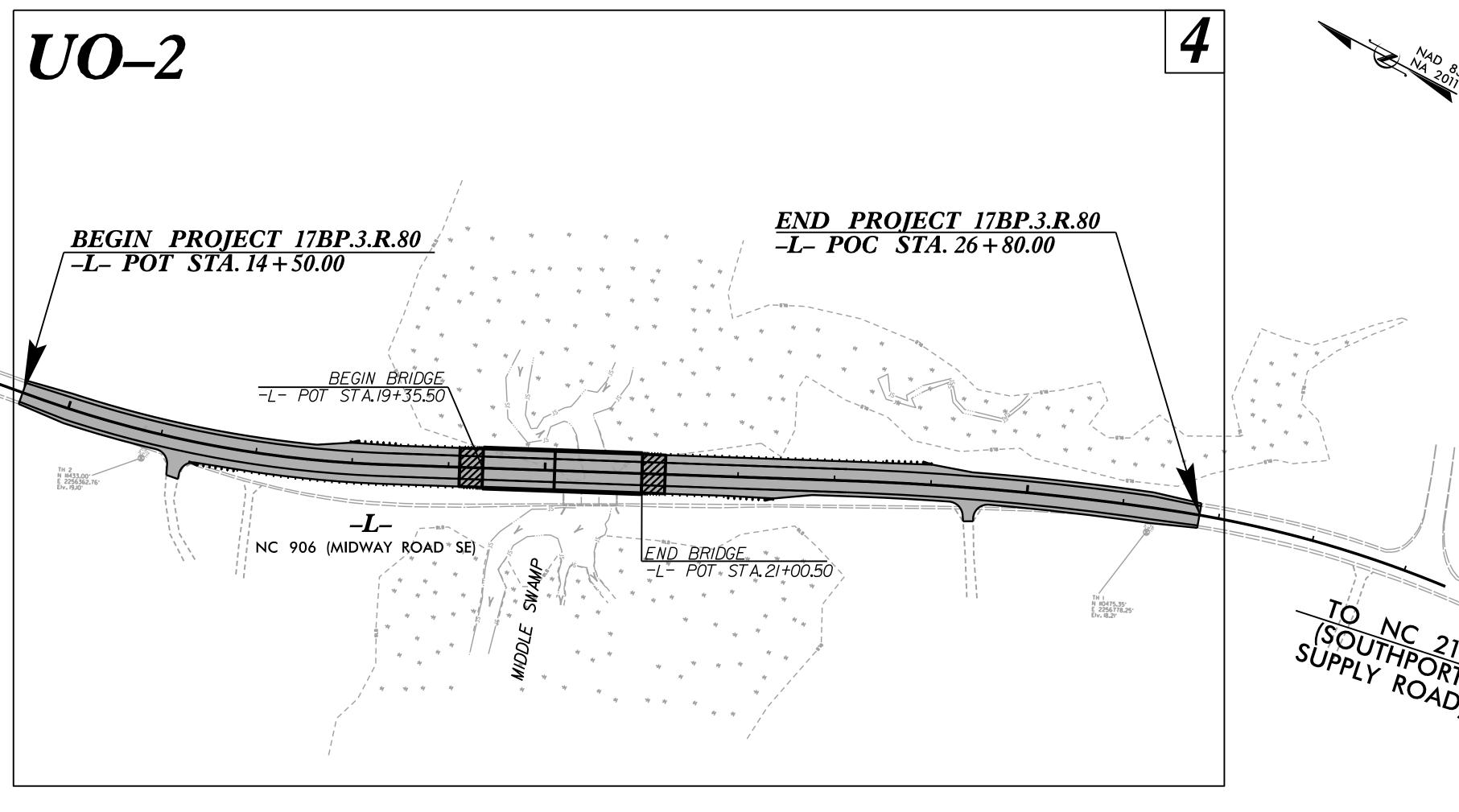
BRUNSWICK COUNTY

T.I.P. NO. UO-1 17BP.3.R.80

(NOTE: ALL UTILITY WORK SHOWN ON THIS SHEET WILL BE DONE BY OTHERS. NO PAYMENT WILL BE MADE TO THE CONTRACTOR FOR UTILITY WORK SHOWN ON THIS SHEET.

LOCATION: REPLACE BRIDGE 104 OVER MIDDLE SWAMP ON NC 906 (MIDWAY ROAD SE)

TYPE OF WORK: RELOCATION OF POWER AND COMMUNICATION



**GRAPHIC SCALES** PROFILE (HORIZONTAL) PROFILE (VERTICAL)

See Sheet IA For Index of Sheets See Sheet IB For Conventional Symbols

VICINITY MAP

• • DETOUR ROUTE

END PROJECT

**BEGIN CONSTRUCTION** 

-L-POTSTA.14+00.00

*N.T.S.* 

INDEX OF SHEETS

SHEET NO.: **DESCRIPTION:** *UO-1* TITLE SHEET

**UO**–2 UBO PLAN SHEET UTILITY OWNERS WITH CONFLICTS

(A) POWER – BRUNSWICK EMC (B) COMMUNICATIONS - ATMC

PREPARED IN THE OFFICE OF:



Scott Williford

PROJECT UTILITY COORDINATOR

UTILITY PROJECT MANAGER



**DIVISION OF HIGHWAYS DIVISION** 3

5501 Barbados Blvd. Castle Hayne, NC 28429

Lonny Sleeper Derek Pielech, PE

**DIVISION UTILITY ENGINEER** BRIDGE PROGRAM MANAGER

PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.80 UO-2 THIS SHEET CORRESPONDS TO RDY-4 UTILITIES BY OTHERS ALL PROPOSED UTILITY WORK SHOWN ON THIS SHEET WILL BE DONE BY OTHERS. NO PAYMENT WILL BE MADE TO THE CONTRACTOR FOR PROPOSED UTILITY WORK SHOWN ON THIS SHEET. CAMERON PROPERTIES LAND COMPANY LLC ATMC 3'X3'X3' HH WITH (2)1.25" AND (1) 2" CONDUITS DRY BORED MIN. 4' BELOW ROADWAY E BARKZ LLC BEMC O/H GIN PROJECT 17BP.3.R.80 - POT STA. 14 + 50.00 END PROJECT 17BP.3.R.80 -L- POC STA. 26 + 80.00 PT Sta. 18+35.94 PC Sta. 23+13,16 BEGIN \*BRIDGE SYKES WADE L ETUX TRACY S 15" WITH ELBOW ---PCC Sta. 26+52.93 ATMC 3'X3'X3' HH WITH (2)1.25" CONDUITS DRY BORED MIN. 4' BELOW ROADWAY ABANDONED 🕇 ABANDONED — REM PC Sta. 14+62.28 ABANDONED. BEGIN CONSTRUCTION -L- POT STA 14 + 00.00 WOODS BEMC/ATMC U/G
DIRECTIONAL BORE (4)4"
DUCTS MIN.15' BELOW CREEK BEMC O/H ATMC DIRECT BURY 2.5' FROM R/W BEMC/ATMC U/G DIRECTIONAL BORE (4)4" DUCTS MIN.15' BELOW CREEK ATMC DIRECTIONAL BORE
(2)1.25" DUCTS (1) 2" DUCT CHARLES M. SCHWARTZ, ET.UX. ATMC DIRECT BURY 2.5 FROM R/W GLOER, RONALD L. ET SHERRY M.

Clements

BEGIN PROJECT

• DETOUR ROUTE

[17]

VICINITY MAP

END PROJECT

*N.T.S.* 

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

SHEET NO 17BP.3.R.80 UC-1

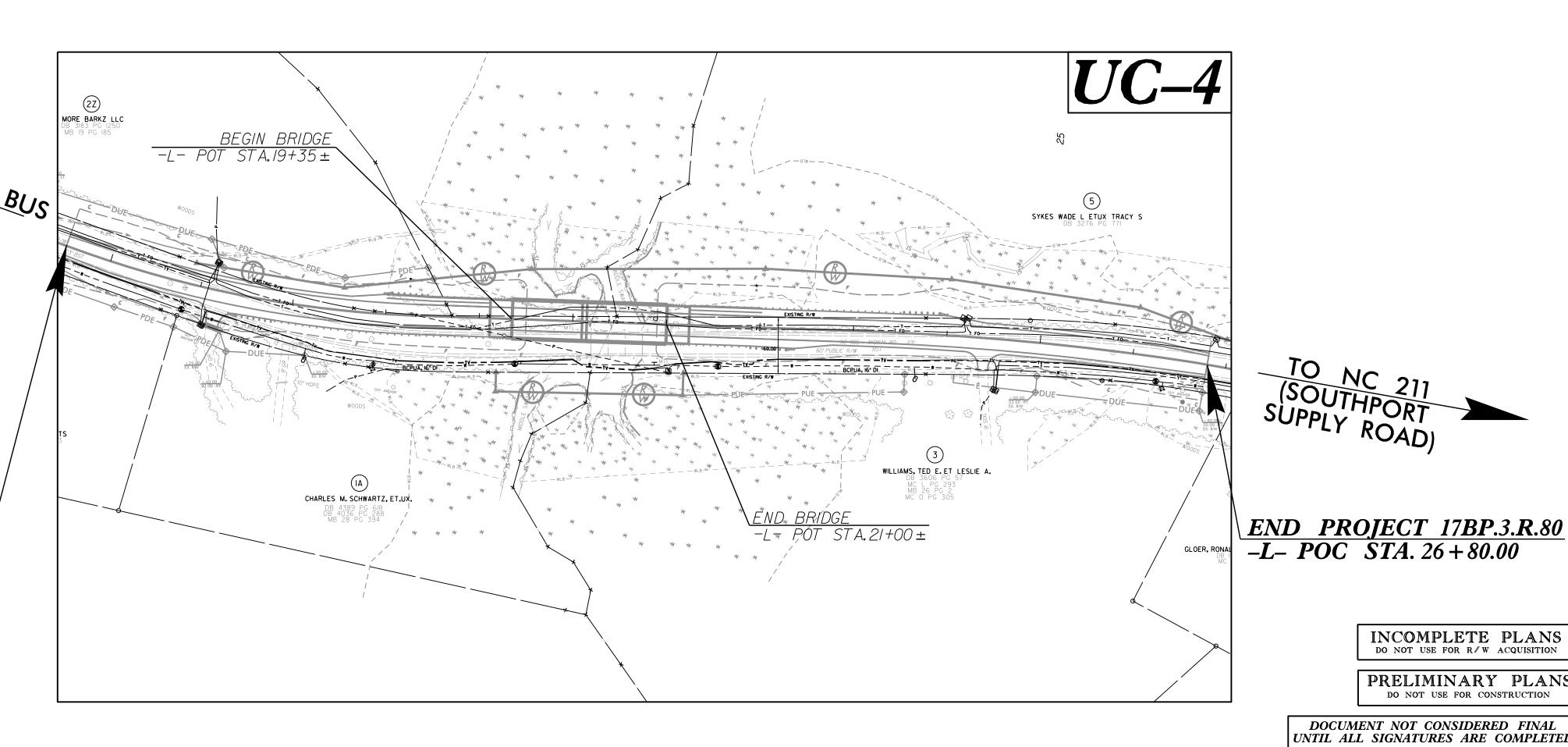
T.I.P. NO.

# UTILITY CONSTRUCTION PLANS BRUNSWICK COUNTY

LOCATION: REPLACE BRIDGE 104 OVER MIDDLE SWAMP ON NC 906 (MIDWAY ROAD SE)

TYPE OF WORK: WATER LINE RELOCATION





**GRAPHIC SCALES PLANS** PROFILE (HORIZONTAL) PROFILE (VERTICAL)

### INDEX OF SHEETS

SHEET NO.:

BEGIN PROJECT 17BP.3.R.80
-L- POT STA. 14 + 50.00

*UC-2* UC-3A THRU UC-3C *UC-4* 

TITLE SHEET UTILITY SYMBOLOGY **NOTES DETAILS** PLAN /PROFILE SHEET

**DESCRIPTION:** 

### WATER AND SEWER OWNERS ON PROJECT

(A) WATER LINE – BRUNSWICK COUNTY (B) SANITARY SEWER - N/A

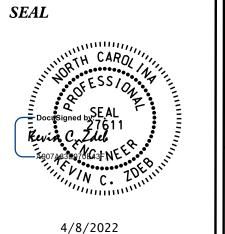
PREPARED IN THE OFFICE OF



C-4647 Cary, NC 27511 Fax: 919.297.0221



KEVIN ZDEB, PE PROJECT ENGINEER AARON COLLINS, EI PROJECT DESIGN ENGINEER





DIVISION OF HIGHWAYS HIGHWAY DIVISION 3

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

PRELIMINARY PLANS

5501 BARBADOS BLVD. CASTLE HAYNE, NC 28429 PHONE (910) 341–2001 FAX (910) 675–0143

DOCUMENT NOT CONSIDERED FINAL UNTIL ALL SIGNATURES ARE COMPLETED

DEREK PIELECH

ROY SUTTON

D. CHAD KIMES, PE DIVISION ENGINEER DIVISION BRIDGE PROGRAM ENGINEER LONNY SLEEPER DIVISION UTILITY ENGINEER

DIVISION UTILITY COORDINATOR

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## PROJECT REFERENCE NO. SHEET NO. 17BP.3.R.80 UC-2

# STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

# UTILITIES PLAN SHEET SYMBOLS

### PROPOSED WATER SYMBOLS

## Water Line (Sized as Shown) 11<sup>1</sup>⁄<sub>4</sub> Degree Bend 22½ Degree Bend 45 Degree Bend 90 Degree Bend Plug Tee · Cross. Reducer Gate Valve Butterfly Valve Tapping Valve Line Stop Line Stop with Bypass Blow Off Fire Hydrant ··· Relocate Fire Hydrant REM FH Remove Fire Hydrant Water Meter Relocate Water Meter Remove Water Meter Water Pump Station RPZ Backflow Preventer DCV Backflow Preventer Relocate RPZ Backflow Preventer Relocate DCV Backflow Preventer PROPOSED SEWER SYMBOLS Gravity Sewer Line (Sized as Shown) Force Main Sewer Line ... (Sized as Shown)

### PROPOSED MISCELLANOUS UTILITIES SYMBOLS

Power Pole	Thrust Block ·····
Telephone Pole	Air Release Valve ····································
Joint Use Pole ····································	Utility Vault
Telephone Pedestal ····································	Concrete Pier
Jtility Line by Others (Type as Shown)	Steel Pier
renchless Installation	Plan Note
Encasement by Open Cut	Pay Item Note
ncasement ·····	PAY ITEM

### EXISTING UTILITIES SYMBOLS

		*	
Power Pole	•	*Underground Power Line	P ————————————————————————————————————
Telephone Pole	-	*Underground Telephone Cable	т ———
Joint Use Pole	<b>→</b>	*Underground Telephone Conduit	тс
Utility Pole	•	*Underground Fiber Optics Telephone Cable —	T F0
Utility Pole with Base		*Underground TV Cable	тv
H-Frame Pole	•—•	*Underground Fiber Optics TV Cable	TV FO
Power Transmission Line Tower		*Underground Gas Pipeline	c
Water Manhole	(W)	Aboveground Gas Pipeline	A/G Gas
Power Manhole	℗	*Underground Water Line	
Telephone Manhole	lacktriangle	Aboveground Water Line	A/G Water
Sanitary Sewer Manhole	⊕	*Underground Gravity Sanitary Sewer Line ——	ss
Hand Hole for Cable	FH	Aboveground Gravity Sanitary Sewer Line —	A/G Sanitary Sewe
Power Transformer		*Underground SS Forced Main Line	FSS
Telephone Pedestal	T	Underground Unknown Utility Line	?UTL
CATV Pedestal		SUE Test Hole	
Gas Valve	<b>♦</b>	Water Meter $\odot$	
Gas Meter	<b>\phi</b>	Water Valve ····································	
Located Miscellaneous Utility Object	⊙	Fire Hydrant •••	
Abandoned According to Utility Records	AATUR	Sanitary Sewer Cleanout ⊕	
End of Information	E.O.I.		

(Type as Shown)

Manhole

(Sized per Note)

Sewer Pump Station

## **UTILITY CONSTRUCTION**

### **GENERAL NOTES:**

- 1. THE PROPOSED UTILITY CONSTRUCTION SHALL MEET THE APPLICABLE REQUIREMENTS OF THE NC DEPARTMENT OF TRANSPORTATION'S "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" DATED JANUARY 2018.
- 2. THE EXISTING WATER LINE UTILITIES BELONG TO BRUNSWICK COUNTY PUBLIC UTILITIES.

CONTACT: BRENT LOCKAMY, PE PHONE: 910-253-2460

- 3. ALL WATER LINES TO BE INSTALLED WITHIN COMPLIANCE OF THE RULES AND REGULATIONS OF THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENTAL AND NATURAL RESOURCES, DIVISION OF ENVIRONMENTAL HEALTH.
- 4. THE UTILITY OWNER OWNS THE EXISTING UTILITY FACILITIES AND WILL OWN THE NEW UTILITY FACILITIES AFTER ACCEPTANCE BY THE DEPARTMENT. THE DEPARTMENT OWNS THE CONSTRUCTION CONTRACT AND HAS ADMINISTRATIVE AUTHORITY. COMMUNICATIONS AND DECISIONS BETWEEN THE CONTRACTOR AND UTILITY OWNER ARE NOT BINDING UPON THE DEPARTMENT OR THIS CONTRACT UNLESS AUTHORIZED BY THE ENGINEER. AGREEMENTS BETWEEN THE UTILITY OWNER AND CONTRACTOR FOR THE WORK THAT IS NOT PART OF THIS CONTRACT OR IS SECONDARY TO THIS CONTRACT ARE ALLOWED. BUT ARE NOT BINDING UPON THE DEPARTMENT.
- 5. PROVIDE ACCESS FOR THE DEPARTMENT PERSONNEL AND THE OWNER'S REPRESENTATIVES TO ALL PHASES OF CONSTRUCTION. NOTIFY DEPARTMENT PERSONNEL AND THE UTILITY OWNER TWO WEEKS PRIOR TO COMMENCEMENT OF ANY WORK AND ONE WEEK PRIOR TO SERVICE INTERRUPTION. KEEP UTILITY OWNERS' REPRESENTATIVES INFORMED OF WORK PROGRESS AND PROVIDE OPPORTUNITY FOR INSPECTION OF CONSTRUCTION AND TESTING.

- 6. THE PLANS DEPICT THE BEST AVAILABLE INFORMATION FOR THE LOCATION, SIZE, AND TYPE OF MATERIAL FOR ALL EXISTING UTILITIES. MAKE INVESTIGATIONS FOR DETERMINING THE EXACT LOCATION, SIZE, AND TYPE MATERIAL OF THE EXISTING FACILITIES AS NECESSARY FOR THE CONSTRUCTION OF THE PROPOSED UTILITIES AND FOR AVOIDING DAMAGE TO EXISTING FACILITIES. REPAIR ANY DAMAGE INCURRED TO EXISTING FACILITIES TO THE ORIGINAL OR BETTER CONDITION AT NO ADDITONAL COST TO THE DEPARTMENT.
- 7. MAKE FINAL CONNECTIONS OF THE NEW WORK TO THE EXISTING SYSTEM WHERE INDICATED ON THE PLANS, AS REQUIRED TO FIT THE ACTUAL CONDITIONS, OR AS DIRECTED.
- 8. MAKE CONNECTIONS BETWEEN EXISTING AND PROPOSED UTILITIES AT TIMES MOST CONVENIENT TO THE PUBLIC, WITHOUT ENDANGERING THE UTILITY SERVICE, AND IN ACCORDANCE WITH THE UTILITY OWNER'S REQUIREMENTS. MAKE CONNECTIONS ON WEEKENDS, AT NIGHT, AND ON HOLIDAYS IF NECESSARY.
- 9. ALL UTILITY MATERIALS SHALL BE APPROVED PRIOR TO DELIVERY TO THE PROJECT. SEE 1500-7, "SUBMITTALS AND RECORDS" IN SECTION 1500 OF THE STANDARD SPECIFICATIONS.
- 10. CONTRACTOR SHALL NOT OPERATE ANY VALVES ON THE EXISTING UTILITY SYSTEMS. CONTRACTOR SHALL CONTACT THE UTILITY OWNER TO CONDUCT STRATEGIC OPERATION OF VALVES FOR SERVICE INTERRUPTION IN ORDER TO PERFORM SPECIFIC WORK.

### PROJECT SPECIFIC NOTES:

- 1. PROPOSED OPEN TRENCH WATER LINE SHALL BE 16" FUSIBLE PVC C-905 DR-18 D.I.P.S. THAT CONFORMS TO NSF-61, NSF-14, AND ASTM CELL CLASSIFICATION 12454.
- 2. PROPOSED WATER LINE FOR DIRECTIONAL DRILLING SHALL BE 16" FUSIBLE PVC C-905 DR-18 D.I.P.S. THAT CONFORMS TO NSF-61, NSF-14, AND ASTM CELL CLASSIFICATION 12454.
- 3. ALL WATER LINE FITTINGS 14-INCH THROUGH 20-INCH SHALL BE DUCTILE IRON AWWA C110 WITH A MINIMUM PRESSURE RATING OF 250 PSI.
- 4. ALL PROPOSED FITTINGS (BENDS, TEES, CROSSES, REDUCERS, PLUGS, ETC.) SHALL BE ADEQUATELY RESTRAINED BY THE USE OF A RESTRAINED JOINT SYSTEM AND/OR CAST IN PLACE CONCRETE THRUST RESTRAINTS AS DETAILED ON THESE DRAWINGS, AS DIRECTED BY THE RESIDENT ENGINEER AND APPROVED BY THE UTILITY OWNER REPRESENTATIVE.
- 5. JOINT RESTRAINT DEVICES FOR VALVES
  AND FITTINGS ON PVC PIPE SHALL HAVE
  DUCTILE IRON GLANDS THAT CAN BE USED
  WITH THE STANDARD MECHANICAL JOINT OF
  THE VALVES AND FITTINGS. TWIST OFF
  NUTS WITH PRESET FACTORY TORQUE SETTING
  SHALL BE USED TO ENSURE PROPER
  ACTUATION OF THE RESTRAINT DEVICE. ALL
  NUTS, BOLTS, AND FASTENERS SHALL BE
  HIGH STRENGTH ALLOY STEEL
- 6. ALL DUCTILE IRON PIPE AND FITTINGS
  INSTALLED IN CORROSIVE, CONTAMINATED,
  AND/OR DREDGED SOILS SHALL BE WRAPPED
  IN A POLYETHYLENE ENCASEMENT PER ANSI /
  AWWA A21.5 / C105 "POLYETHYLENE
  ENCASEMENT FOR DUCTILE IRON PIPING FOR
  WATER AND OTHER LIQUIDS".
  ALL FASTENERS SHALL BE STAINLESS STEEL
  TYPE 304, MINIMUM GRADE.

PROJECT REFERENCE NO. SHEET NO.

17BP.3.R.80

DESIGNED BY: KCZ

DRAWN BY: AJC

CHECKED BY: KCZ

APPROVED BY: KCZ

REVISED:

NORTH CAROL INA
DEPARTMENT OF
TRANSPORTATION

UTILITIES ENGINEERING SEC.
PHONE: (919)707-6690
FAX: (919)250-4151

DUIC-3

UC-3

UC-3

DOCUMENTAL CAROL INALITY

APPROVED BY: KCZ

UTILITY CONSTRUCTION
PLANS ONLY

UTILITY CONSTRUCTION

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Weston Sampson

WSE of North Carolina, PC

598 East Chatham Street Suite 137

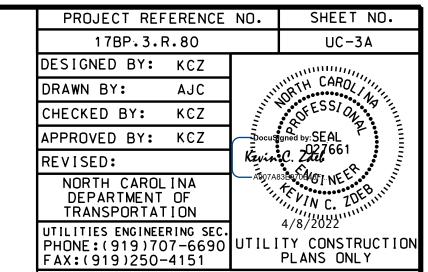
License: C-4647 7 Cary, NC 27511 Fax: 919.297.022

7. CONTRACTOR'S ATTENTION IS DIRECTED TO SECTIONS 102, 107, AND 1550 OF THE STANDARD SPECIFICATIONS CONCERNING TRENCHLESS INSTALLATION. IT IS CONTRACTOR'S RESPONSIBILITY TO HAVE BORE DESIGNED AND SEALED BY A LICENSED NORTH CAROLINA PROFESSIONAL ENGINEER. NO DAMAGE IS ALLOWED TO RIVER, STREAM, CREEK, WETLANDS, OR BUFFER ZONES.

- 8. TRACER WIRE SHALL BE INSTALLED ALONG ALL WATER LINE, INCLUDING PIPE INSTALLED BY HDD. TRACER WIRE SHALL BE BROUGHT UP INSIDE OF VALVE BOXES. CONTRACTOR SHALL PERFORM A SIGNAL STRENGTH TEST OF THE INSTALLED TRACER WIRE AT END OF PROJECT WITH UTILITY OWNER REPRESENTATIVE PRESENT.
- 9. IN ADDITION TO TRACER WIRE, CONTRACTOR SHALL ALSO INSTALL ELECTRONIC MARKER BALLS ALONG THE WATER LINE AS REQUIRED BY THE UTILITY OWNER. REFER TO THE SPECIAL PROVISIONS AND DETAIL ON SHEET UC-3C.
- 10. ALL HYDRANT AND BLOW-OFF OPERATIONS SHALL BE CONDUCTED IN SUCH A MANNER THAT: PRIVATE PROPERTY IS NOT IMPACTED; THERE SHALL BE NO FLOODING OF STREETS AND DRIVEWAYS; AND NO OTHER TRAFFIC PROBLEMS ARE CREATED.
- 11. VALVES ON WATER LINES 16-INCH AND LARGER SHALL BE BUTTERFLY VALVES CONFORMING TO AWWA C504 WITH MECHANICAL JOINT ENDS AND RATED FOR 150 PSI WORKING PRESSURE.

# PROJECT QUANTITIES

ITEM NUMBER	DESCRIPTION	QUANTITY	
-	-	•	-
5326600000-E	16" WATER LINE	479	LF
5329000000-E	DUCTILE IRON WATER PIPE FITTINGS	2,070	POUNDS
5558600000-E	16" VALVE	2	EA
5810000000-E	ABANDON 16" UTILITY PIPE	477	LF
5872608000-E	DIRECTIONAL DRILLING OF 16"	343	LF



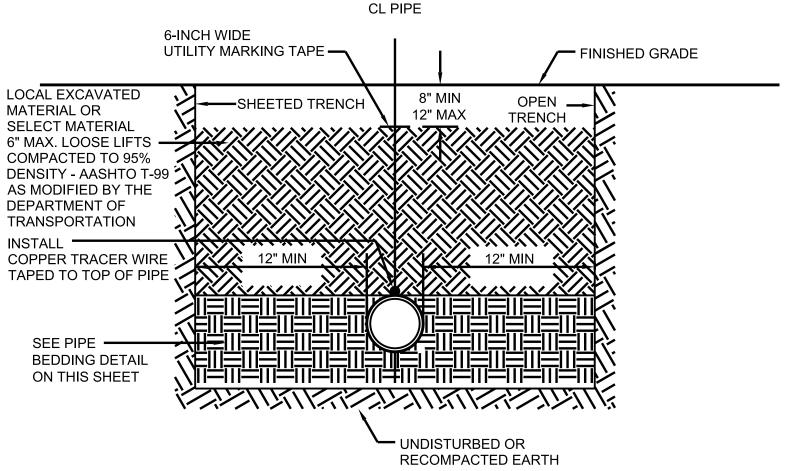
### UTILITY CONSTRUCTION

NC

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Weston & Sampson

WSE of North Carolina, PC
598 East Chatham Street Suite 137 Cary, NC 27511
Phone: 919.297.0220 Fax: 919.297.0220



RECOMMENDED OPEN TRENCH WIDTH AT TOP OF PIPE			
NOMINAL	TRENCH		
PIPE SIZE	WIDTH		
(INCHES)	(INCHES)		
4	29		
6	31		
8	33		
10	35		
12	37		
14	40		
16	42		
18	44		

PIPE BEDDING

FOUNDATION CONDITIONING FABRIC AS REQUIRED

FOUNDATION CONDITIONING AS REQUIRED

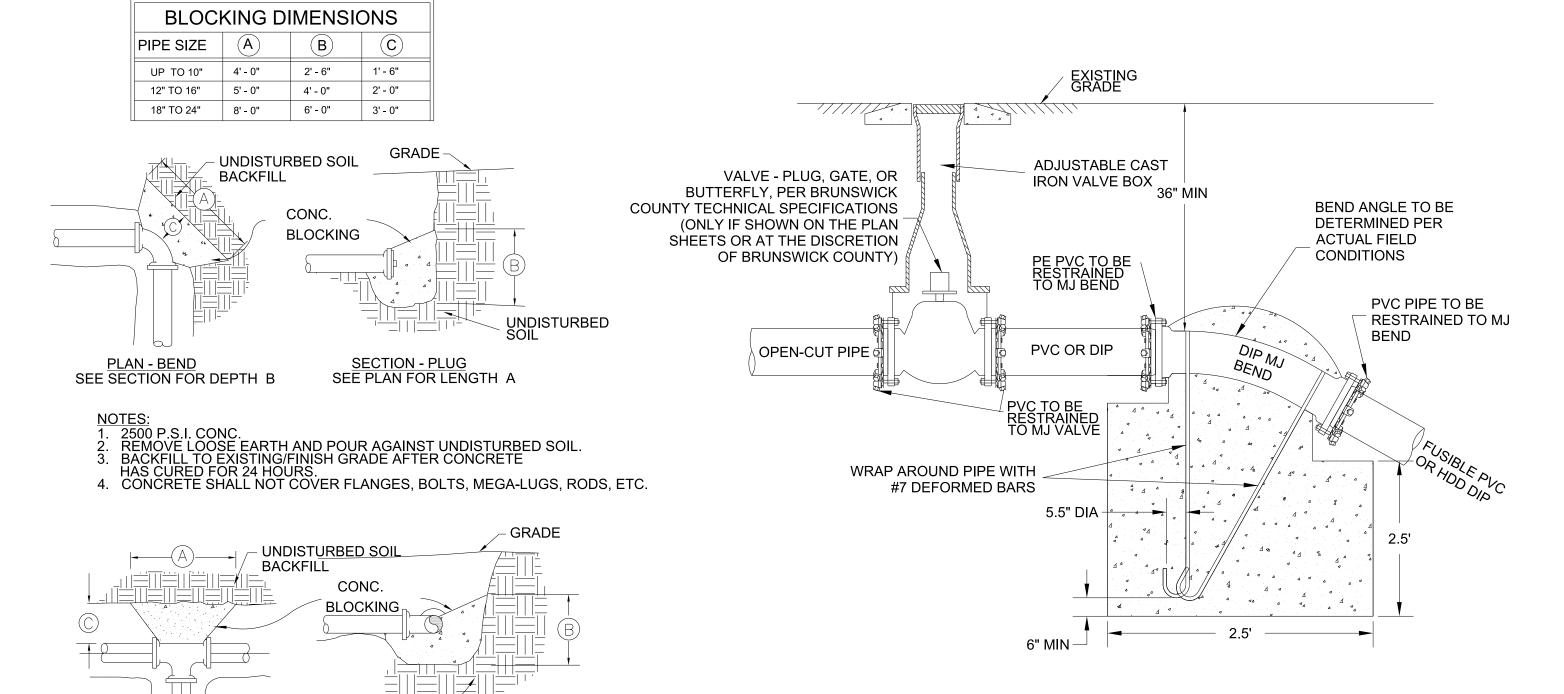
### NOTES:

- 1. IF REQUIRED, PLACE FOUNDATION CONDITIONING MATERIAL BELOW BEDDING AS
- DIRECTED BY ENGINEER.
  2. PIPE BEDDING SHALL BE
- 2. PIPE BEDDING SHALL BE SELECT MATERIAL, EITHER CLASS II (TYPE 1) OR CLASS III, AS PER SECTION 1016.
- 3. TRENCH SHALL BE BACKFILLED IN LOOSE 6" LAYERS COMPACTED TO TOP OF TRENCH USING LOCAL EXCAVATED MATERIAL IF APPROVED BY THE ENGINEER, OR SELECT MATERIAL
- 3. LOCAL EXCAVATED MATERIAL IS DEFINED AS NATIVE SOIL EXCAVATED FROM THE TRENCH THAT IS FREE OF: ROCKS; FOREIGN MATERIAL; AND FROZEN EARTH.
- 4. COMPACTION SHALL BE TO APPROXIMATELY 95% DENSITY IN ACCORDANCE WITH AASHTO T-99 AS MODIFIED BY THE DEPARTMENT OF TRANSPORTATION.

GENERAL TRENCH DETAIL

NOT TO SCALE

PIPE BEDDING DETAIL
NOT TO SCALE



UNDISTURBED SOIL

THRUST BLOCKS
NOT TO SCALE

SECTION - BEND & TEE

SEE PLAN FOR LENGTH A

PLAN - TEE

SEE SECTION FOR DEPTH B

OPEN-CUT PIPE TO FUSIBLE PVC OR HDD DIP TRANSITION ASSEMBLY NOT TO SCALE PLAN VIEW

CONCRETE
COLLAR
WIDTH
WIDTH
3000
P.S.I. MIN
P.S.I. MIN
CROSS SECTION

 UNRESTRAINED SEWER LINES ON 20% SLOPE OR GREATER SHALL BE ANCHORED SECURELY WITH CONCRETE COLLARS. MINIMUM 1 CONCRETE COLLAR PER PIPE JOINT. ADDITIONAL COLLARS MAY BE REQUIRED BY ENGINEER.

NOTES:

2. COLLARS FOR ANTI-SEEPAGE (PIPING) CONTROL FOR PIPELINES CONSTRUCTED THROUGH WETLANDS SHALL BE INSTALLED AT LOCATIONS SHOWN ON THE PLANS.

<u>PIPE COLLAR</u> NOT TO SCALE PROJECT REFERENCE NO. SHEET NO.

17BP.3.R.80 UC-3B

DESIGNED BY: KCZ

DRAWN BY: AJC

CHECKED BY: KCZ

APPROVED BY: KCZ

REVISED:

NORTH CAROLINA
DEPARTMENT OF
TRANSPORTATION

UTILITIES ENGINEERING SEC.
PHONE: (919)707-6690
FAX: (919)250-4151

UC-3B

UC-3B

UC-3B

Docusioned by: KCZ

RESSION

ASDITABLE PROVED BY: KCZ

WITH CAROLINA
DEPARTMENT OF
TRANSPORTATION

UTILITY CONSTRUCTION
PLANS ONLY

## UTILITY CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL UNTIL ALL SIGNATURES ARE COMPLETED

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WSE of North Carolina, PC

598 East Chatham Street Suite 137

Phone: 919.297.0220

License: C-4647 Suite 137 Cary, NC 27511 Fax: 919.297.022

NC

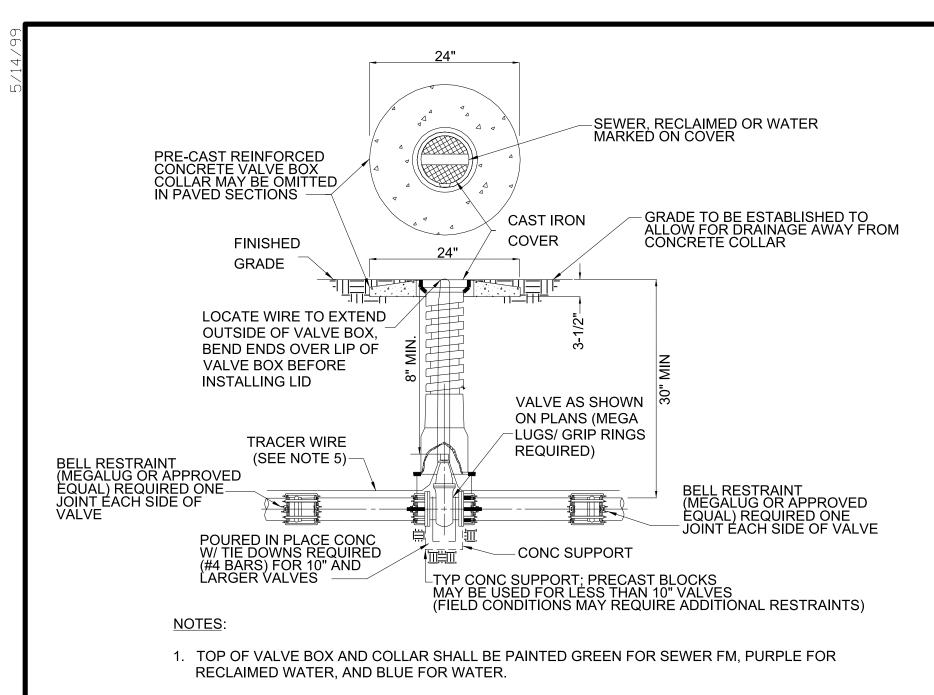
 THIS DETAIL IS A GENERAL SCHEMATIC SHOWING ISOLATION VALVE LOCATIONS AND AIR RELEASE VALVES.
 FINAL LOCATION OF PIPE APPURTENANCES SHALL BE COORDINATED WITH RESIDENT ENGINEER AND BRUNSWICK COUNTY REPRESENTATIVE VALVE & BOX - VALVE & BOX - PULL TRACER WIRE INTO VALVE BOX FLOW 10' MIN,/ FLOW AIR RELEASE VALVE (FORCE MAIN) OR FIRE HYDRANT (WATER) AT HIGH POINT FOR FLUSHING (PER TRACER WIRE DETAIL) HDPE, DIP OR FUSIBLE PVC DIRECTIONALLY DRILLED PIPE

VALVE LOCATIONS FOR DIRECTIONAL DRILL OF

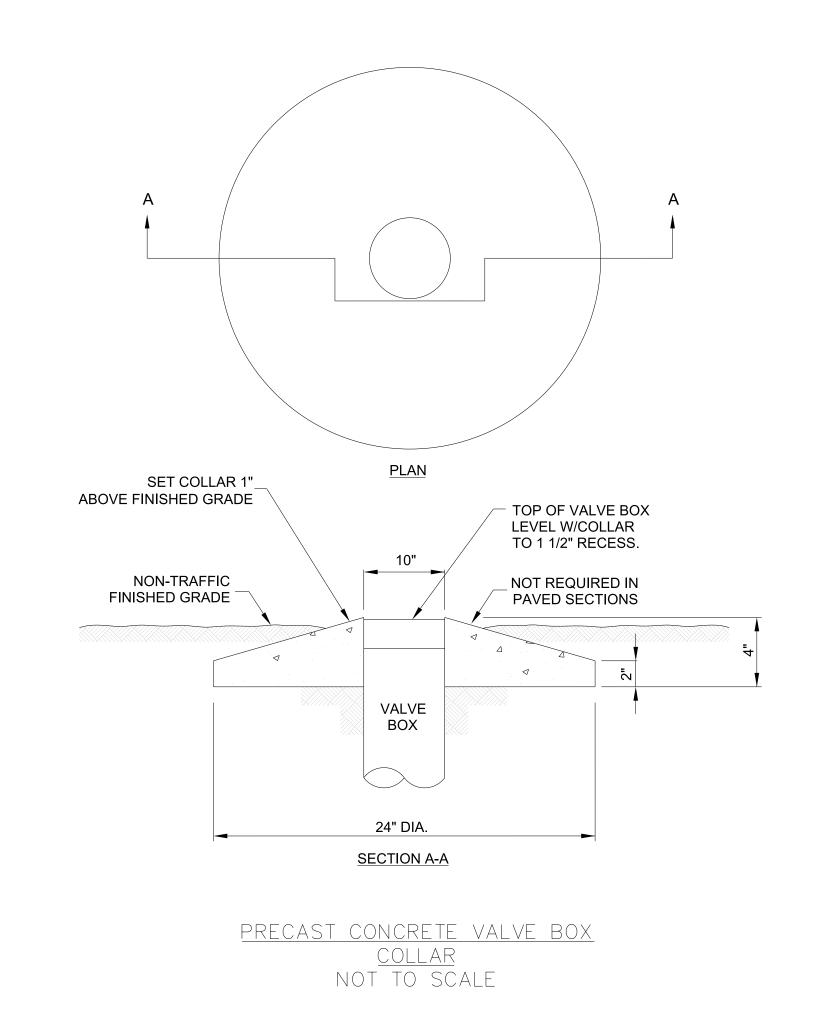
PIPE UNDER WETLANDS/WATER BODY NOT TO SCALE "X" BAR

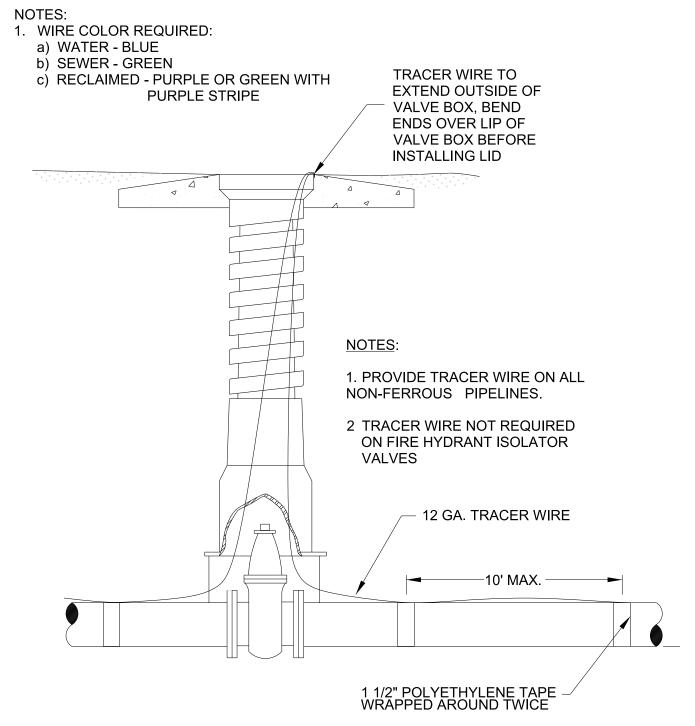
ts/NC\\_Caru01dServer\Engineer\JOBS\0754\006\10000\Utilities\Engineering\UC\Proj\17BP3R80\_

/2022 sela 3 local / WSE / Projects/

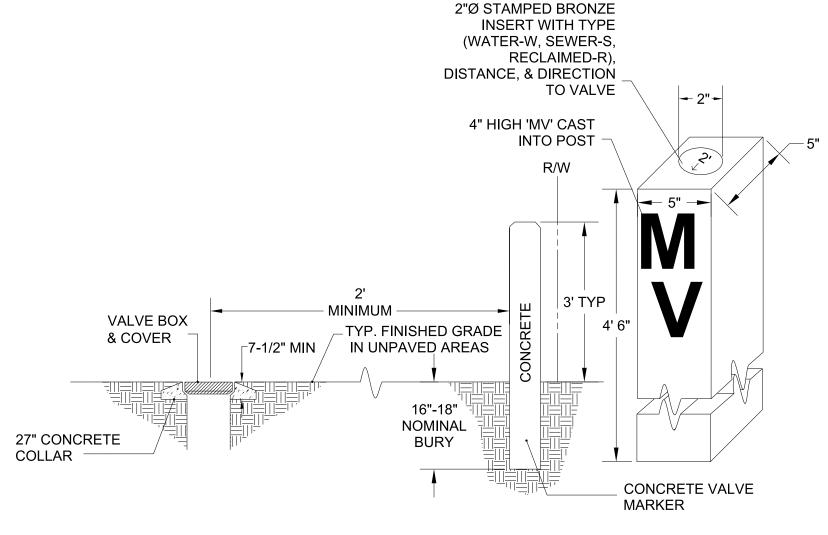


- 2. EXTENSION STEM WILL BE REQUIRED TO BE WITHIN 2 FEET OF THE SURFACE IF OPERATING NUT WOULD OTHERWISE BE OVER 5 FEET BELOW GRADE. EXTENSIONS SHALL BE PERMANENTLY ATTACHED TO VALVE NUT AND SHALL BE PROVIDED WITH HORIZONTAL SPACERS FOR VERTICAL ALIGNMENT WITHIN THE VALVE BOX.
- 3. MINIMUM CLEARANCE TO EXTENSION IS 8".
- 4. WIRE COLORS REQUIRED: SEWER-GREEN/WATER-BLUE/RECLAIMED WATER-PURPLE OR GREEN WITH PURPLE STRIPE





TRACER WIRE AT VALVES
NOT TO SCALE



PROJECT REFERENCE NO. | SHEET NO.

PHONE: (919)707-6690 UTILITY CONSTRUCTION FAX: (919)250-4151 PLANS ONLY

UTILITY CONSTRUCTION

DOCUMENT NOT CONSIDERED FINAL

UNTIL ALL SIGNATURES ARE COMPLETEL

AJC

UC-3C

NC

License:

C-4647

Cary, NC 27511

Fax: 919.297.022

17BP.3.R.80

DESIGNED BY: KCZ

CHECKED BY: KCZ

APPROVED BY: KCZ

NORTH CAROLINA DEPARTMENT OF

TRANSPORTATION

ITILITIES ENGINEERING SEC

Weston & Sampson

598 East Chatham Street Suite 137

WSE of North Carolina, PC

Phone: 919.297.0220

DRAWN BY:

REVISED:

NOTES:

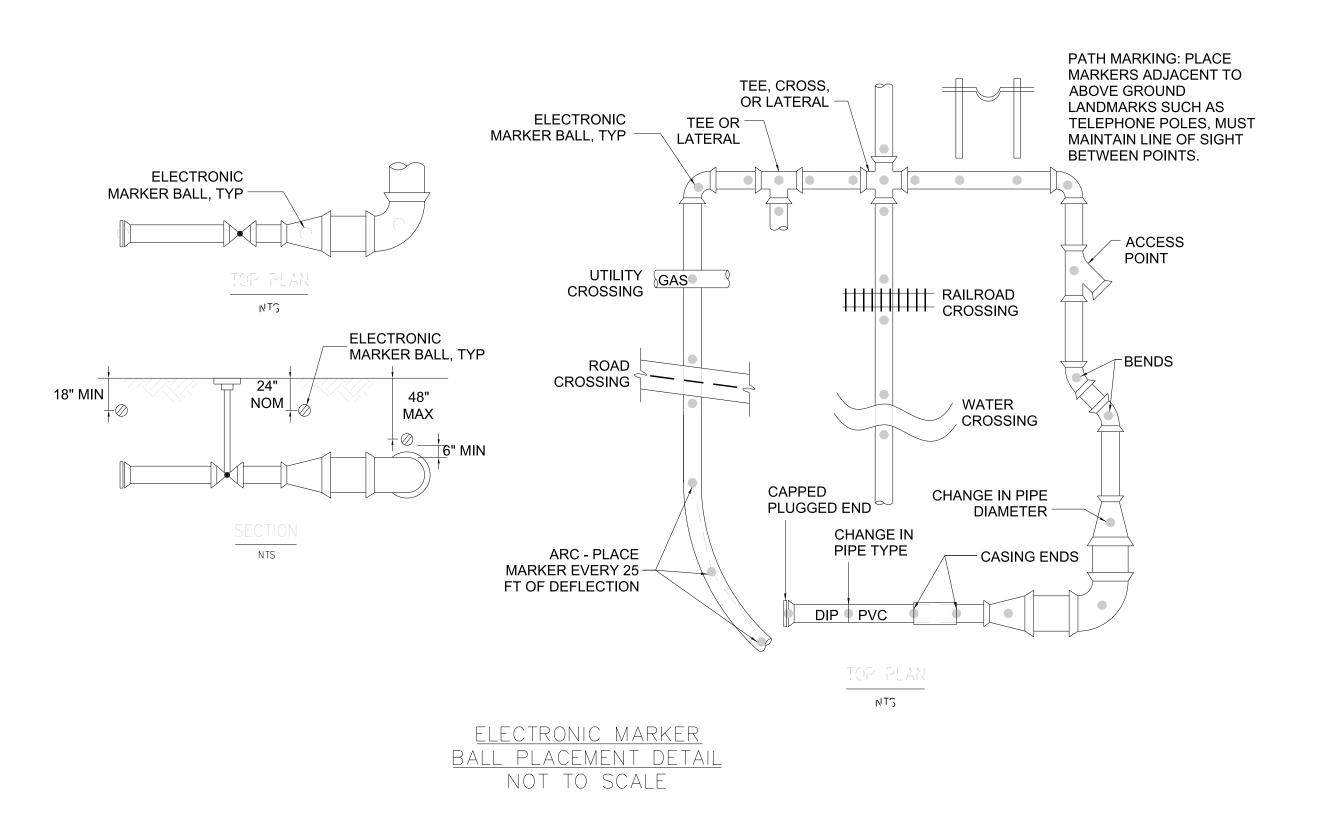
1. VALVE MARKER TO BE INSTALLED UPRIGHT WITH THE CAST 'MV' FACING THE NEAREST STREET.

- 2. DIRECTION AND DISTANCE MUST BE ETCHED OR STAMPED INTO THE BRONZE INSERT.
- 3. ONE POST TO BE INSTALLED FOR EACH VALVE LOCATED OUTSIDE OF THE PAVEMENT.
- 4. VALVE MARKER TO BE INSTALLED AT THE RIGHT OF
- 5. NO VALVE MARKER REQUIRED FOR FIRE HYDRANT ISOLATION VALVES.
- 6. PAINT VALVE MARKER USING STANDARD LOCATING COLORS:
  a) WATER BLUE
  b) SEWER GREEN
  c) RECLAIMED PURPLE

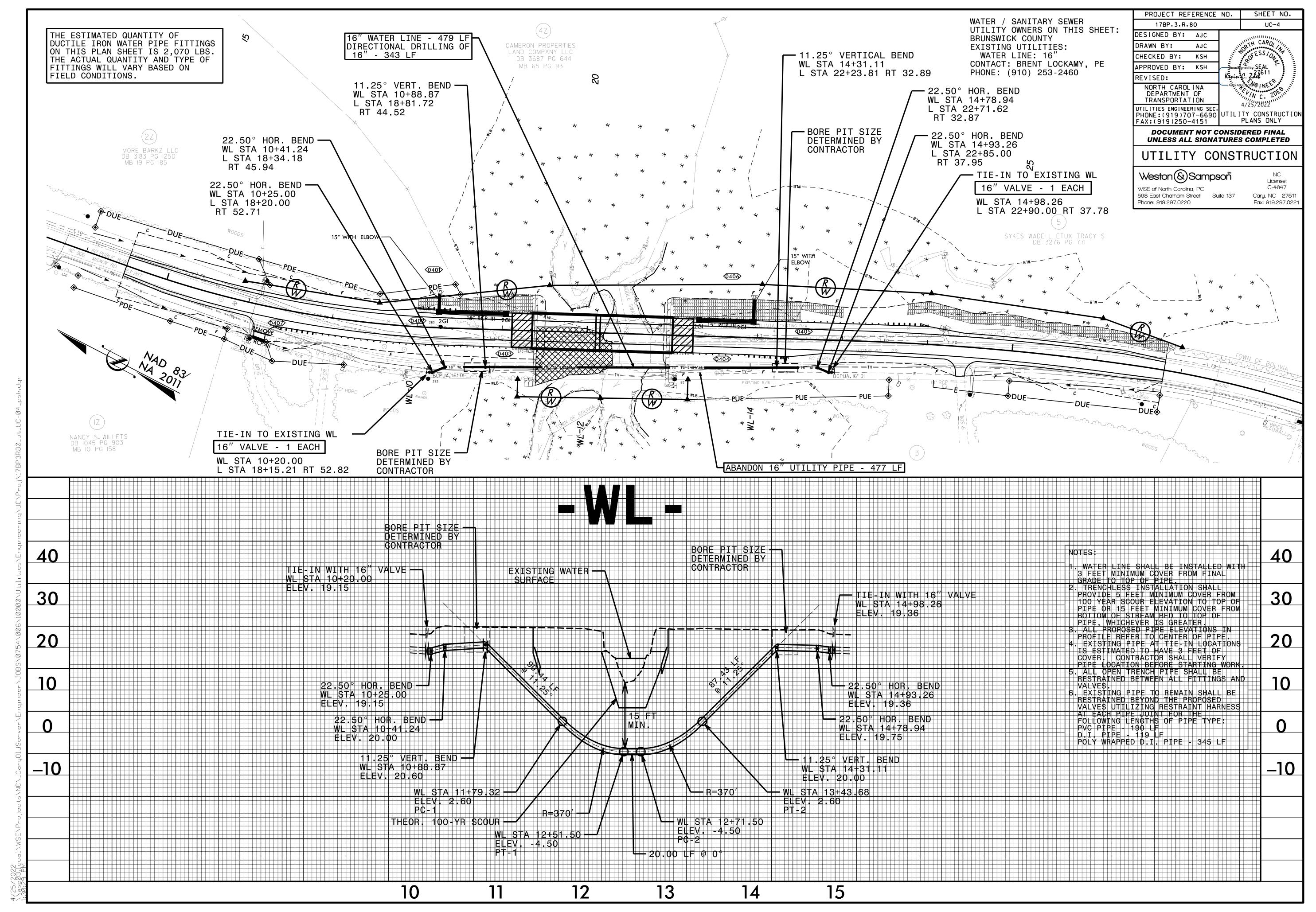
VALVE MARKER IN RIGHT OF WAY

(PAVED)

NOT TO SCALE



/8/2022

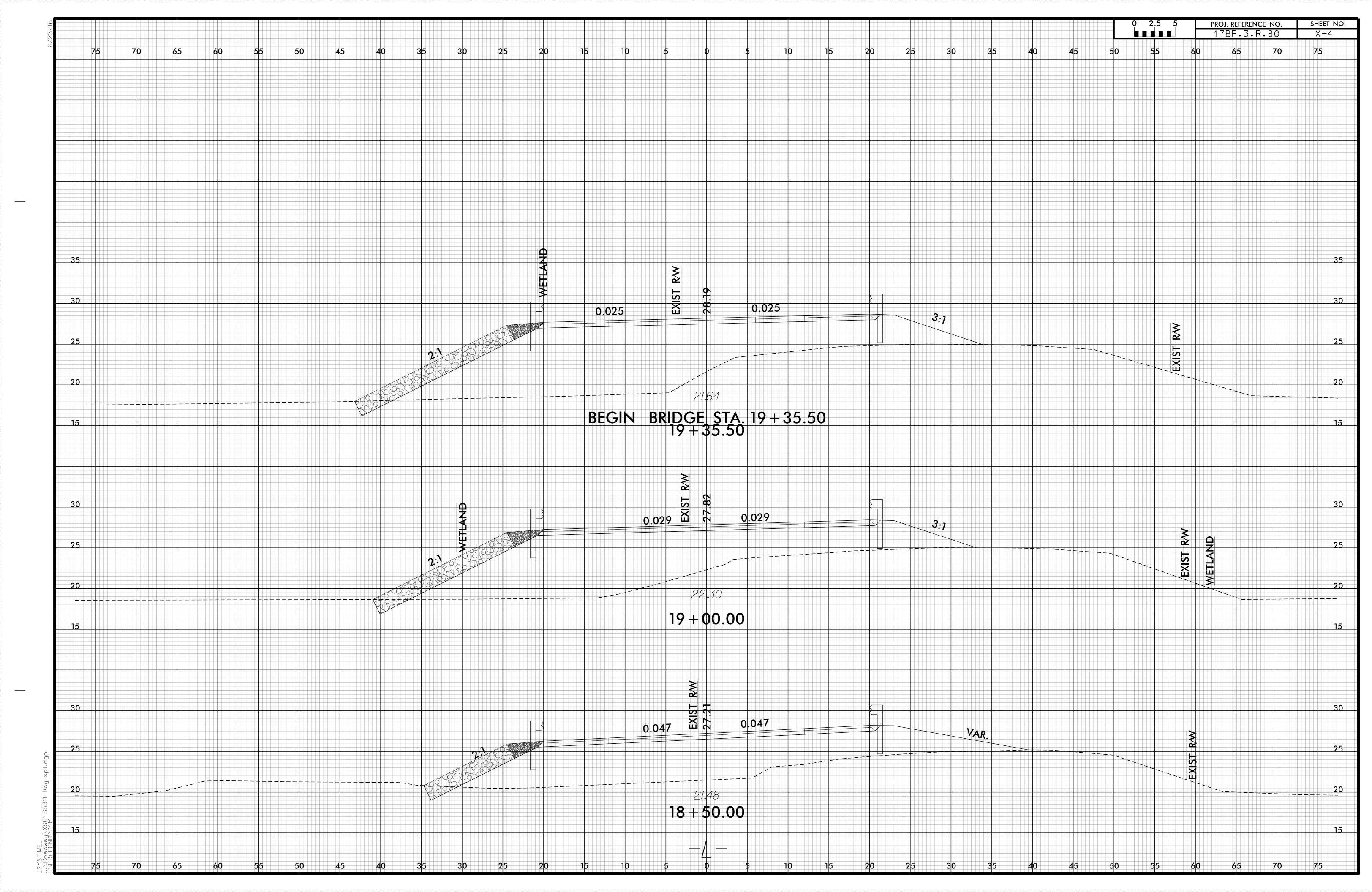


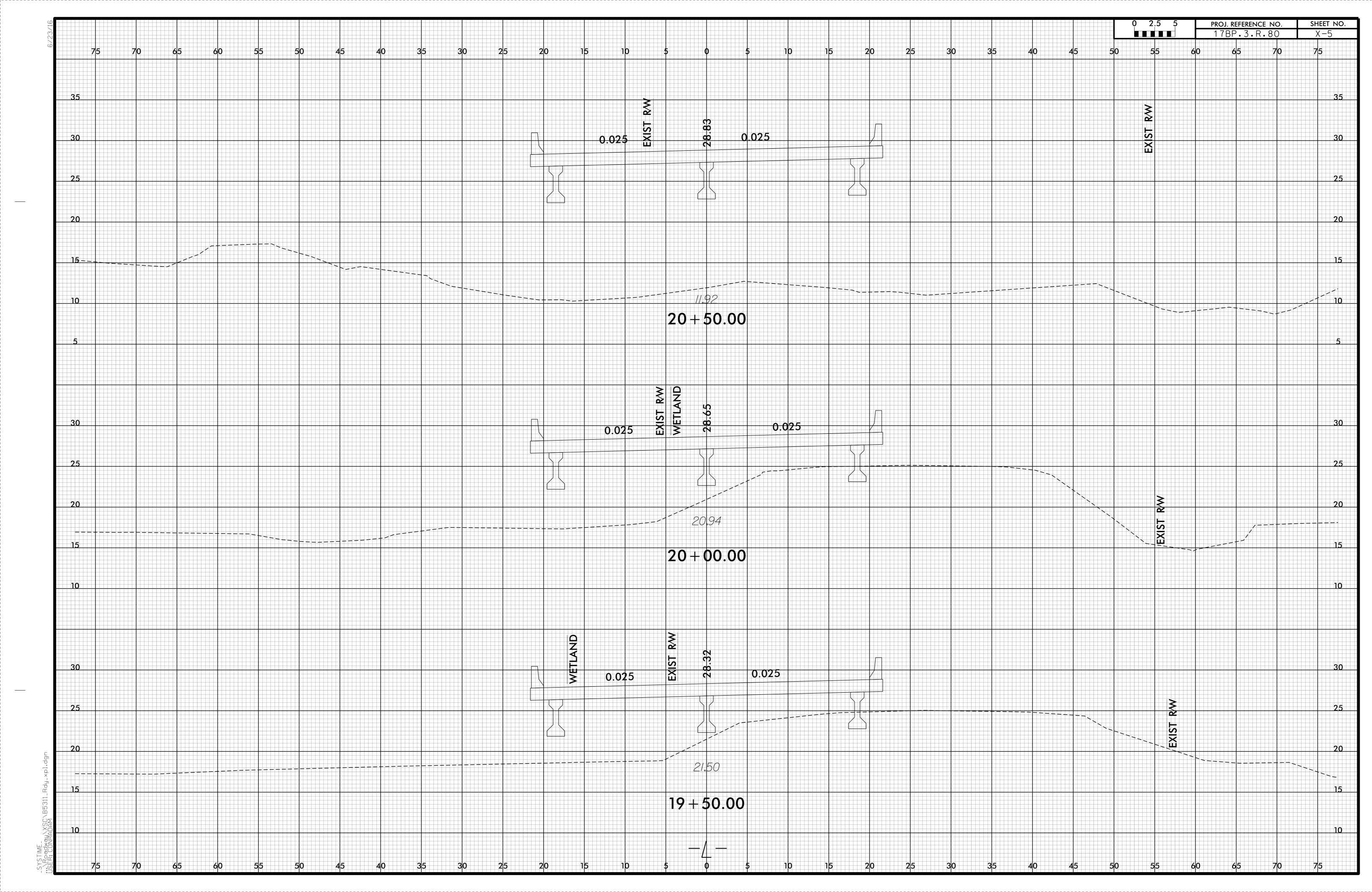
DJECT REFERENCE NO. SHEET NO. X-/

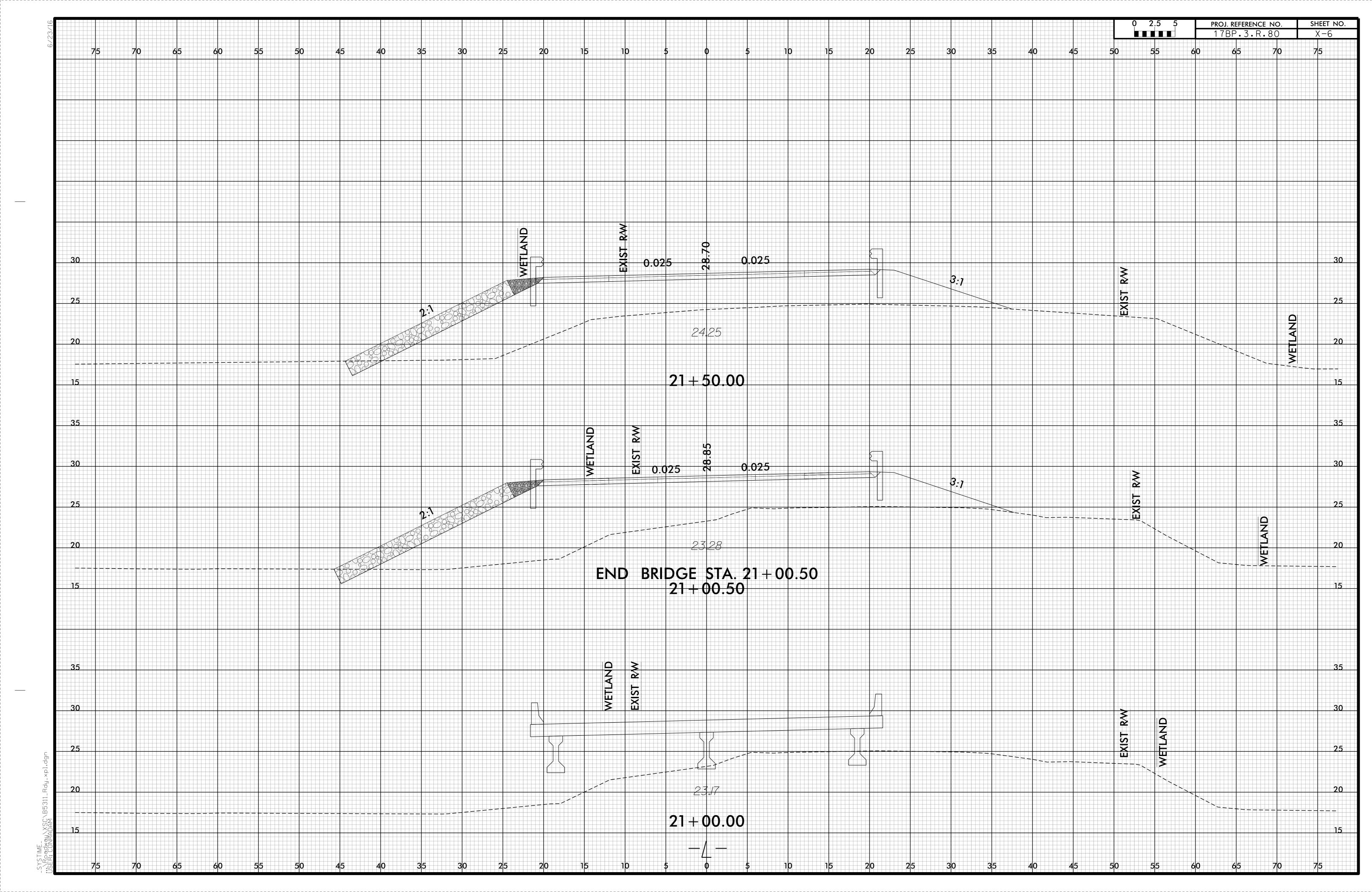
# CROSS SECTION SHEET INDEX

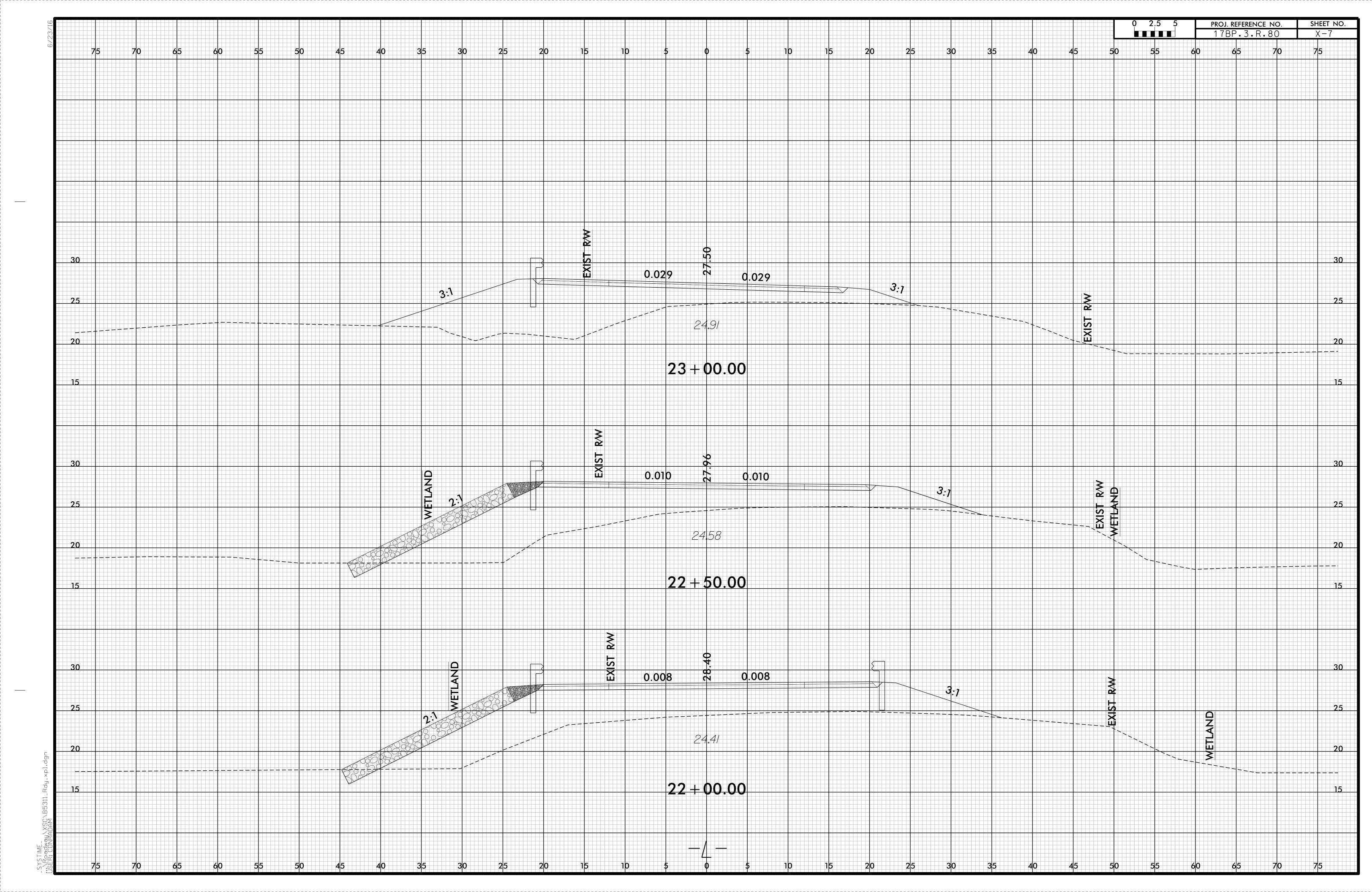
- X-1 CROSS SECTION SHEET INDEX
- X-2 THRU X-9 -L-

Note: "Quantities are approximate only. The Resident Engineer will re–cross–section the work accurately when the project is staked out. These cross–section notes will be used in computing the final quantities for which the contractor will be paid."









\_END PROJECT

DESIGN DATA

ADT 2021 = 5924 VPD

ADT 2040 = 9800 VPD

TTST = 1% DUALS = 4%

**REGIONAL TIER** 

MAJOR COLLECTOR

FUNC CLASS =

V = 60 MPH

VICINITY MAP

STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

# 17BP.3.R.80 (FORMERLY B-5311) STATE PROJ.NO. 17BP.3.PE.80 17BP.3.ROW.80 N/A 17BP.3.R.80 N/A CONST.

Prepared in the Office of:

2018 STANDARD SPECIFICATIONS

LETTING DATE: AUGUST 18, 2022

CDM SMITH

FOR THE NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DAVID Z. KEISER, P.E.

PROJECT ENGINEER

TING H. FANG, P.E.

PROJECT DESIGN ENGINEER

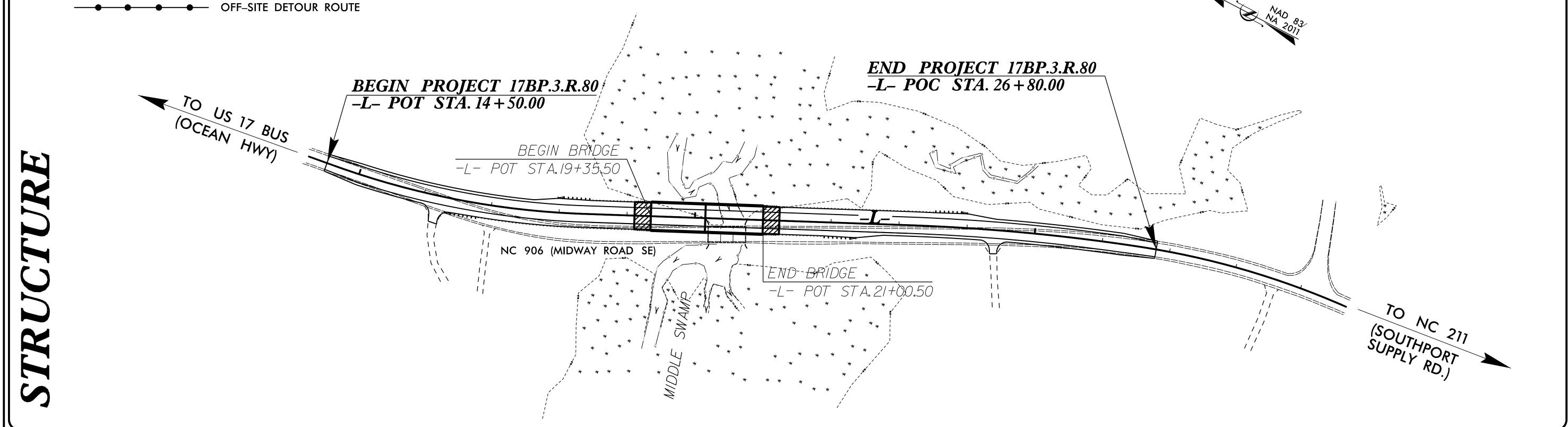
5400 Glenwood Avenue, Suite 400

Raleigh, NC 27612–3228 NC COA No. F–1255

# BRUNSWICK COUNTY

LOCATION: REPLACE BRIDGE 104 OVER MIDDLE SWAMP ON NC 906 (MIDWAY ROAD SE)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE

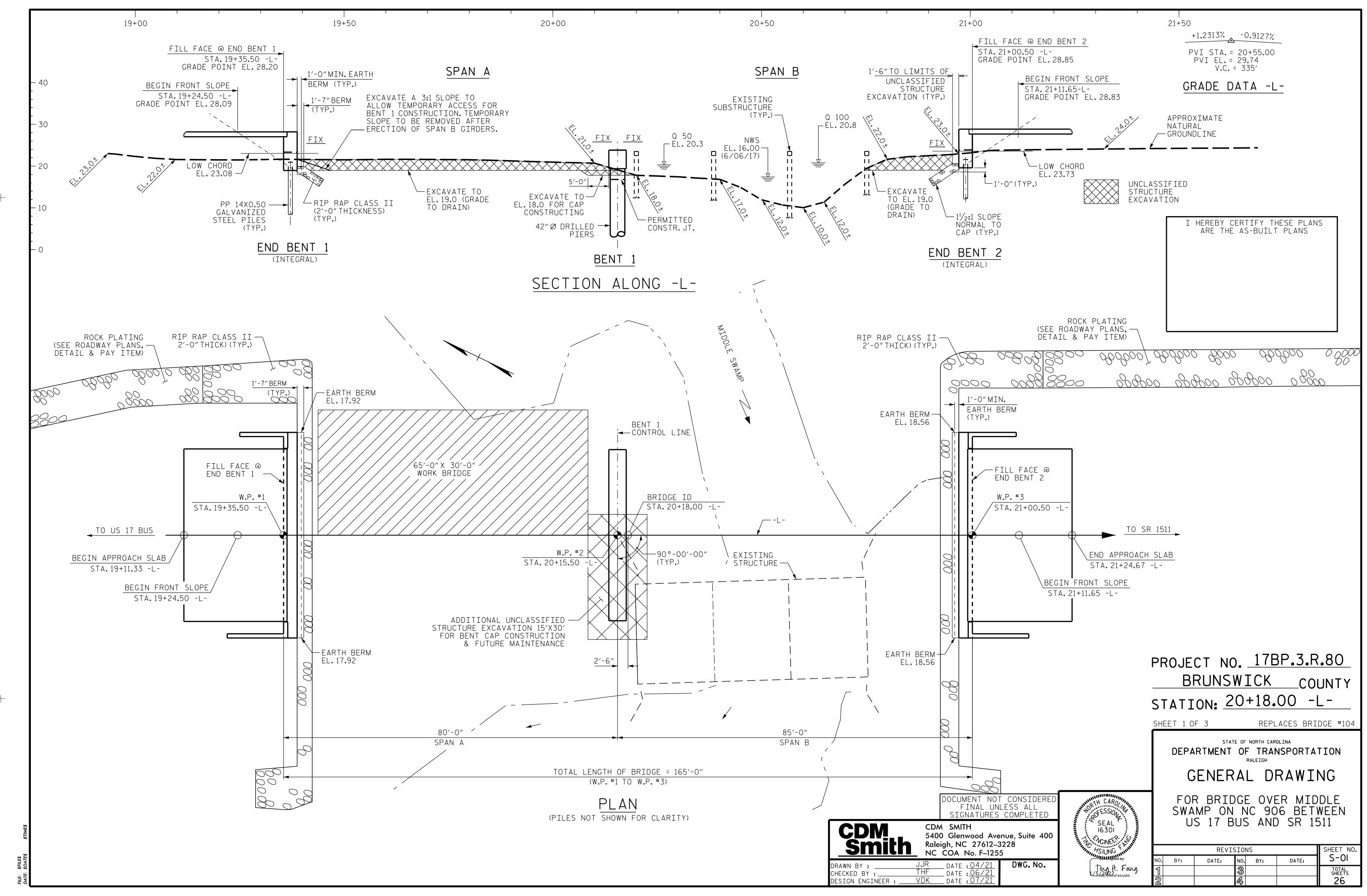


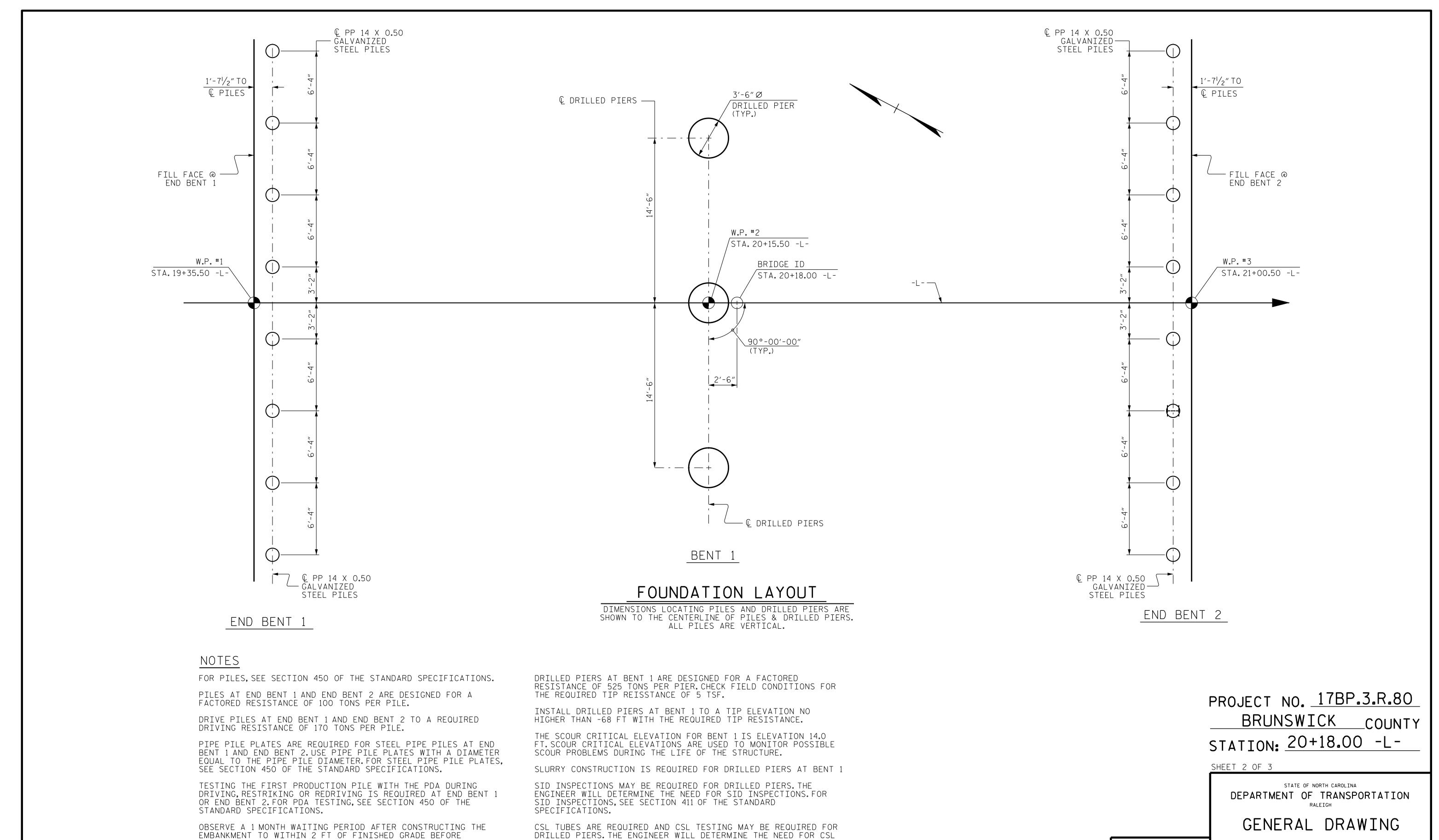
PROJECT LENGTH

LENGTH ROADWAY TIP PROJECT 17BP.3.R.80 = 0.202 MILES

LENGTH STRUCTURE TIP PROJECT 17BP.3.R.80 = 0.031 MILES

TOTAL LENGTH OF TIP PROJECT 17BP.3.R.80 = 0.233 MILES





TESTING. FOR CSL TESTING, SEE SECTION 411 OF THE STANDARD

SPECIFICATIONS.

DOCUMENT NOT CONSIDERED

FINAL UNLESS ALL

5400 Glenwood Avenue, Suite 400

Raleigh, NC 27612-3228

NC COA No. F-1255

JJR DATE : 04/21

THF DATE: 06/21 VDK DATE: 07/21

CHECKED BY : \_\_

DESIGN ENGINEER: \_

CDM SMITH

SIGNATURES COMPLETED

DWG. No.

ON ESSION IN

16301

1 CHOINEER

HSIUNG

Ting H. Fang 1×5/20223C4F9AA469...

SEAL

FOR BRIDGE OVER MIDDLE

SWAMP ON NC 906 BETWEEN

US 17 BUS AND SR 1511

NO. BY:

REVISIONS

DATE:

BY:

SHEET NO.

S-02

TOTAL SHEETS

26

DATE:

.

BEGINNING END BENT CONSTRUCTION AT END BENT 2. FOR BRIDGE

WAITING PERIODS, SEE ROADWAY PLANS AND SECTION 235 OF THE

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD

STANDARD SPECIFICATIONS.

SPECIFICATIONS.

\$FILE\$ E: \$DATE\$

	—— TOTAL BILL OF MATERIAL ——																							
	CONST., MAINT. &REMOVAL OF TEMP. ACCESS	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	3'-6"DIA DRILLED PIERS	PDA TESTING	SID INSPECTIONS	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	REINFORCED CONCRETE DECK SLAB	GROOVING BRIDGE FLOORS	CLASS A CONCRETE	BRIDGE APPROACH I SLABS	EPOXY COATED REINFORCING STEEL	EPOXY COATED SPIRAL COLUMN REINFORCING STEEL	45" PRESTRESSED CONCRETE GIRDERS	PILE DRIVING EQUIPMENT SETUP FOR PP 14 X 0.50 GALVANIZED STEEL PILES	PP 14 GALVA STEEL	X 0.50 ANIZED PILES	STEEL PILE PLATES	PILE REDRIVES	CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS
	LUMP SUM	LUMP SUM	LUMP SUM	LIN.FT.	EA.	EA.	EA.	LUMP SUM	SQ.FT.	SQ.FT.	CU. YDS.	LUMP SUM	LBS.	LBS.	NO. LIN.FT.	EA.	NO. L	IN.FT.	EA.	EA.	LIN.FT.	TON	SQ. YD.	LUMP SUM
SUPERSTRUCTURE									7,064	7,807					12 976.50						326.67			LUMP SUM
END BENT 1								LUMP SUM			35.7		5,554			8	8	360	8	4		122	135	
BENT 1				261.0				LUMP SUM			31.6		22,876	5,765										
END BENT 2								LUMP SUM			35.7		5,554			8	8	320	8	4		131	145	
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	261.0	1	1	1	LUMP SUM	7,064	7,807	103.0	LUMP SUM	33,984	5,765	12 976.50	16	16	680	16	8	326.67	253	280	LUMP SUM

#### B.M. #2: R/R SPIKE SET IN POWER POLE, 50.79' RIGHT OF STA. 21+04.52 -L-, EL. 24.89' $\Rightarrow$ WOODS SWAMP SWAMP -ROCK PLATING -RIP RAP (SEE ROADWAY PLANS. CLASS II ROCK PLATING — DETAIL, & PAY ITEM) CLASS II (TYP.) SEE ROADWAY PLANS, (TYP.) DETAIL & PAY ITEM) BRIDGE ID TO SR 1511 TO US 17 BUS STA. 20+18.00 -L--L- (NC 906) -90°-00′-00″l ∽ EXISTING STRUCTURE MIDDLE SWAMP ■ B.M. #2 PROPOSED GUARDRAIL (ROADWAY DETAILS-& PAY ITEM)(TYP.) SWAMP FOR UTILITY INFORMATION, SEE UTILITY PLANS AND SPECIAL PROVISIONS LOCATION SKETCH

#### HYDRAULIC DATA

= 20.8 FT.

= 1000 CFS DESIGN DISCHARGE FREQUENCY OF DESIGN FLOOD = 50 YR. DESIGN HIGH WATER ELEVATION = 20.3 FT. DRAINAGE AREA = 7.5 SQ. MI. BASE DISCHARGE (Q100) = 1300 CFS

BASE HIGH WATER ELEVATION

#### OVERTOPPING FLOOD DATA

= 6100 CFS OVERTOPPING DISCHARGE FREQUENCY OF OVERTOPPING FLOOD = 500+ YRS. OVERTOPPING FLOOD ELEVATION = 26.1 FT. \*

\* ELEVATION IS TAKEN AT STA. 26+20.53 -L-

#### NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 2.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN. FOR EROSION CONTROL MEASURES SEE EROSION CONTROL PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC-18 EVALUATING SCOUR AT BRIDGES".

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

FOR END BENTS 1 & 2, ONLY PARTIAL GALVANIZING OF THE PILES IS REQUIRED. SEE END BENT SHEETS FOR REQUIRED GALVANIZED LENGTHS. PAYMENT FOR PARTIALLY GALVANIZED PILES WILL BE MADE UNDER THE CONTRACT UNIT PRICE FOR GALVANIZED STEEL PILES.

THE CLASS AA CONCRETE IN THE BRIDGE DECK SHALL CONTAIN FLY ASH OR GROUND GRANULATED BLAST FURNACE SLAG AT THE SUBSTITUTION RATE SPECIFIED IN ARTICLE 1024-1 AND IN ACCORDANCE WITH ARTICLES 1024-5 AND 1024-6 OF THE STANDARD SPECIFICATIONS. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE COST OF THE REINFORCED CONCRETE DECK SLAB.

THIS STRUCTURE CONTAINS THE NECESSARY CORROSION PROTECTION REQUIRED FOR A CORROSIVE SITE.

ALL METALLIZED SURFACES SHALL RECEIVE A SEAL COATING AS SPECIFIED IN THE SPECIAL PROVISION FOR THERMAL SPRAYED COATINGS (METALLIZATION).

CLASS AA CONCRETE SHALL BE USED IN ALL CAST-IN-PLACE COLUMNS, BENT CAPS AND PILE CAPS, SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR. FOR CALCIUM NITRITE CORROSION INHIBITOR, SEE SPECIAL PROVISIONS.

ALL BAR SUPPORTS AND ALL INCIDENTAL REINFORCING STEEL SHALL BE EPOXY COATED IN ACCORDANCE THE STANDARD SPECIFICATIONS.

THE CONCRETE IN THE END BENT AND BENT CAPS OF BENT 1 SHALL CONTAIN SILICA FUME. SILICA FUME SHALL BE SUBSTITUTED FOR 5% OF THE PORTLAND CEMENT BY WEIGHT. IF THE OPTION OF ARTICLE 1024-1 OF THE STANDARD SPECIFICATIONS TO PARTIALLY SUBSTITUTE CLASS F FLY ASH FOR PORTLAND CEMENT IS EXERCISED, THEN THE RATE OF FLY ASH SUBSTITUTION SHALL BE REDUCED TO 1.0 LB OF FLY ASH PER 1.0 LB. NO PAYMENT WILL BE MADE FOR THIS SUBSTITUTION AS IT IS CONSIDERED INCIDENTAL TO THE VARIOUS PAY ITEMS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 5 FT. LEFT SIDE AND 65 FT. RIGHT SIDE OF CENTERLINE ROADWAY AT END BENT 1 AND 14 FT. LEFT SIDE AND 60 FT. RIGHT SIDE OF CENTERLINE ROADWAY AT END BENT 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTING OF 3 SPANS: 1 @ 18'-5", 1 @ 18'-3" AND 1 @ 18'-6" WITH A CLEAR ROADWAY WIDTH OF 24'-0" AND A 5" REINFORCED CONCRETE DECK ON 12 LINES OF W12X16.5; SUBSTRUCTURE CONSISTING OF RC CAPS ON TIMBER PILES AT END BENTS AND BENT 2, BENT 1 WITH STEEL CAP AND PILE CRUTCHES LOCATED AT THE SITE OF THE PROPOSED BRIDGE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

NEEDLE BEAMS WILL NOT BE ALLOWED UNLESS OTHERWISE CALLED FOR ON THE PLANS OR APPROVED BY THE ENGINEER.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 20+18.00 -L-."

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY. SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR BRIDGE APPROACH FILLS TYPE 5 GEOTEXTILE, SEE SPECIAL PROVISIONS.

FOR CONSTRUCTION, MAINTENANCE & REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

> PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

SHEET 3 OF 3

OFESSION

SEAL

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Z. NGINEER.

HSILING

Ting Fang

-60E43C9AEA60462 4/20/2022

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER MIDDLE SWAMP ON NC 906 BETWEEN BUS 17 AND SR 1511

SHEET NO REVISIONS S-03 NO. BY: DATE: BY: DATE: SHEETS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

CDM SMITH 5400 Glenwood Avenue, Suite 400 Raleigh, NC 27612-3228 NC COA No. F-1255

\_\_ DATE :<u>04/21</u> THF DATE : 06/21 VDK DATE : 07/21

CHECKED BY :

DESIGN ENGINEER : .

DWG. No.

		LOAD AN	ID RES	SIST	ANCE	FA(	CTOR	RAT	ING	(LRF	D) S	UMMA	RY F	OR I	PRES	TRES	SSED	CON	CRET	E GI	RDEF	RS		
										STRE	NGTH	I LIM	MIT S	ГАТЕ				SE	RVICE	III	LIMI	T STA	TE	
										MOMENT					SHEAR						MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A		1.46		1.75	0.684	1.75	85′	EL	41.3	0.684	1.75	85′	EL	7.8	0.80	0.684	1.46	85′	EL	41.3	
DESIGN		HL-93(0pr)	N/A		1.99		1.35	0.621	2.45	85′	I	41.3	0.777	1.99	85′	I	7.8	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.95	70.20	1.75	0.684	2.34	85′	EL	41.3	0.684	2.29	85′	EL	7.8	0.80	0.684	1.95	85′	EL	41.3	
RATING		HS-20(0pr)	36.000		2.59	93.24	1.35	0.621	3.28	85′	I	41.3	0.777	2.59	85′	I	7.8	N/A						
		SNSH	13.500	1	4.51	60.89	1.4	0.684	6.77	85′	EL	41.3	0.684	7.23	85′	EL	7.8	0.80	0.684	4.51	85′	EL	41.3	
		SNGARBS2	20.000	-	3.32	66.40	1.4	0.684	4.98	85′	EL	41.3	0.684	5.06	85′	EL	7.8	0.80	0.684	3.32	85′	EL	41.3	
		SNAGRIS2	22.000	-	3.13	68.86	1.4	0.684	4.69	85′	EL	41.3	0.684	4.67	85′	EL	7.8	0.80	0.684	3.13	85′	EL	41.3	
		SNCOTTS3	27.250	-	2.25	61.31	1.4	0.684	3.37	85′	EL	41.3	0.684	3.53	85′	EL	7.8	0.80	0.684	2.25	85′	EL	41.3	
	S S	SNAGGRS4	34.925	-	1.86	64.96	1.4	0.684	2.79	85′	EL	41.3	0.684	2.88	85′	EL	7.8	0.80	0.684	1.86	85′	EL	41.3	
		SNS5A	35.550		1.82	64.70	1.4	0.684	2.73	85′	EL	41.3	0.684	2.90	85′	EL	7.8	0.80	0.684	1.82	85′	EL	41.3	
		SNS6A	39.950		1.66	66.32	1.4	0.684	2.49	85′	EL	41.3	0.684	2.62	85′	EL	7.8	0.80	0.684	1.66	85′	EL	41.3	
LEGAL		SNS7B	42.000		1.58	66.36	1.4	0.684	2.37	85′	EL	41.3	0.684	2.57	85′	EL	7.8	0.80	0.684	1.58	85′	EL	41.3	
LOAD		TNAGRIT3	33.000		2.03	66.99	1.4	0.684	3.04	85′	EL	41.3	0.684	3.16	85′	EL	7.8	0.80	0.684	2.03	85′	EL	41.3	
RATING		TNT4A	33.075		2.03	67.14	1.4	0.684	3.05	85′	EL	41.3	0.684	3.09	85′	EL	7.8	0.80	0.684	2.03	85′	EL	41.3	
		TNT6A	41.600		1.65	68.64	1.4	0.684	2.48	85′	EL	41.3	0.684	2.72	85′	EL	7.8	0.80	0.684	1.65	85′	EL	41.3	
	LS.	TNT7A	42.000		1.66	69.72	1.4	0.684	2.49	85′	EL	41.3	0.684	2.66	85′	EL	7.8	0.80	0.684	1.66	85′	EL	41.3	
		TNT7B	42.000		1.71	71.82	1.4	0.684	2.56	85′	EL	41.3	0.684	2.50	85′	EL	7.8	0.80	0.684	1.71	85′	EL	41.3	
		TNAGRIT4	43.000		1.63	70.09	1.4	0.684	2.45	85′	EL	41.3	0.684	2.43	85′	EL	7.8	0.80	0.684	1.63	85′	EL	41.3	
		TNAGT5A	45.000		1.54	69.30	1.4	0.684	2.31	85′	EL	41.3	0.684	2.40	85′	EL	7.8	0.80	0.684	1.54	85′	EL	41.3	
		TNAGT5B	45.000	3	1.53	68.86	1.4	0.684	2.29	85′	EL	41.3	0.684	2.31	85′	EL	7.8	0.80	0.684	1.53	85′	EL	41.3	

LOAD FACTORS:

DESIGN	LIMIT STATE	$\gamma_{DC}$	$\gamma_{\sf DW}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

**COMMENTS:** 

(#) CONTROLLING LOAD RATING

(1) DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

 $\sqrt{3}$  LEGAL LOAD RATING \*\*

\*\* SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

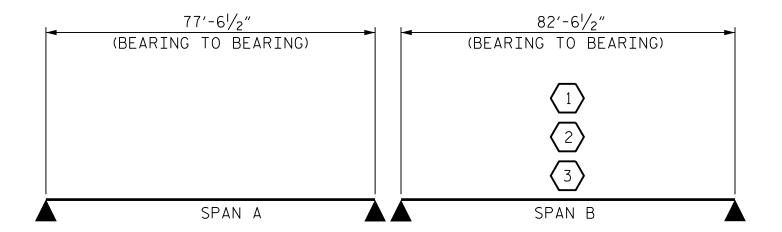
SEAL 16301

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Ting H. Fang 1×5/20022204F9AA469...

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER



LRFR SUMMARY

PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK COUNTY STATION: 20+18.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
STANDARD

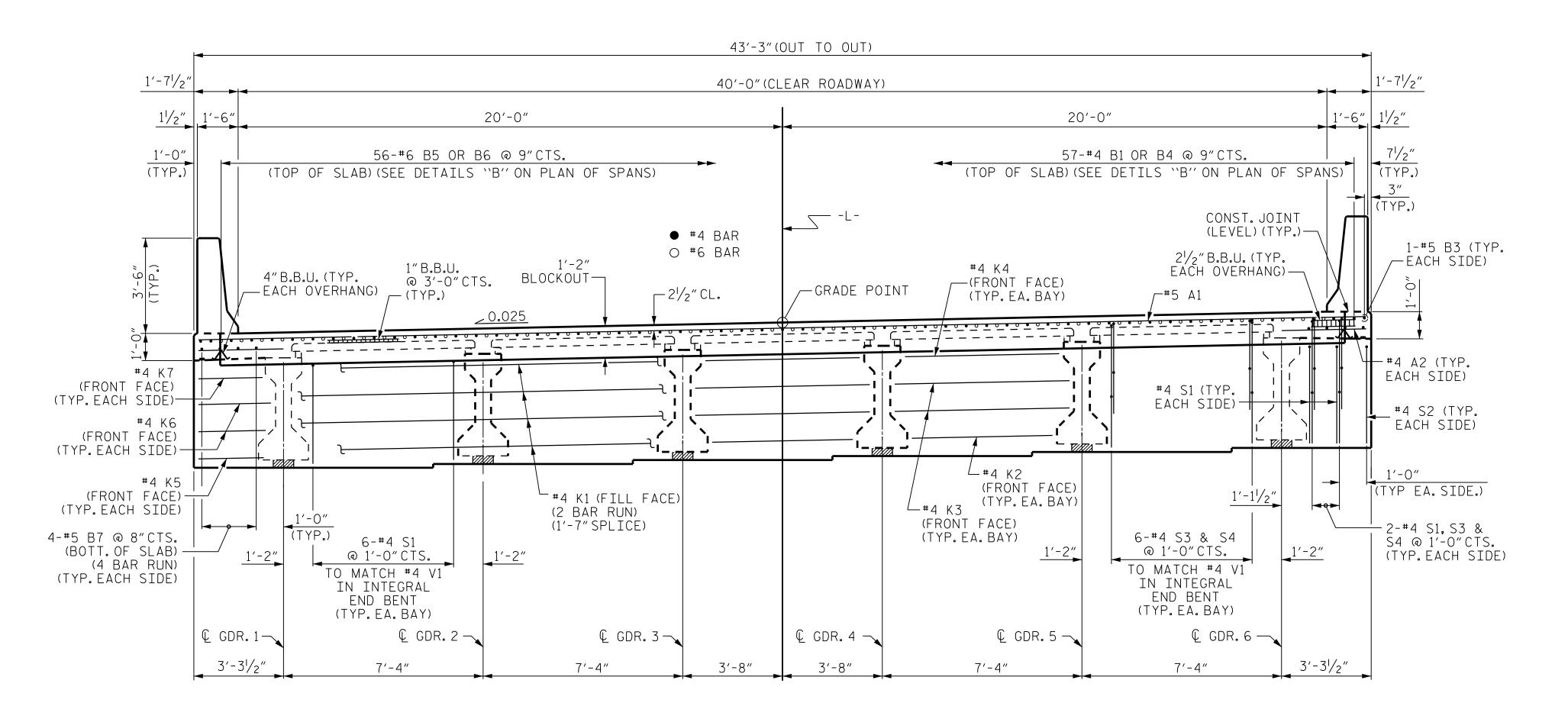
LRFR SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

(NON-INTERSTATE TRAFFIC)

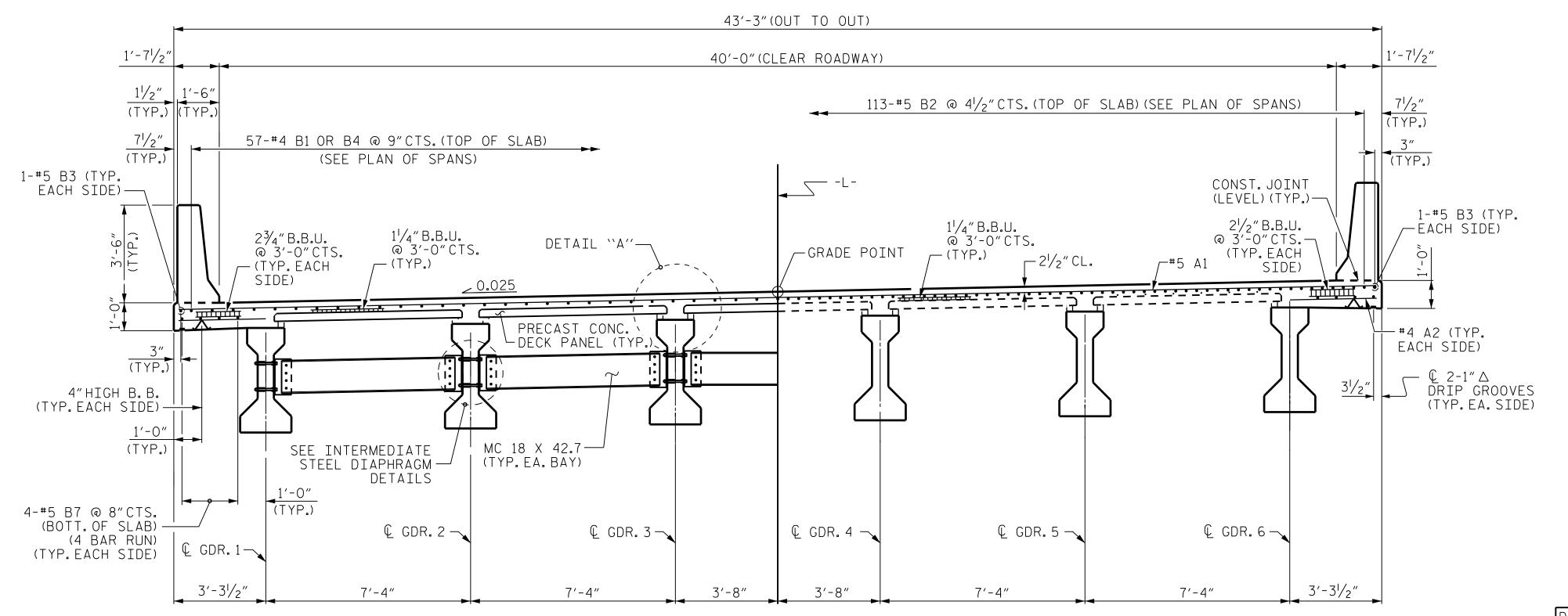
REVISIONS S-04 NO. BY: DATE: DATE: BY: TOTAL SHEETS 26

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THF DATE : 06/21
VDK DATE : 07/21 DWG. No. CHECKED BY : \_\_\_

DESIGN ENGINEER : \_



#### TYPICAL SECTION SHOWING ABUTMENT WALL AT END BENT



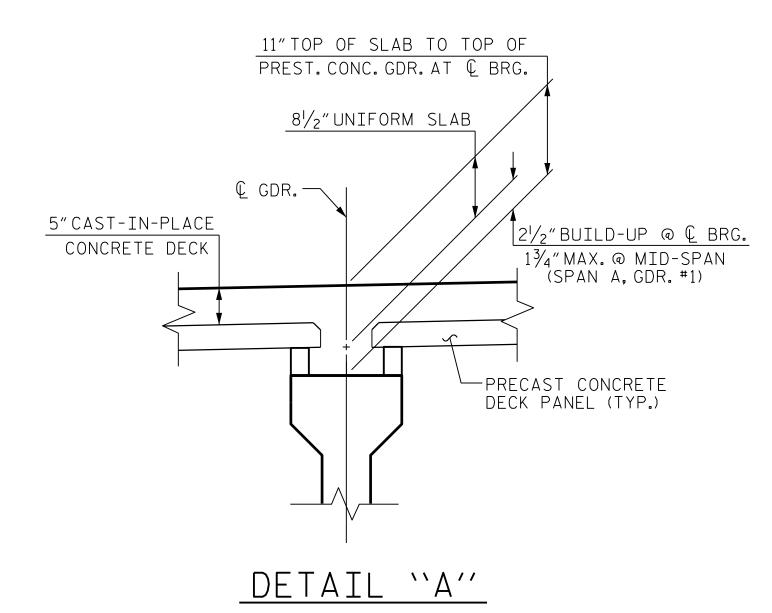
NOTES

LONGITUDINAL STEEL MAY BE SHIFTED SLIGHTLY, AS NECESSARY, TO AVOID INTERFERENCE WITH STIRRUPS IN PRESTRESSED CONCRETE GIRDERS.

PREVIOUSLY CAST CONCRETE IN A CONTINUOUS UNIT SHALL HAVE ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI BEFORE ADDITIONAL CONCRETE IS CAST IN THE UNIT.

BARRIER RAILS IN A CONTINUOUS UNIT SHALL NOT BE CAST UNTIL ALL SLAB CONCRETE IN THE UNIT HAS BEEN CAST AND HAS REACHED A MINIMUM COMPRESSIVE STRENGTH OF 3,000 PSI.

FOR INTERMEDIATE STEEL DIAPHRAGMS DETAILS, SEE "INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE III PRESTRESSED CONCRETE GIRDERS" SHEET.



PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE

TYPICAL SECTION

SHEET NO

S-05

TOTAL SHEETS 26

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<u> VDK \_\_\_ date :07/21</u>

**CDM** 

CHECKED BY : \_

DESIGN ENGINEER : .

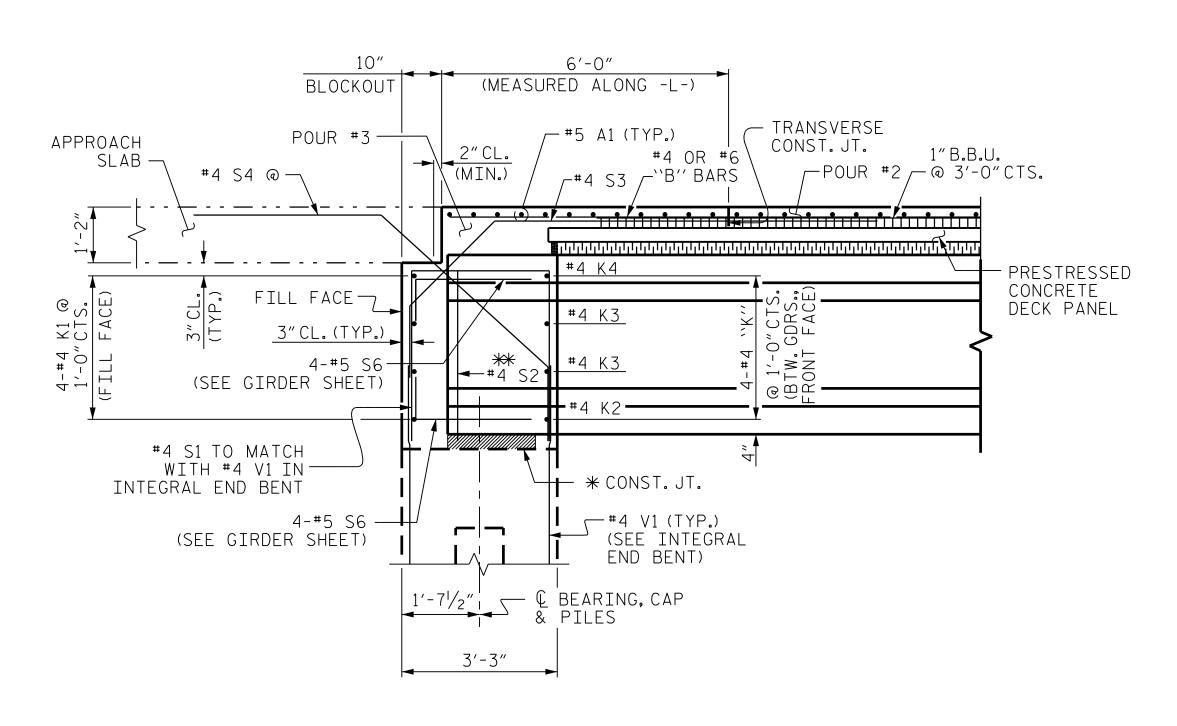
SEAL 16301 1 CONEER DATE : 04/21 DWG. No. THF DATE : 06/21

REVISIONS NO. BY: DATE: BY: DATE: Ting H. Fang 175695769624F9AA469...

HALF TYPICAL SECTION

SHOWING INTERMEDIATE DIAPHRAGMS

TYPICAL SECTION SHOWING LINK SLAB AT BENT



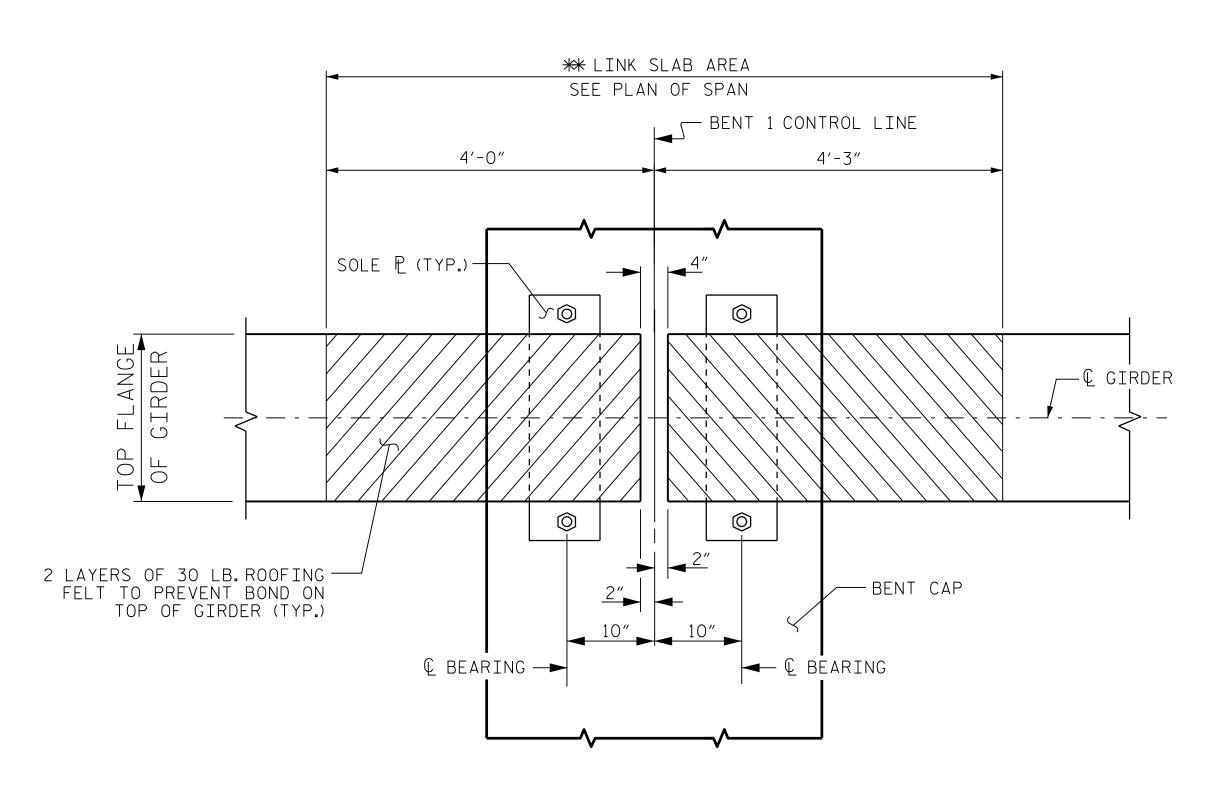
#### CONTROL LINE ( LINK SLAB AREA SEE PLAN OF SPAN PRESTRESSED SEE DETAIL "A" CONCRETE 1<sup>1</sup>/<sub>4</sub>"HIGH B.B.U. ┌─**#**5 B2 #5 A1 (TYP.) — DECK PANEL $\sim$ 0 3'-0" CTS. (TYP.) 2 LAYERS OF 30 LB. ROOFING 1"(TYP.)→ FELT TO PREVENT BOND ON TOP OF GIRDER (TYP.) 10" 10" ┿┸— Ç BRG.

# SECTION THROUGH LINK SLAB

BENT 1

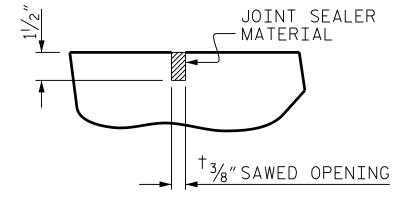
#### SECTION THROUGH INTEGRAL END BENT

- \* THE TOP SURFACE OF THE END BENT CAP EXCLUDING THE BEARING AREA SHALL BE RAKED TO A DEPTH OF 1/4"
- \*\* #4 S2 LOCATED AT OUTSIDE EDGES OF INTEGRAL END BENT DIAPHRAGM. SEE PLAN OF SPANS AND TYPICAL SECTION FOR PLACEMENT DETAILS.



## PLAN AT BENT

\*\* THE TOP OF GIRDER IN THE AREA OF THE LINK SLAB SHALL BE SMOOTH AND FREE OF STIRRUPS OR ANCHOR STUDS.



#### DETAIL "A"

A 11/2" DEEP CONTRACTION JOINT AT EACH END BENT SHALL BE SAWED NO MORE THAN 12 HOURS AFTER THE APPROACH SLAB IS CAST. THE JOINT SHALL BE CLEANED OF ALL DEBRIS BEFORE THE SEALANT IS APPLIED. THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF SECTION 1028 OF THE STANDARD SPECIFICATIONS.

PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUPERSTRUCTURE

TYPICAL SECTION

TOTAL SHEETS 26

**CDM** 

DESIGN ENGINEER: \_

CDM SMITH 5400 Glenwood Avenue, Suite 400 Raleigh, NC 27612–3228 NC COA No. F–1255

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

JJR DATE : 04/21
THF DATE : 06/21
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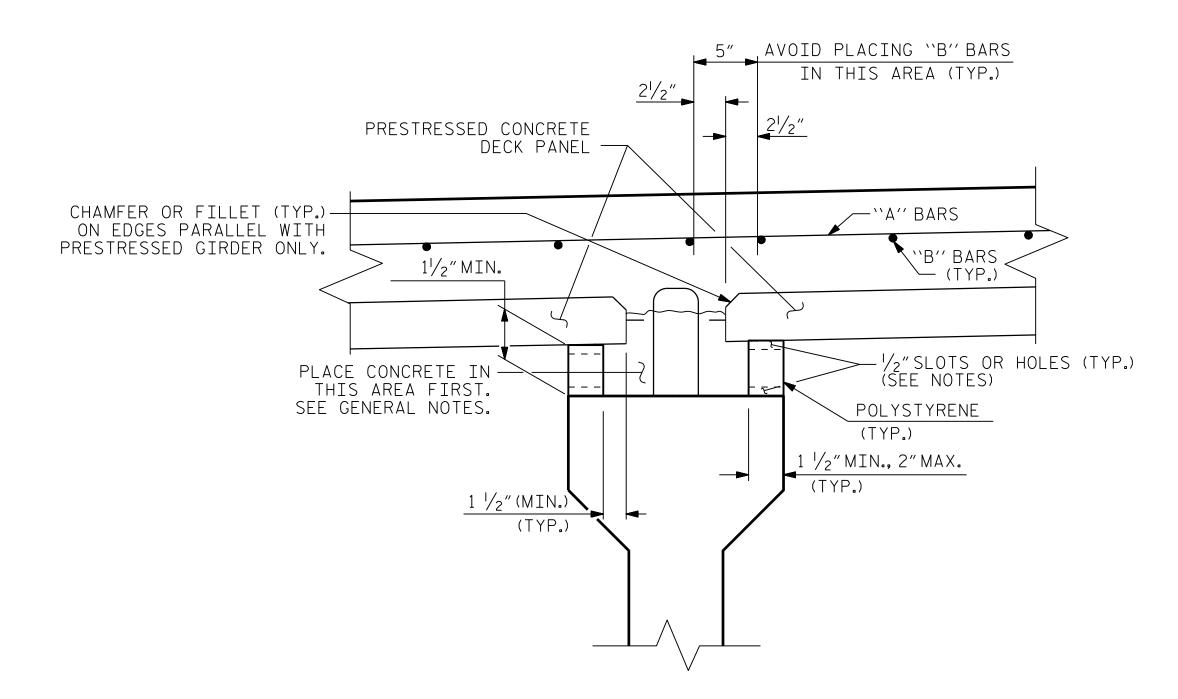
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#### DECK PANEL SUPPORTS

THE CONTRACTOR SHALL PROVIDE THE DECK PANEL SUPPORT SYSTEM SHOWN OR HE MAY SUBMIT A DECK PANEL SUPPORT SYSTEM OF HIS OWN DESIGN TO THE ENGINEER FOR APPROVAL.

#### POLYSTYRENE SUPPORT SYSTEM

- 1. ALL POLYSTYRENE SHALL BE DOW STYROFOAM 60 HIGH-LOAD, UC INDUSTRIES FOAMULAR 600 OR APPROVED EQUAL.
- 2. THE POLYSTYRENE SUPPORT SYSTEM SHALL CONSIST OF ONE LAYER WITH A MINIMUM WIDTH OF  $1\frac{1}{2}$  AND A MAXIMUM WIDTH OF 2". THE POLYSTYRENE SHALL HAVE  $\frac{1}{2}$ " X  $\frac{1}{2}$ " WIDE SLOTS OR 1/2" DIAMETER HOLES AT 4'-0" CENTERS STAGGERED ALONG THE TOP AND BOTTOM.
- 3. THE POLYSTYRENE MAY BE CUT AND PLACED ON EDGE AS NECESSARY TO MATCH THE REQUIRED BUILDUP PROFILE ALONG THE GIRDER.
- 4. ADHESIVE, AS APPROVED BY THE ENGINEER, SHALL BE APPLIED TO THE TOP OF THE GIRDER IN A CONTINUOUS BEAD AND IN SUFFICIENT AMOUNT TO PREVENT THE POLYSTYRENE FROM BLOWING OUT AND TO PREVENT GAPS FROM FORMING BETWEEN THE POLYSTYRENE AND THE GIRDER. PRIOR TO PLACEMENT OF THE DECK PANELS, THE ADHESIVE SHALL ALSO BE APPLIED TO THE TOP OF THE POLYSTYRENE.
- 5. CONCRETE-FILLED BUCKETS, STACKS OF DECK PANELS, BUNDLED REINFORCING BARS OR OTHER HEAVY CONCENTRATED LOADS WILL NOT BE PERMITTED ON THE DECK PANEL ONCE THE PANEL HAS BEEN PLACED ON THE POLYSTYRENE SUPPORT SYSTEM.



POLYSTYRENE SUPPORT

#### GENERAL NOTES

- 1. THE DESIGN COMPRESSIVE STRENGTH (f'c) FOR THE CONCRETE IN PRESTRESSED PANELS SHALL BE 5000 PSI MINIMUM AT 28 DAYS. COMPRESSIVE STRENGTH OF CONCRETE AT TIME OF RELEASE OF STRANDS SHALL BE 4000 PSI MINIMUM.
- 2. THE PRECAST PRESTRESSED PANEL SHALL HAVE A THICKNESS OF 3  $\frac{1}{2}$ " WITH THE PRESTRESSED STRANDS LOCATED AT HALF THE DEPTH OF THE PANEL.
- 3. FOR SKEWED SPANS, TRAPEZOIDAL CLOSURE PANELS SHALL HAVE A MINIMUM WIDTH OF 2 FEET ON THE SHORT SIDE.
- 4. ALL PRESTRESSING STRANDS SHALL EXTEND 2" BEYOND THE PANEL EDGES.
- 5. SHEAR REINFORCING OF 0.60 SQ. INCHES OF REINFORCING STEEL PER 10 SQ. FEET OF PANEL SURFACE SHALL BE PROVIDED IN THE PANEL TO ENSURE COMPOSITE ACTION BETWEEN PANEL AND THE CAST-IN-PLACE CONCRETE. SHEAR REINFORCEMENT SHALL BE MADE OF WELDED WIRE HAVING A MINIMUM YIELD STRENGTH OF 60 KSI.
- 6. SHEAR REINFORCEMENT AND LIFTING DEVICES SHALL BE CONSTRUCTED AND PLACED SO AS TO AVOID ANY INTERFERENCE WITH REINFORCING STEEL IN THE CAST-IN-PLACE DECK SLAB AND TO ALLOW FOR PROPER CONCRETE CONSOLIDATION IN THE DECK PANEL.
- 7. SHIFT LONGITUDINAL ''B'' BARS AS NECESSARY TO OBTAIN A MINIMUM CLEAR DISTANCE OF 2 1/2" TO THE RIGHT OR LEFT OF THE EDGE OF THE DECK PANEL. IF, IN SHIFTING TO OBTAIN THIS CLEARANCE, THE "B" BAR INTERFERES WITH THE STIRRUP IN THE TOP OF THE GIRDER THE "B" BAR MAY BE ELIMINATED.
- 8. WHEN CASTING THE DECK, PLACE CONCRETE FIRST OVER THE GIRDERS IN CONTINUOUS STRIPS A MINIMUM OF THREE PANEL LENGTHS AHEAD OF THE REST OF THE CONCRETE. CAREFULLY VIBRATE THE CONCRETE OVER THE GIRDERS SO THAT CONCRETE COMPLETELY FILLS THE AREA UNDER THE DECK PANEL OVERHANGS. THEN PLACE AND VIBRATE THE REMAINING DECK CONCRETE.
- 9. PRECAST PANELS SHALL BE DESIGNED FOR AN ALLOWABLE TENSILE STRESS OF O PSI IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.
- 10. PRECAST DECK PANELS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.
- 11. ALL BAR SUPPORTS AND INCIDENTAL REINFORCING STEEL USED IN THE PRECAST DECK PANELS SHALL BE EPOXY COATED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

PRECAST PRESTRESSED CONCRETE DECK PANELS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED SEAL 16301 NOINEER ting H. Fang

1 57 686 7E9C4F9AA469...

CHECKED BY : \_

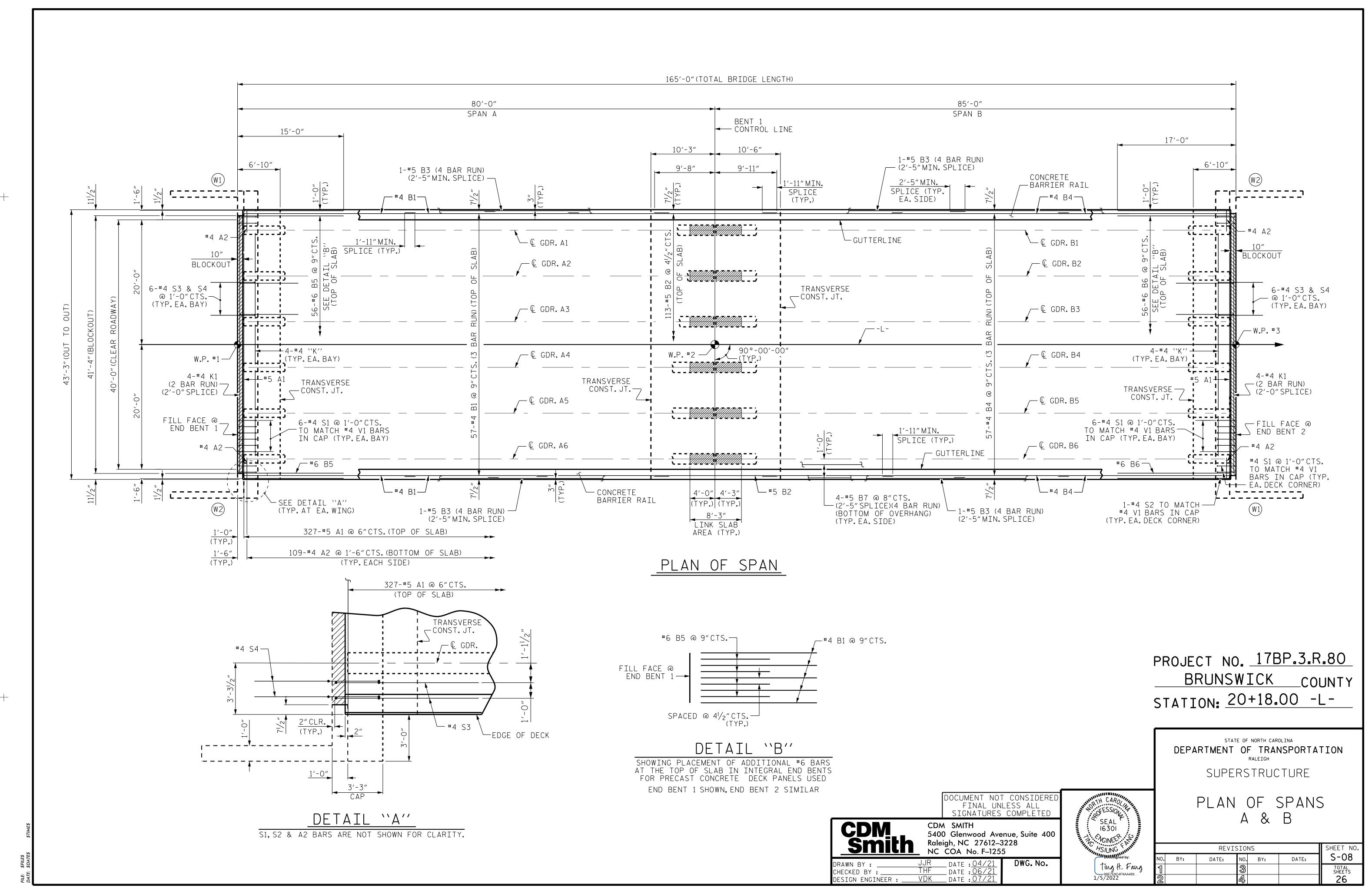
DESIGN ENGINEER:

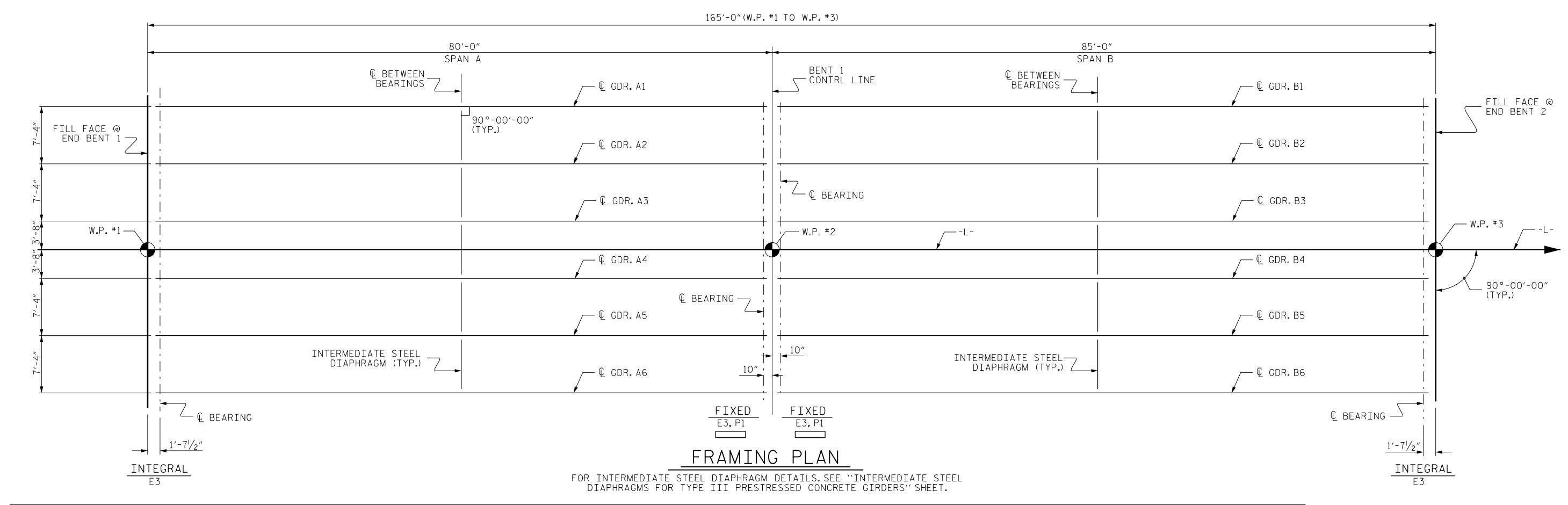
CDM SMITH 5400 Glenwood Avenue, Suite 400 Raleigh, NC 27612–3228 NC COA No. F-1255

JJR DATE : 04/21 THF DATE : 06/21 DWG. No. <u>VDK</u> DATE : <u>07/2</u>

SHEET NO REVISIONS S-07 NO. BY: DATE: BY: DATE: TOTAL SHEETS

STD. NO. PDP1





DEAD LOAD DEFLECTION TABLE																					
	SPAN A																				
											GIRDEF	RS 1 &	6								
	BRG.	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	BRG.
CAMBER (GIRDER ALONE IN PLACE)	0	0.030	0.059	0.086	0.111	0.134	0.152	0.167	0.178	0.185	0.187	0.185	0.178	0.167	0.152	0.134	0.111	0.086	0.059	0.030	0
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0	0.016	0.031	0.047	0.062	0.075	0.086	0.095	0.101	0.105	0.106	0.105	0.101	0.095	0.086	0.075	0.062	0.047	0.031	0.016	0
FINAL CAMBER	0	3/16"	5/16"	7/16"	9/16"	11/16"	13/16"	7/8″	15/16"	15/16"	1"	15/16"	15/16"	7/8"	13/16"	11/16"	9/16"	7/16"	5/16"	3/16"	0
GIRDERS 2 THRU 5																					
	BRG.	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	BRG.
CAMBER (GIRDER ALONE IN PLACE)	0	0.030	0.059	0.086	0.111	0.134	0.152	0.167	0.178	0.185	0.187	0.185	0.178	0.167	0.152	0.134	0.111	0.086	0.059	0.030	0
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0	0.017	0.035	0.052	0.068	0.082	0.095	0.105	0.112	0.117	0.118	0.117	0.112	0.105	0.095	0.082	0.068	0.052	0.035	0.017	0
FINAL CAMBER	0	1/8"	5/ <sub>16</sub> "	7/16"	1/2"	5/8"	11/16"	3/4"	13/16"	13/16"	13/16"	13/16"	13/16"	3/4"	11/16"	5/8"	1/2"	7/16"	5/16"	1/8"	0
											SPA	AN B									
											GIRDEF	RS 1 &	6								
	BRG.	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	BRG.
CAMBER (GIRDER ALONE IN PLACE)	0	0.032	0.063	0.092	0.119	0.142	0.162	0.178	0.190	0.197	0.200	0.197	0.190	0.178	0.162	0.142	0.119	0.092	0.063	0.032	0
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0	0.021	0.041	0.061	0.081	0.097	0.112	0.123	0.132	0.137	0.139	0.137	0.132	0.123	0.112	0.097	0.081	0.061	0.041	0.021	0
FINAL CAMBER	0	1/8"	1/4"	3/8"	7/16"	9/16"	5/8″	11/16"	11/16"	3/4"	3/4"	3/4"	11/16"	11/16"	5/8″	9/16"	7/16"	3/8"	1/4"	1/8"	0
										G I	IRDERS	2 THRU	5								
	BRG.	0.05	0.10	0.15	0.20	0.25	0.30	0.35	0.40	0.45	0.50	0.55	0.60	0.65	0.70	0.75	0.80	0.85	0.90	0.95	BRG.
CAMBER (GIRDER ALONE IN PLACE)	0	0.032	0.063	0.092	0.119	0.142	0.162	0.178	0.190	0.197	0.200	0.197	0.190	0.178	0.162	0.142	0.119	0.092	0.063	0.032	0
* DEFLECTION DUE TO SUPERIMPOSED D.L. ↓	0	0.023	0.045	0.067	0.088	0.107	0.123	0.135	0.145	0.151	0.152	0.151	0.145	0.135	0.123	0.107	0.088	0.067	0.045	0.023	0
FINAL CAMBER	0	1/8"	3/16"	5/16"	3/8"	7/16"	7/16"	1/2"	9/16"	9/16"	9/16"	9/16"	9/16"	1/2"	7/16"	7/16"	3/8"	5/16"	3/16"	1/8"	0

\* INCLUDES FUTURE WEARING SURFACE

ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM) EXCEPT "FINAL CAMBER," WHICH IS GIVEN IN INCHES (FRACTION FORM).

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CDM SMITH 5400 Glenwood Avenue, Suite 400 Raleigh, NC 27612–3228 NC COA No. F–1255

JJR DATE : 04/21
THF DATE : 06/21
VDK DATE : 07/21 DWG. No. CHECKED BY : \_\_\_ DESIGN ENGINEER : \_

SEAL 16301 Ting H. Fang 1/5/985/2904F9AA469...

STATION: 20+18.00 -L-STATE OF NORTH CAROLINA

PROJECT NO. <u>17BP.3.R.80</u>

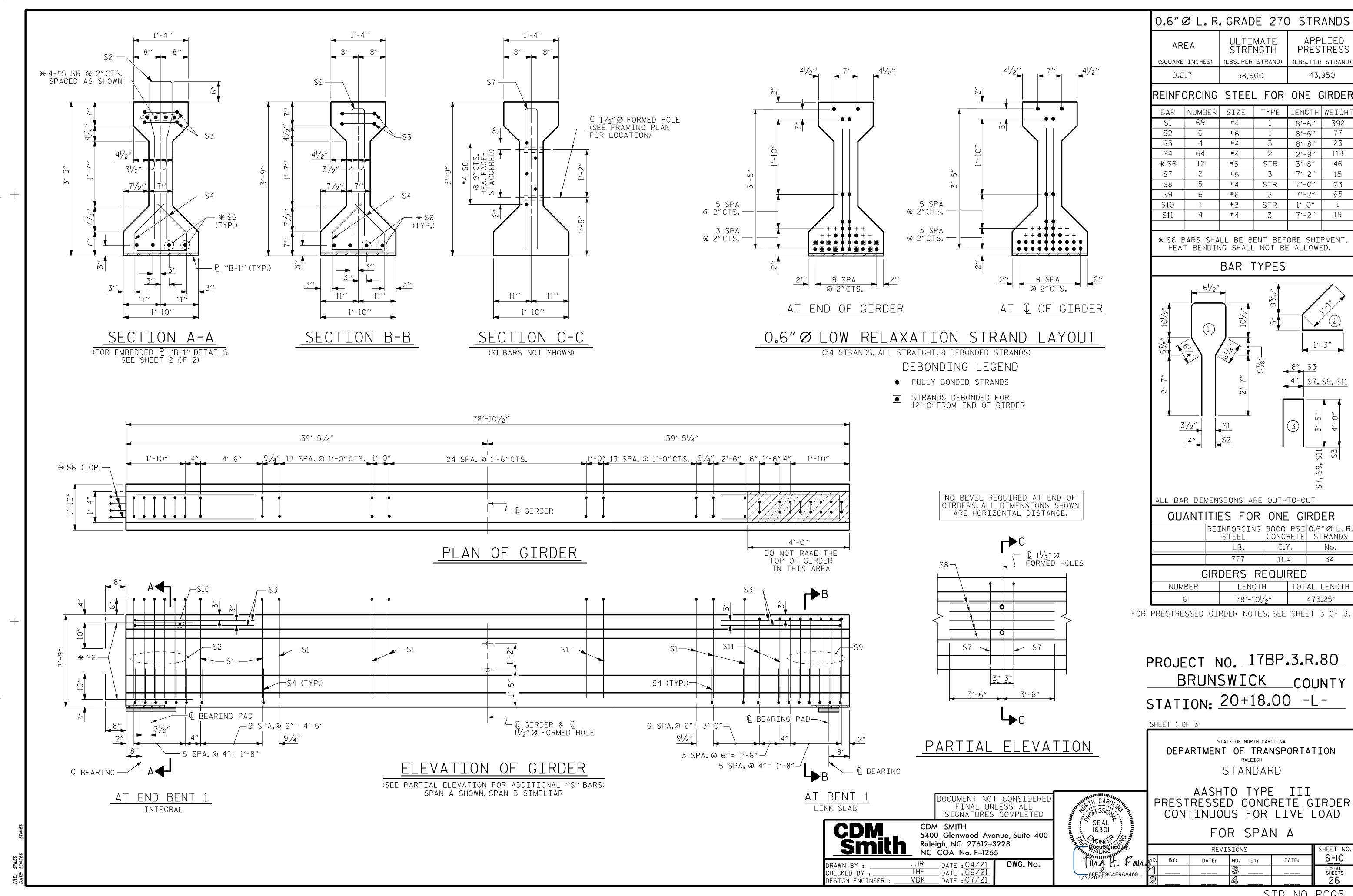
BRUNSWICK COUNTY

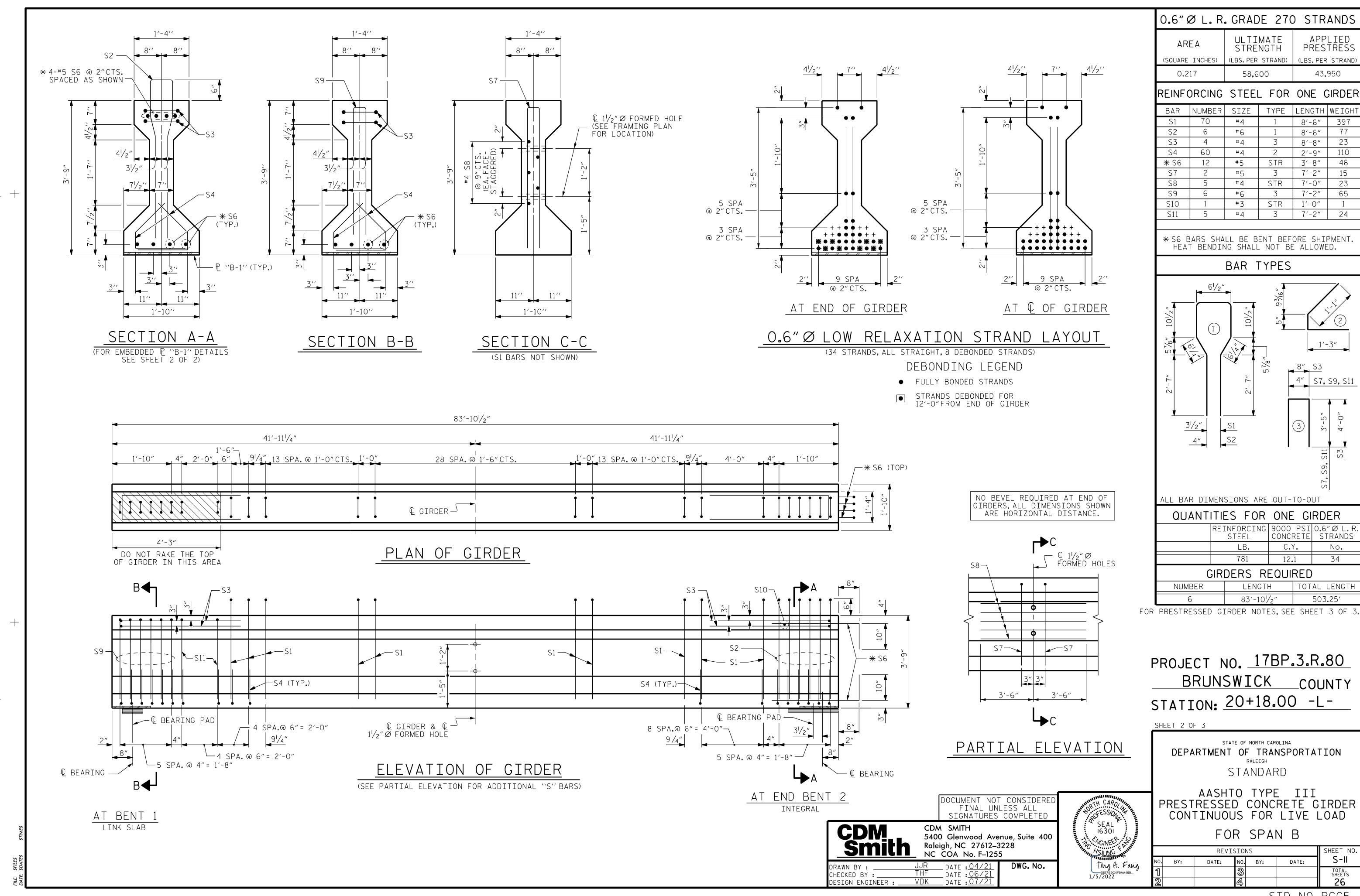
DEPARTMENT OF TRANSPORTATION
RALEIGH

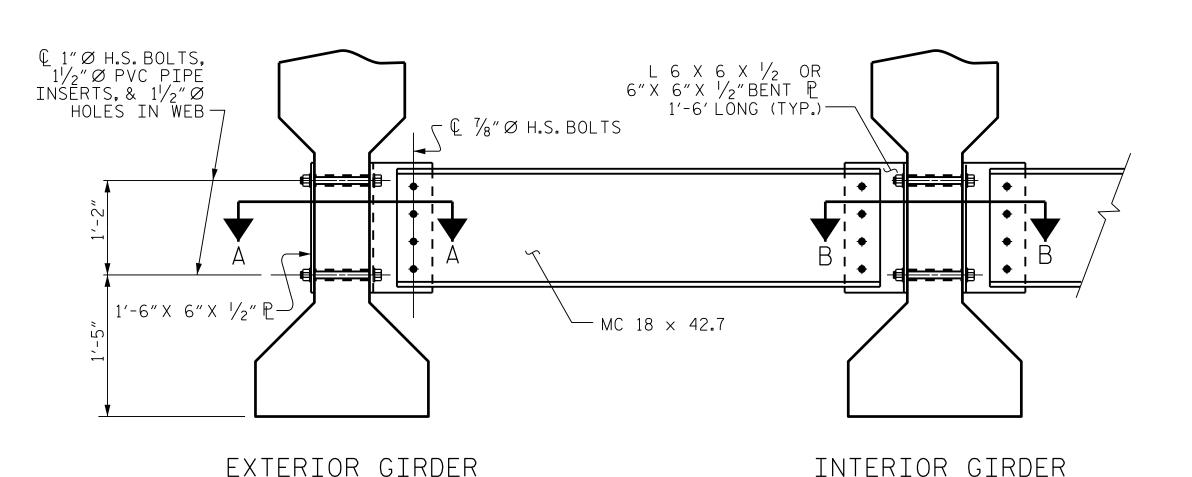
SUPERSTRUCTURE

FRAMING PLAN AND DEAD LOAD DEFLECTIONS

SHEET NO. REVISIONS S-09 NO. BY: DATE: DATE: TOTAL SHEETS 26







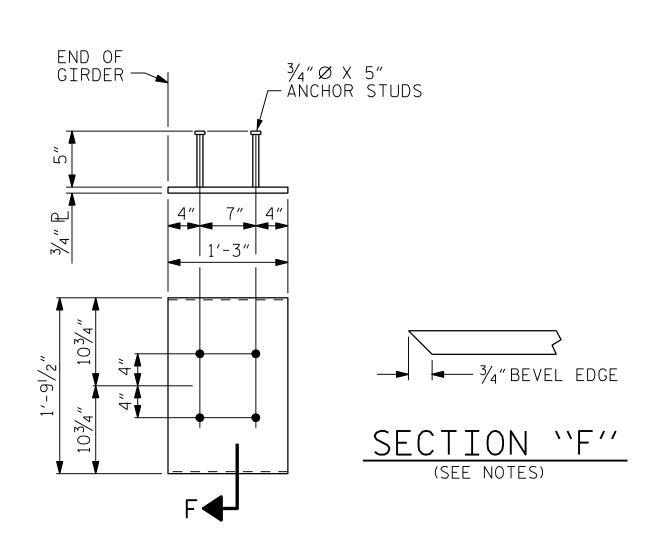
# PART SECTION AT INTERMEDIATE DIAPHRAGM

(TYPICAL FOR EACH BAY)

L 6" X 6" X 1/2" OR BENT 6" X 6" X 1/2" P - 1'-6"LONG (TYP.) -90°-00'-00" 一(TYP。) FOR BOLT CONNECTION, · (£ 1"Ø H.S. BOLT AND — -SEE TYPICAL BOLT WITH 2 HARDENED WASHERS (TYP.) DTI ASSEMBLY DETAIL 1'-6" X 6" X 1/2" ₽ 7/8″∅ H.S. BOLT,— 2 HARDENED WASHERS AND DTI (TYP.) -MC 18 X 42.7 (TYP.) SECTION A-A SECTION B-B

# CONNECTION DETAILS

FOR LOCATION OF INTERMEDIATE DIAPHRAGMS, SEE "FRAMING PLAN" SHEET.



EMBEDDED PLATE "B-1" DETAILS

#### PRESTRESSED CONCRETE GIRDER NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL SHALL BE GRADE 60.

EMBEDDED PLATE "B-1" SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE "B" REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6900 PSI.

THE TOP SURFACE OF THE GIRDER SHALL BE RAKED TO A DEPTH OF  $\frac{1}{4}$ "

PRESTRESSED CONCRETE GIRDERS ARE DESIGNED FOR O PSI TENSION IN THE PRECOMPRESSED TENSILE ZONE UNDER ALL LOADING CONDITIONS.

PRESTRESSED CONCRETE GIRDERS SHALL CONTAIN CALCIUM NITRITE CORROSION INHIBITOR IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

# 21/4", 33/4" $- \bigoplus$ $\tau \oplus \tau$ - (£ <sup>15</sup>/<sub>16</sub>" X 1<sup>1</sup>/<sub>8</sub>" SLOTTED HOLES └Û 1¼6″X 15¼6″ SLOTTED HOLES WEB FACE DIAPHRAGM FACE

# CONNECTOR PLATE DETAILS

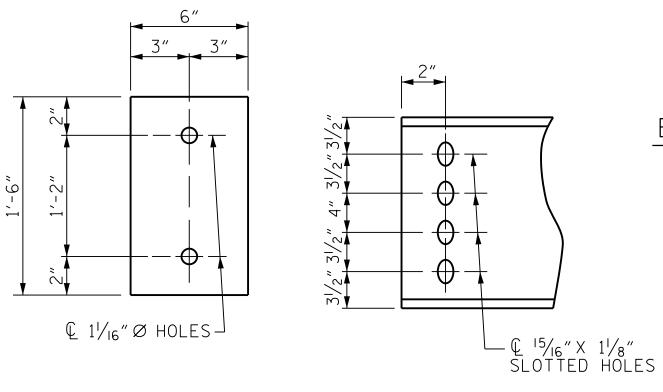


PLATE DETAILS CHANNEL END

#### STRUCTURAL STEEL NOTES

ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL.

TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN

TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL 1/4 TURN.

THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS.

FOR METALLIZATION, APPLY AN 8 MIL THICK 99.50 PERCENT 1350 ALUMINUM (W-AL-1350) THERMAL SPRAY COATING WITH A 0.5 MIL THICK SEAL COAT TO ALL PLATES, BENT PLATES, CHANNELS, ANGLES, AND PLATE WASHERS IN ACCORDANCE WITH THE THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS.

APPLY 1 COAT EACH OF 1080-12 BROWN AND 1080-12 GRAY PAINT ON THE EDGES AND THE WEB FACE OF THE CONNECTOR PLATE WHICH COMES IN CONTACT WITH THE CONCRETE GIRDER IN ACCORDANCE WITH SECTION 442 OF THE STANDARD SPECIFICATIONS.

GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT.

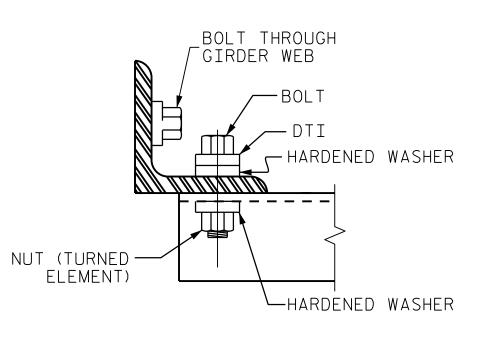
FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST  $\frac{1}{4}$  PROJECTION BEYOND THE NUT.

INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS.

SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION.

IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED.

THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS.



BOLT WITH DTI ASSEMBLY DETAIL

PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

INTERMEDIATE STEEL DIAPHRAGMS E III CONCRETE

Ting H. Fang 1/598605994F9AA469...

W.CW.W.W.W.W.W.W.W.W.W.W.W.W.W.W.W	FOR TYPE PRESTRESSED GIRDE
FIRITI	REVISIONS
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SHEET NO S-I2 TOTAL SHEETS 26

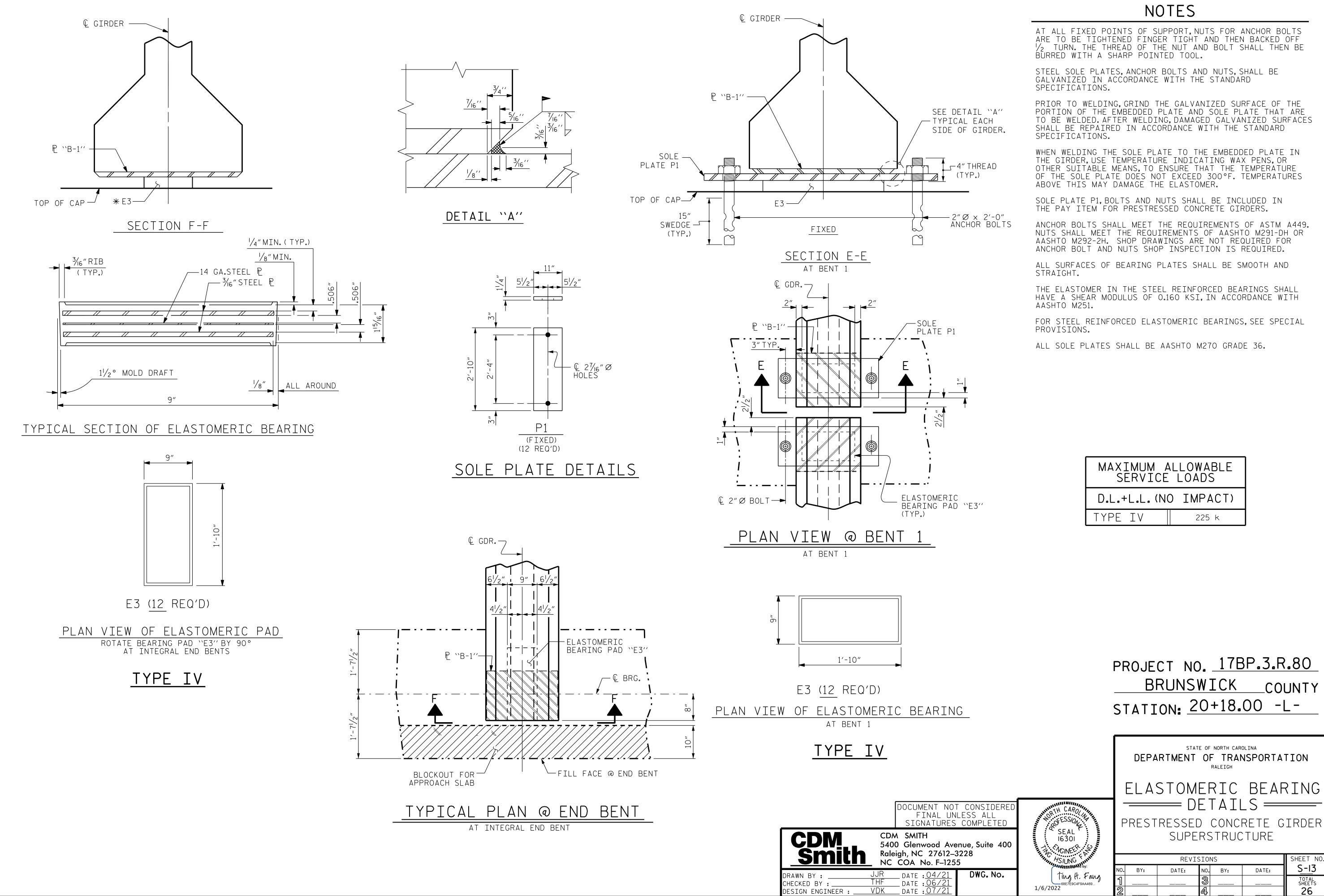
(2 REQ'D PER GIRDER)

**CDM** 

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THF DATE : 06/21 VDK DATE : 07/21 CHECKED BY : DESIGN ENGINEER : .

DWG. No.

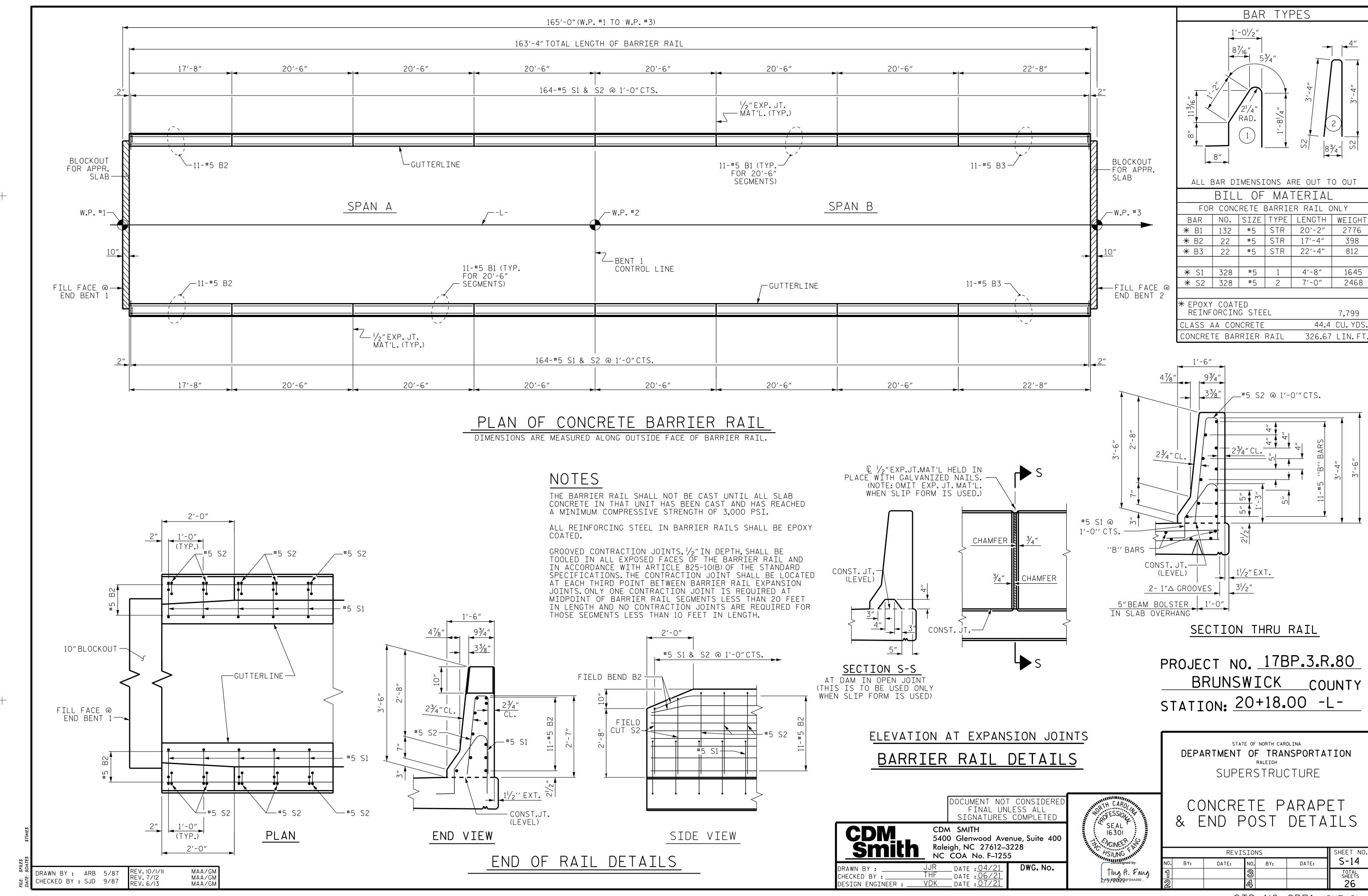


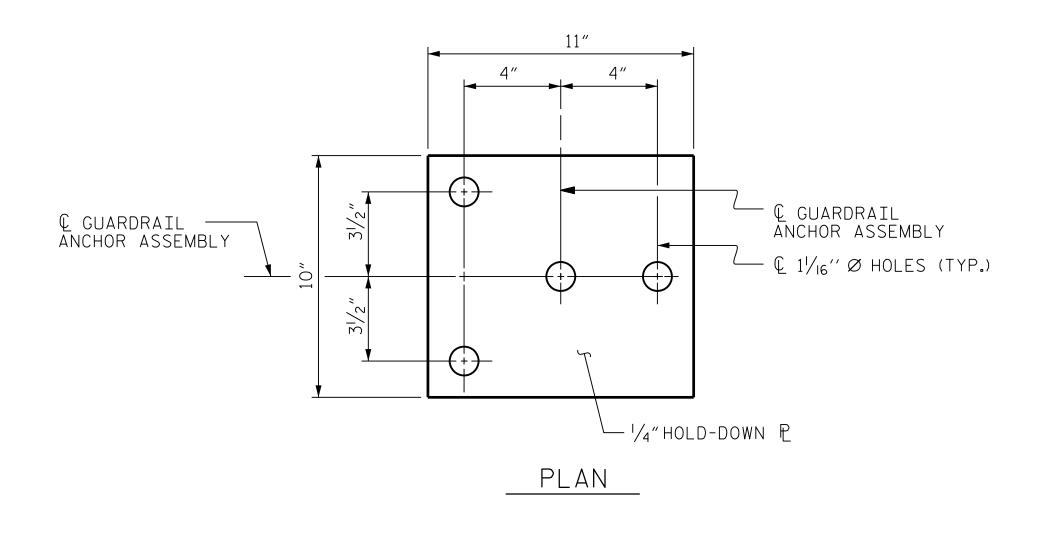
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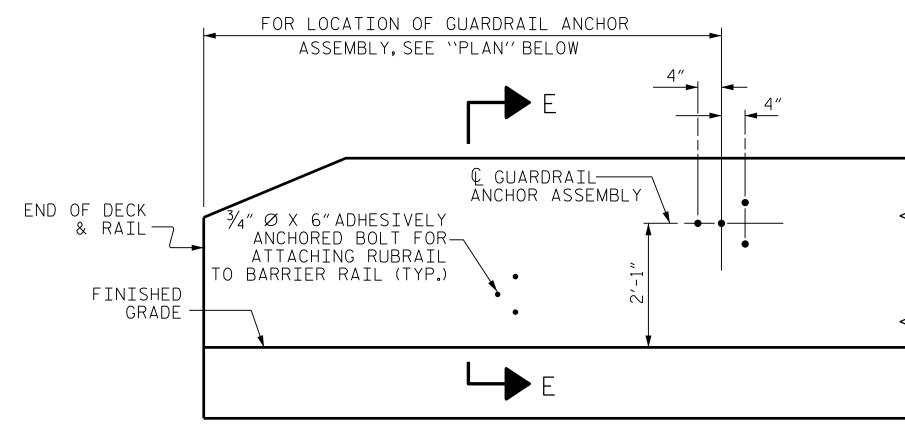
SHEET NO. S-I3

TOTAL SHEETS 26

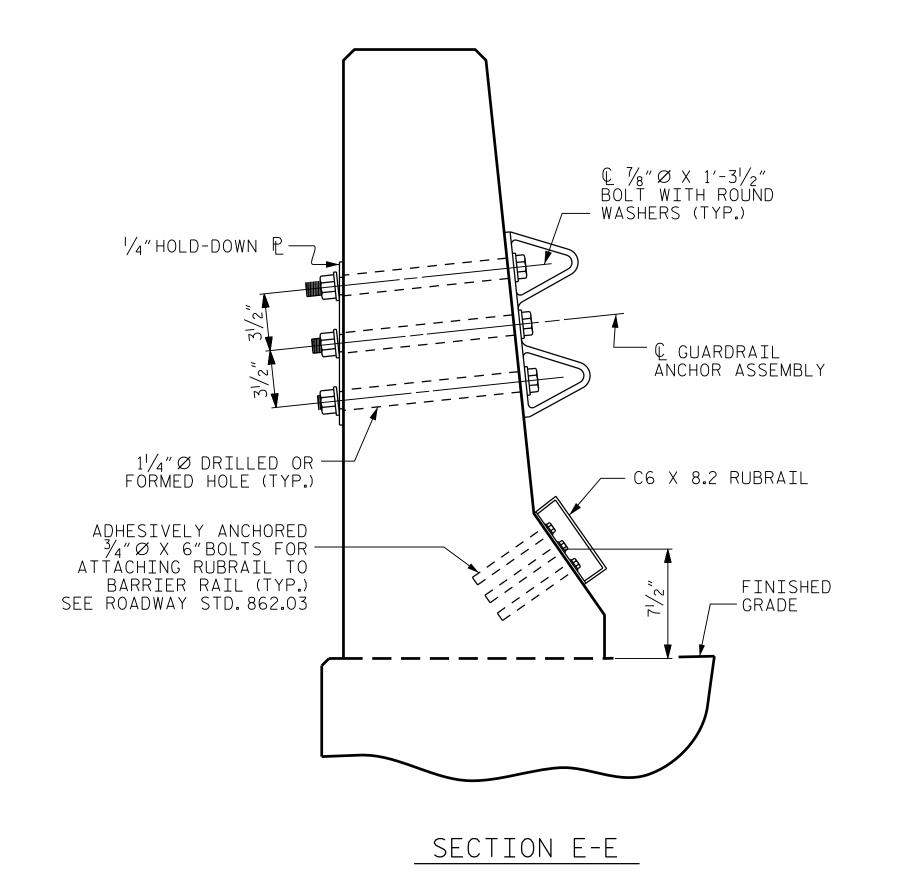
DATE:

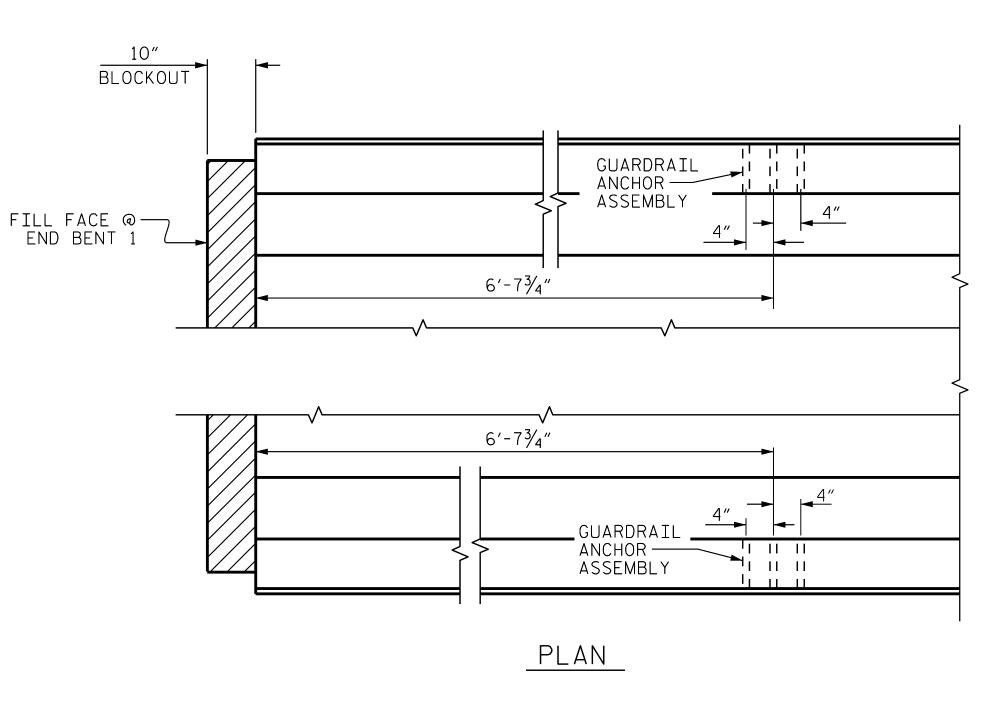






#### ELEVATION





LOCATION OF ANCHORS FOR GUARDRAIL

END BENT 1 SHOWN, END BENT 2 SIMILAR.

GUARDRAIL ANCHOR ASSEMBLY DETAILS

#### NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A 1/4" HOLD-DOWN PLATE AND 4 - 1/8" Ø BOLTS WITH NUTS AND WASHERS, RUBRAIL, AND ADHESIVELY ANCHORED

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 7/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

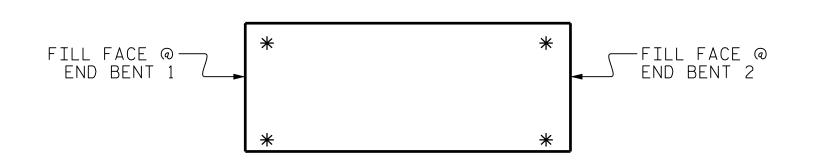
AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR CONCRETE BARRIER RAIL.

THE 1  $\frac{1}{4}$ "  $\varnothing$  HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

THE C6 X 8.2 RUBRAIL IS TO BE ADHESIVELY ANCHORED TO THE RAIL USING THREE  $rac{3}{4}$ "arphi x 6"bolts with washers. Level one field testing is required, and

YIELD LOAD OF THE  $\frac{3}{4}$ "  $\varnothing$  BOLT IS 12 KIPS. FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS. SEE ROADWAY STANDARD 862.03 FOR DETAILS AND LOCATION OF THE RUBRAIL.



SKETCH SHOWING POINTS OF ATTACHMENTS \* DENOTES GUARDRAIL ANCHOR ASSEMBLY

> PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

DETAILS FOR CONCRETE BARRIER RAIL

SEAL 16301 1 NOINEER

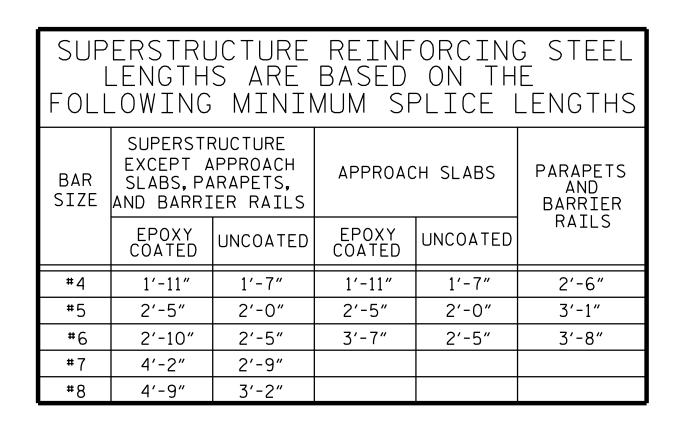
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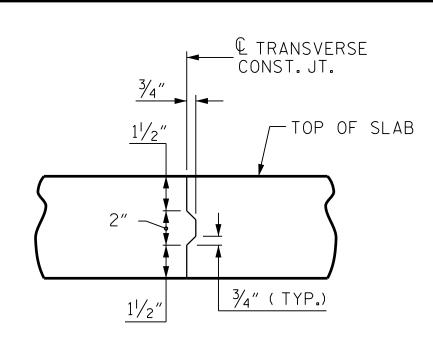
ting H. Fang 1/5/2022

GUARDRAIL ANCHORAGE

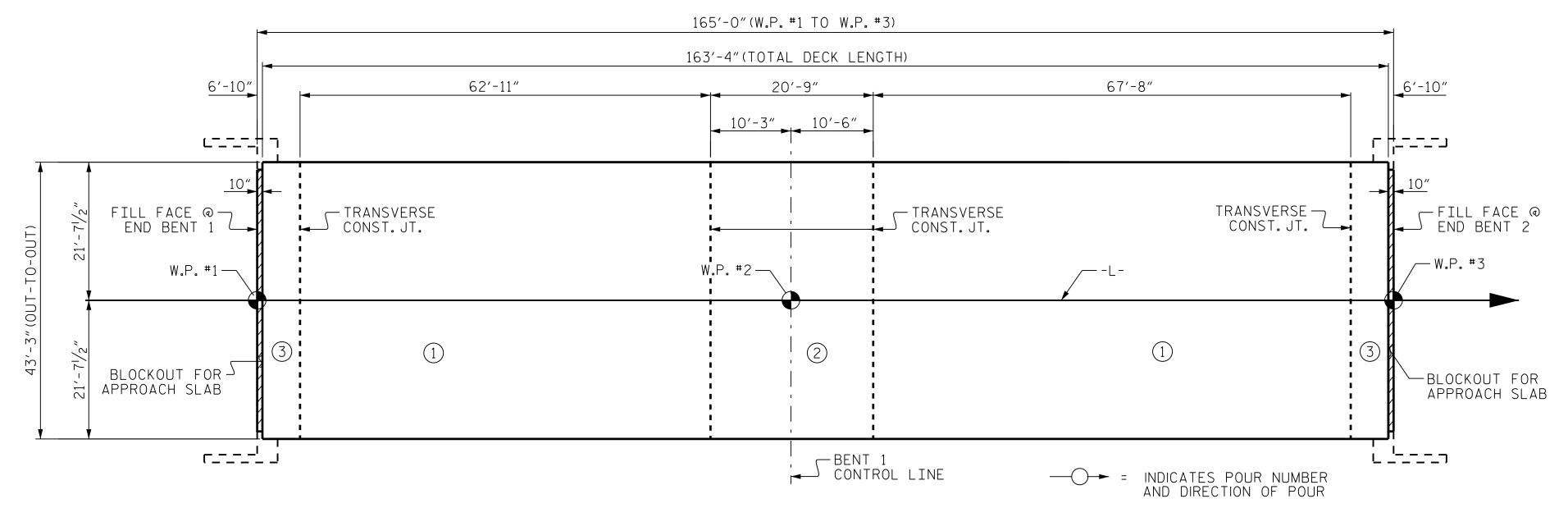
REVISIONS SHEET NO S-I5 NO. BY: DATE: DATE: BY: TOTAL SHEETS 26

CHECKED BY : \_ DESIGN ENGINEER : .



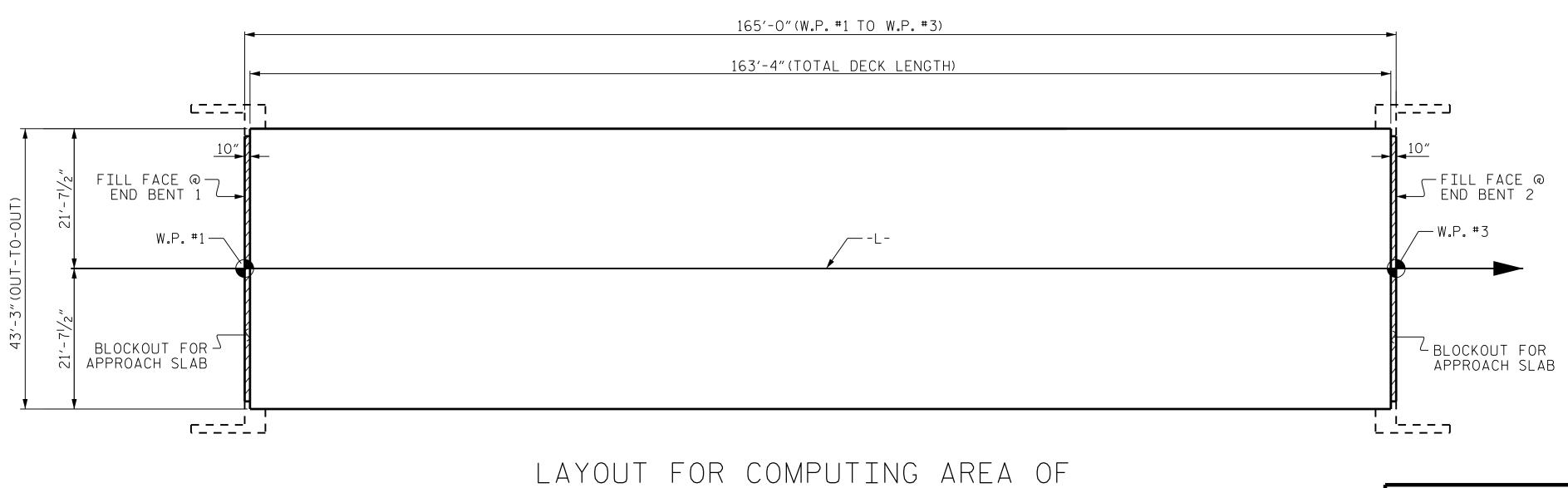


#### TRANSVERSE CONSTRUCTION JOINT DETAIL———

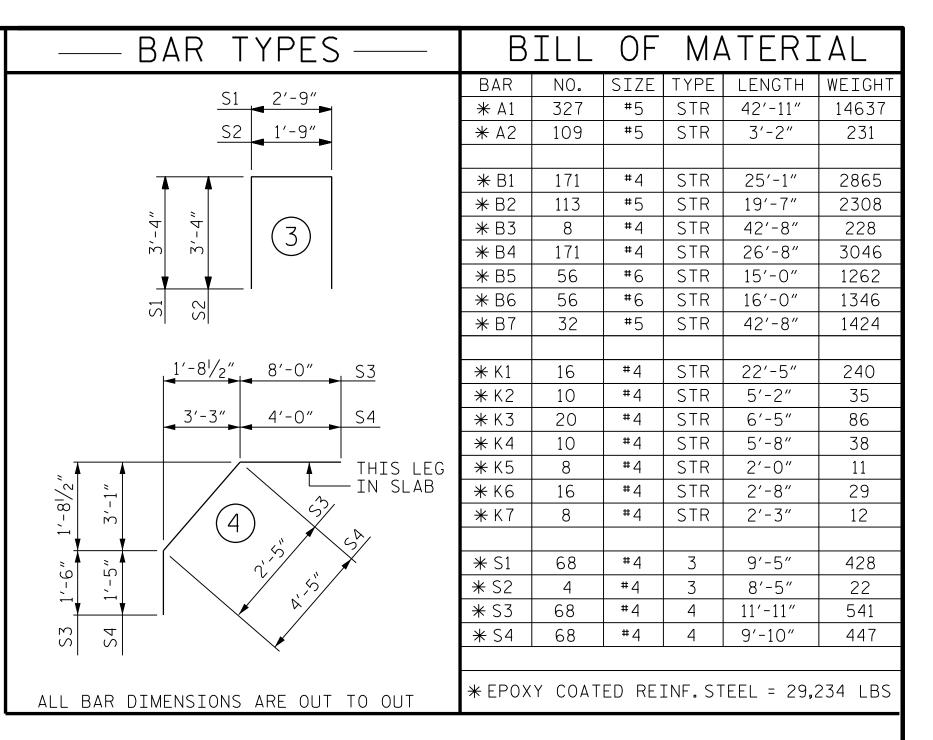


# OPTIONAL POURING SEQUENCE

POUR 2 CANNOT BE STARTED UNTIL BOTH ADJACENT POUR 1 REACH A MINIMUM OF 3000 PSI RESPECTIVELY. SEE TRANSVERSE CONSTRUCTION JOINT DETAIL



(SQ.FT. = 7,064)



SUPERSTRL	ICTURE B	ILL OF M	ATERIAL
	CLASS AA CONCRETE	REINFORCING STEEL	EPOXY COATED REINFORCING STEEL
	(CU.YDS.)	(LBS.)	(LBS.)
POUR 1	107.2		
POUR 2	17.2		
POUR 3	39.7		
TOTAL **	164.1		29,234

\*\* QUANTITY FOR CONCRETE PARAPETS ARE NOT INCLUDED

GROOVING BRID	GE FL	OORS
APPROACH SLABS	1,776	SQ.FT.
BRIDGE DECK	6,031	SQ.FT.
TOTAL	7,807	SQ.FT.

PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

> > SUPERSTRUCTURE

BILL OF MATERIAL & POUR SEQUENCE

SHEET NO.	REVISIONS											
S-I6	DATE:	BY:	NO.	DATE:	BY:							
TOTAL SHEETS			3									
26			4		·							

REINFORCED CONCRETE DECK SLAB ----

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SEAL 16301

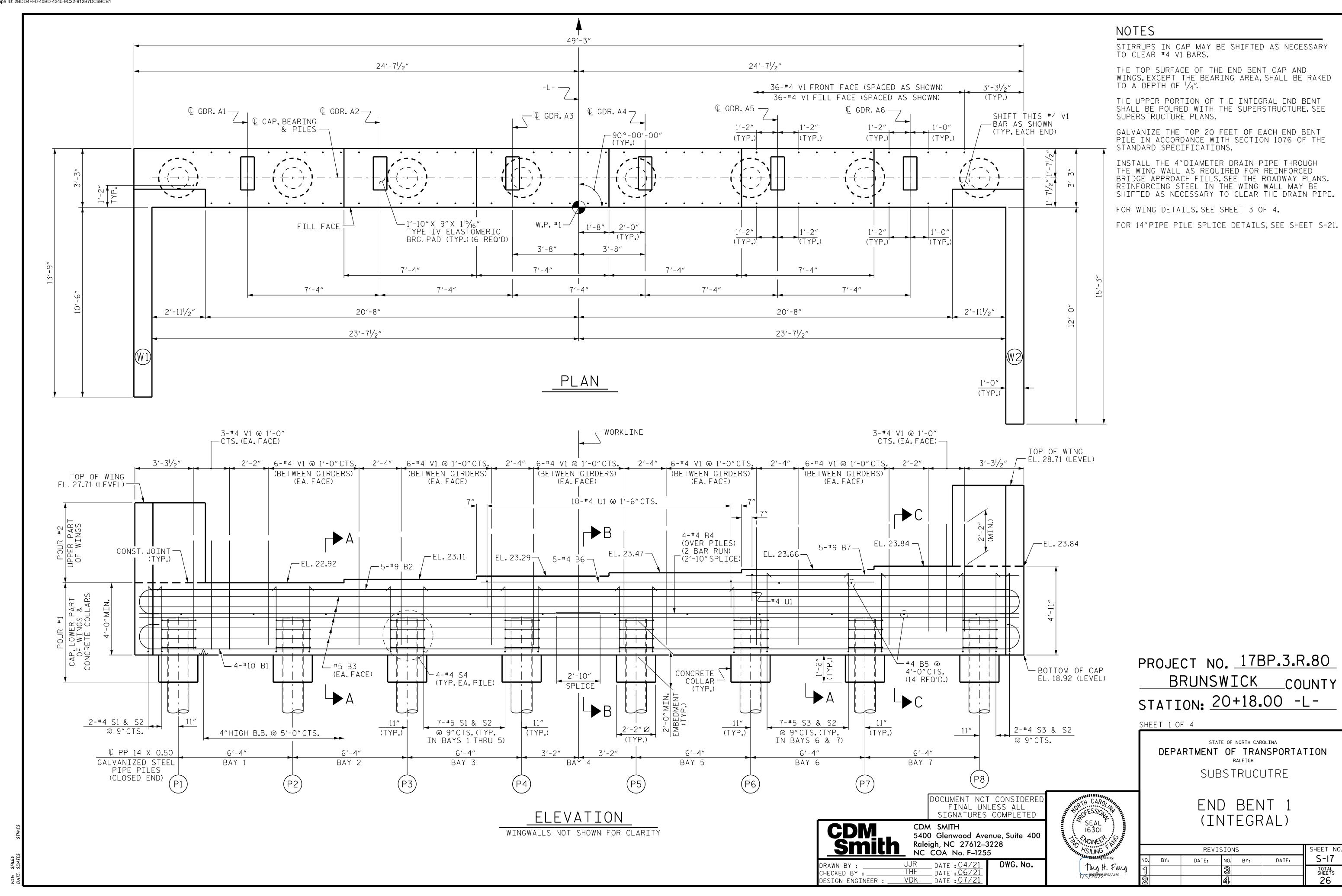
1 NOINEER

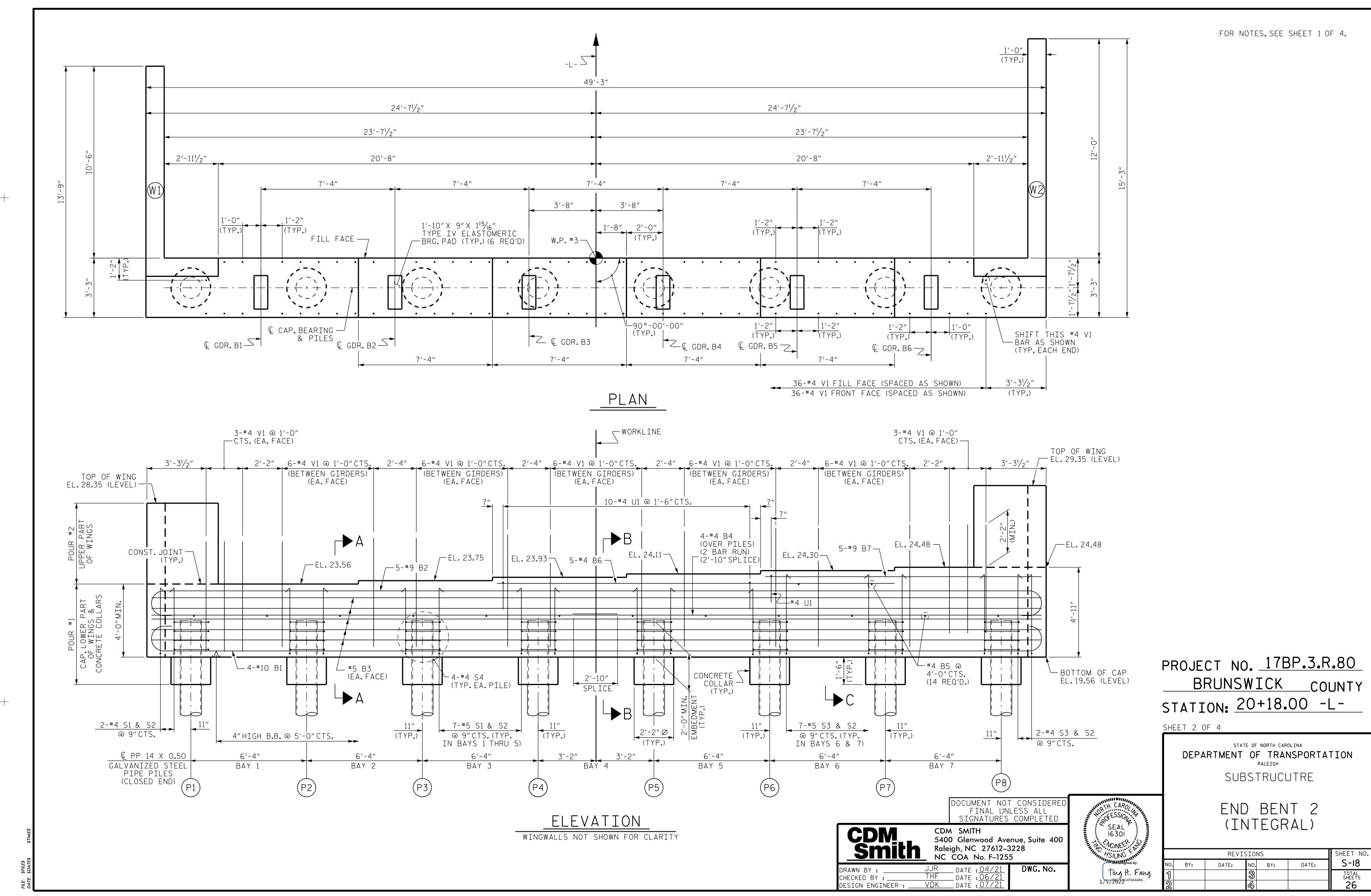
Ting H. Fang 1/5/2022

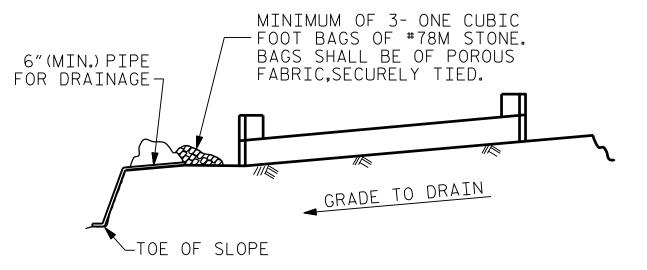
CDM Smith

JJR DATE : 04/21
THF DATE : 06/21
VDK DATE : 07/21 CHECKED BY : \_ DESIGN ENGINEER : .

DWG. No.





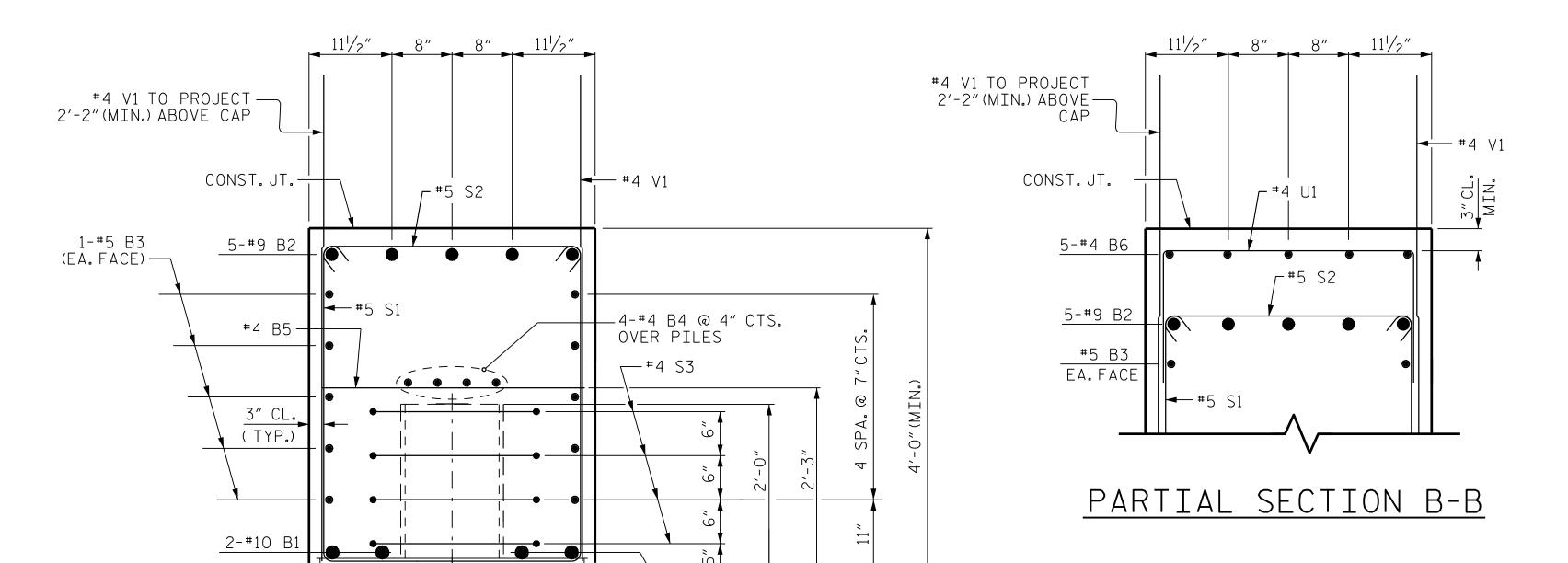


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

### TEMPORARY DRAINAGE AT END BENT



<u>\_\_2-#10 B1</u>

−4" HIGH B.B.

SECTION A-A

2'-2" Ø

3'-3"

1'-71/2"

FILL FACE

© PP 14 X 0.50 GALVANIZED -STEEL PILE

CONCRETE -COLLAR

10"

1'-71/2"

PARTIAL SECTION C-C

r#5 S2

#4 V1

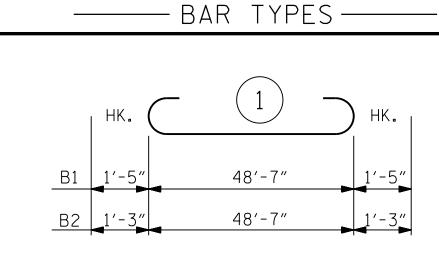
5-#9 B7

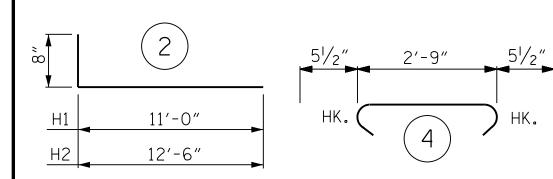
5-#9 B2

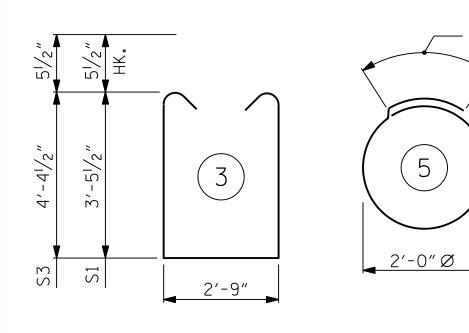
#5 B3 EA.FACE

**−**#5 S3

CONST.JT.







ALL BAR DIMENSIONS ARE OUT TO OUT.

	<b>₩</b> B3	10	#5	STR	48'-9"	508
1	<b></b> ₩ B4	8	#4	STR	25′-10″	138
<u> </u>	<b>∗</b> B5	14	#4	STR	2′-9″	26
	<b>∗</b> B6	5	#4	STR	21′-9″	73
	<b>∗</b> B7	5	#9	STR	15′-1″	256
5 <sup>1</sup> / <sub>2</sub> "	<b>∗</b> H1	38	#5	2	11'-8"	462
<b>→</b>   <b>→</b>   <b>→</b>	<b>∗</b> H2	42	#5	2	13'-2"	577
HK.	<b>∗</b> K1	44	#4	STR	3′-5″	100
	<b>∗</b> S1	37	#5	3	10'-7"	408
	* S2	53	#5	4	3′-8″	203
	<b>*</b> S3	16	#5	3	12′-5″	207
1'-3"LAP	<b>*</b> S4	32	#4	5	7′-7″	162
T 5 LAI						
<b>&gt;</b>	* V1	72	#4	STR	6′-1″	293
$\searrow$	* V2	32	#4	STR	8′-3″	176
`	* V3	34	#4	STR	9'-3"	210
\ \						
)	* EPOX	Y COATED	REINFO	RCING S	TEEL LB	S. 5,554

BILL OF MATERIAL

FOR ONE END BENT

#10

#9

BAR

**∗** B1

NO.

(2 REQUIRED)

SIZE TYPE LENGTH WEIGHT

51′-5″

51′-1″

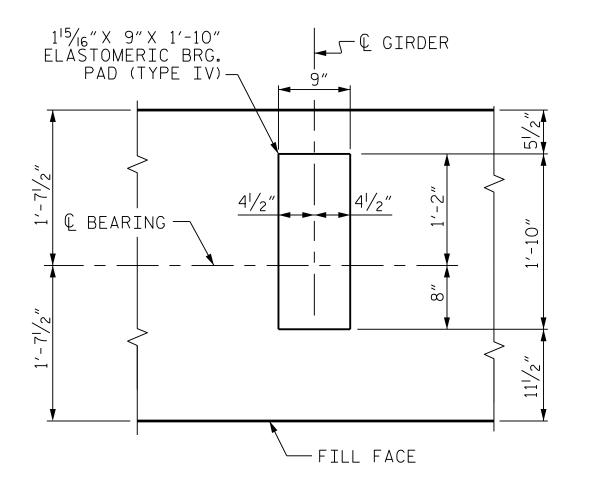
885

868

Ø	POUR #1 - CAP, COLLARS, & LOWER WINGS	C.Y.	30.4
	POUR #2 - UPPER WINGS	C.Y.	5.3
Т.	TOTAL:	C.Y.	35.7
PILE Q	UANTITY		

CLASS A CONCRETE BREAKDOWN:

END BENT :	[		END BENT	2	
PILE DRIVING EQUIPMENT SETUP FOR PP 14X0.5 GALVANIZ STEEL PILES	EA. ED	8	PILE DRIVING EQUIPMENT SETUP FOR PP 14X0.5 GALVANIZED STEEL PILES	EA.	8
PP 14X0.5 GALVANIZED STEEL P	ILES		PP 14X0.5 GALVANIZED STEEL PILE	ES	
NO. = 8	LIN.FT.	360	NO. = 8	LIN.FT.	320
STEEL PILE PLATES	EA.	8	STEEL PILE PLATES	EA.	8
PILE REDRIVES	EA.	4	PILE REDRIVES	EA.	4



DETAIL ''A''

(END BENT 1 SHOWN, END BENT 2 SIMILAR BY ROTATION)

PROJECT NO. 17BP.3.R.80

BRUNSWICK COUNTY

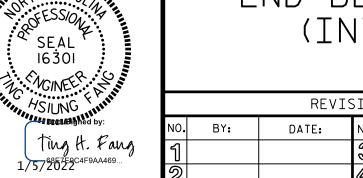
STATION: 20+18.00 -L-

SHEET 4 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENTS 1 & 2 (INTEGRAL)



CDM SMITH
5400 Glenwood Avenue, Suite 400
Raleigh, NC 27612–3228
NC COA No. F–1255

DRAWN BY:

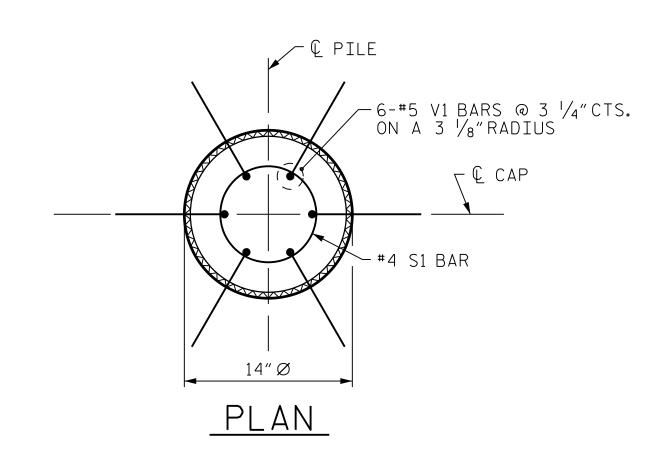
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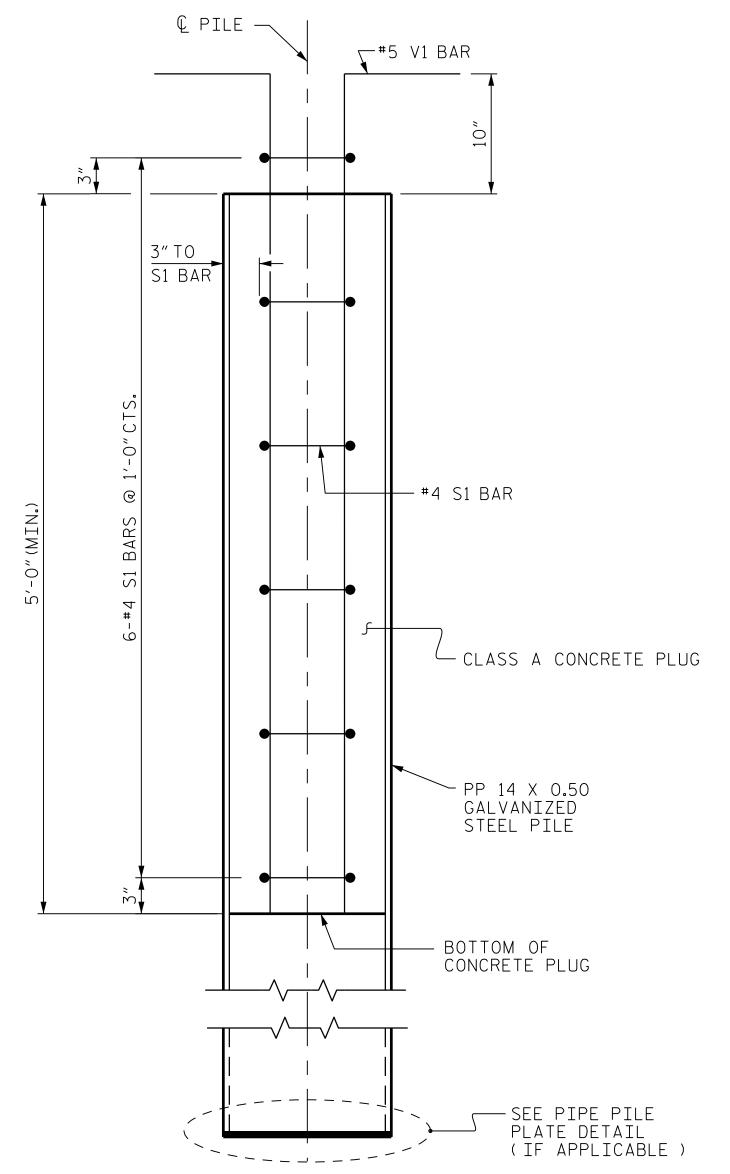
CDM SMITH
5400 Glenwood Avenue, Suite 400
Raleigh, NC 27612–3228
NC COA No. F–1255

:: \$FILE\$ TE: \$DATE\$ \$TIM

| SINIO | NC COA No. F-1255 | NC COA No. F-1255 | NC COA No. F-1255 | NO. BY: | JJR | DATE : 04/21 | CHECKED BY : | THF | DATE : 06/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | VDK | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | VDK | VDK | DATE : 07/21 | DESIGN ENGINEER : | VDK | VDK

#4 V1 TO PROJECT 2'-2"(MIN.) ABOVE





# ELEVATION

PP 14 X 0.50 GALVANIZED STEEL PILE (OPEN OR CLOSED END )

#### NOTES

— 14′′ Ø X ¾′′ ₽

PIPE PILE PLATE DETAIL

(IF APPLICABLE)

PIPE PILE SPLICE DETAIL

€ PILE SPLICE 7

PP 14 X 0.50 — GALVANIZED

STEEL PILE

PIPE PILES SHALL BE IN ACCORDANCE WITH SECTION 1084 OF THE STANDARD SPECIFICATIONS.

GALVANIZE STEEL PIPE PILES IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS UNLESS METALLIZING IS REQUIRED. GALVANIZING OR METALLIZING PIPE PILE PLATES IS NOT REQUIRED.

PIPE PILE PLATES, IF REQUIRED, SHALL BE IN ACCORDANCE WITH SECTION 450 OF THE STANDARD SPECIFICATIONS.

REMOVE AND REPLACE OR REPAIR TO THE SATISFACTION OF THE

ENGINEER PILES THAT ARE DAMAGED, DEFORMED OR COLLAPSED DURING INSTALLATION OR DRIVING.

PILE SPLICES SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS AND AWS D1.1.

FOR CLOSED END PIPE PILES, REMOVE ALL SOIL AND WATER FROM INSIDE THE PILES JUST PRIOR TO PLACING REINFORCING STEEL AND CONCRETE FOR THE CONCRETE PLUG.

FOR OPEN END PIPE PILES. REMOVE ENOUGH SOIL AND WATER FROM INSIDE THE PILES TO CONSTRUCT THE CONCRETE PLUG WITHOUT FOULING THE CONCRETE.

FORM THE CONCRETE PLUG SUCH THAT THE REINFORCING STEEL OR CONCRETE DOES NOT MOVE AND THE CLEARANCE FROM THE REINFORCING STEEL TO THE INSIDE OF THE PILE IS MAINTAINED AFTER CONCRETE PLACEMENT. DO NOT PLACE CONCRETE IN THE BENT CAP UNTIL THE CONCRETE PLUG HAS ATTAINED A MINIMUM COMPRESSIVE STRENGTH OF 1500 PSI.

THE REINFORCING STEEL, CLASS A CONCRETE, AND GALVANIZING ARE CONSIDERED INCIDENTAL TO THE CONTRACT UNIT PRICE BID PER LINEAR FOOT FOR PP 14 X 0.50 GALVANIZED STEEL PILES.

BILL OF MATERIAL FOR ONE PP 14 X 0.50 GALVANIZED STEEL PIL

NO. | SIZE | TYPE LENGTH WEIGHT S1 6 #4 3′-5″ 14 1 #5 42 6′-8″ 6 V1

LBS

0.2 CY

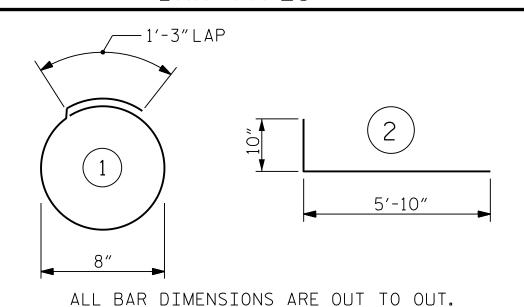
56

CLASS A CONCRETE

5'-0"MINIMUM PLUG

REINFORCING STEEL =

#### BAR TYPES



PROJECT NO. <u>17BP.3.R.80</u> BRUNSWICK \_COUNTY STATION: 20+18.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD

14" STEEL PIPE PILE

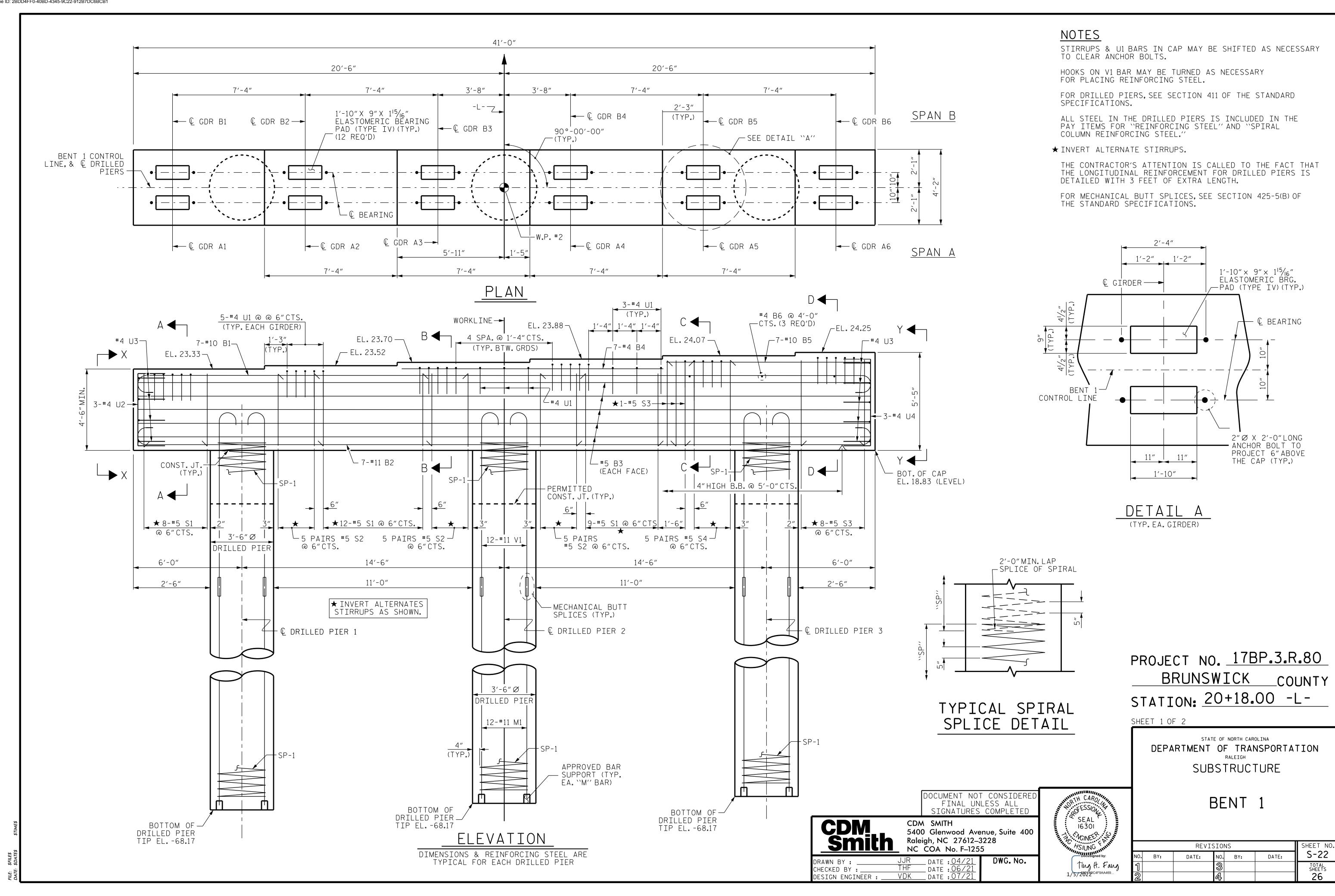
ORTH CAROL OFESSION SEAL 16301 1 NOINEER Ting H. Fang 1/5/2007220C4F9AA469...

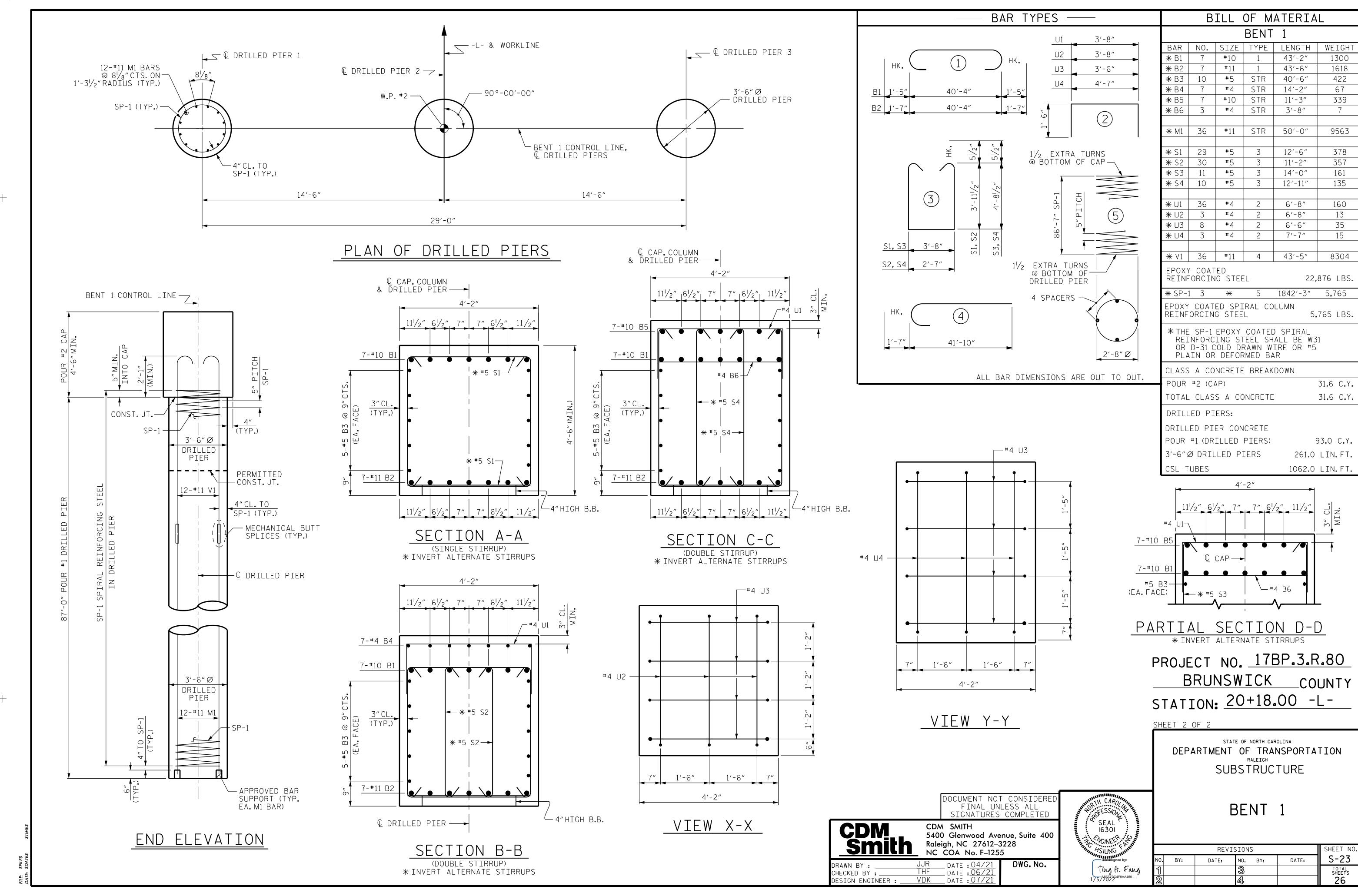
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED **CDM** CDM SMITH 5400 Glenwood Avenue, Suite 400

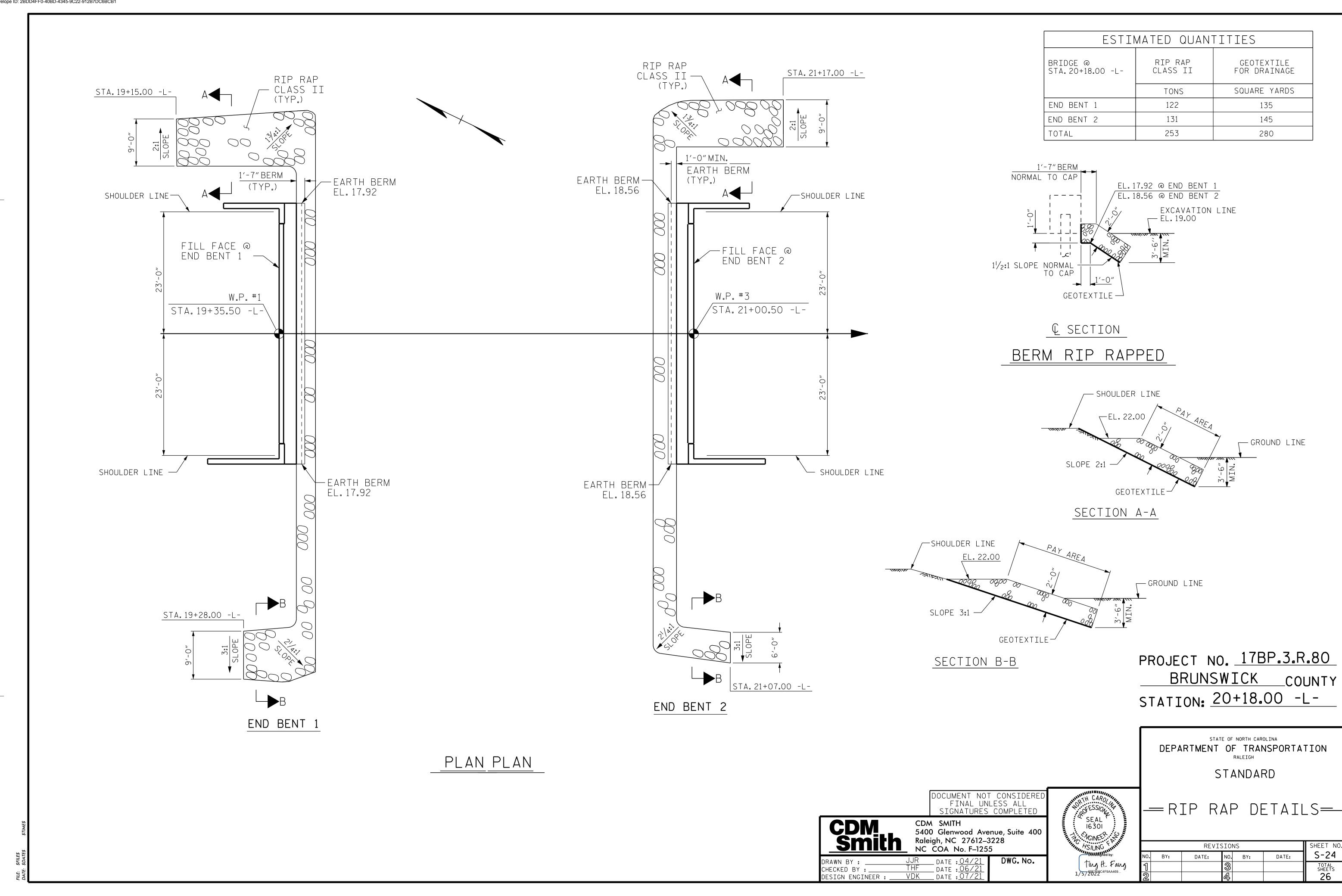
JJR DATE : 04/21
THF DATE : 06/21
VDK DATE : 07/21 CHECKED BY : \_ DESIGN ENGINEER : .

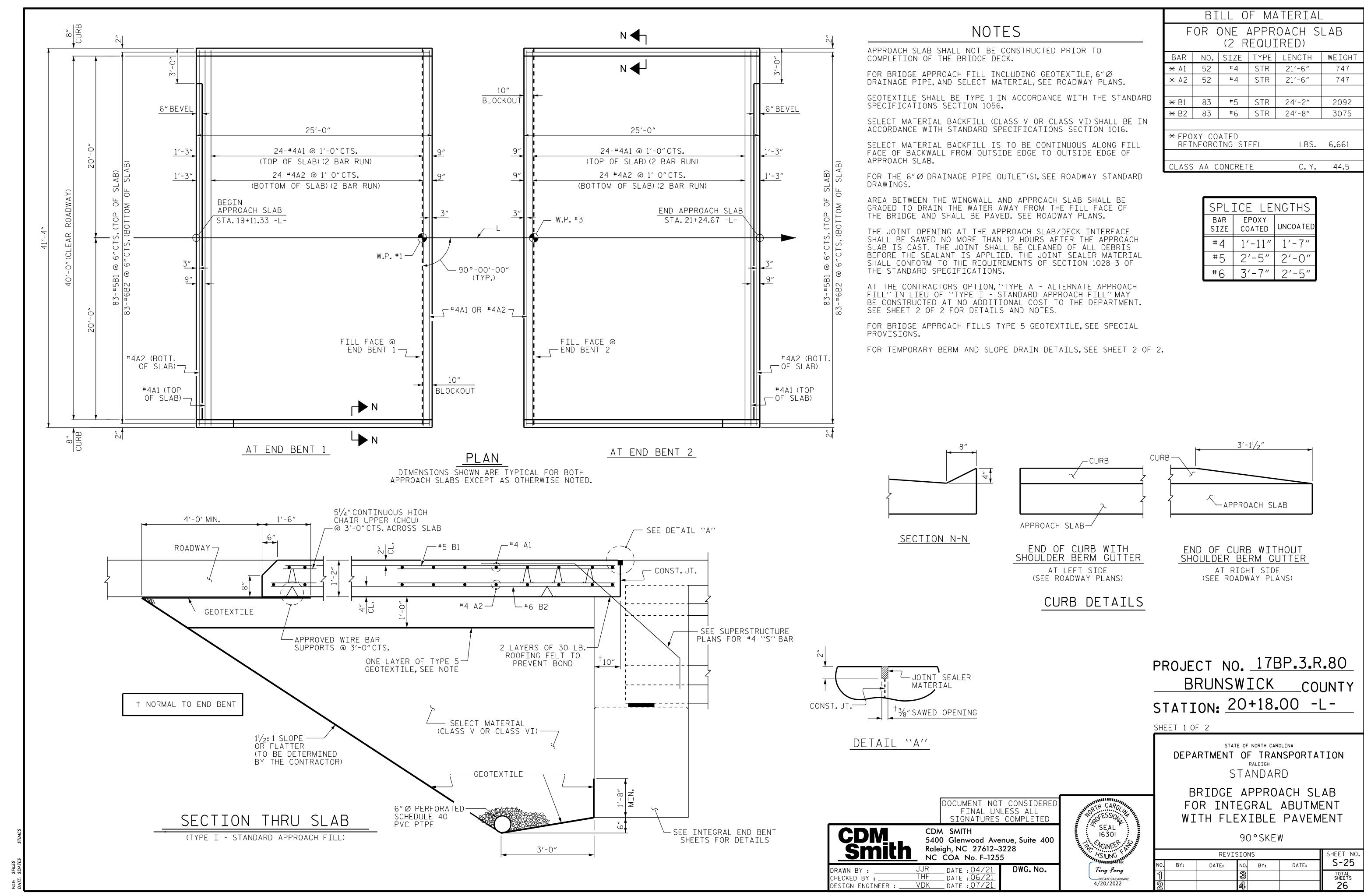
REVISIONS SHEET NO S-21 NO. BY: DATE: DATE: TOTAL SHEETS **26** 

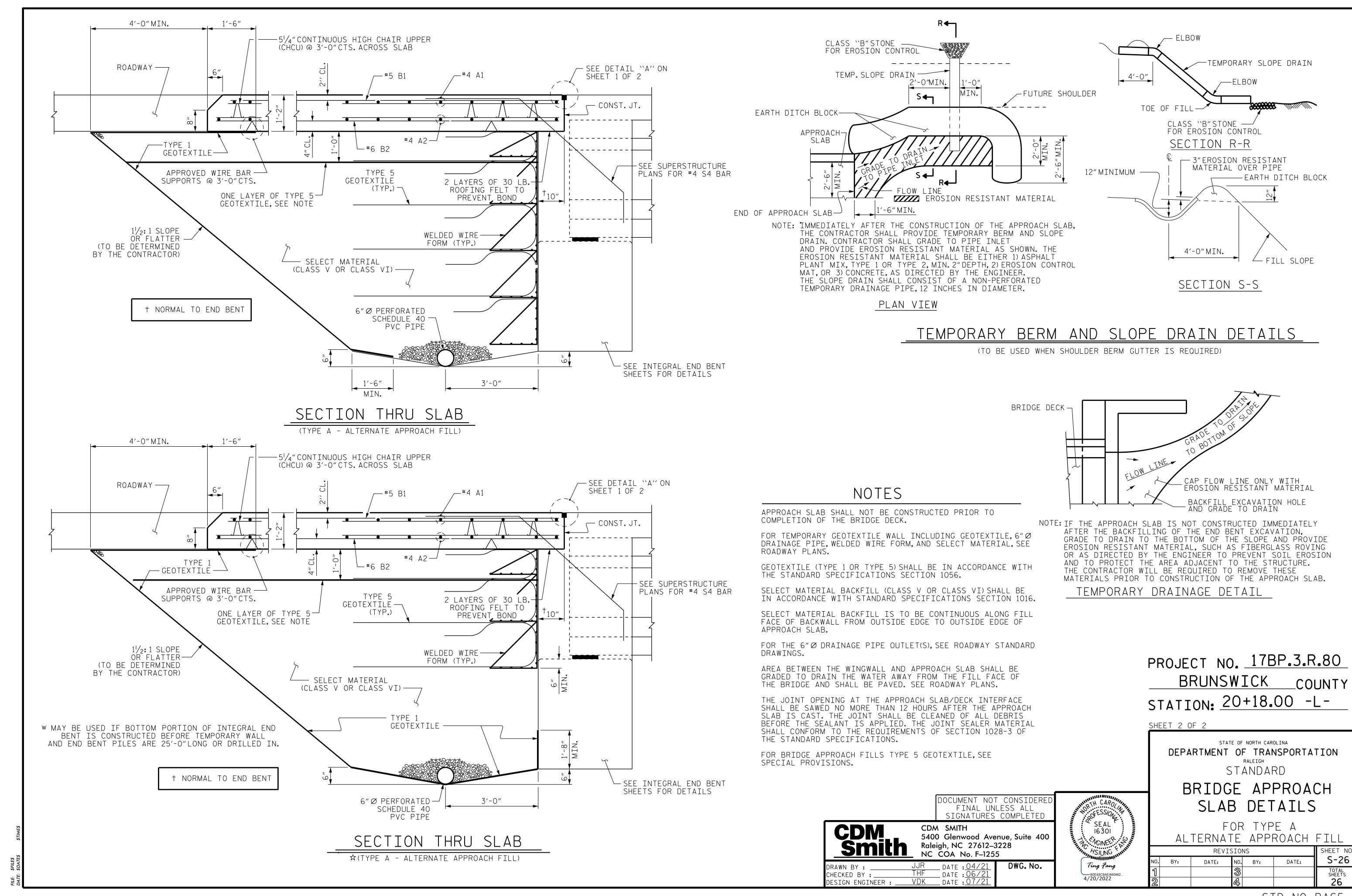
Raleigh, NC 27612–3228 NC COA No. F–1255 DWG. No.











#### STANDARD NOTES

#### DESIGN DATA:

SPECIFICATIONS ---- A.A.S.H.T.O. (CURRENT) LIVE LOAD ---- SEE PLANS IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50W - 27,000 LBS.PER SQ.IN. - AASHTO M270 GRADE 50 - 27,000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN 375 LBS. PER SQ. IN. OF TIMBER ----EQUIVALENT FLUID PRESSURE OF EARTH 30 LBS. PER CU. FT.

#### MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

(MINIMUM)

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

#### CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

#### DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

#### ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

#### REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

#### STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE  $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

#### HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

#### SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS. BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH