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STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

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STRUCTURE SUBSURFACE INVESTIGATION

COUNTY NASH

PROJECT DESCRIPTION BRIDGE NO. 111 ON -L-(SR 1704) OVER SAPONY CREEK AT STA. 15 + 75.5 STATE PROJECT REFERENCE NO. 630111

CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT 1991 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE DESTREY OF MATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MOY LONG ECONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

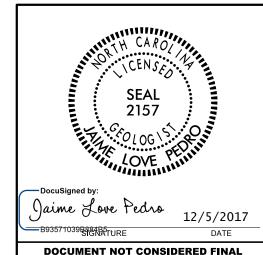
THE BIDDER OR CONTRACTOR IS CAUTIONED THAT DETAILS SHOWN ON THE SUBSURFACE PLANS ARE PRELIMINARY ONLY AND IN MANY CASES THE FINAL DESIGN DETAILS ARE DIFFERENT, FOR BIDDING AND CONSTRUCTION PURPOSES, REFER TO THE CONSTRUCTION PLANS AND DOCUMENTS FOR FINAL DESIGN INFORMATION ON THIS PROJECT. THE DEPARTMENT DOES NOT WARRANT OR GUARANTEE THE SUFFICIENCY OR ACCURACY OF THE INVESTIGATION MADE, NOR THE INTERRETATIONS MADE, OR OPINION OF THE DEPARTMENT AS TO THE TYPE OF MATERIALS AND CONDITIONS TO BE ENCOUNTERED. THE BIDDER OR CONTRACTOR IS CAUTIONED TO MAKE SUCH INDEPENDENT SUBSURFACE INVESTIGATIONS AS HE DEEMS NECESSARY TO SATISFY HIMSELF AS TO CONDITIONS TO BE ENCOUNTERED ON THE PROJECT. THE CONTRACTOR SHALL HAVE NO CLAIM FOR ADDITIONAL COMPENSATION OR FOR AN EXTENSION OF TIME FOR ANY REASON RESULTING FROM THE ACTUAL CONDITIONS ENCOUNTERED AT THE SITE DIFFERING FROM THOSE INDICATED IN THE SUBSURFACE INFORMATION.

- NOTES:

 1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.

 2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL N. T. ROBERSON J. L. PEDRO A. N. KINTNER D. G. PINTER INVESTIGATED BY J. L. PEDRO DRAWN BY _A. N. KINTNER CHECKED BY N. T. ROBERSON SUBMITTED BY N. T. ROBERSON DATE OCTOBER 2017



UNLESS ALL SIGNATURES COMPLETED

PROJECT REFERENCE NO. SHEET NO. 2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

| COLL DECODINATION | | Tooly or only by the same of t | TERMS AND DEFINITIONS |
|--|--|--|---|
| SOIL DESCRIPTION SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN | GRADATION | ROCK DESCRIPTION HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED | TERMS AND DEFINITIONS |
| BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT | <u>WELL GRADED</u> - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE. <u>UNIFORMLY GRADED</u> - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE. | ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. | ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER. |
| ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: | GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES. | SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL, THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN | AQUIFER - A WATER BEARING FORMATION OR STRATA. |
| CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH | ANGULARITY OF GRAINS | REPRESENTED BY A ZONE OF WEATHERED ROCK. ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS: | ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND. |
| AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE, VERY STIFF.GRAY.SILTY CLAY.MOIST WITH INTERBEDDED FINE SAND LAYERS.HIGHLY PLASTIC.A-7-6 | THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: | SU//2SU//A | ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC. |
| SOIL LEGEND AND AASHTO CLASSIFICATION | ANGULAR, <u>SUBANGULAR</u> , <u>SUBROUNDED</u> , OR <u>ROUNDED</u> . | WEATHERED V// NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED. | ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT |
| CENEDAL CRANIII AR MATERIALS SILT-CLAY MATERIALS | MINERALOGICAL COMPOSITION | THE TO COADSE CRAIN IGNERIES AND METAMORPHIC ROCK THAT | WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND |
| CLASS. (\$\leq 35% PASSING *200) (> 35% PASSING *200) ORGANIC MATERIALS | MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. | CRYSTALLINE ROCK (CR) WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GMEISS, GABBRO, SCHIST, ETC. | SURFACE. |
| GROUP A-1 A-3 A-2 A-4 A-5 A-6 A-7 A-1, A-2 A-4, A-5 | ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE. | FINE TO COARSE CRAIN METAMORPHIC AND NON-COASTAL PLAIN | CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE. |
| CLASS. A-1-6 A-1-6 A-2-4 A-2-5 A-2-6 A-2-7 A-7-6 A-3 A-6, A-7 | COMPRESSIBILITY | NON-CRYSTALLINE ROCK (NCR) SEDIMENTARY ROCK THAT WOULD YELLD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC. | COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE. |
| SYMBOL 000000000000000000000000000000000000 | SLIGHTLY COMPRESSIBLE LL < 31 MODERATELY COMPRESSIBLE LL = 31 - 50 | COASTAL PLAIN COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD | CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED |
| 7. PASSING Stu T- | HIGHLY COMPRESSIBLE LL > 50 | SEDIMENTARY ROCK SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC. | BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE. |
| *10 50 MX GRANULAR CLAY MUCK, *40 30 MX 150 MX 51 MN SOILS PEAT | PERCENTAGE OF MATERIAL | WEATHERING | DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT |
| 200 15 MX 25 MX 10 MX 35 MX 35 MX 35 MX 35 MX 36 MN 36 MN 36 MN 36 MN 36 MN | GRANULAR SILT - CLAY ORGANIC MATERIAL SOILS SOILS OTHER MATERIAL | FRESH ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING, ROCK RINGS UNDER | ROCKS OR CUTS MASSIVE ROCK. |
| MATERIAL | TRACE OF ORGANIC MATTER 2 - 3% 3 - 5% TRACE 1 - 10% | HAMMER IF CRYSTALLINE. | DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL. |
| PASSING *40 SOILS WITH | LITTLE ORGANIC MATTER 3 - 5% 5 - 12% LITTLE 10 - 20% MODERATELY ORGANIC 5 - 10% 12 - 20% SOME 20 - 35% | VERY SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, | DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE |
| LL 40 MX 41 MN LITTLE OR PI 6 MX NP 10 MX 10 MX 11 MN 11 MN 10 MX 10 MX 11 MN 11 | HIGHLY ORGANIC > 10% > 20% HIGHLY 35% AND ABOVE | (V SLI.) CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY, ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE. | LINE OF DIP, MEASURED CLOCKWISE FROM NORTH. |
| CROUP INDEX 9 9 9 4 MY 8 MY 12 MY 16 MY NO MY AMOUNTS OF | GROUND WATER | SLIGHT ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO | FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE |
| USUAL TYPES STONE FRAGS. SILE OF THE CONTROL OF THE | ▼ WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING | (SLI.) 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR | SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE. |
| OF MAJOR GRAVEL, AND FAINE SILIY OF CLAYEY SILIY CLAYEY MATTER | ▼ STATIC WATER LEVEL AFTER 24 HOURS | CRYSTALS ARE DULL AND DISCOLORED, CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS. | FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES. |
| MATERIALS SANU | → PW PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA | MODERATE SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS | FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL. |
| GEN. RATING EXCELLENT TO GOOD FAIR TO POOR POOR POOR UNSUITABL | E | DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED | FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM. |
| PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30 | — SPRING OR SEEP | WITH FRESH ROCK. | FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE |
| CONSISTENCY OR DENSENESS | MISCELLANEOUS SYMBOLS | MODERATELY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL SEVERE AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH | FIELD. |
| COMPACTNESS OR RANGE OF STANDARD RANGE OF UNCONFINED | TT 25,425 | (MOD. SEV.) AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK, ROCK GIVES "CLUNK" SOUND WHEN STRUCK. | JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED. |
| PRIMARY SOIL TYPE COMPRESSIVE STRENGTH (N-VALUE) COMPRESSIVE STRENGTH (TONS/FT ²) | ROADWAY EMBANKMENT (RE) OF ROCK STRUCTURES | IF TESTED, WOULD YIELD SPT REFUSAL | LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO |
| VERY LODGE 4.4 | SPI CO CLODE INDICATOR | SEVERE ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT (SEV.) REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED | ITS LATERAL EXTENT. |
| GENERALLI LOOSE 4 TO 10 | SOIL SYMBOL OPT DAT TEST BORING SLOPE INDICATION INSTALLATION | TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. | LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS. MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS |
| MATERIAL DENSE 30 TO 50 | ARTIFICIAL FILL (AF) OTHER AUGER BORING CONE PENETROMETER THAN ROADWAY EMBANKMENT AUGER BORING | IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF VERY ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE | USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE. |
| (NON-COHESIVE) VERY DENSE > 50 | THAN ROADWAY EMBANKMENT THAN ROADWAY EMBANKMENT TEST | SEVERE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK | PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE |
| VERY SOFT < 2 < 0.25 | ── INFERRED SOIL BOUNDARY ———————————————————————————————————— | (V SEV.) REMAINING, SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR | OF AN INTERVENING IMPERVIOUS STRATUM. |
| GENERALLY SOFT 2 TO 4 0.25 TO 0.5 SILT-CLAY MEDIUM STIFF 4 TO 8 0.5 TO 1.0 | INFERRED ROCK LINE MM MONITORING WELL TEST BORING HITH CORE | VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. <u>IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF</u> COMPLETE ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND | RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK. |
| MATERIAL STIFF 8 TO 15 1 TO 2 | NI TONE | SCATTERED CONCENTRATIONS, QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS, SAPROLITE IS | ROCK QUALITY DESIGNATION (ROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE |
| (COHESIVE) VERY STIFF 15 TO 30 2 TO 4 HARD > 30 > 4 | PIEZOMETER INSTALLATION SPT N-VALUE | ALSO AN EXAMPLE. | RUN AND EXPRESSED AS A PERCENTAGE. |
| TEXTURE OR GRAIN SIZE | RECOMMENDATION SYMBOLS | ROCK HARDNESS | SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT |
| | THE ACCIPIES EVEN AT THE THE PROPERTY OF THE P | VERY HARD CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK, BREAKING OF HAND SPECIMENS REQUIRES | ROCK. |
| U.S. STD. SIEVE SIZE 4 10 40 60 200 270 OPENING (MM) 4.76 2.00 0.42 0.25 0.075 0.053 | UNDERCOT AND UNDERCOTAL AND UN | SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK. HARD CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY, HARD HAMMER BLOWS REQUIRED | SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO |
| COARSE FINE | SHALLOW UNCLASSIFIED EXCAVATION - USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL | TO DETACH HAND SPECIMEN. | THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS. |
| BOULDER COBBLE GRAVEL SAND SAND (SL.) (CL.) | | MODERATELY CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE | SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT |
| (CSE, SU.) (F SU.) | ABBREVIATIONS | HARD EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK, HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS. | OR SLIP PLANE. |
| GRAIN MM 305 75 2.0 0.25 0.05 0.005 SIZE IN. 12 3 | AR - AUGER REFUSAL MED MEDIUM VST - VANE SHEAR TEST BT - BORING TERMINATED MICA MICACEOUS WEA WEATHERED | MEDIUM CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. | STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB, HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL |
| | CL CLAY MOD MODERATELY γ - UNIT WEIGHT | HARD CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE | WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER, SPT REFUSAL IS PENETRATION EQUAL |
| SOIL MOISTURE - CORRELATION OF TERMS SOIL MOISTURE SCALE FIELD MOISTURE CAUSE FOR SUITE ASSOCIATION. | CPT - CONE PENETRATION TEST NP - NON PLASTIC CSE COARSE ORG ORGANIC | POINT OF A GEOLOGIST'S PICK. | TO OR LESS THAN 0.1 FOOT PER 60 BLOWS. |
| (ATTERBERG LIMITS) OBSCRIPTION GUIDE FOR FIELD MOISTURE DESCRIPTION | DMT - DILATOMETER TEST PMT - PRESSUREMETER TEST SAMPLE ABBREVIATIONS | SOFT CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN | STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE. |
| - SATURATED - USUALLY LIQUID; VERY WET, USUALLY | DPT - DYNAMIC PENETRATION TEST SAP SAPROLITIC S - BULK e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON | PIECES CAN BE BROKEN BY FINGER PRESSURE. | STRATA ROCK QUALITY DESIGNATION (SROD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY |
| (SAT.) FROM BELOW THE GROUND WATER TABLE | e - VOID RATIO SD SAND, SANDY SS - SPLIT SPOON F - FINE SL SILT, SILTY ST - SHELBY TUBE | VERY CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK, PIECES 1 INCH SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY | LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE. |
| PLASTIC LIQUID LIMIT | FOSS FOSSILIFEROUS SLI SLIGHTLY RS - ROCK | SOFT OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL. | TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER. |
| PANCE / - WET - (W) SEMISULID; REGUIRES DRIING TO | FRAC FRACTURED, FRACTURES TCR - TRICONE REFUSAL RT - RECOMPACTED TRIAXIAL FRAGS FRAGMENTS | FRACTURE SPACING BEDDING | BENCH MARK: BL-102 REBAR WITH CAP AT -L- STA, 15+06 14LT |
| (PI) PL _ PLASTIC LIMITATTAIN OPTIMUM MOISTURE | HI HIGHLY V - VERY RATIO | TERM SPACING TERM THICKNESS | BENCH MARK: BL-102 REBAR WITH CAP AT -L- STA. 15+06 14LT |
| - MOIST - (M) COLID. AT OR NEAR ORTHMIN MOISTINE | EQUIPMENT USED ON SUBJECT PROJECT | VERY WIDE MORE THAN 10 FEET VERY THICKLY BEDDED 4 FEET WIDE 3 TO 10 FEET THICKLY BEDDED 1.5 - 4 FEET | ELEVATION: 136.56 FEET |
| OM _ OPTIMUM MOISTURE SL _ SHRINKAGE LIMIT | DRILL UNITS: ADVANCING TOOLS: HAMMER TYPE: | WIDE | NOTEC |
| PERHIPES ANDITIONAL WATER TO | CME-45C CLAY BITS X AUTOMATIC MANUAL | CLOSE 0.16 TO 1 FOOT VERY THINLY BEDDED 0.03 - 0.16 FEET | NOTES: |
| - DRY - (D) ATTAIN OPTIMUM MOISTURE | 6 CONTINUOUS FLIGHT AUGER CORE SIZE: | VERY CLOSE LESS THAN 0.16 FEET THICKLY LAMINATED 0.008 - 0.03 FEET THINLY LAMINATED < 0.008 FEET | TOP OF RAIL AT EBISTA. 15+19, 11'RT ELEV.= 139.9 |
| PLASTICITY | X CME-55 X 8*HOLLOW AUGERS CORE SIZE: -BH | INDURATION | 1 · · · · · · · |
| | | FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC. | TOP OF RAIL AT EB2 STA.16+21, 11'RT ELEV.= 139.9 |
| PLASTICITY INDEX (PI) DRY STRENGTH NON PLASTIC 0-5 VERY LOW | TUNG-CARRIDE INSERTS | RUBBING WITH FINGER FREES NUMEROUS GRAINS; | |
| SLIGHTLY PLASTIC 6-15 SLIGHT | VANE SHEAR TEST X CASING X W/ ADVANCER HAND TOOLS: | GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE. | |
| MODERATELY PLASTIC 16-25 MEDIUM | CASING X W/ ADVANCER POST HOLE DIGGER | MODERATELY INDURATED GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; | |
| HIGHLY PLASTIC 26 OR MORE HIGH | | PIODENHIELI INDONHIED | |
| | PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER | BREAKS EASILY WHEN HIT WITH HAMMER. | |
| HIGHLY PLASTIC 26 OR MORE HIGH COLOR | PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER TRICONE TUNGCARB. SOUNDING ROD | BREAKS EASILY WHEN HIT WITH HAMMER. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. | |
| COLOR DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY). | PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER | INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER. CHARR NAMMER BLOWS BEGUIRED TO BREAK SAMPLE. | |
| COLOR | PORTABLE HOIST TRICONE STEEL TEETH HAND AUGER TRICONE TUNGCARB. SOUNDING ROD | BREAKS EASILY WHEN HIT WITH HAMMEN. INDURATED GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; | DATE: 8-15-14 |

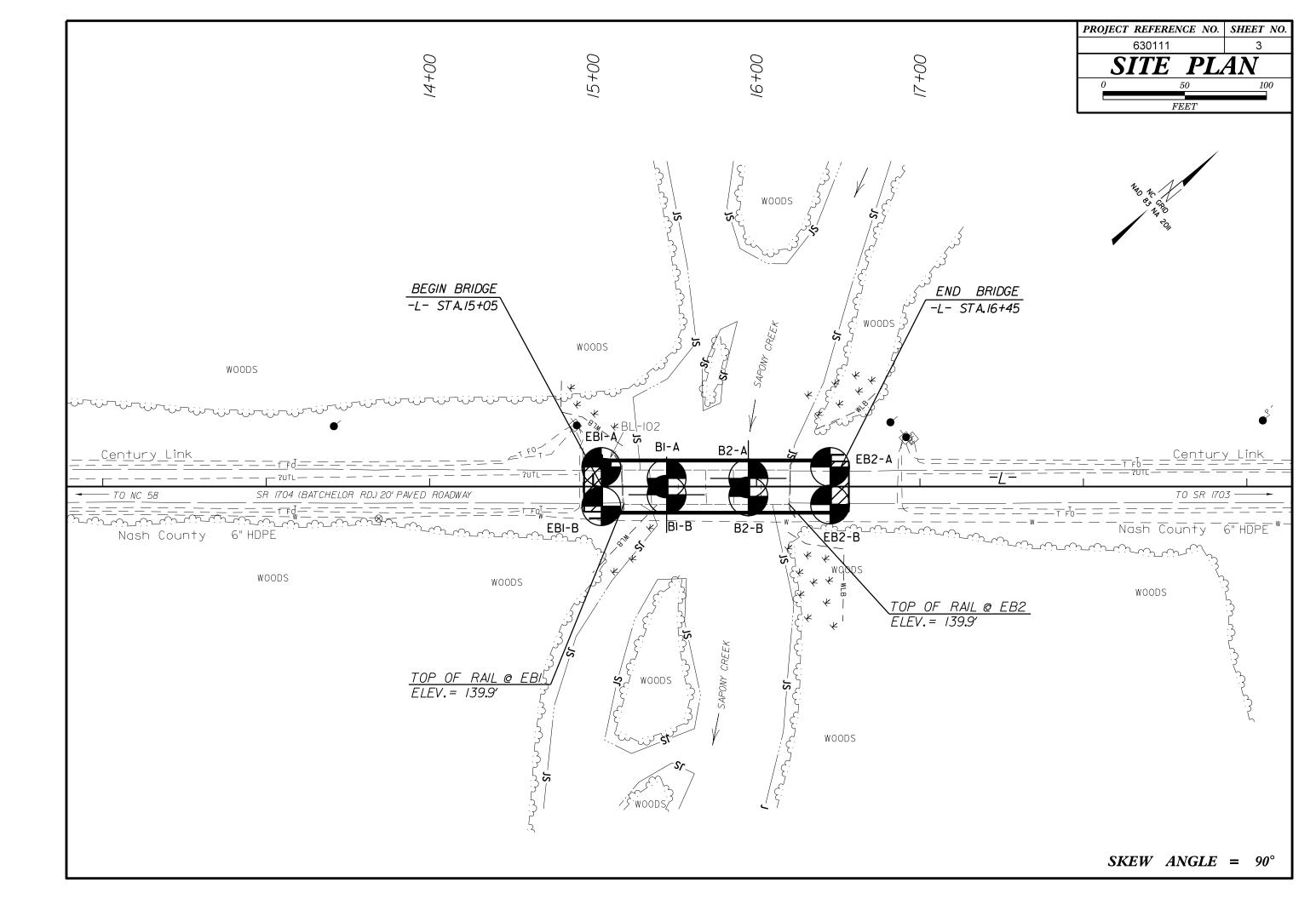
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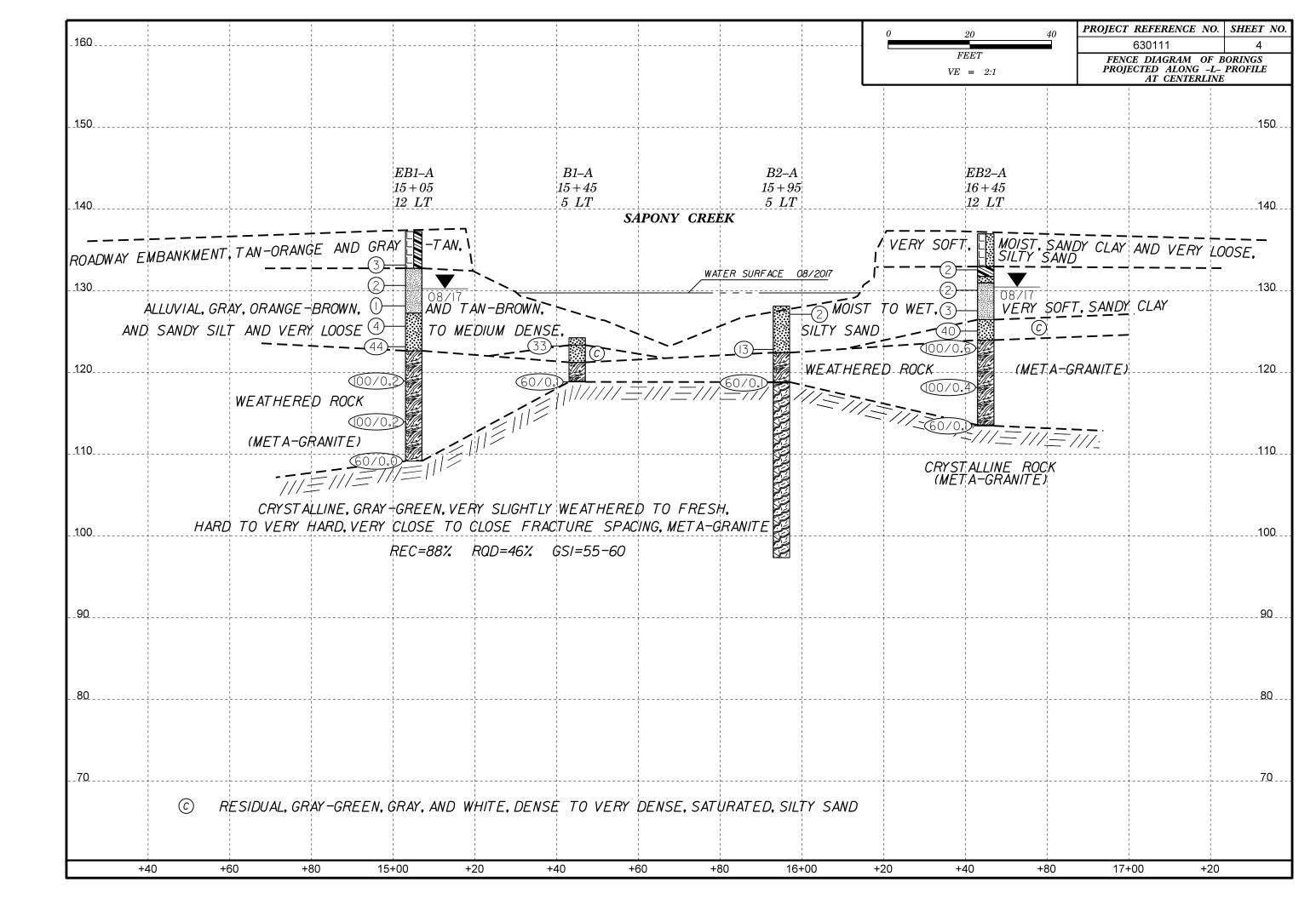
NORTH CAROLINA DEPARTMENT OF TRANSPORTATION **DIVISION OF HIGHWAYS** GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

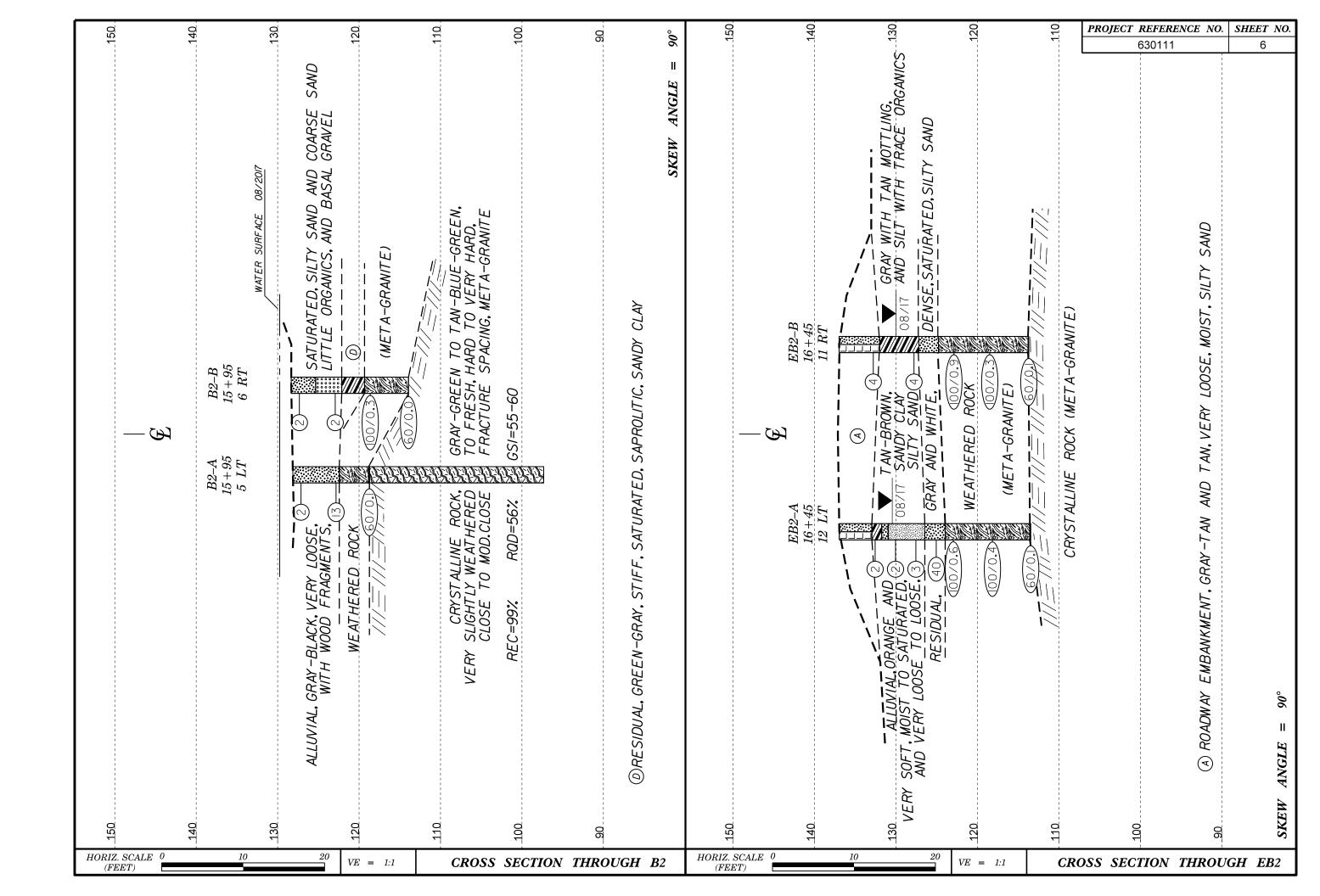
SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES

| AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Join | nted Ro | ock Mass (Marınos and Hoek, 2 | 2000) | | | AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Def | ormed Hetero | geneous Rock | Masses (Marı | nos and Hoek | , 2000) |
|--|--------------------|--|--|---|-------------------------------------|--|---|--|---|---|--|
| GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000) | | o ce | | 8 9 0 0 | | GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos.P and Hoek E., 2000) | | | | | |
| From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis. | SURFACE CONDITIONS | VERY GOOD Very rough, fresh unweathered surf COOD Rough, slightly weathered, iron stai | FAIR Smooth, moderately weathered and altered surfaces | Slickensided, highly weathered surfa with compact coatings or fillings or angular fragments VERY POOR Slickensided, highly weathered surfa | with soft clay coatings or fillings | From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis. | VERY GOOD - Very Rough, fresh unweathered surfaces | 600D - Rough, slightly weathered surfaces | FAIR - Smooth, moderately weathered and altered surfaces | POOR - Very smooth, occasionally slickensided surfaces with compact coatings or fillings with angular fragments | VERY POOR - Very smooth, slicken- sided or highly weathered surfaces with soft clay coatings or fillings |
| STRUCTURE | | DECREASING SU | JRFACE QU | | | COMPOSITION AND STRUCTURE | | | | | |
| INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities BLOCKY - well interlocked un- | PIECES | 90 80 | | N/A N/A | | A. Thick bedded, very blocky sandstone The effect of pelitic coatings on the bedding planes is minimized by the confinement of the rock mass. In shallow tunnels or slopes these bedding planes may cause structurally controlled instability. | 70 60 | A | | | |
| disturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets | OF ROCK | 70 60 | | | | 8. Sand- stone with thin inter- Solution Sol | | 50 B | C / I |) E | |
| VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets | OCKING | 5 | 0 | | | layers of shale with sandstone layers Statione Sta | | 40 | | | |
| BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity | ASING INTERL | | 40 | 30 | | C. D. E. and G - may be more or less folded than illustrated but this does not change the strength. Iectonic deformation, faulting and loss of continuity moves these categories to F and H. | | | 30 | F 20 | |
| DISINTEGRATED - poorly inter- locked, heavily broken rock mass with mixture of angular and rounded rock pieces | | | | 20 | | G. Undisturbed silty or clayey shale with or without a few very thin sandstone layers W. Tectonically deformed silty or clayey shale forming a chaotic structure with pockets of clay. Thin layers of sandstone are transformed | , , | | / | | 10 |
| LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes | Ÿ | N/A N/A | | 10 | $/\!\!\mid$ | Means deformation after tectonic disturbance | | | | | |

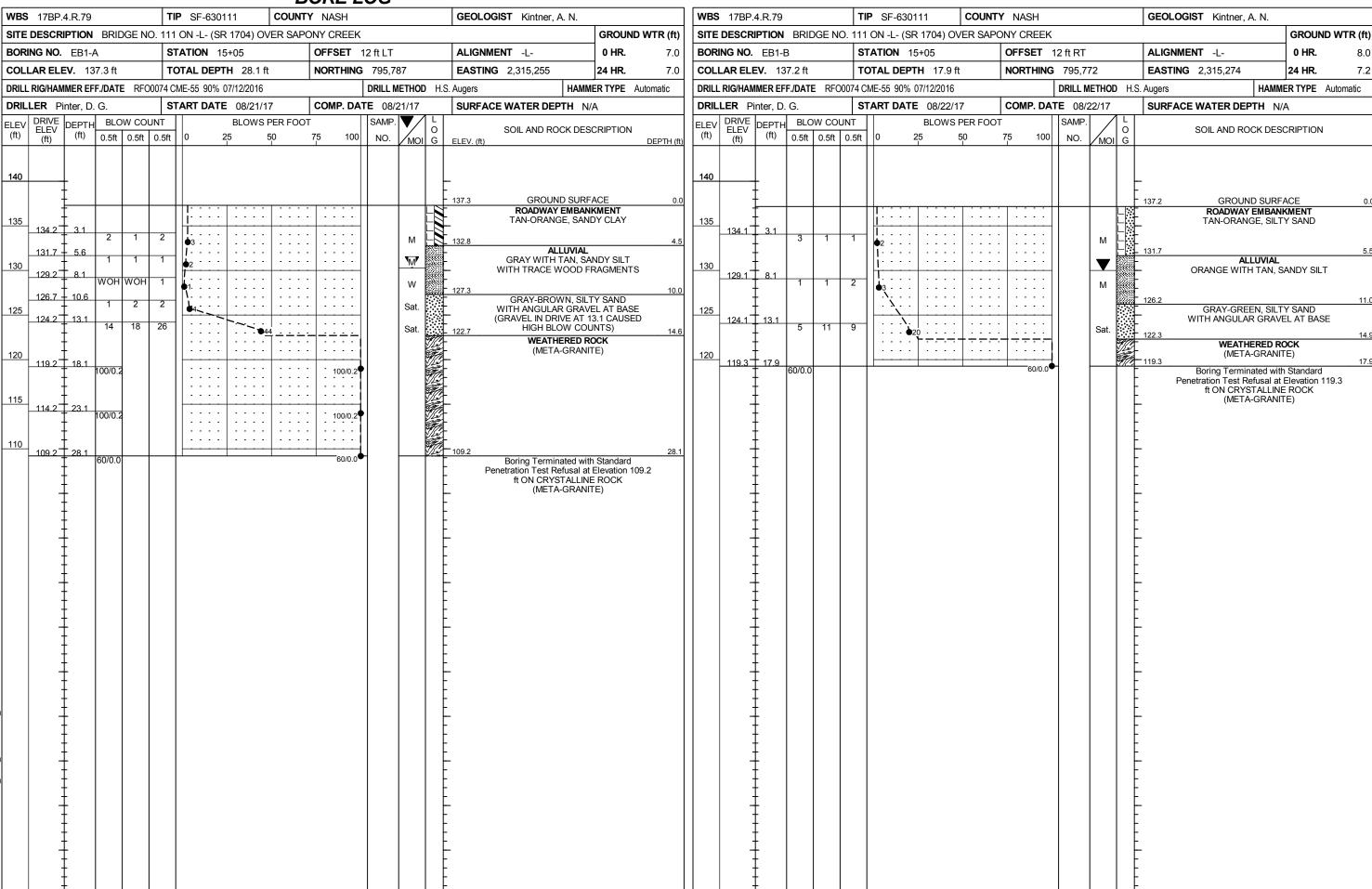




| 150 | 130 77 S .D. | 100 100 = 90° | 140 | 120 | PROJECT REFERENCE NO. SHE |
|--------------------------|---|---------------------------|--|---|--|
| | ► TO WET, TO WET, ATURATE | ry SAND | | | E) SLIGHTLY D-70 -GRAWITE 5-60 |
| | MOJST WOOD F ENSE, S, YLAR GF | WIT-E) LOOSE, SILTY SKEW | 08/2017 | i | -GRANITE) N. VERY SI SH. HARD CLOSE TO IG. META-C |
| | 7 SOFT. TRACE TRACE F TO D SAL ANG | -ROCK(MET-A-GRANITE) | R SURFACE | l l ! | (META =16% NY-GREE TO FRE D, VERY SPACIN 1D=43% |
| | WITH BAY | K-(MET-A | WATER | RED-ROCK GRAWITE) | |
| EBI-B 15+05 | | LLINE-ROCK-SANDY CLAY | | ATHE TA- | WE AT HERED CRYST ALLINE WE AT HE VERY CLOSE FRAC REC=77% |
| | 101-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1 | CRY-STALLINE. | B1–B 15+45 5 RT | | |
| $\mathcal{E}-$ | (META-GRAVIT | SOFT, MOIST. | $egin{pmatrix} & & & & & & & & & & & & & & & & & & $ | | |
| EB1-A 15+05 | 6RA) S1L7 | E,VERY | BI 15+ | E ROCK | IE SAND VTS |
| EI 15 | | O/0.0 | | CRYSTALLINE (META-GRAN | SE TO ILTY FINE GMENTS, FRAGMENT |
| | E-W/ EN A ROC | ENT, TAW | | CRY | TRY LOO'S ATED, SED FRA WOOD |
| | \\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\ | =_///_=_//// | | | TAW, VE SATUF TATHER CS AND |
| | VIAL. GRAY-AND ALLUVIAL, GRA | ROADWAY EI | | | ALLUVIAL, GRAY –T AN, VERY LOO MEDIUM DENSE, SATURATED, S WITH SOME WEATHERED FRA LITTLE ORGANICS AND WOOD RESIDUAL, GRAY –GREEN, DENSI VERY DENSE, SATURATED, SILT |
| | LEUVIAL. | A ROA | | | ALLUVIA MEDIUM WITH S LITTLE RESIDU |
| 150. | 130A | 100. | 150 | 120 | 100 80 © |
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GEOTECHNICAL BORING REPORT BORE LOG



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SHEET 8

| WBS 17BP.4.R.79 | TIP SF-630111 COUNTY | Y NASH | GEOLOGIST Kintner, A. N. | |
|------------------------------------|----------------------|---------------------|---|--|
| SITE DESCRIPTION BRIDGE NO. | l | | Tantalor, 7 a. 14. | GROUND WTR (ft) |
| BORING NO. B1-A | STATION 15+45 | OFFSET 5 ft LT | ALIGNMENT -L- | 0 HR. N/A |
| COLLAR ELEV. 124.3 ft | TOTAL DEPTH 5.4 ft | NORTHING 795,813 | EASTING 2,315,286 | 24 HR. N/A |
| DRILL RIG/HAMMER EFF./DATE RF0007 | | DRILL METHOD NW | l | ER TYPE Automatic |
| DRILLER Pinter, D. G. | START DATE 08/23/17 | COMP. DATE 08/23/17 | SURFACE WATER DEPTH 5.4 | |
| FLEY DRIVE DEDTU BLOW COUNT | L | | OOK AGE WATER DEI III 5.4 | rit. |
| (ft) ELEV (ft) (ft) 0.5ft 0.5ft 0. | | 75 100 NO. MOI G | SOIL AND ROCK DESC | CRIPTION |
| 125 | | Sat. | 124.3 GROUND SURFA 123.4 ALLUVIAL GRAY, SILTY SA WITH LITTLE ORGANICS FRAGMENTS RESIDUAL GRAY-GREEN, SILTY WEATHERED RC (META-GRANIT) CRYSTALLINE RC (META-GRANIT) Boring Terminated with Penetration Test Refusal at If IN CRYSTALLINE (META-GRANIT) (META-GRANIT) | ND 3.0 AND WOOD 3.0 S 7 SAND 5.3 Y SAND 5.4 E) 0CK E) 0CK E) Standard Elevation 118.9 ROCK |

GEOTECHNICAL BORING REPORT

BORE LOG WBS 17BP.4.R.79 **COUNTY** NASH **TIP** SF-630111 GEOLOGIST Kintner, A. N. SITE DESCRIPTION BRIDGE NO. 111 ON -L- (SR 1704) OVER SAPONY CREEK **GROUND WTR (ft)** OFFSET 5 ft RT BORING NO. B1-B STATION 15+45 ALIGNMENT -L-0 HR. N/A COLLAR ELEV. 125.1 ft TOTAL DEPTH 42.7 ft **NORTHING** 795,807 **EASTING** 2,315,294 N/A 24 HR. DRILL RIG/HAMMER EFF./DATE RFO0074 CME-55 90% 07/12/2016 **DRILL METHOD** NW Casing W/SPT & Core HAMMER TYPE Automatic DRILLER Pinter, D. G. **START DATE** 08/22/17 COMP. DATE 08/22/17 **SURFACE WATER DEPTH** 4.7ft ELEV DRIVE DEPTH BLOW COUNT SAMP. **BLOWS PER FOOT** SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft 75 100 NO. MOI G (ft) ELEV. (ft) DEPTH (ft 130 **GROUND SURFACE** 125 125.1 T Sat. ALLUVIAL GRAY-TAN, FINE SAND WITH SOME WEATHERED FRAGMENTS WEATHERED ROCK 120 119.0 T 6.1 92 8/0.1 100/0.6 115 114.0 11.1 60/0.0 60/0.0 CRYSTALLINE ROCK GRAY-GREEN. VERY SLIGHTLY WEATHERED TO FRESH, HARD TO VERY 110 HARD, VERY CLOSE TO CLOSE RS-1 FRACTURE SPACING, META-GRANITE REC=88% RQD=39% GSI=55-60 WEATHERED ROCK 105 GRAY-GREEN, SEVERELY WEATHERED, META-GRANITE REC=16% 100 CRYSTALLINE ROCK GRAY-GREEN, VERY SLIGHTLY WEATHERED TO FRESH, HARD TO VERY 97.9 27.2 100/0.4 HARD. VERY CLOSE TO CLOSE 100/0.4 FRACTURE SPACING, META-GRANITE WITH WEATHERED ROCK ZONE (27.3-27.7) REC=77% RQD=43% GSI=55-60 90 85 Boring Terminated at Elevation 82.4 ft IN CRYSTALLINE ROCK (META-GRANITE)

GEOTECHNICAL BORING REPORT CORF LOG

| | | | | | | | | | C | :C | ORE LOG | |
|--------------|----------------|---------------|-------------|---|-----------------------|------------------|-----------------|-------------------|------------------|-------|---|----------|
| | 17BP | | | | | SF-63 | | | | | Y NASH GEOLOGIST Kintner, A. N. | |
| | | | | DGE NO | 1 | | • | 4) OVI | ER SA | _ | PONY CREEK GROUND WTF | ` ' |
| - | ING NO | | | | | | 15+45 | | | + | | N/A |
| | LAR EL | | | | | | PTH 42 | | | N | | N/A |
| | | | | TE RFO | | | | | | 1 _ | DRILL METHOD NW Casing W/SPT & Core HAMMER TYPE Automa | atic |
| — | LER P | | | | | | TE 08/2 | | | C | COMP. DATE 08/22/17 SURFACE WATER DEPTH 4.7ft | |
| | E SIZE RUN | | | DRILL | RI | JN | N 31.2 f | | ATA | + | L | |
| ELEV (ft) | ELEV (ft) | DEPTH (ft) | RUN (ft) | RATE (Min/ft) | REC. (ft) % | RQD (ft) % | SAMP. NO. | REC. (ft) % | RQD (ft) % | | O DESCRIPTION AND REMARKS | PTH (ft) |
| 114 | 114.0 112.4 | 11:7 12:7 | 1.6 | N=60/0.0 | (1.6) | (0.0) | | (6.3) | (2.8) | | Begin Coring @ 11.1 ft CRYSTALLINE ROCK | 11.1 |
| 110 | 112.4 | 12.7 | 5.0 | N=60/0.0 1:32/1.0 0:59/0.6 1:21/1.0 1:18/1.0 1:16/1.0 0:53/1.0 | (5.0) 100% 100% | (2.8) 56% | RS-1 | 107% | 47% | | GRAY-GREEN, VERY SLIGHTLY WEATHERED TO FRESH, HARD TO VERY HARD, VERY CLOSE TO CLOSE FRACTURE SPACING, META-GRANITE | |
| | 107.4 | 17.7 | 5.0 | 1:16/1.0 0:53/1.0 | (0.5) | (0.0) | | (0.8) | | 7 | 108.1 GSI=55-60 WEATHERED ROCK | 17.0 |
| 105 | | ‡ | 5.0 | 2:05/1.0 2:10/1.0 2:52/1.0 | (0.5) 10% | 0% | | 16% | | | GRAY-GREEN, SEVERELY WEATHERED, META-GRANITE | |
| | 102.4 | 22.7 | | 2:52/1.0 3:42/1.0 1:45/1.0 | | | | (45.0) | (0.0) | | 103.1 | 22.0 |
| 100 | - | | 4.6 | 1:17/1.0 1:12/1.0 1:23/1.0 1:20/1.0 | (1.4) 30% | (0.0) 0% | | (15.9) 77% | (8.9) 43% | N. N. | CRYSTALLINE ROCK GRAY-GREEN, VERY SLIGHTLY WEATHERED TO FRESH, HARD TO VERY HARD, VERY CLOSE TO CLOSE FRACTURE SPACING, META-GRANITE WITH WEATHERED ROCK ZONE (27.3-27.7) | |
| | 97.8 97.4 | 27.3 27.7 | 5.0 | 1:17/0.6 N=100/0.4 | (4.4) | (2.5) | | | | | GSI=55-60 | |
| 95 | 92.4 | 32.7 | 0.0 | 1:17/0.6 N=100/0.4 1:24/1.0 1:43/1.0 1:48/1.0 1:53/1.0 2:08/1.0 | 88% | 50% | | | | | | |
| | 32.4 | 52.7 | 5.0 | 1:36/1.0 1:32/1.0 | (4.6) 92% | (2.6) 52% | | | | | | |
| 90 | 87.4 | 37.7 | | 1:35/1.0 1:19/1.0 1:15/1.0 | | | | | | KIK | | |
| 85 | | Ŧ | 5.0 | 1:24/1.0 1:33/1.0 | (5.0) 100% | (3.8) 76% | | | | | | |
| |] · | Ŧ | | 1:28/1.0 1:42/1.0 | | | | | | | | |
| | 82.4 | + 42.7 + | | 2:23/1.0 | | | | | | 1 | 82.4 Boring Terminated at Elevation 82.4 ft IN CRYSTALLINE ROCK | 42.7 |
| | - | | | | | | | | | | Boring Terminated at Elevation 82.4 ft IN CRYSTALLINE ROCK (META-GRANITE) | |

GEOTECHNICAL BORING REPORT

BORE LOG WBS 17BP.4.R.79 **TIP** SF-630111 COUNTY NASH GEOLOGIST Kintner, A. N. SITE DESCRIPTION BRIDGE NO. 111 ON -L- (SR 1704) OVER SAPONY CREEK **GROUND WTR (ft)** ALIGNMENT -L-**STATION** 15+95 OFFSET 5 ft LT BORING NO. B2-A 0 HR. N/A COLLAR ELEV. 128.1 ft TOTAL DEPTH 30.8 ft **NORTHING** 795,852 **EASTING** 2,315,318 24 HR. N/A DRILL RIG/HAMMER EFF./DATE DRILL METHOD NW Casing W/SPT & Core HAMMER TYPE Automatic RFO0074 CME-55 90% 07/12/2016 DRILLER Pinter, D. G. **START DATE** 08/23/17 COMP. DATE 08/23/17 **SURFACE WATER DEPTH** 1.5ft ELEV DRIVE ELEV (ft) DEPTH BLOW COUNT (ft) 0.5ft 0.5ft 0.5ft **BLOWS PER FOOT** SAMP. SOIL AND ROCK DESCRIPTION (ft) 0.5ft 0.5ft 0.5ft NO. 75 100 MOI G ELEV. (ft) 130 GROUND SURFACE 128.1 T ALUVIAL GRAY-BLACK, SILTY SAND WITH WOOD FRAGMENTS, LITTLE ORGANICS AND BASAL GRAVEL Sat. 125 123.8 Sat. WEATHERED ROCK (META-GRANITE) 120 118.8 + 9.3 60/0. 60/0.1 CRYSTALLINE ROCK (META-GRANITE) GRAY-GREEN TO TAN-BLUE-GREEN, 115 RS-2 VERY SLIGHT WEATHERING TO FRESH, HARD TO VERY HARD, CLOSE TO MOD. CLOSE FRACTURE SPACING, META-GRANITE 110 REC=99% RQD=56% GSI=55-60 105 100 Boring Terminated at Elevation 97.3 ft IN CRYSTALLINE ROCK (META-GRANITE)

GEOTECHNICAL BORING REPORT CORFIGG

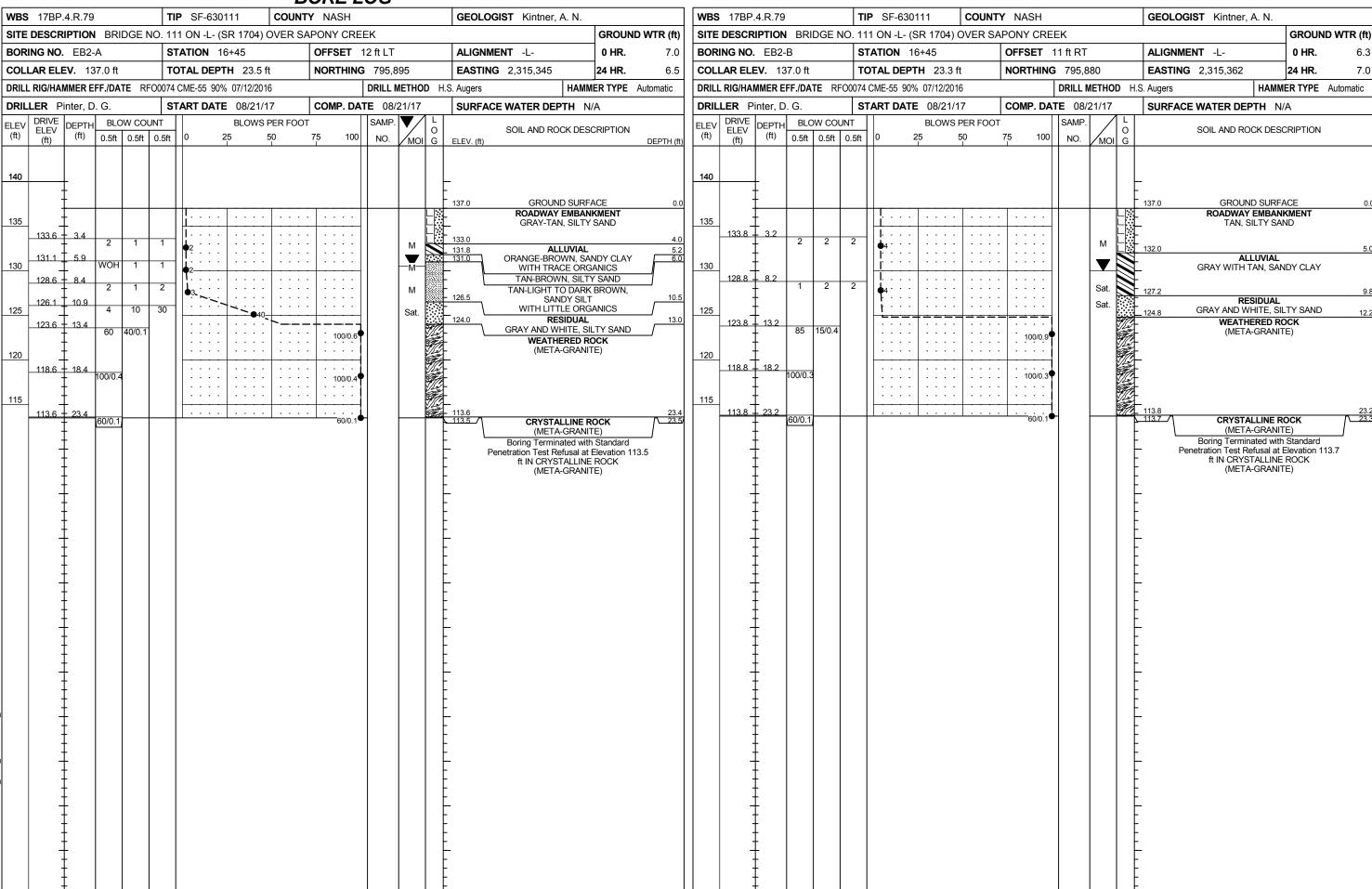
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|--------------|---------------------|---------------|-------------|--|--------------------------------|-----------------------|-----------------|-------------------|------------------|-------------|----------|------------------|-----------|--|---------------|----------|-------------|
| WBS | 17BP.4 | I.R.79 | | | TIP | SF-63 | 80111 | C | OUNT | Y 1 | NASH | | | GEOLOGIST Kintner | A. N. | | |
| SITE | DESCRI | PTION | BRI | DGE NO. | 111 C | N -L- (| (SR 1704) | OVEF | R SAP | ONY | Y CREEK | | | | | GROUN | ID WTR (ft) |
| BOR | ING NO. | B2-A | | | STA | TION | 15+95 | | | OF | FSET 5 | ft LT | | ALIGNMENT -L- | | 0 HR. | N/A |
| COL | LAR ELE | V . 12 | 8.1 ft | | TOT | AL DE | PTH 30. | .8 ft | | NC | DRTHING | 795,852 | | EASTING 2,315,318 | | 24 HR. | N/A |
| - | RIG/HAM | | | RF00 | 074 CM | IE-55 9 | 0% 07/12/2 | 2016 | | | | DRILL METH | | Casing W/SPT & Core | | ER TYPE | Automatic |
| | LER Pi | | | | | | TE 08/2 | | | CC | OMP. DAT | E 08/23/1 | 17 | SURFACE WATER DE | PTH 1. | 5ft | |
| | E SIZE | | | DDILL | | AL RUI Un | N 21.4 f | | ATA | ļ., | | | | | | | |
| ELEV (ft) | RUN ELEV (ft) | DEPTH (ft) | RUN (ft) | DRILL RATE (Min/ft) | REC. (ft) % | RQD (ft) % | SAMP. NO. | REC. (ft) % | RQD (ft) % | L O G | ELEV. (f | t) | DI | ESCRIPTION AND REMAR | KS | | DEPTH (ft) |
| 118.7 | 118.7 | . 9.4 | 1.8 | 1:18/1.0 | (1.5) | (1.1) | | (21.1) | (12.0) | | 118.7 | CRYS | TΔI I INF | Begin Coring @ 9.4 ft GRAY-GREEN TO TAN-B | LIE-GRE | EN VERY | 9.4 |
| 115 | 118.7 | 11.2 | 5.0 | 1:26/0.8 1:42/1.0 1:38/1.0 1:48/1.0 1:44/1.0 1:33/1.0 1:52/1.0 1:47/1.0 | (5.0) 100% (5.0) 100% | (2.8) 56% (3.8) | RS-2 | (21.1) 99% | 56% | | 118.7 | SLIGHTLY | WEATHE | GRED TO FRESH, HARD T OSE FRACTURE SPACING GSI=55-60 | O VERY I | HARD, CL | |
| 105 | 106.9 | 21.2 | 5.0 | 1:37/1.0 1:47/1.0 1:24/1.0 1:38/1.0 1:23/1.0 1:14/1.0 1:33/1.0 1:37/1.0 | (5.0) 100% | (3.7) 74% | | | | | | | | | | | |
| 100 | 97.3 | 30.8 | 4.6 | 1:36/1.0 1:27/1.0 1:07/1.0 2:11/1.0 1:12/0.6 | (4.6) 100% | (0.6) 13% | | | | | 97.3 | Boring | Terminat | ed at Elevation 97.3 ft IN CI | RYSTALLI | NE ROCK | 30.8 |
| | | | | | | | | | | | | | | (META-GRANITE) | | | |

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SHEET 11

| WBS | 17BP. | 4 R 79 | | | Т | IP | SF-630 | 111 | cc | OUNT | r NA | SH | | | | GEOLOGIST Kintner, A. N. | |
|--------------|---------------|---------------|-----------|---------|-------|----|-----------|--|---|---------------|-------------|---------|---------------|------|----------|---|-------------------|
| | | | | DGE N | | | ON -L- (| | | | | | FK | | | SECESIOT TURBER, 74. 14. | GROUND WTR (ft) |
| | NG NO. | | | | _ | | TION 1 | | ., 0 1 2 | | | | ft RT | | | ALIGNMENT -L- | 0 HR. N/A |
| | AR ELE | | | | + | | TAL DEP | | 4 ft | | | | 795,8 | 245 | | | 24 HR. N/A |
| | | | | TE DE | | | ME-55 90% | | | | 11011 | 1111110 | | | D NIV | <u> </u> | ER TYPE Automatic |
| | | | | IL IXIX | | | | | | | COM | D DA | TE 08/ | | ואו | | |
| | LER Pi | | | W COLL | | IΑ | RT DATI | | S PER | FOOT | COIVI | P. DA | SAMP. | | 1 L T | SURFACE WATER DEPTH 1.3 | SIL . |
| ELEV (ft) | ELEV (ft) | DEPTH (ft) | 0.5ft | 0.5ft | 0.5ft | - | 0 | 25 1 | 50 | | 75 | 100 | NO. | моі | 0 | SOIL AND ROCK DESC | RIPTION |
| 130 | | | | | | | | | | | | | | | | | |
| | 128.3 | 0.0 | WOH | WOH | 2 | Ц | 1 | 1 | | | 1 | | | | | 128.3 GROUND SURFA ALLUVIAL | ACE 0.0 |
| 125 | - | - | WOII | VVOIT | 2 | | •2 | | | | | | | Sat. | - | GRAY-BLACK, SILTY SAND ORGANICS | WITH LITTLE3.0 |
| 120 | 123.9 | 4.4 | 2 | 1 | 1 | ij | 1 • | | | | 1:: | | | Sat. | 0000 | GRAY-BLACK, COARSE S WOOD FRAGMENTS AN | ND LITTLE |
| 400 | - | - | | | | | 1 | | | | | | | Out. | | ORGANICS WITH BASAI RESIDUAL | |
| 120 | 118.9 | 9.4 | 100/0.3 | | | ╟ | <u> </u> | | | - | | 00/0.2 | , | | | 119.3 GREEN-GRAY, SAPROLITIC WEATHERED RO | |
| |] | | . 55, 6.5 | | | | | | | | : ' | 00/0.3 | | | | (META-GRANIT | |
| 115 | 113.9 | 14.4 | | | | 止 | | | <u>. </u> | | \pm | 00/0 | | | | 113.9 | 14.4 |
| | - | - | 60/0.0 | | | | | | | | | 60/0.0 | | | | Boring Terminated with Penetration Test Refusal at E | Elevation 113.9 |
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GEOTECHNICAL BORING REPORT BORE LOG



PROJ. NO. - 17BP.4.R.79 ID NO. - 630111 COUNTY - NASH

B1-B -L-

| ROCK TEST RESULTS | | | | | | | |
|-------------------|--------|---------|-----------|-------------|--------------------|------------------|--------------|
| SAMPLE | | | DEPTH | ROCK | UNIT WT | UNCONFINED COMP. | SECTION MOD. |
| NO. | OFFSET | STATION | INTERVAL | TYPE | LB/FT ³ | STRENGTH, KSI | @ 40% MPSI |
| RS-1 | 5 RT | 15+45 | 15.1-15.7 | METAGRANITE | 168.2 | 11.05 | 7.65 |

SHEET 13

B2-A -L-

| ROCK TEST RESULTS | | | | | | | |
|-------------------|--------|---------|-----------|-------------|--------------------|------------------|--------------|
| SAMPLE | | | DEPTH | ROCK | UNIT WT | UNCONFINED COMP. | SECTION MOD. |
| NO. | OFFSET | STATION | INTERVAL | TYPE | LB/FT ³ | STRENGTH, KSI | @ 40% MPSI |
| RS-2 | 5 LT | 15+95 | 12.1-12.9 | METAGRANITE | 167.8 | 15.01 | 9 |

CORE PHOTOGRAPHS

B1-BBOXES 1, 2 & 3: 11.1 - 42.7 FEET



FEET

B2-ABOXES 1, 2 & 3: 9.4 - 30.8 FEET



FEET

SITE PHOTOGRAPH

Bridge No. 111 on –L– (SR 1704) over Sapony Creek

