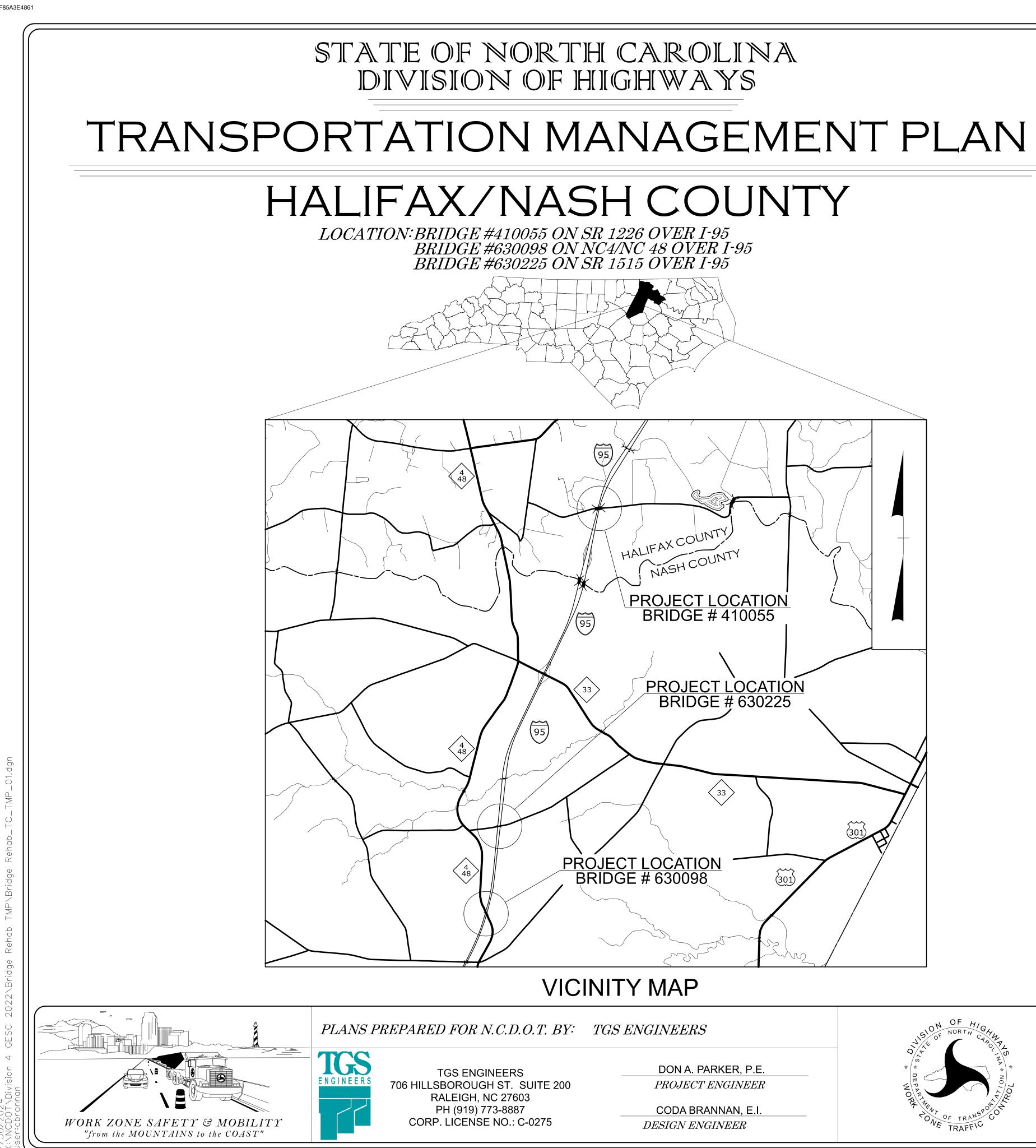
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SHEET NO.

TMP-1

TMP-1A

TMP-1B

TMP-2 TMP-2A

TMP-2B

TMP-2C

TMP-3

INDEX OF SHEETS	SHEET NO. TMP-1	
LIDEX OF SHEETS IT LE SHEET, VICINITY MAP, AND INDEX OF SHEETS IS OF APPLICABLE ROADWAY STANDARD DRAWINGS, AND LEGEND IRANSPORTATION OPERATIONS PLAN: (MANAGEMENT IRANSPORTATION OPERATIONS PLAN: (MANAGEMENT IRANSPORTATION OPERATIONS PLAN: MANAGEMENT IRANSPORTATION OPERATIONS PLAN: MANAGEMENT IRANSPORTATIONS PLAN: MANAGEMENT IRANSPORTATI		
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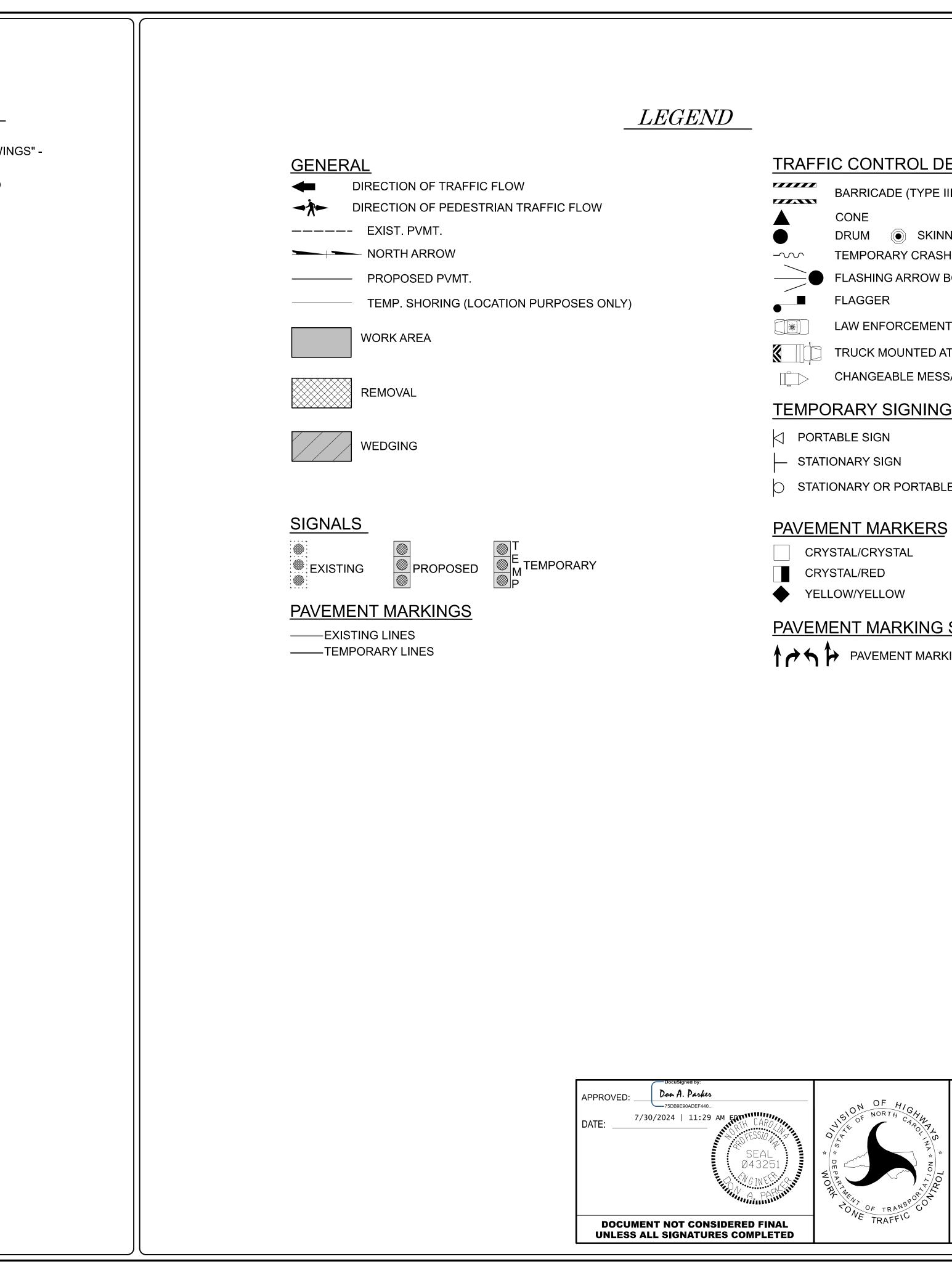
ROADWAY STANDARD DRAWINGS

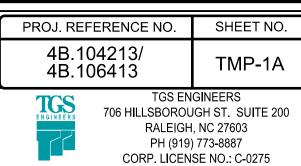
THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2024 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.

TITLE

1101.01	WORK ZONE WARNING SIGNS
1101.02	TEMPORARY LANE CLOSURES
1101.03	TEMPORARY ROAD CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1115.01	FLASHING ARROW BOARDS
1130.01	DRUMS
1135.01	CONES
1145.01	BARRICADES
1150.01	FLAGGING DEVICES
1165.01	TRUCK MOUNTED ATTENUATOR
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO LANE AND MULTILANE ROADWAYS
1205.04	PAVEMENT MARKINGS - INTERSECTIONS
1205.05	PAVEMENT MARKINGS - TURN LANES
1205.08	PAVEMENT MARKINGS - SYMBOLS AND WORD MESSAGES
1205.12	PAVEMENT MARKINGS - BRIDGES





TRAFFIC CONTROL DEVICES

	BARRICADE (TYPE III)
	CONE
	DRUM 💿 SKINNY DRUM 💿 TUBULAR MARKER
-~~	TEMPORARY CRASH CUSHION
	FLASHING ARROW BOARD
	FLAGGER
*	LAW ENFORCEMENT
	TRUCK MOUNTED ATTENUATOR (TMA)
	CHANGEABLE MESSAGE SIGN
TEMPO	DRARY SIGNING

- STATIONARY OR PORTABLE SIGN

PAVEMENT MARKING SYMBOLS

PAVEMENT MARKING SYMBOLS

ROADWAY STANDARD DRAWINGS & LEGEND

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

TIME RESTRICTIONS

A) DO NOT CLOSE OR NARROW TRAVEL LANES AS FOLLOWS:

ROAD NAME	DAY AND TIME RESTRICTIONS

-95

MONDAY THRU THURSDAY 7:00 AM TO 8:00 PM AND FRIDAY THRU SUNDAY

7:00 AM TO 10:00 PM

B) DO NOT CLOSE OR NARROW TRAVEL LANES DURING HOLIDAYS AND SPECIAL EVENTS AS FOLLOWS:

ROAD NAME

I-95

HOLIDAY

- 1. FOR ANY UNEXPECTED OCCURRENCE THAT CREATES UNUSUALLY HIGH TRAFFIC VOLUMES, AS DIRECTED BY THE ENGINEER.
- 2. FOR NEW YEAR'S, BETWEEN THE HOURS OF 6:30 A.M. DECEMBER 31st TO 8:00 P.M. JANUARY 2ND. IF NEW YEAR'S DAY IS ON A FRIDAY. SATURDAY, SUNDAY, OR MONDAY THEN UNTIL 8:00 P.M. THE FOLLOWING TUESDAY.
- 3. FOR EASTER, BETWEEN THE HOURS OF 6:30 A.M. THURSDAY AND 8:00 P.M. MONDAY.
- 4. FOR MEMORIAL DAY, BETWEEN THE HOURS OF 6:30 A.M. FRIDAY TO 8:00 P.M. TUESDAY.
- 5. FOR INDEPENDENCE DAY, BETWEEN THE HOURS OF 6:30 A.M. THE DAY BEFORE INDEPENDENCE DAY AND 8:00 P.M. THE DAY AFTER INDEPENDENCE DAY.

IF INDEPENDENCE DAY IS ON A FRIDAY, SATURDAY, SUNDAY OR MONDAY THEN BETWEEN THE HOURS OF 6:30 A.M. THE THURSDAY BEFORE INDEPENDENCE DAY AND 8:00 P.M. THE TUESDAY AFTER INDEPENDENCE DAY.

- 6. FOR LABOR DAY, BETWEEN THE HOURS OF 6:30 A.M. FRIDAY AND 8.00 P.M. TUESDAY
- 7. FOR THANKSGIVING DAY, BETWEEN THE HOURS OF 6:30 A.M. TUESDAY TO 8:00 P.M. MONDAY.
- 8. FOR CHRISTMAS. BETWEEN THE HOURS OF 6:30 A.M. THE FRIDAY BEFORE THE WEEK OF CHRISTMAS DAY AND 8:00 P.M. THE FOLLOWING TUESDAY AFTER THE WEEK OF CHRISTMAS.

LANE AND SHOULDER CLOSURE REQUIREMENTS

C) REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED OR AS DIRECTED BY THE ENGINEER.

D) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.

	DED FACILITY AND WITHIN SE THE NEAREST OPEN 1101.02 UNLESS THE WO				Q) 1 L
WHEN PERSONNEL AND/C ADJACENT TO A DIVIDED F					R) F M
TRAVEL LANE, CLOSE THE STANDARD DRAWING NO. BARRIER OR GUARDRAIL.	1101.02 UNLESS THE WO	L LANE USING ROADWAY ORK AREA IS PROTECTED BY			MIS
F) WHEN PERSONNEL AND/ OF AN UNDIVIDED OR DIV THE TRAFFIC CONTROL P	OR EQUIPMENT ARE WO IDED FACILITY, CLOSE T LANS, ROADWAY STAND OUCT THE WORK SO THA	THE LANE ACCORDING TO ARD DRAWINGS, OR AS DIRE T ALL PERSONNEL AND/OR			S) L A
G) DO NOT WORK SIMULTAN TRAVELWAY, RAMP, OR LO WITH GUARDRAIL OR BAR	OOP WITHIN THE SAME L	ON BOTH SIDES OF AN OPEN OCATION UNLESS PROTECTE	ED		LOC 1. Tł I
H) USE SEQUENTIAL FLASH OF NIGHTTIME LANE CLOS STANDARD SPECIFICATIO	SURES IN ACCORDANCE	WITH SECTION 1140 IN THE	RGING TA	APERS	\ ((
	TES PRIOR TO INSTALLIN	N OPERATIONS CENTER (STO IG AND WITHIN 15 MINUTES A ONTROLLED ACCESS FACILIT	FTER RE	MOVING	
TRAFFIC PATTERN ALTERATION	ONS				(S
J) NOTIFY THE ENGINEER T PATTERN ALTERATION.	HIRTY (30) CALENDAR D/	AYS PRIOR TO ANY TRAFFIC			l
SIGNING					((
K) INSTALL ADVANCE WORK40 FT FROM THE EDGE OF(3) DAYS PRIOR TO THE BI	TRAVEL LANE AND NO M	MORE THAN THREE			(L
L) PROVIDE SIGNING AND D REQUIRED TO CLOSE THE AND TRAFFIC CONTROL P	ROAD ACCORDING TO T	THE ROADWAY STANDARD DF	RAWINGS		(
PROVIDE SIGNING REQUI		TRAFFIC CONTROL PLANS.			
M) COVER OR REMOVE ALL TO CLOSE THE ROAD WH					THE
COVER OR REMOVE ALL S DETOUR WHEN THE DETO					REC
N) ENSURE ALL NECESSAR` TRAFFIC PATTERN.	Y SIGNING IS IN PLACE P	RIOR TO ALTERING ANY			TRA F L
TRAFFIC CONTROL DEVICES					S N
O) PLACE TYPE III BARRICAE ATTACHED, OF SUFFICIEN					C WOF S
PAVEMENT MARKINGS AND N	IARKERS				D S
P) INSTALL FINAL PAVEMENT PAVEMENT SURFACE AS F		ENT MARKERS ON FINAL			Ρ
ROAD NAME	MARKING	MARKER		APPROVED:	Doen A. Parker
ALL ROADS (ASPHALT)	THERMOPLASTIC	NONE		7/30 DATE:	0/2024 11:29 AM ED
ALL ROADS (CONCRETE BRIDGE DECKS)	COLD APPLIED PLASTIC	NONE			

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PROJ. REF	ERENCE NO.	SHEET NO.		
4B.10 4B.10	TMP-1B			
TGS		GINEERS IGH ST. SUITE 200		
	RALEIGH, NC 27603 PH (919) 773-8887			
		SE NO.: C-0275		

TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING INES.

REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND IARKERS BY THE END OF EACH DAY'S OPERATION.

CELLANEOUS

AW ENFORCEMENT MAY BE USED TO MAINTAIN TRAFFIC THROUGH THE WORK AREA. ND/ OR INTERSECTIONS AS DIRECTED BY THE ENGINEER.

LOCAL NOTES

AL NOTES

HE FOLLOWING REQUIREMENTS APPLY TO LANE CLOSURES ON I-95:

REDUCE SPEED DISPLAY ON DIGITAL SPEED LIMIT SIGNS (DSLS) AS STATED IN THE WORK ZONE VARIABLE SPEED LIMIT (WZVSL) ORDINANCE. WHEN THE LANE CLOSURE IS REMOVED. RESTORE THE SPEED DISPLAY TO THE POSTED SPEED LIMIT (SEE DSLS SPECIAL PROVISION.)

INCLUDE UP TO TWO ADDITIONAL PCMS TO THE TWO REQUIRED BY RSD 1102.02. SHEET 4 (4 TOTAL). LOCATE AND PROVIDE MESSAGING AS DIRECTED BY THE ENGINEER.

USE A CONNECTED LANE CLOSURE SYSTEM (SEE SPECIAL PROVISION). WHEN THE SYSTEM IS TURNED ON FOR THE LANE CLOSURE, THE CONTRACTOR SHALL CONFIRM THE SYSTEM IS TRANSMITTING INFORMATION PRIOR TO LEAVING THE SYSTEM JNATTENDED.

USE PRESENCE LIGHTING IN ADVANCE OF THE MERGE TAPER FOR NIGHTTIME LANE CLOSURES (SEE SPECIAL PROVISIONS).

USE SEQUENTIAL FLASHING WARNING LIGHTS ON DRUM TAPERS FOR NIGHTTIME ANE CLOSURES.

USE LAW ENFORCEMENT AS DIRECTED BY THE ENGINEER.

MANAGEMENT STRATEGIES

FOLLOWING LISTED WORK ZONE STRATEGIES ARE RECOMMENDED FOR LUSION WITHIN THIS TRANSPORTATION MANAGEMENT PLAN (TMP).

COMMENDED STRATEGIES:

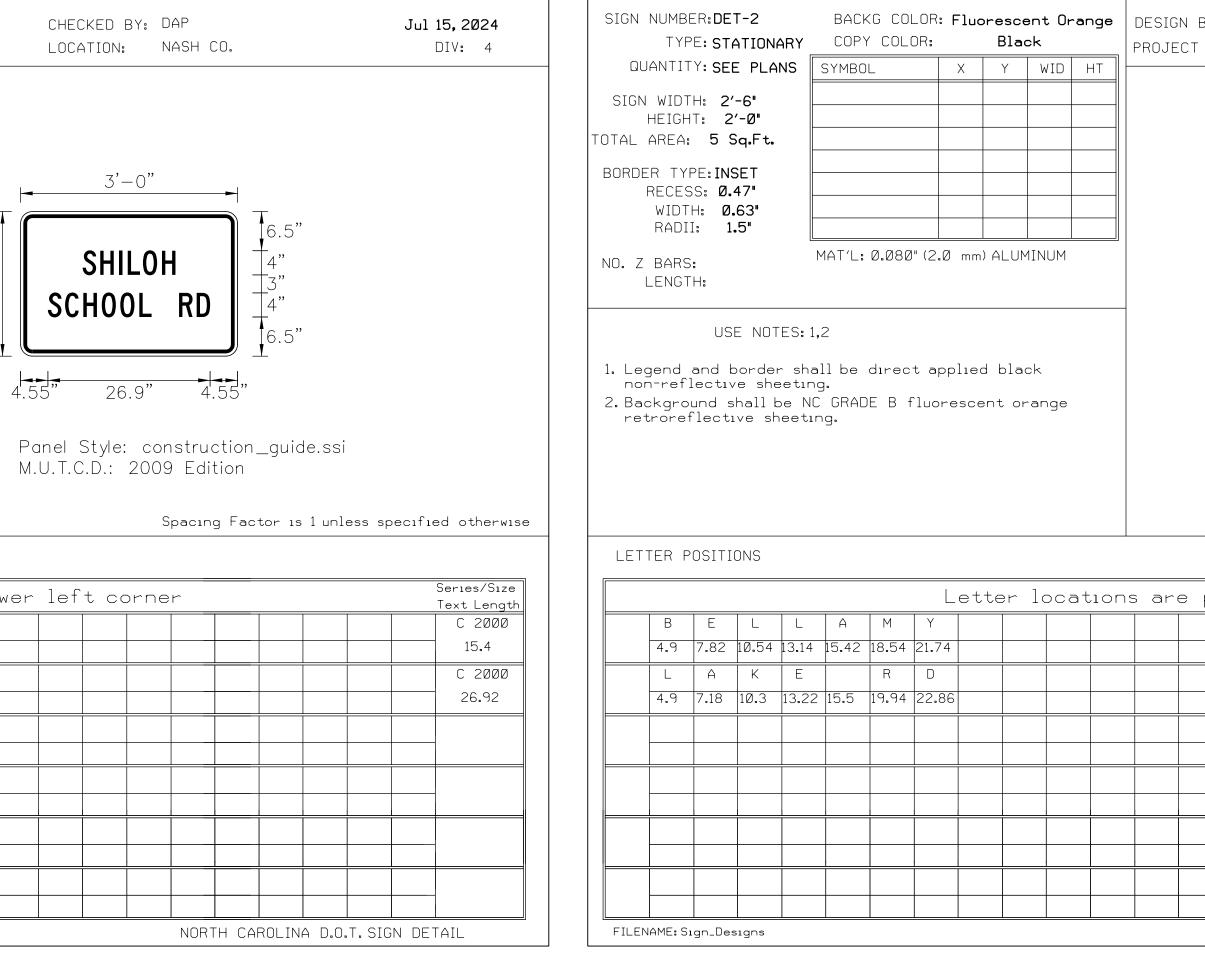
FFIC MANAGEMENT STRATEGIES: FULL ROADWAY CLOSURES ANE SHIFTS OR CLOSURES SHOULDER CLOSURES IIGHT WORK OFF-SITE DETOURS / USE OF ALTERNATIVE ROUTES RK ZONE SAFETY & MOBILITY STRATEGIES: SPEED LIMIT REDUCTION DIGITAL SPEED LIMIT SIGNS / VARIABLE SPEED LIMITS EQUENTIAL LIGHTING RESENCE LIGHTING



TRANSPORTATION **OPERATIONS** PLAN

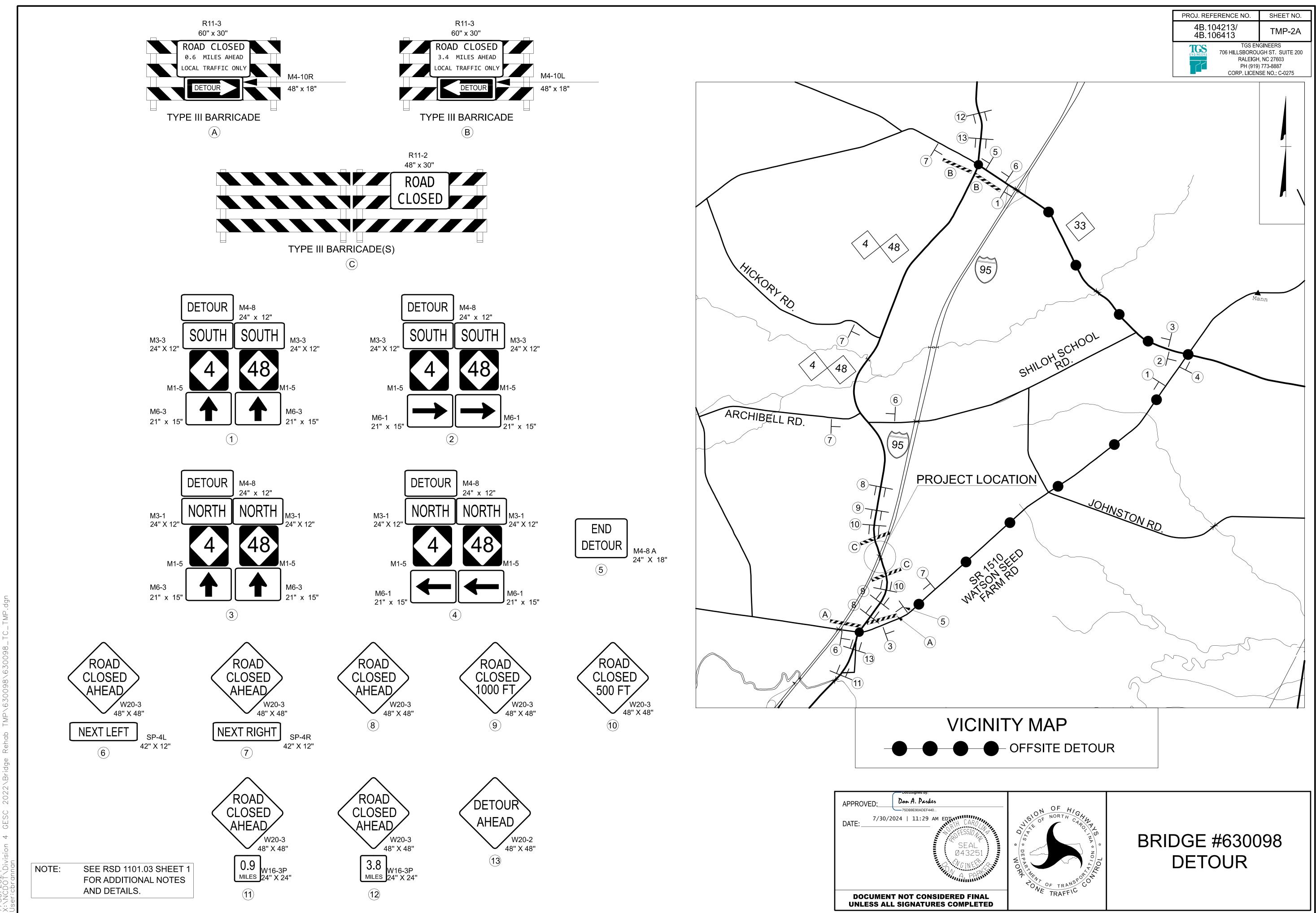
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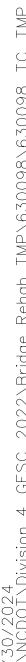
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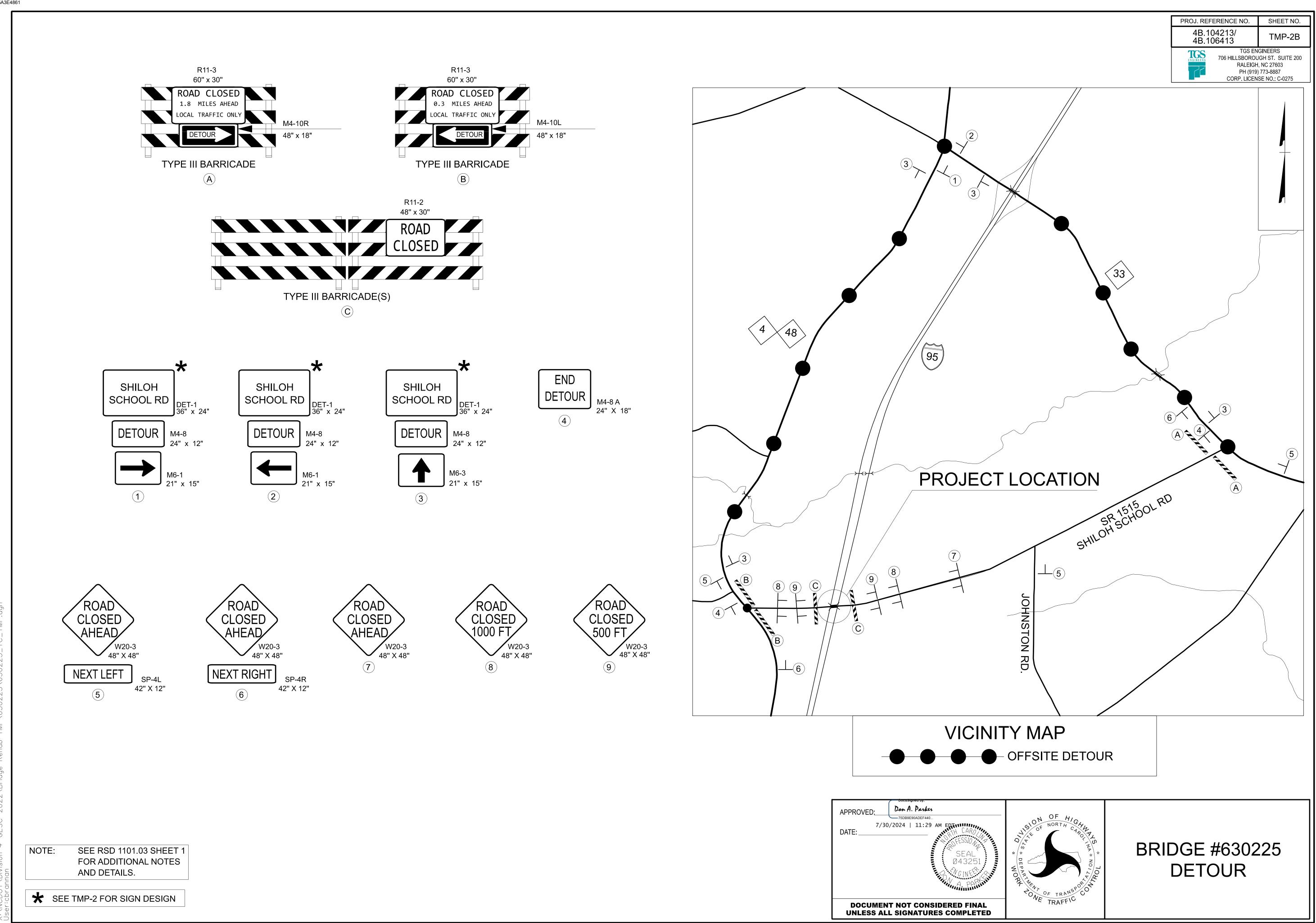


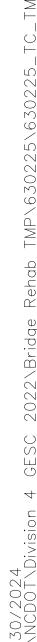
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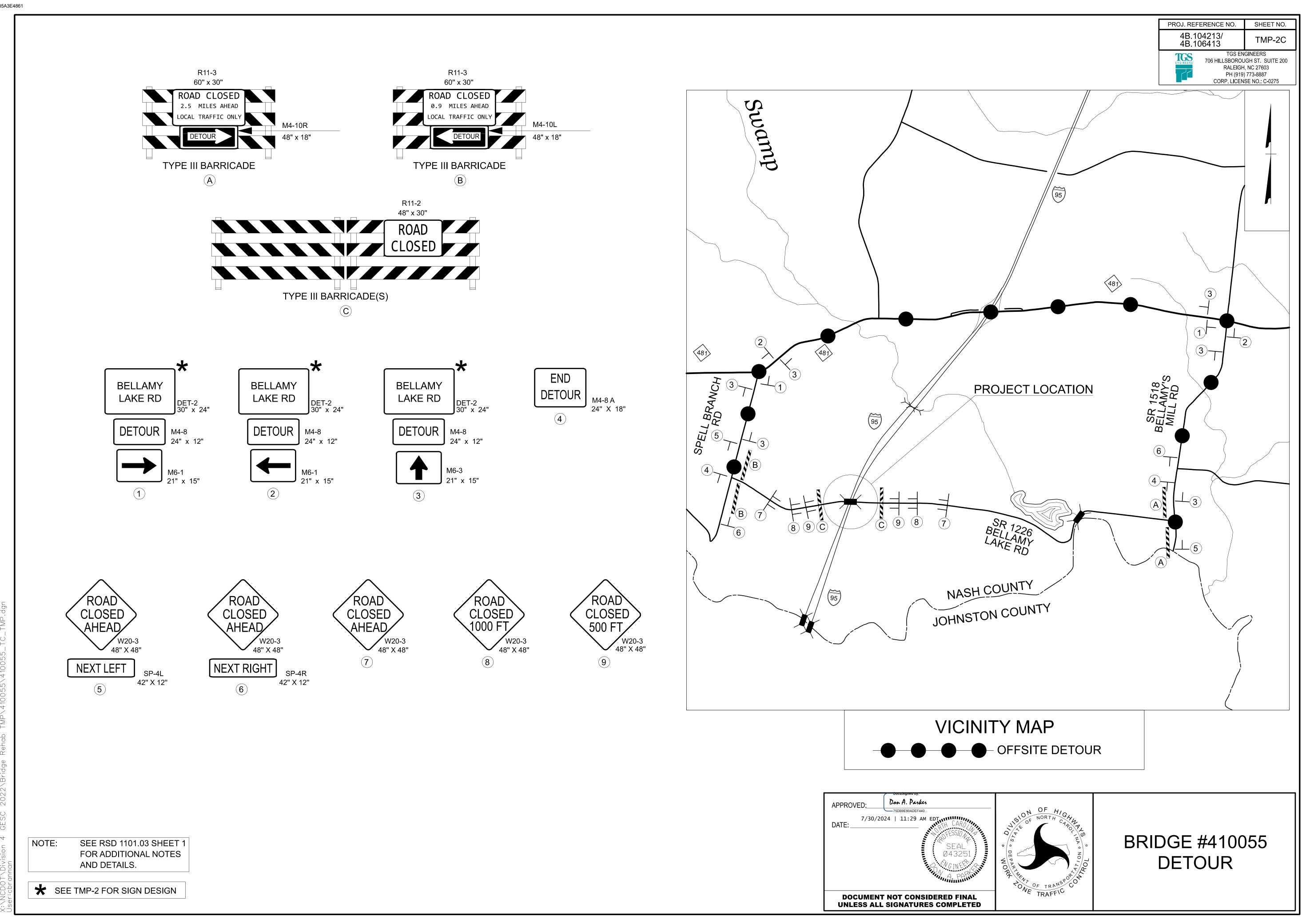
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	4B.106413	TMP-2
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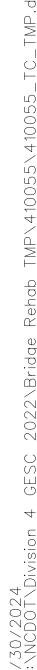


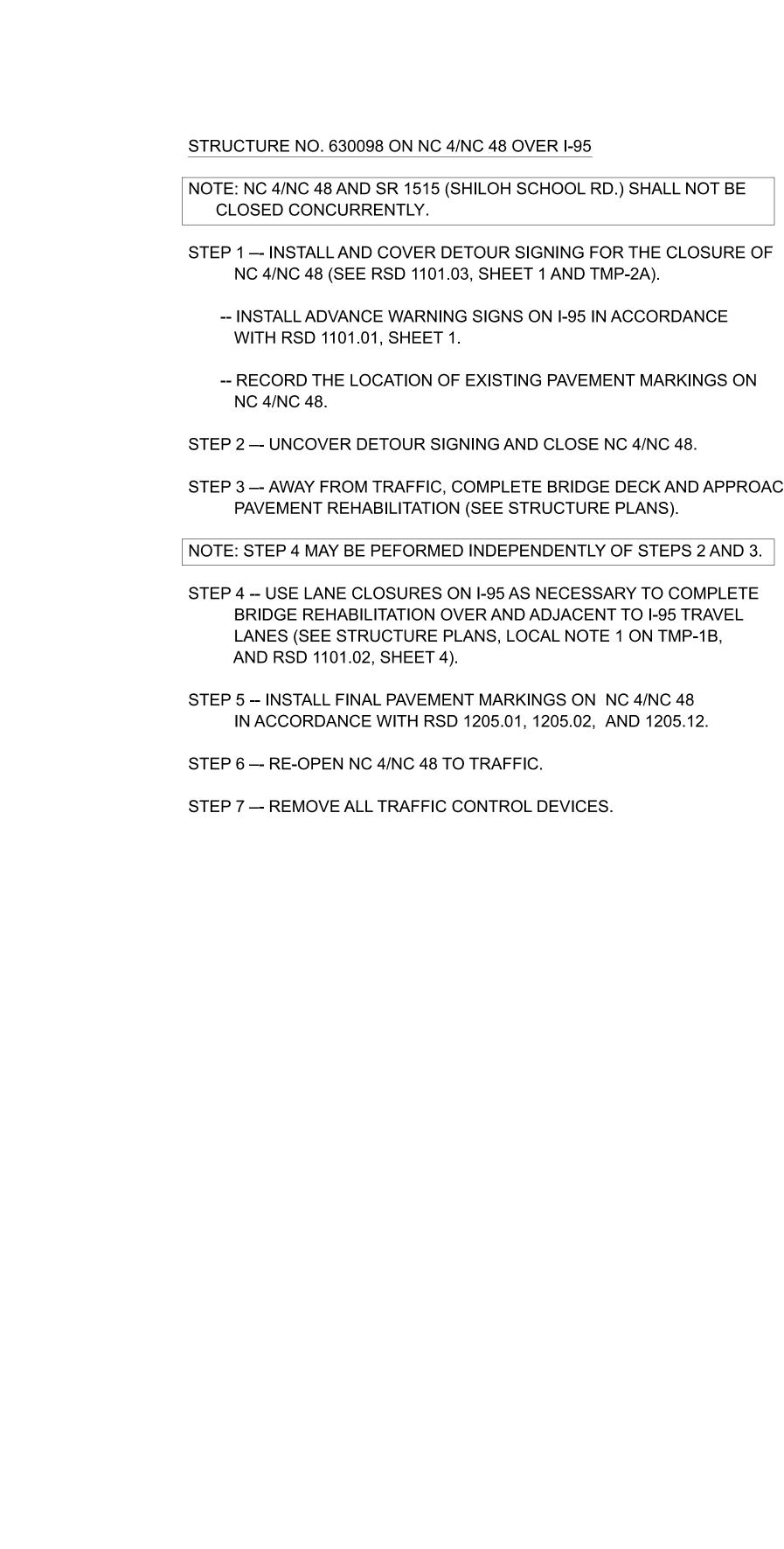












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PHASING

	STRUCTURE NO. 630225 ON SR 1515 (SHILOH SCHOOL RD.) OVER I-95	STRUCTURE NO. 41005
E	NOTE: SR 1515 (SHILOH SCHOOL RD.) AND NC 4/NC 48 SHALL NOT BE CLOSED CONCURRENTLY.	STEP 1 — INSTALL AND SR 1226 (BELLAM) AND TMP-2C).
EOF	STEP 1 INSTALL AND COVER DETOUR SIGNING FOR THE CLOSURE OF SR 1515 (SHILOH SCHOOL RD.) (SEE RSD 1101.03, SHEET 1 AND TMP-2B).	INSTALL ADVANCE WITH RSD 1101.01
ON	INSTALL ADVANCE WARNING SIGNS ON I-95 IN ACCORDANCE WITH RSD 1101.01, SHEET 1.	RECORD THE LOC SR 1226 (BELLAM)
	RECORD THE LOCATION OF EXISTING PAVEMENT MARKINGS ON SR 1515 (SHILOH SCHOOL RD.).	STEP 2 — UNCOVER DE (BELLAMY LAKE RI
ROACH	STEP 2 UNCOVER DETOUR SIGNING AND CLOSE SR 1515 (SHILOH SCHOOL RD.)	STEP 3 — AWAY FROM T PAVEMENT REHAE
D 3.	STEP 3 AWAY FROM TRAFFIC, COMPLETE BRIDGE DECK AND APPROACH PAVEMENT REHABILITATION (SEE STRUCTURE PLANS).	NOTE: STEP 4 MAY BE P
TE	NOTE: STEP 4 MAY BE PEFORMED INDEPENDENTLY OF STEPS 2 AND 3.	STEP 4 USE LANE CLO BRIDGE REHABILI LANES (SEE STRU
	STEP 4 USE LANE CLOSURES ON I-95 AS NECESSARY TO COMPLETE BRIDGE REHABILITATION OVER AND ADJACENT TO I-95 TRAVEL	AND RSD 1101.02,
	LANES (SEE STRUCTURE PLANS, LOCAL NOTE 1 ON TMP-1B, AND RSD 1101.02, SHEET 4).	STEP 5 INSTALL FINAL (BELLAMY LAKE R RSD 1205.01, 1205
	STEP 5 INSTALL FINAL PAVEMENT MARKINGS ON SR 1515 (SHILOH SCHOOL RD.) IN ACCORDANCE WITH RSD 1205.01, 1205.02, AND 1205.12.	STEP 6 — RE-OPEN SR
	STEP 6 RE-OPEN SR 1515 (SHILOH SCHOOL RD.) TO TRAFFIC.	STEP 7 REMOVE ALL

STEP 7 --- REMOVE ALL TRAFFIC CONTROL DEVICES.

APPROVE	Don A. Park	
DATE:	7/30/2024 11:29	AM EDT

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PROJ. REF	SHEET NO.		
4B.1 4B.1	TMP-3		
TGS	TGS EN		
ENGINEERS	706 HILLSBOROL	IGH ST. SUITE 200	
RALEIGH, NC 27603			
	PH (919)	773-8887	
		SE NO.: C-0275	

055 ON SR 1226 (BELLAMY LAKE RD.) OVER I-95

ND COVER DETOUR SIGNING FOR THE CLOSURE OF MY LAKE RD.) (SEE RSD 1101.03, SHEET 1

ICE WARNING SIGNS ON I-95 IN ACCORDANCE .01, SHEET 1.

OCATION OF EXISTING PAVEMENT MARKINGS ON MY LAKE RD.)

DETOUR SIGNING AND CLOSE SR 1226 RD.)

I TRAFFIC, COMPLETE BRIDGE DECK AND APPROACH ABILITATION (SEE STRUCTURE PLANS).

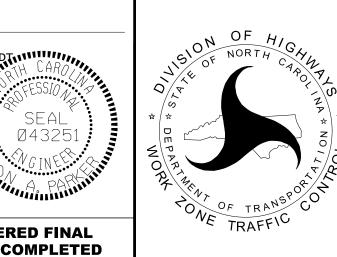
E PEFORMED INDEPENDENTLY OF STEPS 2 AND 3.

CLOSURES ON I-95 AS NECESSARY TO COMPLETE BILITATION OVER AND ADJACENT TO I-95 TRAVEL RUCTURE PLANS, LOCAL NOTE 1 ON TMP-1B, 02, SHEET 4).

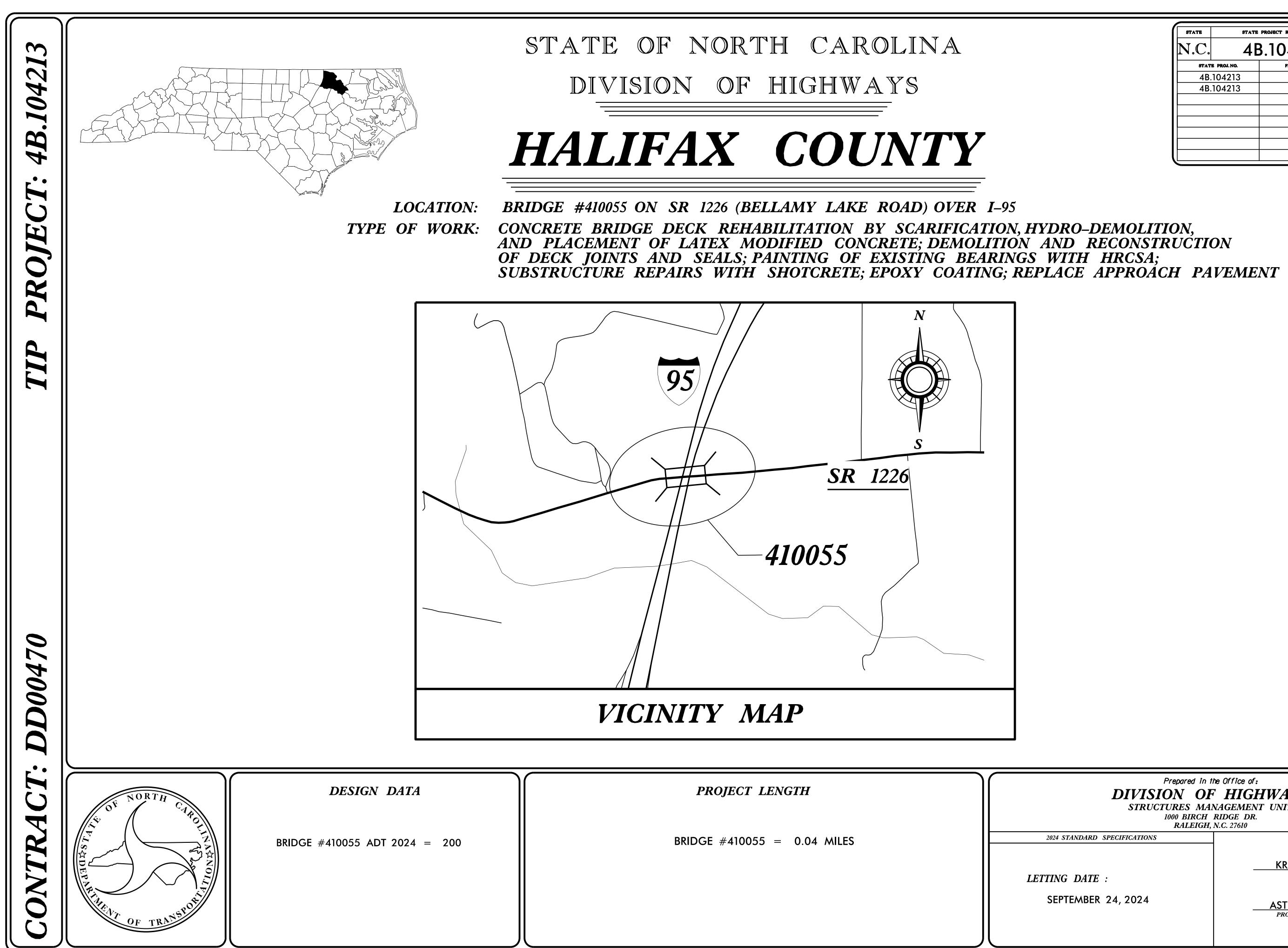
IAL PAVEMENT MARKINGS ON SR 1226 E RD.) IN ACCORDANCE WITH 205.02, AND 1205.12.

SR 1226 (BELLAMY LAKE RD.) TO TRAFFIC.

L TRAFFIC CONTROL DEVICES.

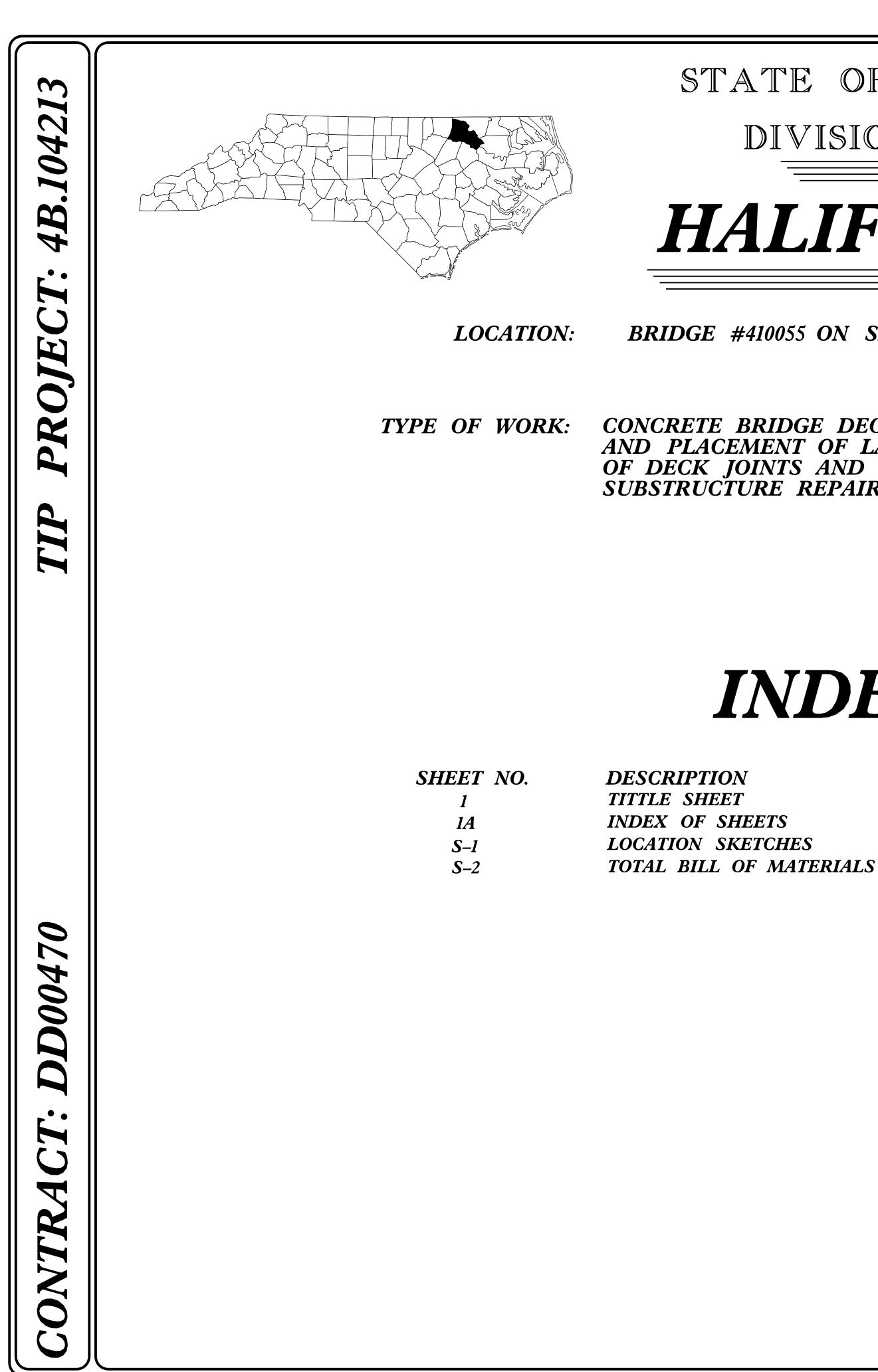






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4B.	104213	_		P.E.	
4B.	104213	_	CONST.		

Prepared in th DIVISION OF STRUCTURES MAN 1000 BIRCH RALEIGH,	HIGHWAYS NAGEMENT UNIT RIDGE DR.
NDARD SPECIFICATIONS	
DATE :	KRISTY ALFORD, PE PROJECT ENGINEER
MBER 24,2024	ASTER G. ABRAHA, PE PROJECT DESIGN ENGINEER



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

HALIFAX COUNTY

BRIDGE #410055 ON SR 1226 (BELLAMY LAKE ROAD) OVER I-95

CONCRETE BRIDGE DECK REHABILITATION BY SCARIFICATION, HYDRO-DEMOLITION, AND PLACEMENT OF LATEX MODIFIED CONCRETE; DEMOLITION AND RECONSTRUCTION OF DECK JOINTS AND SEALS; PAINTING OF EXISTING BEARINGS WITH HRCSA; SUBSTRUCTURE REPAIRS WITH SHOTCRETE; EPOXY COATING; REPLACE APPROACH PAVEMENT

INDEX OF SHEETS

SHEET NO. **STRUCTURE** *S1–1* **S1–**2 *S1–3 THRU S1–6 S1–7 THRU S1–10 S1–11 S1–12 S1–13 S1–14 S1–15 THRU S1–17 S1–18 S1–19* SN

DESCRIPTION #410055 GENERAL DRAWING TYPICAL SECTION DECK SURFACE REPAIRS DECK UNDERSIDE REPAIRS JOINT REPAIR DETAILS DECK REPAIR DETAILS **OVERHANG & DIAPHRAGM REPAIR DETAILS END BENT 1 & 2** BENTS TYPICAL CAP AND COLUMN REPAIR DETAILS APPROACH MILLING & TYPICAL ROADWAY SECTIONS STANDARD NOTES

		PROJECT REPERENCE NO.		SHEET	TOTAL
state N.C.	state 4E	NO.	SHEETS		
STAT	e proj. No.	F. A. PROJ. NO.		DESCRIPT	10N
4B.	.104213	_	P.E.		
4B.	.104213	_	CONST.		Т.



LOCATION SKETCH

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. THE CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING THE BRIDGE, ROADWAYS, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

DRAWN BY :	Q. T. NGUYEN	DATE : 05/2024
CHECKED BY :	F. LEA	DATE : 05/2024

+

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6/18/2024 S:\DPG1\Division4\Halifax 55 & Nash 98 & 225\Plans\400_sheets\Halifax_4B.104213_410055\400_005_4B.104213_SMU_LS_S-01_410055.dgn wcsmith

BRIDGE COORDINATES					
BRIDGE No.	LATITUDE	LONGITUDE			
410055	36°-09′-23.34″	77°-46′-58.00″			

	PROJECT H BRIDGE	IALIF	AX		UNTY
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DocuSigned by William () Smith 054816 6A2A92833F62470 MCNEER MCNEER MCNEER 07/08/2024	LC	DCAT	TON S	KETCI	4
		REVIS	SIONS		SHEET NO.
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FINAL UNLESS ALL	1		3		TOTAL SHEETS
SIGNATURES COMPLETED	2		4		3

GENERAL NOTES:

+

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THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT DUE TO THE NATURE OF PRESERVATION PROJECTS, THE EXTENT OF WORK CANNOT ALWAYS BE ACCURATELY DETERMINED PRIOR TO COMMENCEMENT OF WORK. REPAIR LOCATIONS AND ESTIMATES QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, T ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION THE REPAIRS.

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT FOR DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN WHAT IS SH ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

WORK ON THE BRIDGE(S) SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW, EXCEPT WHERE THE CONTRACTOR'S PLAN USES PLATFORMS, NETS, SCREENS OR OTHER PROTECTIVE DEVICES TO CATCH THE MATERIAL. THE CONTRACTOR SHALL SUBMI PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PERFORM ALL WORK WITH CARE SO THAT THE EXISTING STRUC WHICH IS TO REMAIN IN PLACE WILL NOT BE DAMAGED. IF THE CONTRACTOR DAMAGES PART OF THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE, THE DAMAGED AREA SHALL BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE ENGINEER AT NO ADDITIONAL COST TO THE DEPARTMENT.

ANY DAMAGE TO EXISTING REINFORCING STEEL, DURING CONTRACTOR'S OPERATIONS, SI BE REPAIRED AS DIRECTED BY THE ENGINEER AND PERFORMED AT NO ADDITIONAL COST THE DEPARTMENT.

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE ELSEWHERE THE CONTRACT DOCUMENTS.

PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROV COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR PAVEMENT MARKING SEE ELSEWHERE IN THE CONTRACT DOCUMENTS.

THE ELEVATIONS AND CLEARANCES SHOWN ON THE PLANS AT THE POINT OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION(S) ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

	TOTAL BILL OF MATERIAL									
BRIDGE NO.	INCIDENTAL MILLING	ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B	ASPHALT BINDER FOR PLANT MIX	GROOVING BRIDGE FLOORS	POLLUTIC CONTRO		MODIFIED	PLACING & FINISHING LATEX MODIFIED CONCRETE OVERLAY	SHOTCRETE REPAIRS	PAINTING CONTAINMENT FOR BRIDGE NO
	SQ. YDS.	TONS	TONS	SQ. FT.	LUMP SU	M SQ. YD.	CU. YD.	SQ. YD.	CU. FT.	LUMP SUM
410055	288.1	30	5	4,806.1	LUMP SU	M 63.3	31.2	605.3	27.4	LUMP SUM
BRIDGE NO.	POURABLE SILICONE JOINT SEALANT	EPOXY COATING	ELASTOMERIO CONCRETE FOR PRESERVATIO	JOIN	IT	HYDRO- DEMOLITION OF BRIDGE DECK	SCARIFYING BRIDGE DECK	CLEANING PAINTING EXIS BEARINGS WITH RATIO CALCI SULFONAT	STING H HIGH IUM	
	LN. FT.	SQ. FT.	CU. FT.	SQ.	FT.	SQ. YD.	SQ. YD.	EA.		
410055	95.0	350.8	15.9	69.6	5	605.3	605.3	24		

RAWN BY :	Q. T. NGUYEN	DATE :	05/20
HECKED BY	W. C. SMITH	DATE :	06/20

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	FOR MAII	NTENANCE AND PROTECTION OF TRAFFIC BENEATH	+ PROPOSED
OF HE	THE CON	G JOINTS AND DECK DRAINS SHALL BE SEALED PRIC TRACTOR SHALL TAKE CARE THAT ANY CONSTRUC N SHOULDERS OF ADJACENT TRAVEL LANES SHALL	TION DEBRIS
ON OF	FOR BRIE	OGE JOINT DEMOLITION, SEE SPECIAL PROVISIONS.	
	FOR CLE	ANING AND PAINTING EXISTING BEARINGS WITH HI	RCSA, SEE SP
	FOR ELAS	STOMERIC CONCRETE FOR PRESERVATION, SEE SPI	ECIAL PROVIS
ANY IOWN		ITING CONTAINMENT AND POLLUTION CONTROL, S PROVISION.	EE "CLEANING
	FOR SHO	TCRETE REPAIRS, SEE SPECIAL PROVISIONS.	
	FOR CON	ICRETE REPAIRS, SEE SPECIAL PROVISIONS.	
	FOR VOL	UMETRIC MIXER, SEE SPECIAL PROVISIONS.	
κ Γ	FOR ANC	HOR BOLT REPAIRS, SEE SPECIAL PROVISIONS.	
	FOR EPO	XY RESIN INJECTION, SEE SPECIAL PROVISIONS.	
CTURE ANY		RIFYING BRIDGE DECK, CLASS II SURFACE PREPARA , SEE "LMC OVERLAY SURFACE PREPARATION" SPEC	
)		CONSTRUCTION, APPROPRIATE MEASURES SHALL E MIGRATE INTO ACTIVE TRAVEL LANES.	BE USED TO E
HALL F TO	REQUIRE NECESSA	IME OF PREPARATION OF THESE PLANS, IT WAS NO D. HOWEVER, IT MAY BE DETERMINED IN THE FIEL RY TO PROPERLY COMPLETE THE INTENDED BRIDO	D THAT THE F SE PRESERVAT
EIN	EXTRA W SPECIAL	ARED TO PERFORM SUCH WORK IN A TIMELY MANN ORK AND SHALL BE ADDRESSED AS PER ARTICLE PROVISIONS THAT OUTLINE REQUIREMENTS FOR T JECT DOCUMENTS,BUT NO QUANTITIES HAVE BEEN	104-7 OF THE HESE POTENT
VAL A		IRED, IF EXTRA WORK IS ENCOUNTERED. UNANTIC	
-	ITEM	DESCRIPTION	UNIT
	1.	CLASS III SURFACE PREPARATION	SQ. YDS.
	2. 3.	CONCRETE REPAIR EPOXY RESIN INJECTION	CU. FT. LN. FT.
	9. 4.	ANCHOR BOLT REPAIR	EA.
	5.	VOLUMETRIC MIXER	LS

STRUCTURE, SEE SPECIAL PROVISIONS.

NING SURFACE PREPARATIONS OF THE BRIDGE DECK. THAT COLLECTS IN THE DRAINS IS CONTAINED. EE AND CLEAR OF DEBRIS.

PECIAL PROVISIONS.

SIONS.

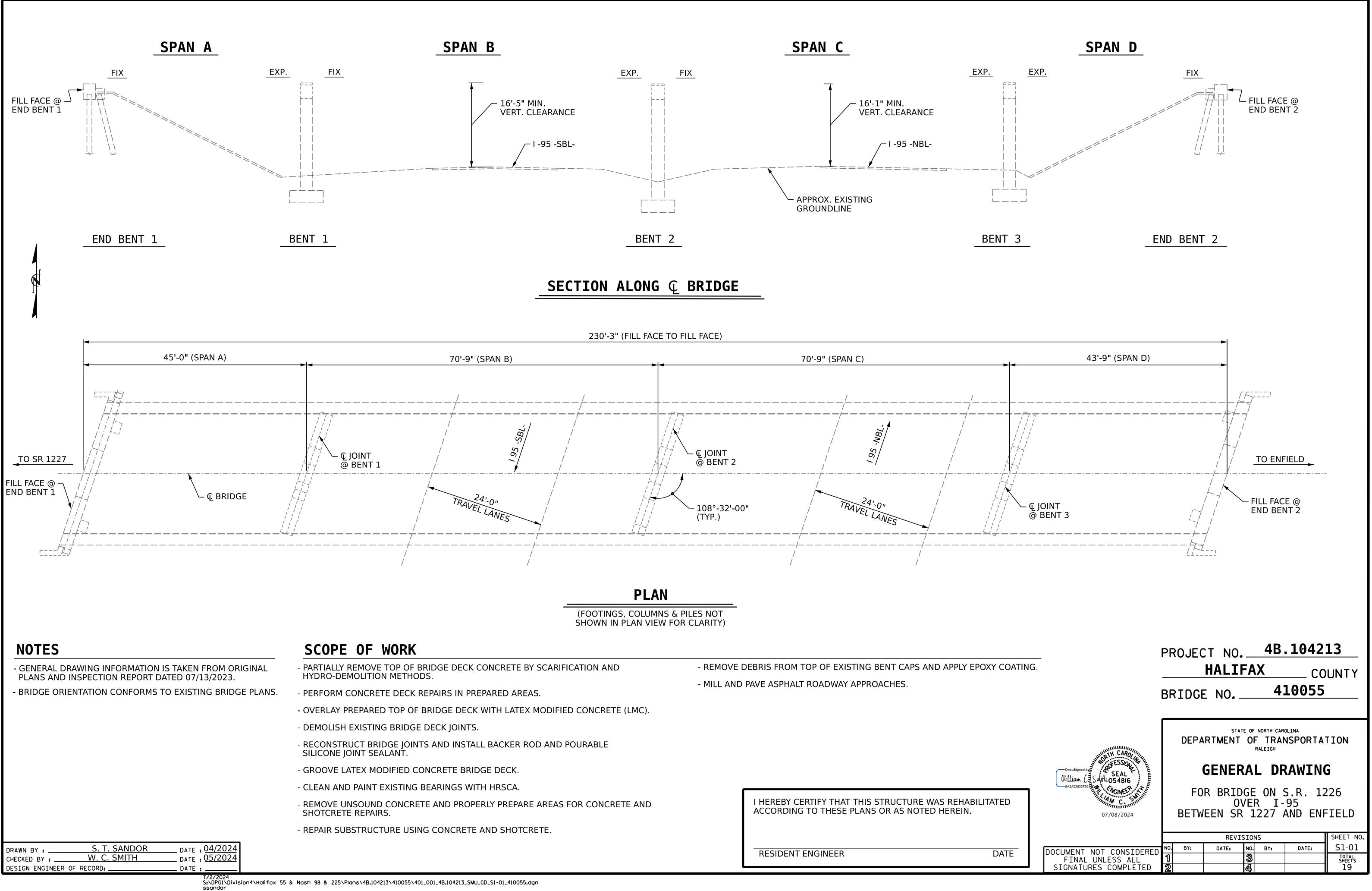
IG AND PAINTING EXISTING BEARINGS WITH HRCSA"

III SURFACE PREPARATION, AND HYDRO-DEMOLITION ON.

ENSURE THAT HYDRO-DEMOLITION WATER DOES NOT

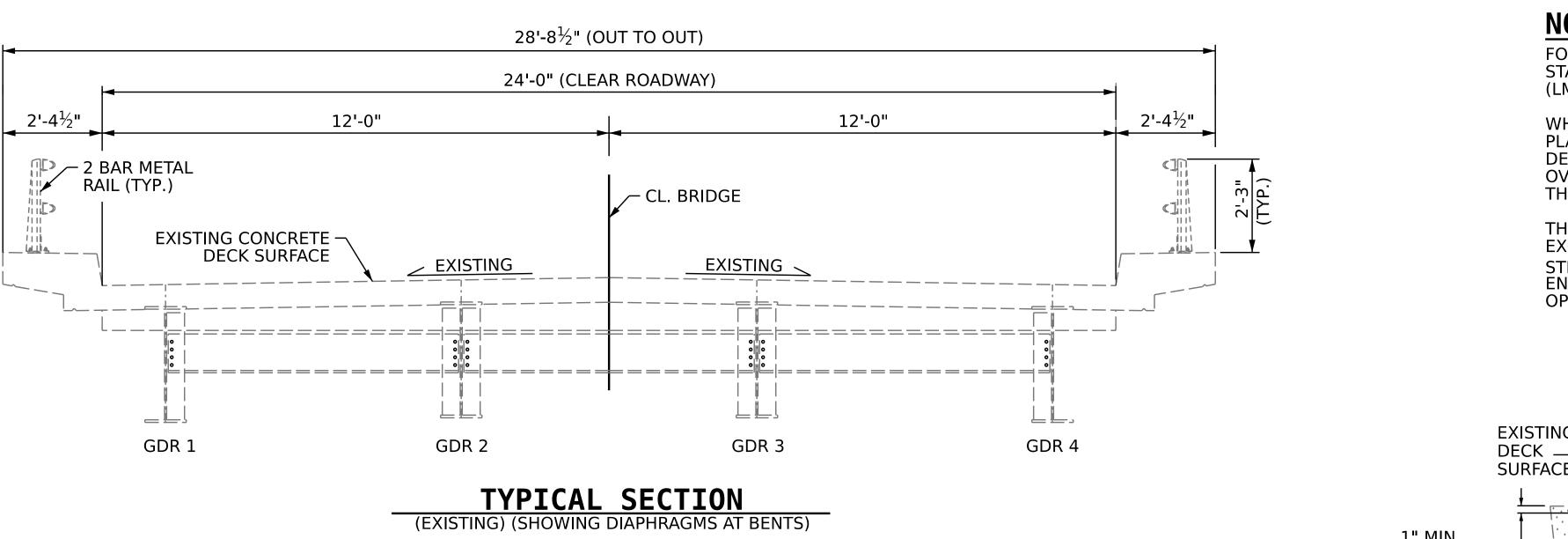
ED THAT THE FOLLOWING ITEM(S) LISTED WOULD BE FOLLOWING ITEM(S) LISTED, OR OTHER WORK WILL BE TION/REHABILITATION WORK. THE CONTRACTOR SHALL RMINED IN THE FIELD. SUCH WORK SHALL BE CONSIDERED STANDARD SPECIFICATIONS. PROJECT TIAL ADDITIONAL WORK ITEMS HAVE BEEN PROVIDED IN TUAL PAY ITEMS, QUANTITIES, AND COSTS WILL BE ESTABLISHED,

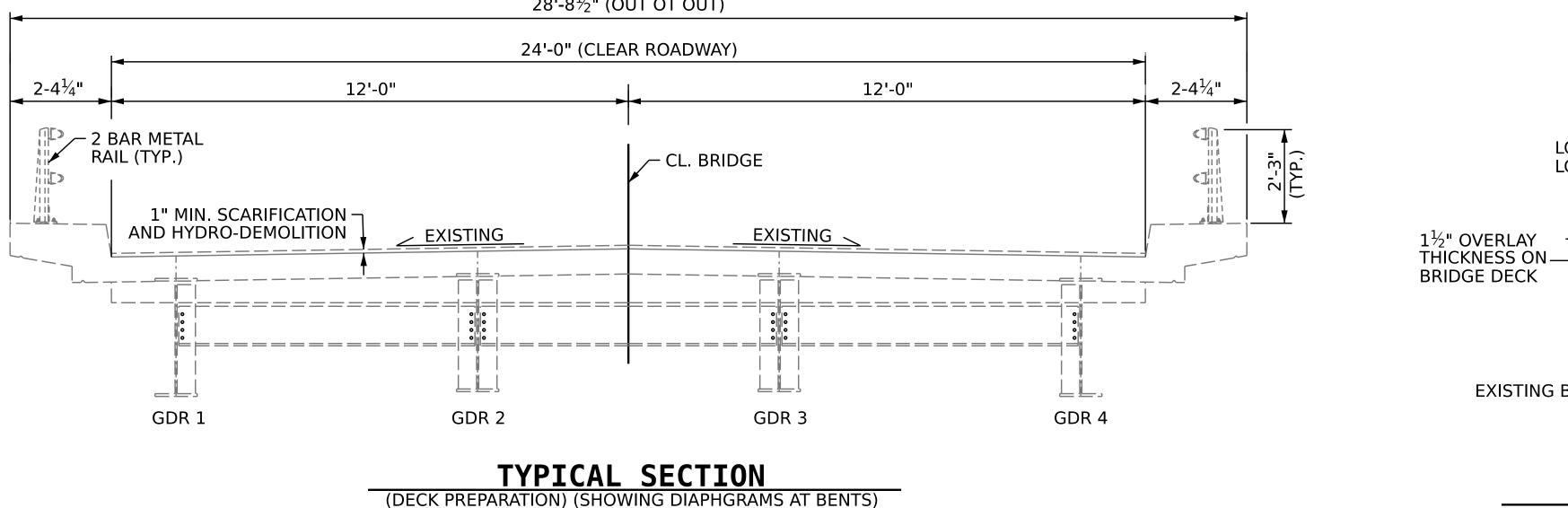
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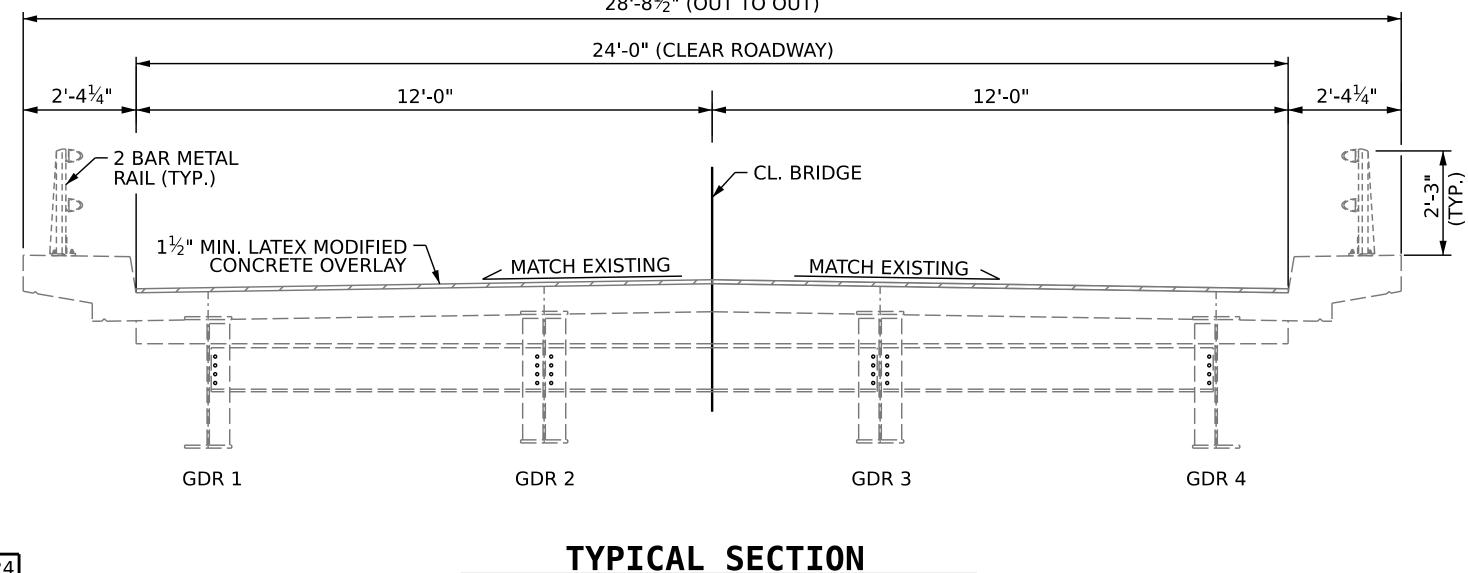


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CHECKED BY :	F.	LEA DA	re : 05/2024
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1" MIN. SCARIFICATION & HYDRO-DEMOLITION

$28'-8\frac{1}{2}"$ (OUT OT OUT)

28'-8¹⁄₂" (OUT TO OUT)

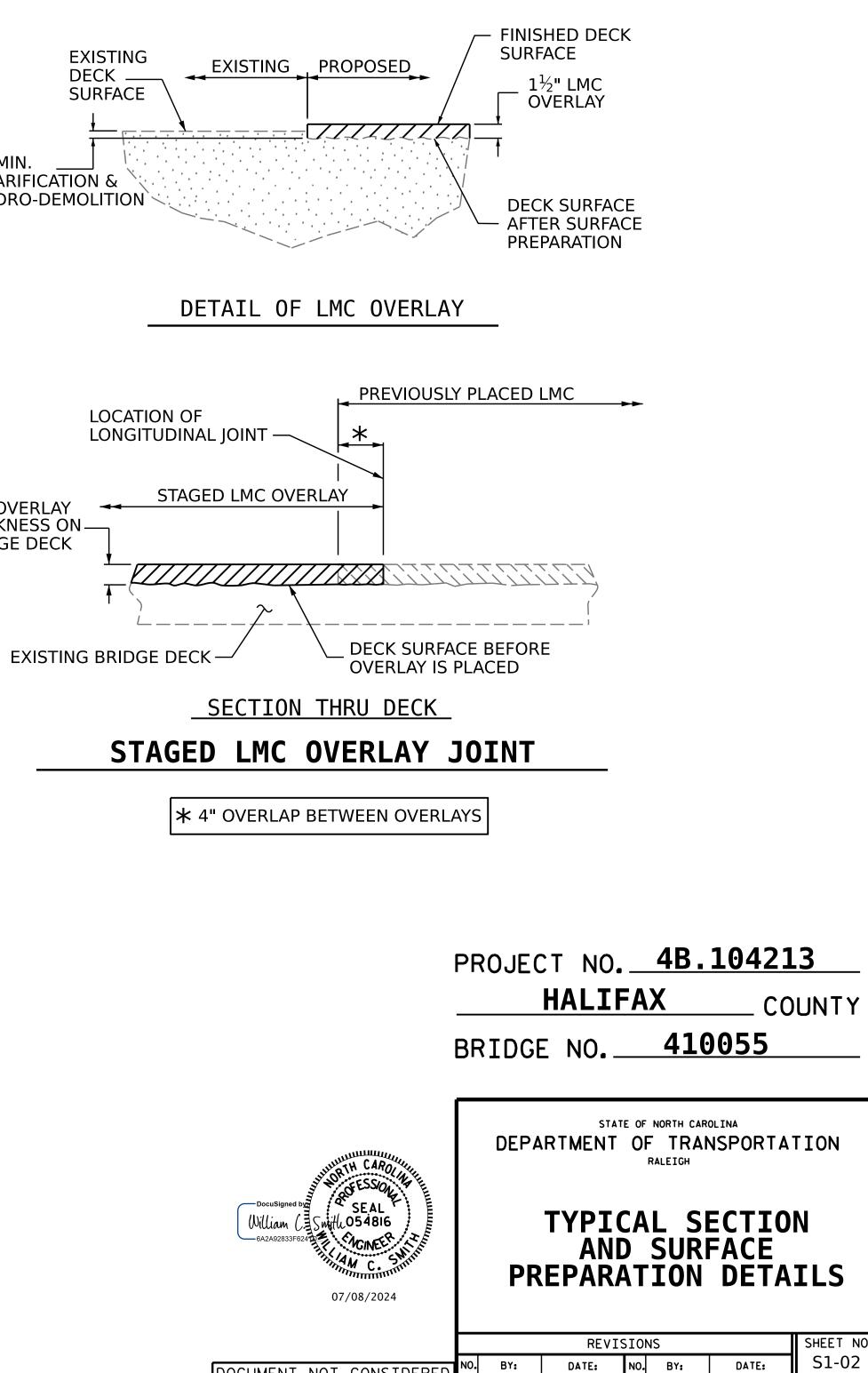
(PROPOSED) (SHOWING DIAPHRAGMS AT BENTS)

NOTES

FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF OVERLAY SURFACE PREPARATION AND LATEX MODIFIED CONCRETE (LMC) PLACEMENT, SEE ELSEWHERE IN THE CONTRACT DOCUMENTS.

WHEN PREPARING THE SURFACE FOR LMC OVERLAY ADJACENT TO THE PREVIOUSLY PLACED LMC STAGE, THE PREVIOUSLY PLACED LMC SHALL BE SAW-CUT TO THE FULL DEPTH OF THE LMC AT THE CENTERLINE OF THE BRIDGE AND ALL LMC IN THE 4" OVERLAP SHALL BE REMOVED WITH HAND TOOLS PRIOR TO PLACEMENT OF LMC IN THE SECOND STAGE.

THE CONTRACTOR IS ALERTED TO THE FACT THAT THERE ARE MANY AREAS IN THE EXISTING BRIDGE DECK WHERE CONCRETE COVER OVER THE TOP MAT OF REINFORCING STEEL IS APPROXIMATELY $\frac{1}{2}$ ". APPROPRIATE CARE AND MEASURES SHALL BE TAKEN TO ENSURE THAT REINFORCING BARS ARE NOT DAMAGED DURING SURFACE PREPARATION OPERATIONS.

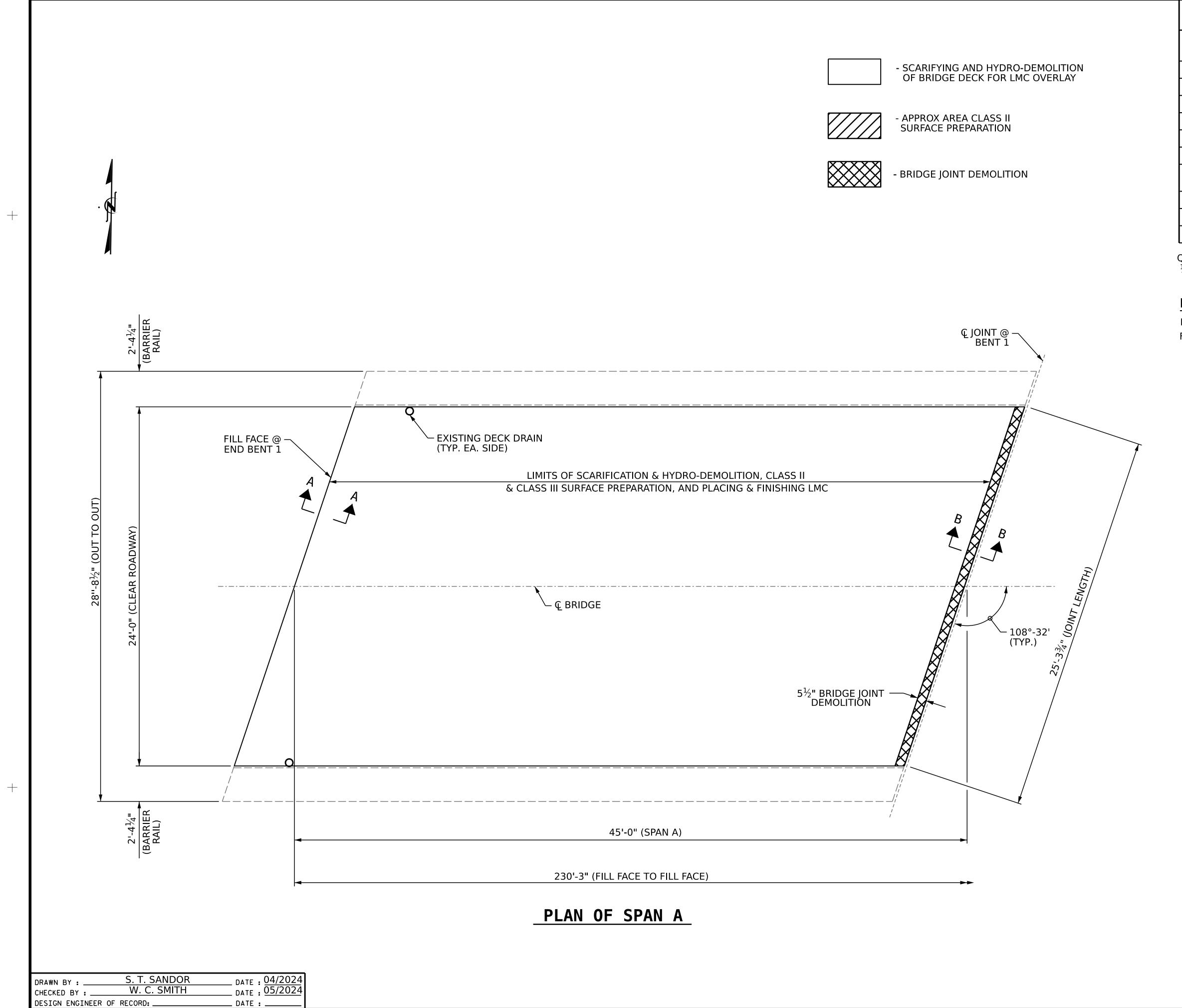


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DECK SURFACE REPAIR QUANTITY TABLE

DECK SURFACE REPAIR - SPAN A

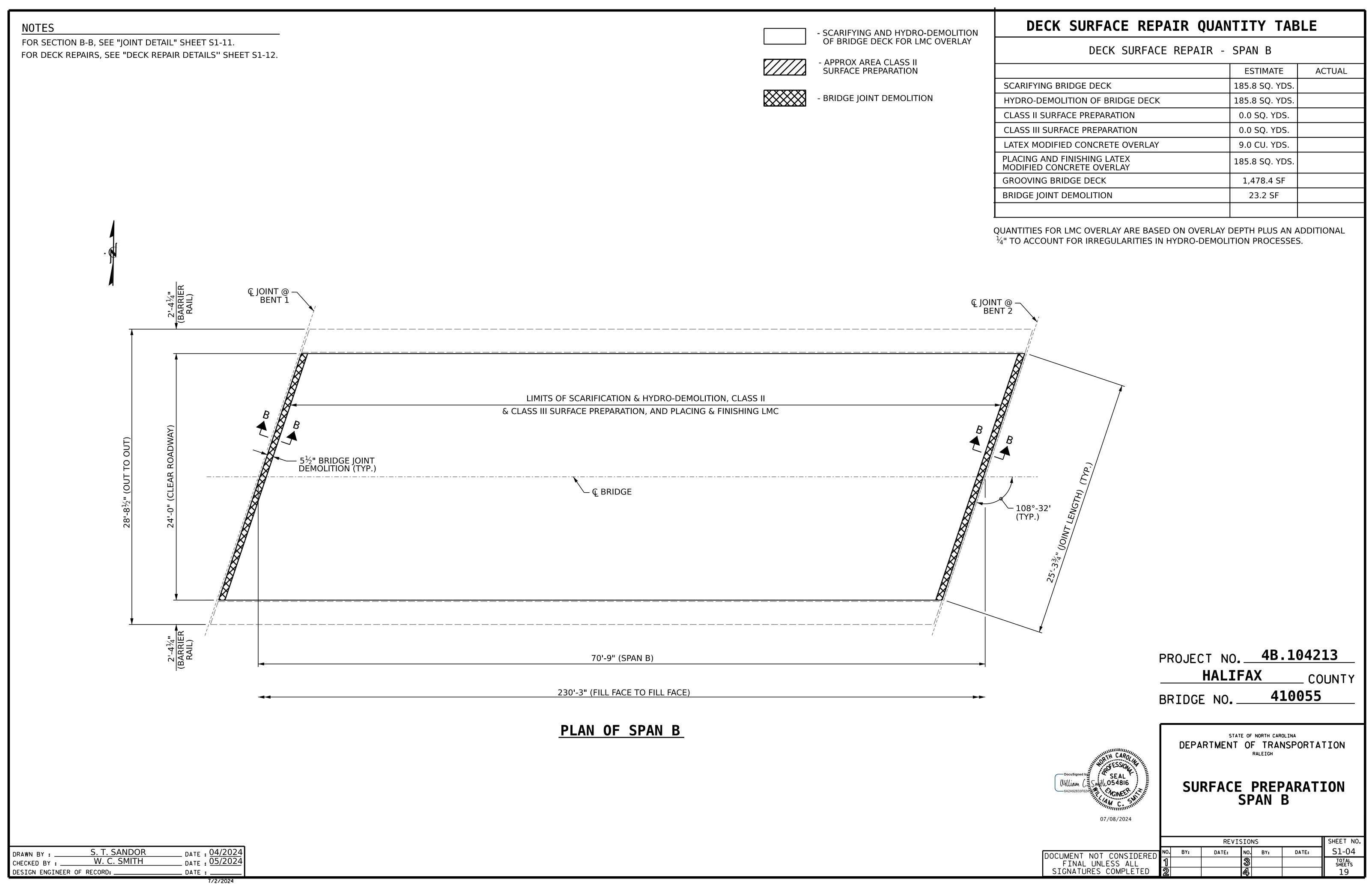
	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	118.6 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	118.6 SQ. YDS.	
CLASS II SURFACE PREPARATION	0.0 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	5.8 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	118.6 SQ. YDS.	
GROOVING BRIDGE DECK	937.6 SF	
BRIDGE JOINT DEMOLITION	11.6 SF	

QUANTITIES FOR LMC OVERLAY ARE BASED ON OVERLAY DEPTH PLUS AN ADDITIONAL $\frac{1}{4}$ " TO ACCOUNT FOR IRREGULARITIES IN HYDRO-DEMOLITION PROCESSES.

NOTES

FOR SECTIONS A-A AND B-B, SEE "JOINT DETAIL" SHEET S1-11. FOR DECK REPAIRS, SEE "DECK REPAIR DETAILS" SHEET S1-12.

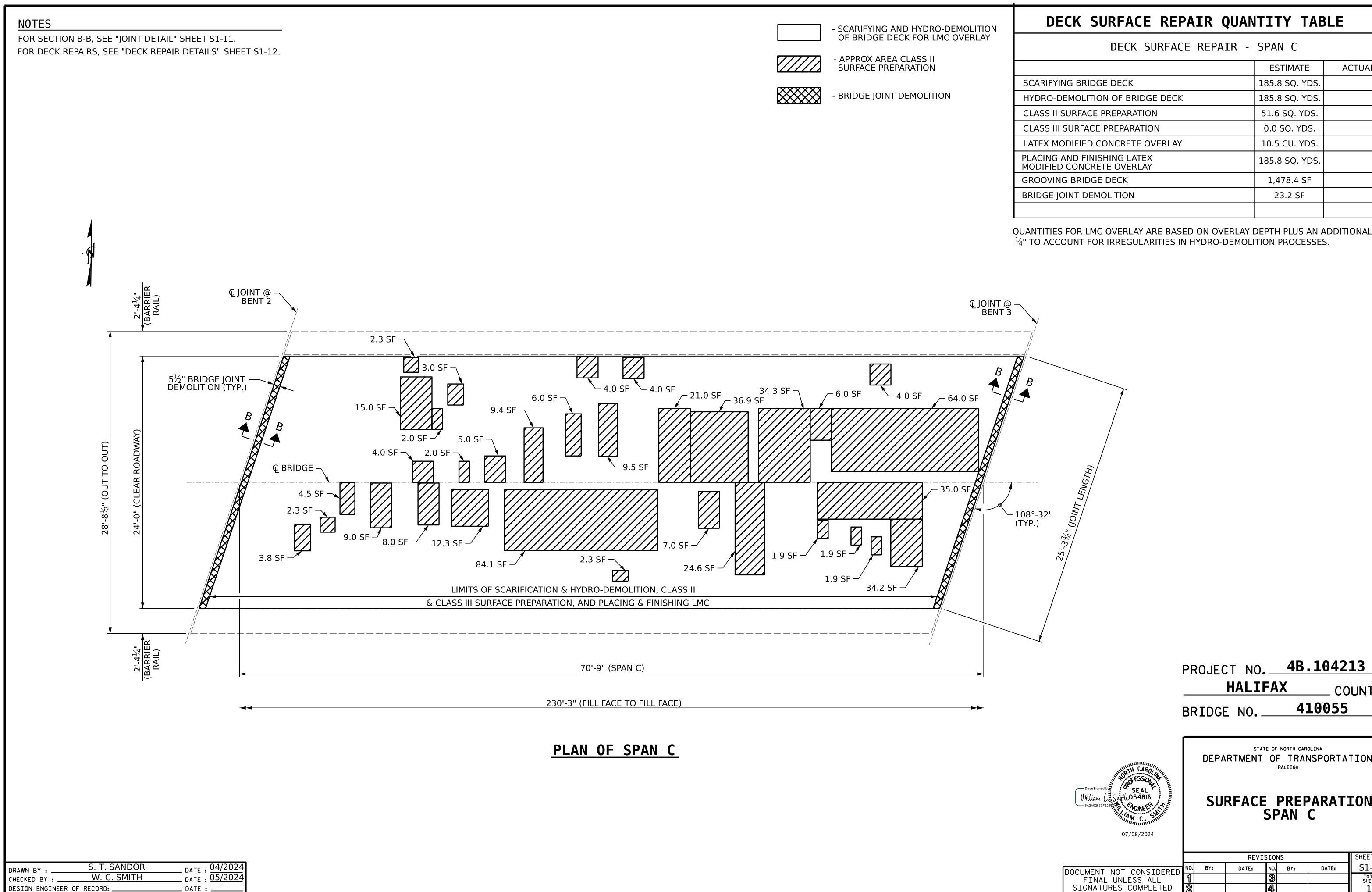
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WRTH CAROLINE		OF NORTH CAROLINA OF TRANSPORTA RALEIGH	TION
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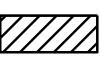
	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	185.8 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	185.8 SQ. YDS.	
CLASS II SURFACE PREPARATION	0.0 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	9.0 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	185.8 SQ. YDS.	
GROOVING BRIDGE DECK	1,478.4 SF	
BRIDGE JOINT DEMOLITION	23.2 SF	



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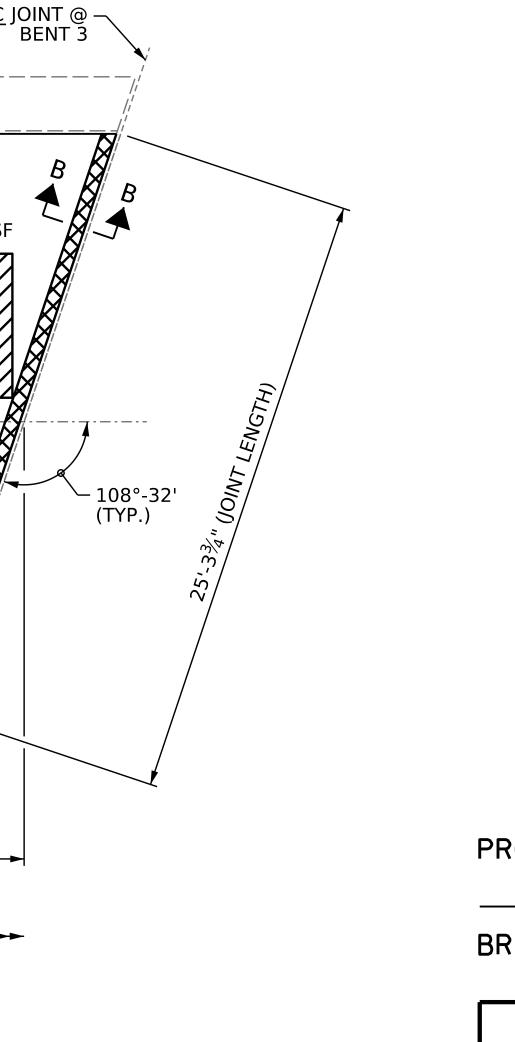
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DECK SURFACE REPAIR QUANTITY TABLE

DECK SURFACE REPAIR - SPAN C

	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	185.8 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	185.8 SQ. YDS.	
CLASS II SURFACE PREPARATION	51.6 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	10.5 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	185.8 SQ. YDS.	
GROOVING BRIDGE DECK	1,478.4 SF	
BRIDGE JOINT DEMOLITION	23.2 SF	

QUANTITIES FOR LMC OVERLAY ARE BASED ON OVERLAY DEPTH PLUS AN ADDITIONAL $\frac{1}{4}$ " TO ACCOUNT FOR IRREGULARITIES IN HYDRO-DEMOLITION PROCESSES.

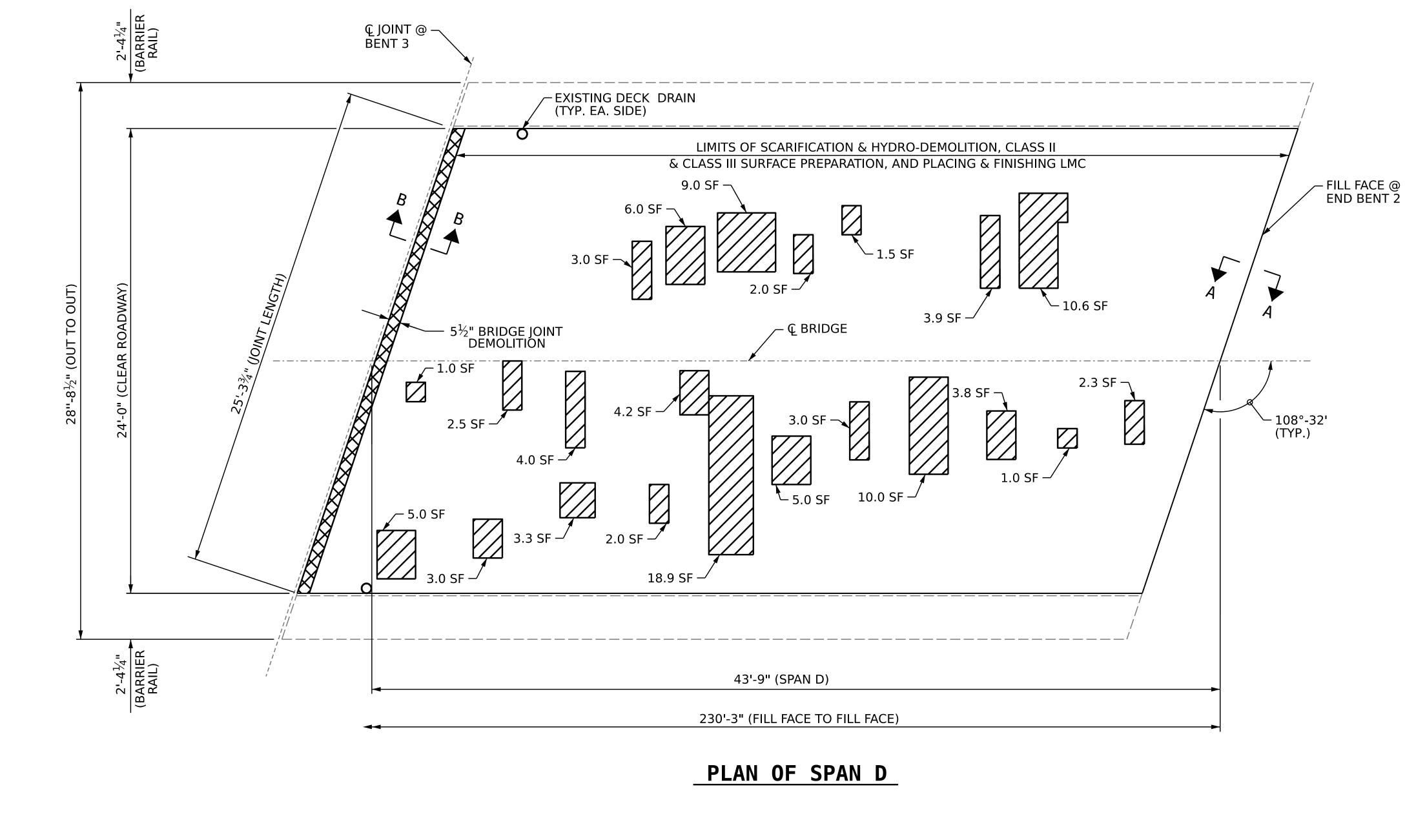
	HALIFAX	COUNTY
	BRIDGE NO	410055
TH CAROLAND	STATE OF NORT	RANSPORTATION
SEAL O54816 MCNEER	SURFACE PR SPAN	
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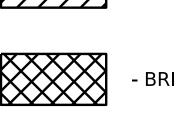
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FOR SECTIONS A-A AND B-B, SEE "JOINT DETAIL" SHEET S1-11. FOR DECK REPAIRS, SEE "DECK REPAIR DETAILS" SHEET S1-12.



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CHECKED BY :	W. C. SMITH	DATE : 05/2024
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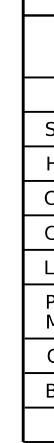
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- BRIDGE JOINT DEMOLITION

- APPROX AREA CLASS II SURFACE PREPARATION

- SCARIFYING AND HYDRO-DEMOLITION OF BRIDGE DECK FOR LMC OVERLAY



DECK SURFACE REPAIR QUANTITY TABLE

DECK SURFACE REPAIR - SPAN D

	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	115.1 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	115.1 SQ. YDS.	
CLASS II SURFACE PREPARATION	11.7 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	5.9 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	115.1 SQ. YDS.	
GROOVING BRIDGE DECK	911.7 SF	
BRIDGE JOINT DEMOLITION	11.6 SF	

QUANTITIES FOR LMC OVERLAY ARE BASED ON OVERLAY DEPTH PLUS AN ADDITIONAL $\frac{1}{4}$ " TO ACCOUNT FOR IRREGULARITIES IN HYDRO-DEMOLITION PROCESSES.

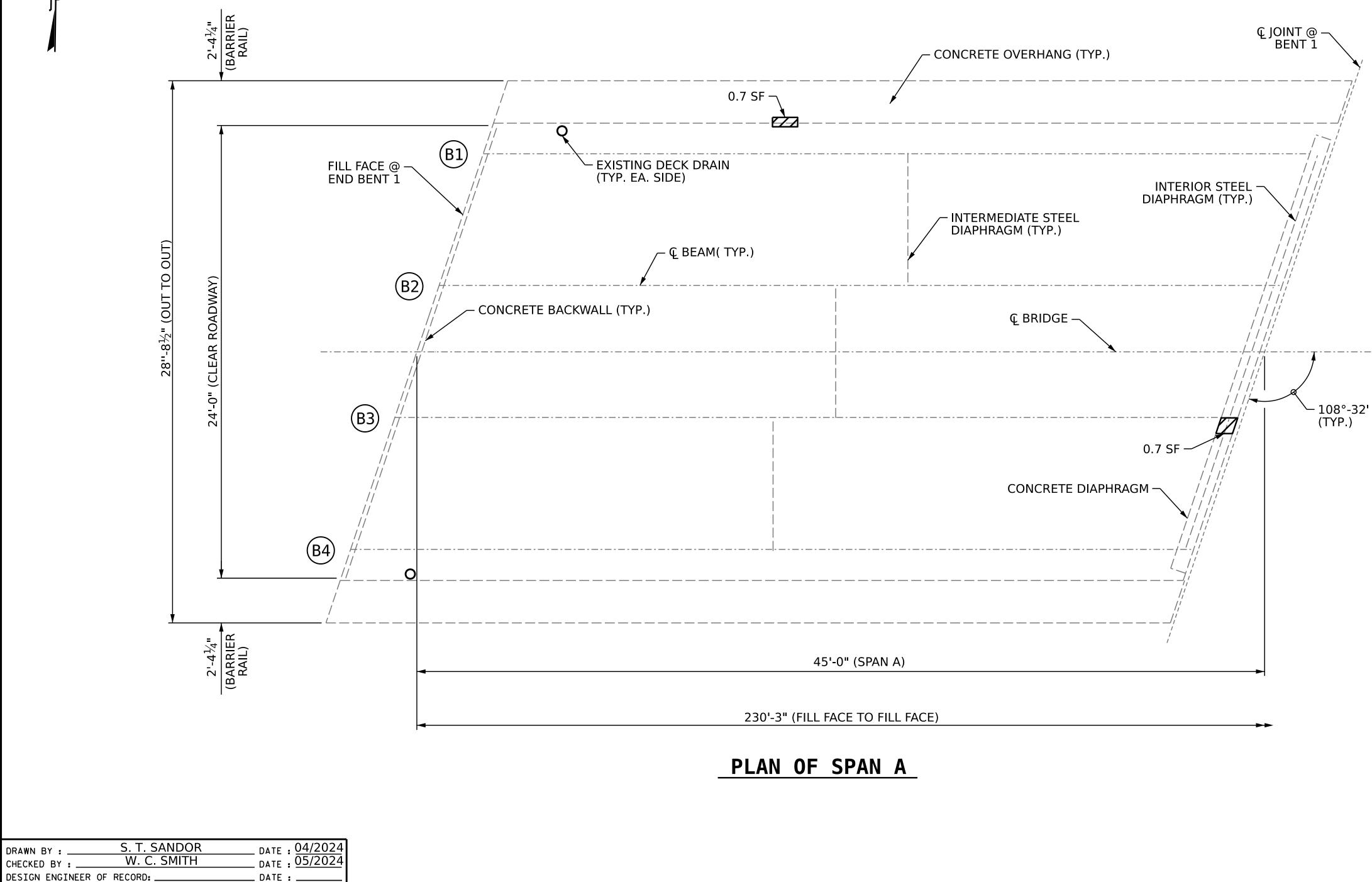
	PROJECT NO. <u>4B.104213</u> <u>HALIFAX</u> COUNTY BRIDGE NO. <u>410055</u>
WRTH CAROLINE	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH
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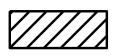
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REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

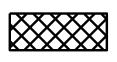
CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.



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SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



BEAM NUMBER



DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE			
DECK UNDERSIDE REPAIRS		QUAN	TITIES	TIES			
SPAN A	ESTI	МАТЕ	ACT	UAL			
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
UNDERSIDE OF DECK	0	0					
CONCRETE DIAPHRAGM	0.7	0.4					
OVERHANG	0.7	0.4					
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
UNDERSIDE OF DECK	0	0					
CONCRETE DIAPHRAGM	0	0					
OVERHANG	0	0					

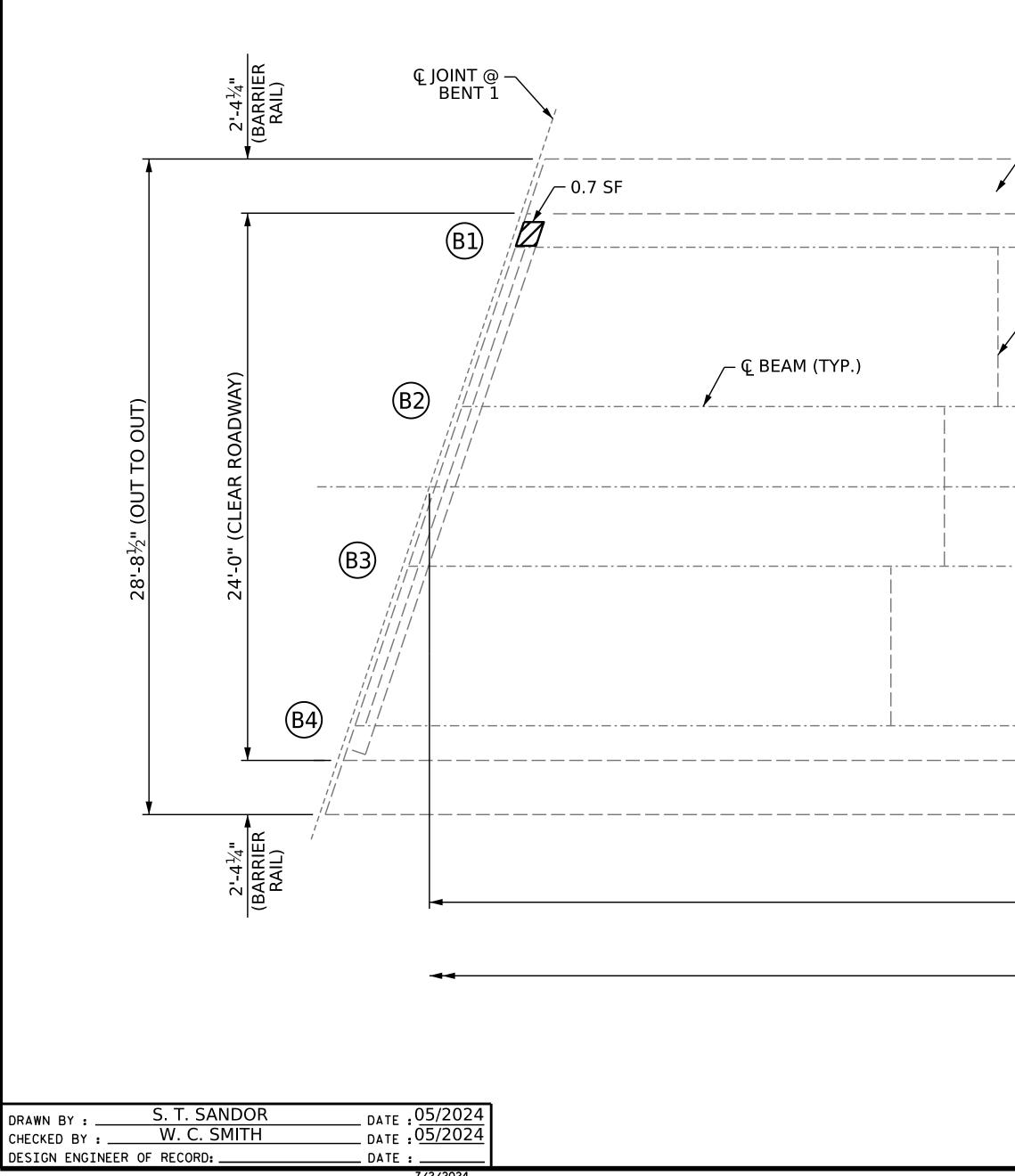
	PROJECT NO HALIFA BRIDGE NO	
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William (5 Smith 054816 6A2A92833F624) 44 C. Smith 44 C. Smith 07/08/2024		DE REPAIRS PAN A
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REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CONCRETE REPAIRS MAY BE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

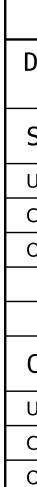


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PLAN OF SPAN B

230'-3" (FILL FACE TO FILL FACE)

CONCRETE OVERHANG (TYP.)		
– INTERMEDIATE STEEL DIAPHRAGM (TYP.)		INTERIOR STEEL – DIAPHRAGM (TYP.)
Ç BRIDGE		
	,	CONCRETE DIAPHRAGM - ///////////////////////////////////
70'-9" (SPAN B)		,



SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

BEAM NUMBER

B#

€ JOINT @ ·

DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE		
DECK UNDERSIDE REPAIRS		QUAN	ITIES			
SPAN B	ESTI	ИАТЕ	ACT	TIES ACTUAL AREA VOLUME SF CF		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF				
UNDERSIDE OF DECK	0	0				
CONCRETE DIAPHRAGM	0.7	0.4				
OVERHANG	0	0				
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
UNDERSIDE OF DECK	0	0				
CONCRETE DIAPHRAGM	0	0				
OVERHANG	0	0				

BENT 2 ____ _ _ _

------ 108°-32' (TYP.)

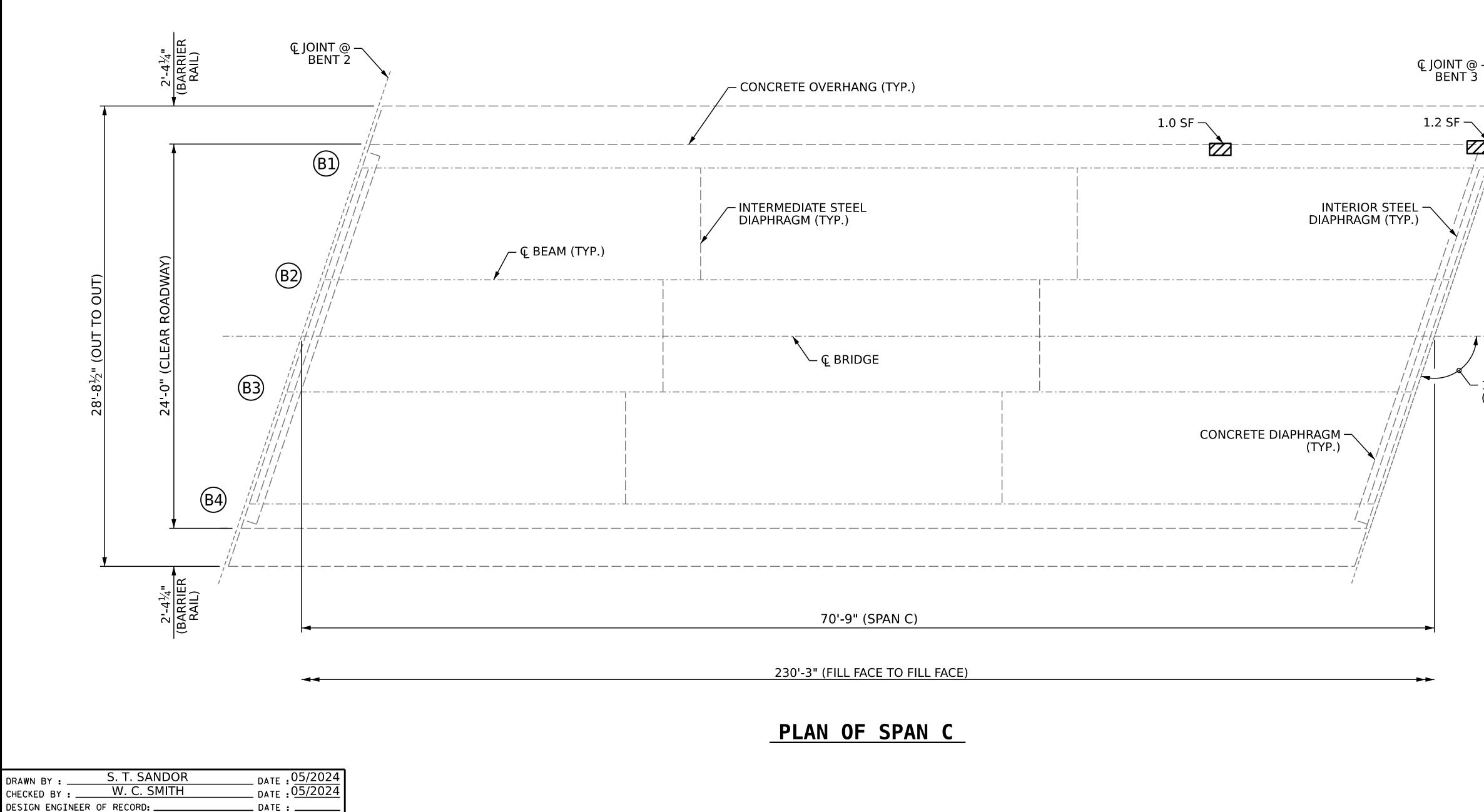
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REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CONCRETE REPAIRS MAY BE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.



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BENT 3



SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA

B# BEAM NUMBER

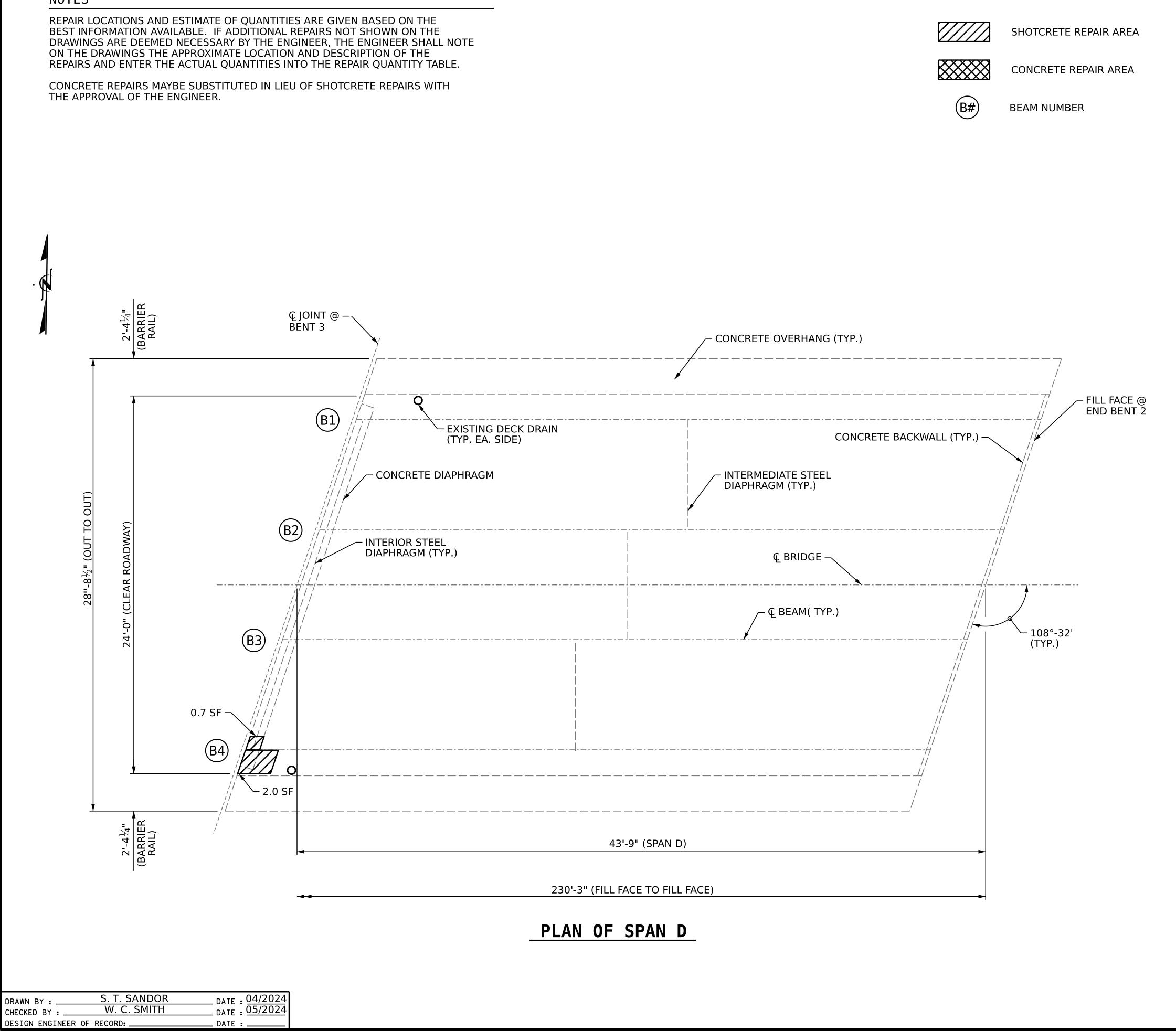
DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE
DECK UNDERSIDE REPAIRS		QUAN	TITIES	
SPAN C	ESTI	МАТЕ	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	2.2	1.1		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		

- 108°-32' (TYP.)

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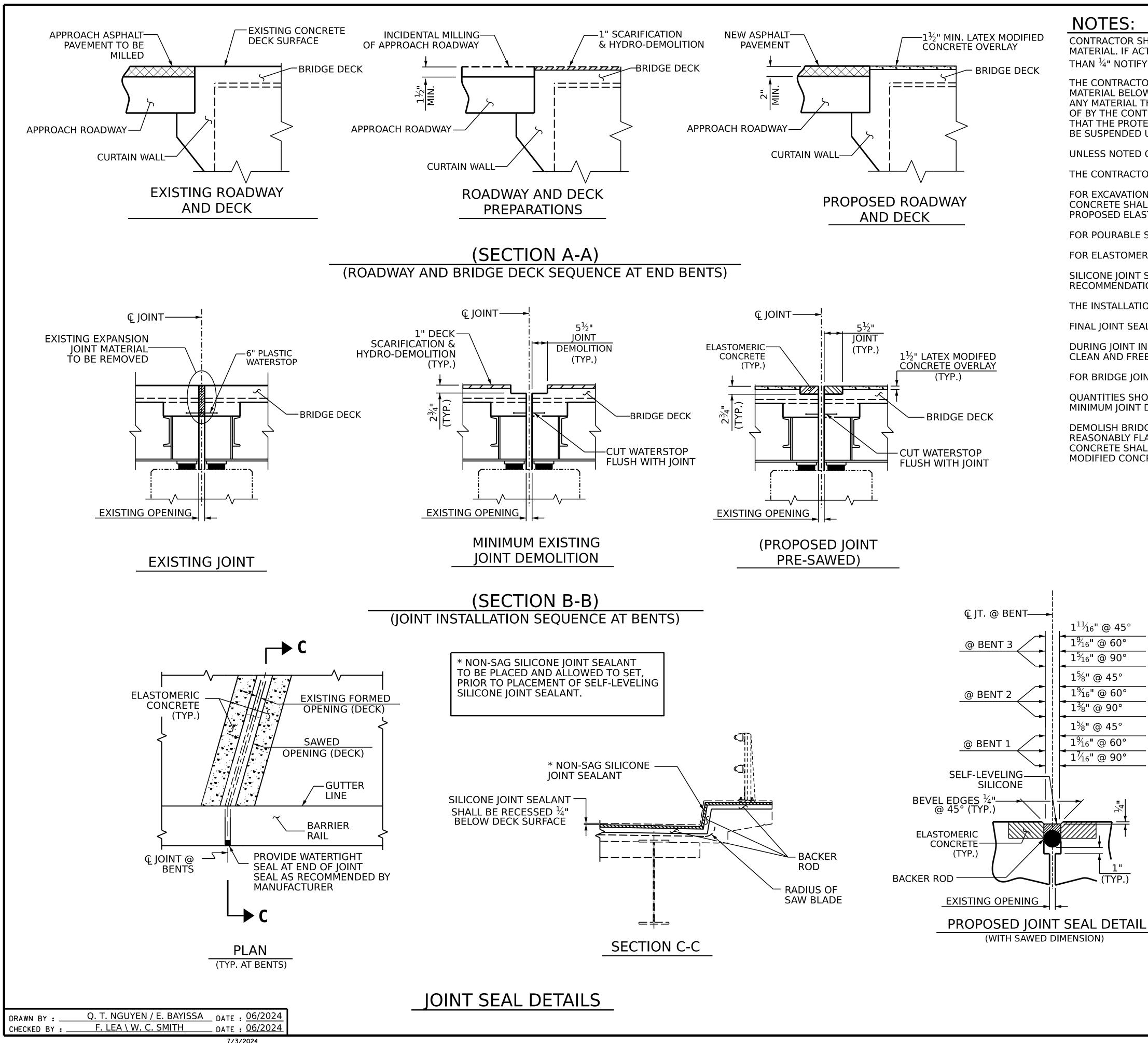
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DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE			
DECK UNDERSIDE REPAIRS		QUANTITIES					
SPAN D	ESTIN	ИАТЕ	ACT	UAL			
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
UNDERSIDE OF DECK	2.0	1.0					
CONCRETE DIAPHRAGM	0.7	0.4					
OVERHANG	0	0					
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
UNDERSIDE OF DECK	0	0					
CONCRETE DIAPHRAGM	0	0					
OVERHANG	0	0					

	PROJEC	HALIF	AX		UNTY
DocuSigned by William () Smith 054816 6A2A92833F624THILLIAN C. SMITHING		RTMENT	RALEIGH	NSPORTA	
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CONTRACTOR SHALL FIELD VERIFY THE EXISTING FORMED OPENING PRIOR TO OBTAINING JOINT MATERIAL. IF ACTUAL JOINT OPENINGS VARIES FROM THE OPENING INDICATED IN DETAIL MORE THAN $\frac{1}{4}$ " NOTIFY ENGINEER. REVISION TO THE JOINT SEAL SIZE MAY BE NECESSARY.

THE CONTRACTOR SHALL TAKE CARE DURING JOINT REHAB OPERATIONS NOT TO DROP ANY MATERIAL BELOW THE BRIDGE WITHOUT PROTECTIVE DEVICES BELOW TO CATCH THE MATERIAL. ANY MATERIAL THAT FALLS BELOW THE BRIDGE SHALL BE CONTAINED, REMOVED AND DISPOSED OF BY THE CONTRACTOR AT NO EXTRA COST TO THE DEPARTMENT. IF THE ENGINEER DETERMINES THAT THE PROTECTIVE DEVICES ARE NOT ADEQUATE OR NOT BEING EMPLOYED, THE WORK SHALL BE SUSPENDED UNTIL ADEQUATE PROTECTION IS PROVIDED.

UNLESS NOTED OTHERWISE RETAIN ALL EXISTING REINFORCING STEEL. CLEAN AND REPAIR AS NEEDED.

THE CONTRACTOR WILL NOT BE PERMITTED TO FORM THE JOINTS IN LIEU OF SAWING THE JOINT.

FOR EXCAVATION BELOW THE BOTTOM OF THE PLANNED JOINT DEMOLITION, APPROVED REPAIR CONCRETE SHALL BE PLACED IN THE EXCAVATED AREA TO THE ELEVATION AT BOTTOM OF THE PROPOSED ELASTOMERIC CONCRETE FOR PRESERVATION HEADERS SHOWN.

FOR POURABLE SILICONE JOINT SEALANT, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC CONCRETE FOR PRESERVATION, SEE SPECIAL PROVISIONS.

RECOMMENDATION.

THE INSTALLATION OF JOINT SEAL SHALL BE WATERTIGHT.

FINAL JOINT SEALS SHALL NOT BE INSTALLED UNTIL THE OVERLAY IS COMPLETE.

DURING JOINT INSTALLATION PROCEDURE, THE JOINT AND SURROUNDING AREA SHALL BE KEPT CLEAN AND FREE OF DEBRIS.

FOR BRIDGE JOINT DEMOLITION, SEE SPECIAL PROVISIONS.

QUANTITIES SHOWN IN THE ELASTOMERIC CONCRETE FOR PRESERVATION ARE BASED ON THE MINIMUM JOINT DEMOLITION SHOWN.

DEMOLISH BRIDGE JOINT AREA SUCH THAT THE BOTTOM OF THE EXCAVATION SHALL BE REASONABLY FLAT AND LEVEL AND TO THE NECESSARY DEPTH. SUCH THAT ELASTOMERIC CONCRETE SHALL BE FOUNDED ON CONCRETE OR REPAIR CONCRETE SUBSTRATE, NOT LATEX MODIFIED CONCRETE.

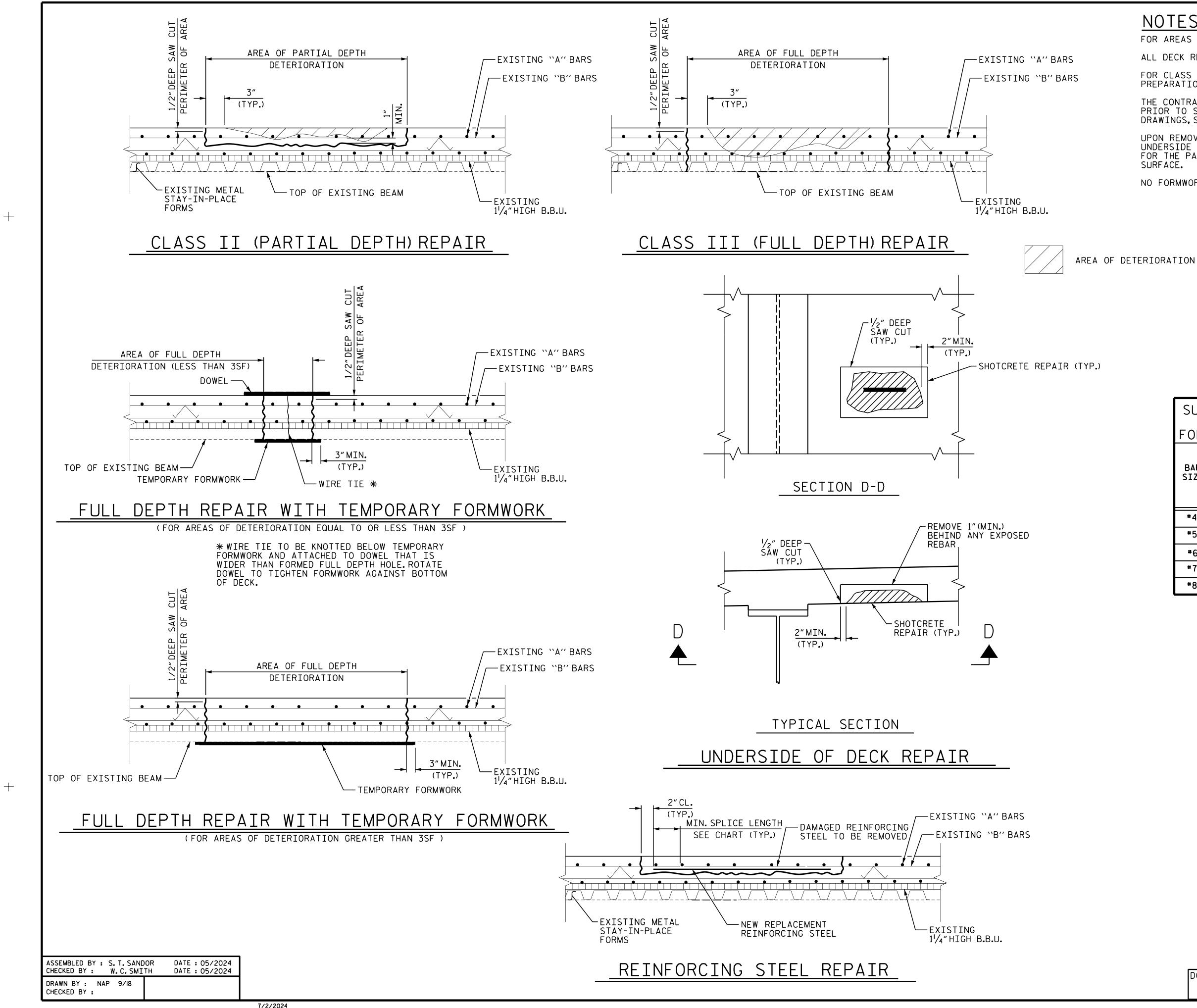
SILICONE JOINT SEALANT AND BACKER ROD SHALL BE INSTALLED AS PER MANUFACTURER'S

SUMMARY OF QUANTITIES			
	ESTIMATE	ACTUAL	
ELASTOMERIC CONCRETE FOR PRESERVATION	15.9 CF		
POURABLE SILICONE JOINT SEALANT	95.0 LF		

	HALIF	AX	COUNTY
	BRIDGE NO.	4100)55
NUMBERSSION AND THE RESSION		E OF NORTH CAROLINA OF TRANSPO RALEIGH	RTATION
SEAL 054816 William Market String 6A2A92833	JOINT F	Repair de	ETAILS

PROJECT NO. 48.104213

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DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	S1-11
FINAL UNLESS ALL	1		3		TOTAL SHEETS
SIGNATURES COMPLETED	2		4		19



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NOTES

FOR AREAS TO BE REPAIRED, SEE "PLAN OF SPAN" SHEETS.

ALL DECK REPAIRS SHALL BE COMPLETED PRIOR TO PLACEMENT OF OVERLAY.

FOR CLASS II AND CLASS III SURFACE PREPARATION, SEE "OVERLAY SURFACE PREPARATIONS" SPECIAL PROVISION.

THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO STARTING WORK FOR TEMPORARY FORMWORK.FOR SUBMITTALS OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

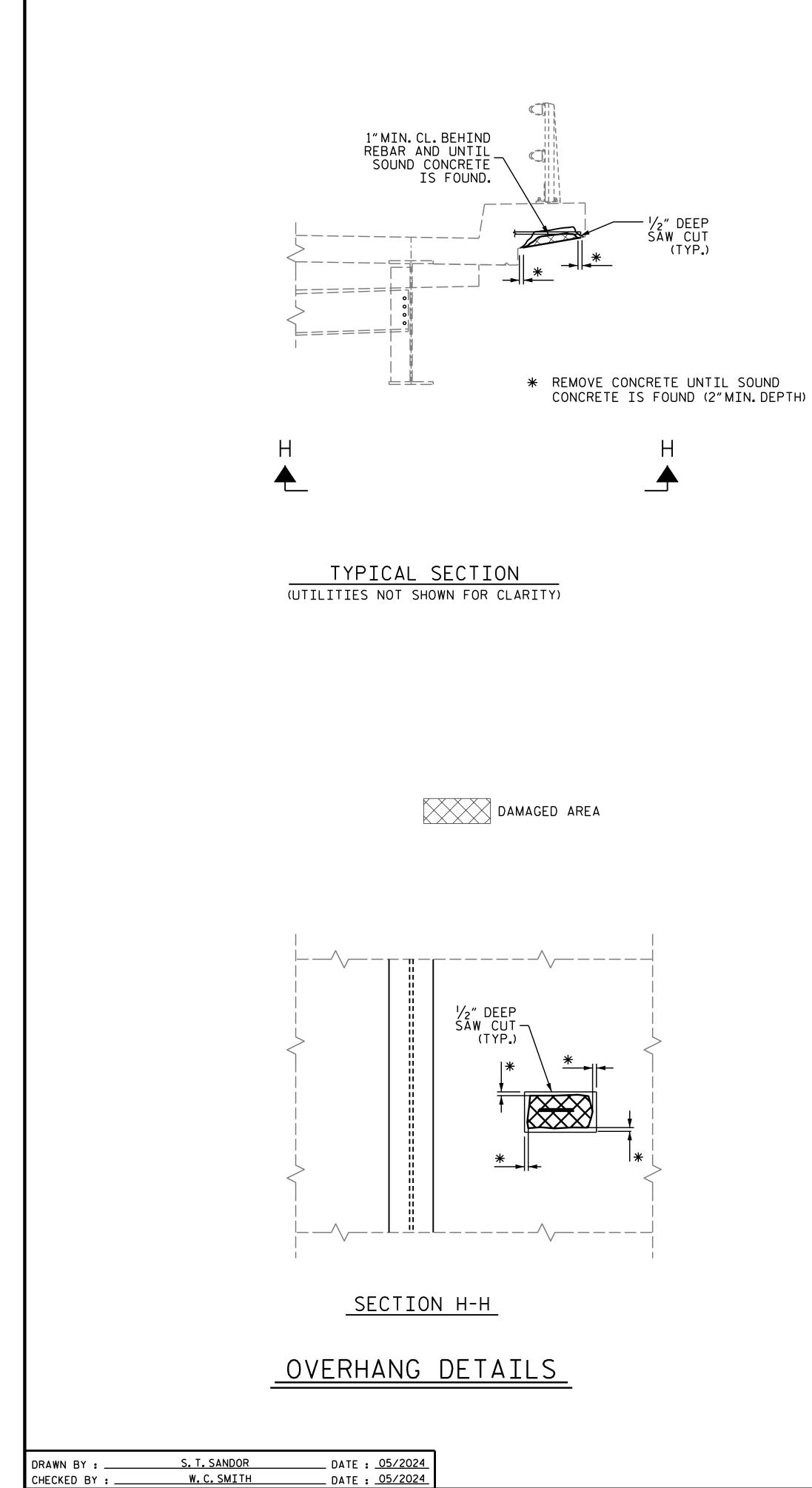
UPON REMOVAL OF TEMPORARY FORMWORK, ALL VOIDS AND HONEYCOMBS ON THE UNDERSIDE OF DECK SURFACE SHALL BE FILLED WITH THE SAME MATERIAL AS USED FOR THE PATCH, AND FINISHED TO CONFORM TO THE SURROUNDING CONCRETE SURFACE.

NO FORMWORK SHALL BE LEFT IN PLACE.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS						
BAR SIZE AND BARRIER RAIL			APPROACH SLABS		PARAPET AND BARRIER	
	EPOXY COATED	UNCOATED	EPOXY COATED UNCOATED		RAIL	
# 4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"	
* 5	2'-5"	2'-0"	2′-5″	2'-0"	3'-1"	
* 6	2'-10"	2'-5"	3'-7"	2'-5"	3′-8″	
# 7	4'-2"	2'-9"				
* 8	4'-9"	3'-2"				

	PROJECT NO. <u>4B</u> .	.104213
	HALIFAX	COUNTY
	BRIDGE NO. <u>41</u> C	055
OFESSION SEAL	STATE OF NORTH CARC DEPARTMENT OF TRAN RALEIGH STANDAR	SPORTATION
Docusioned by 054816 William F Schoon E Structure 6A2A92833 Structure MAN C. Structure	DECK REPAIR	DETAILS
07/08/2024		

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THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

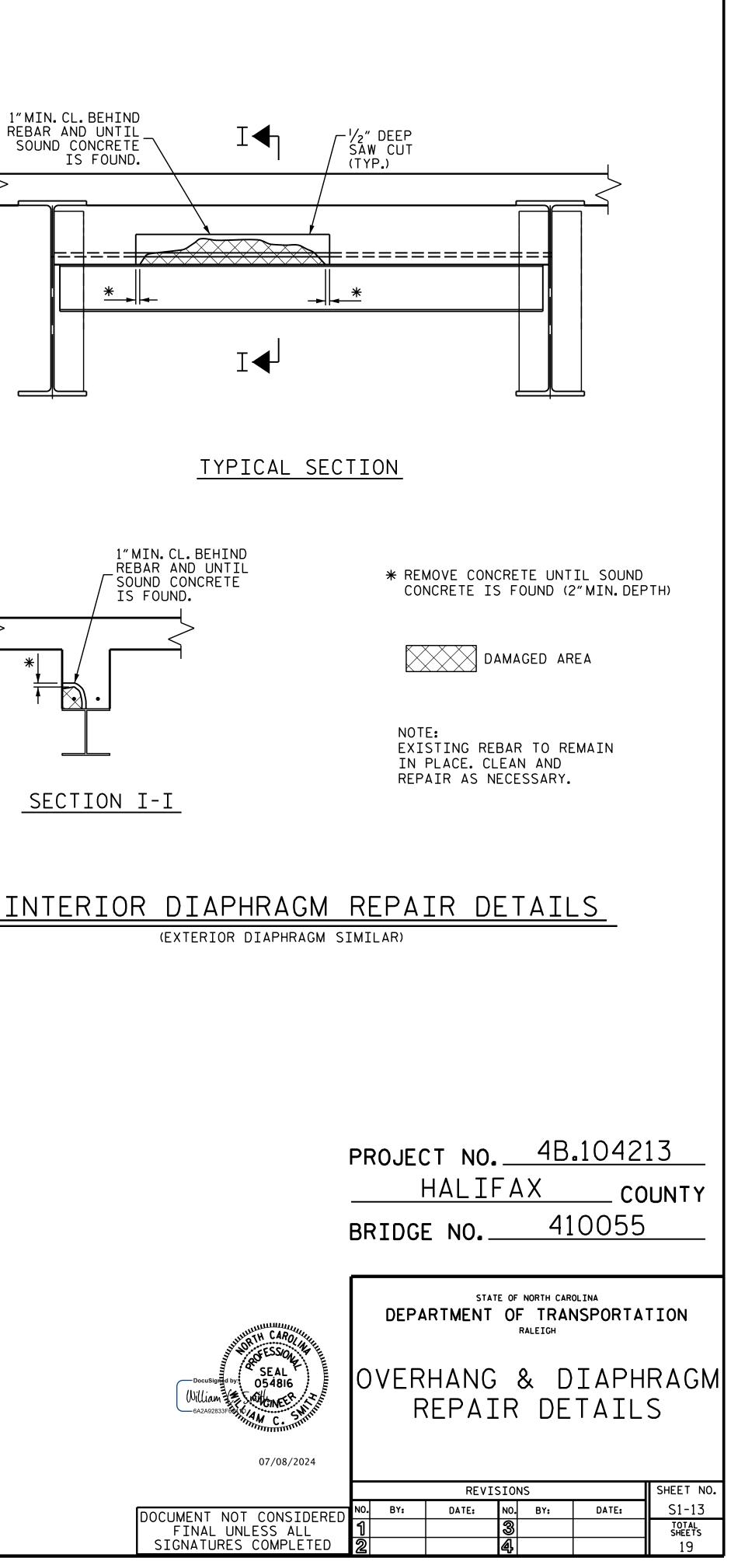
REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON the "DECK REPAIR DETAIL" SHEET.

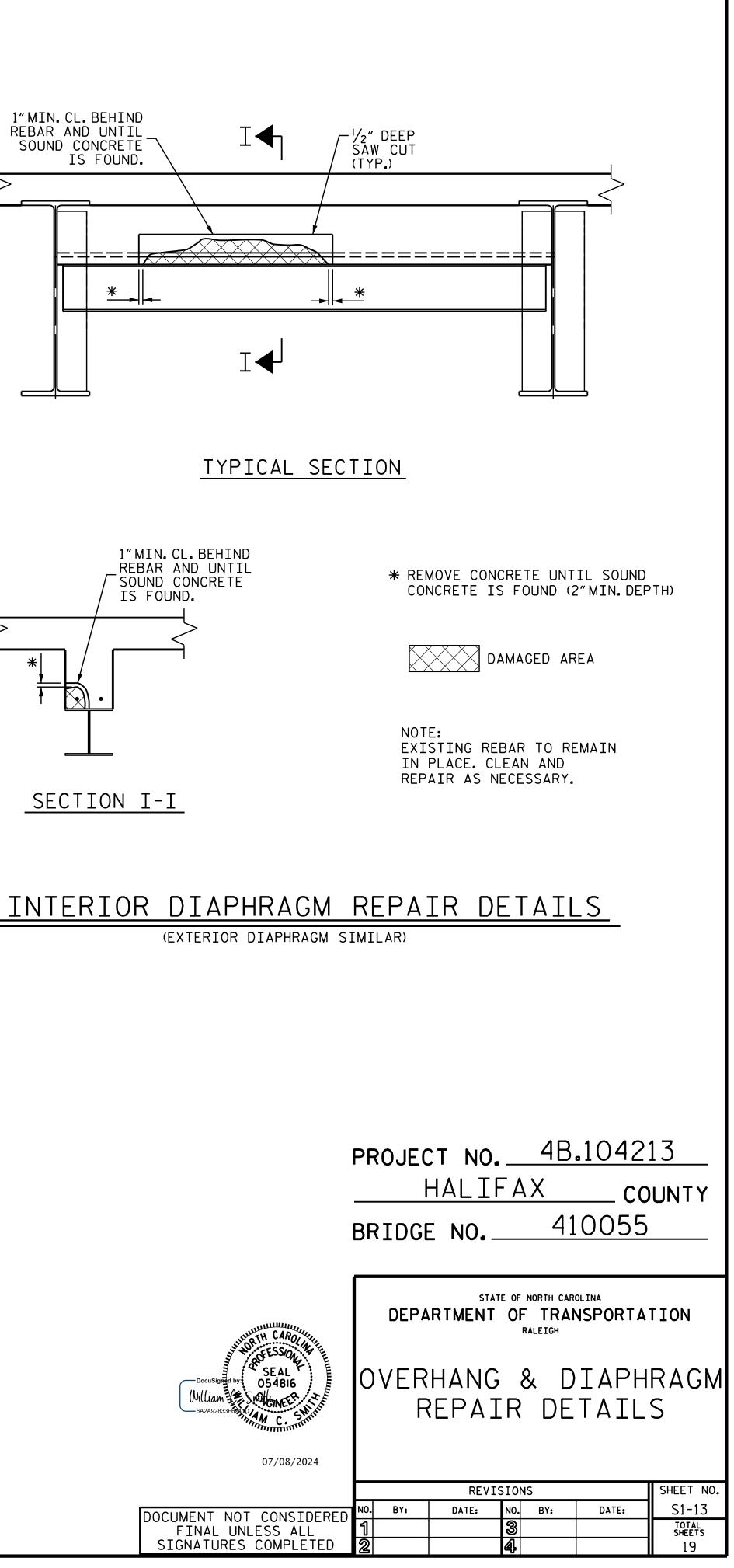
FOR AREAS TO BE REPAIRED, SEE "UNDERSIDE DECK REPAIRS" SHEETS.

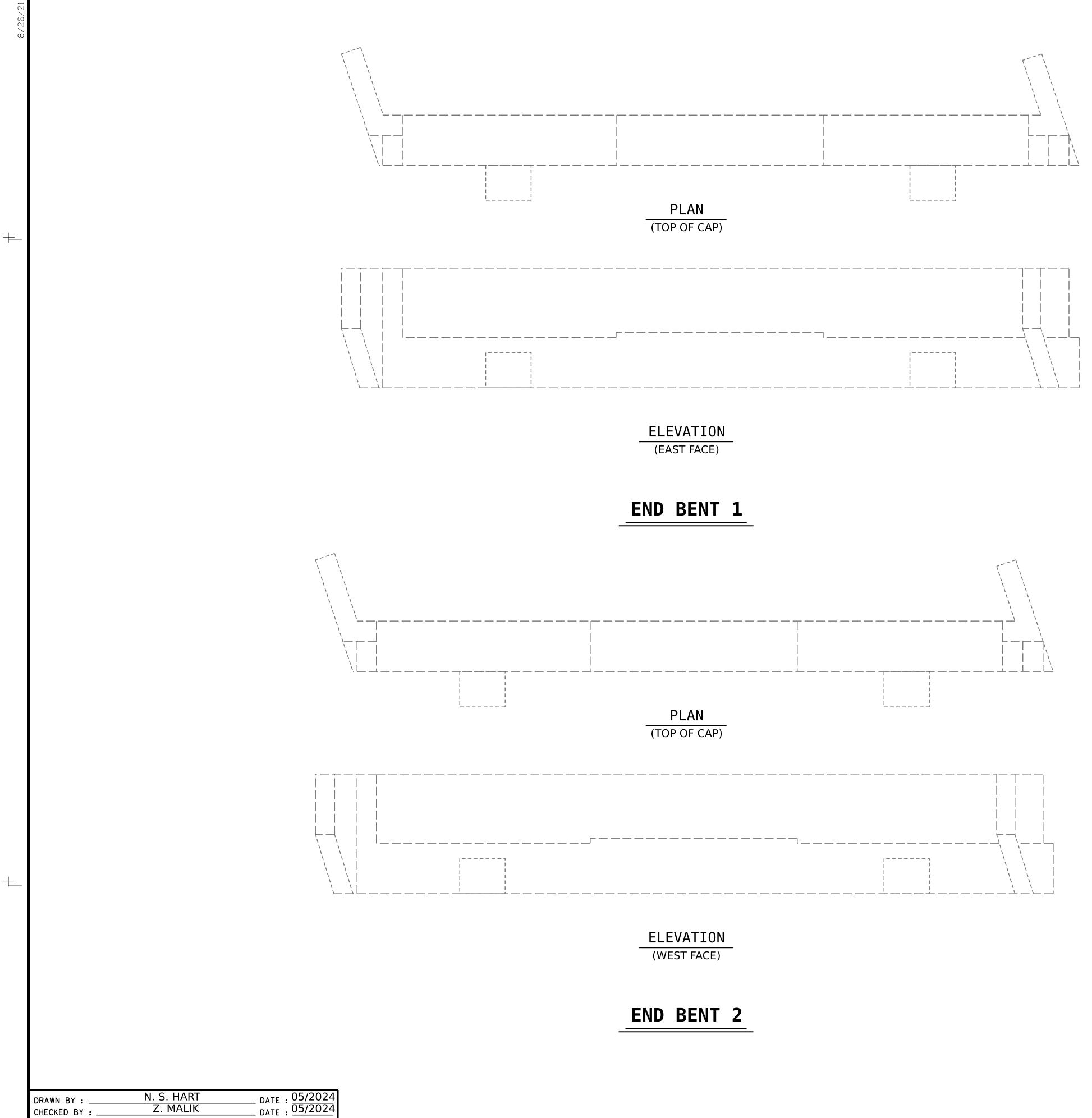
THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO STARTING WORK FOR TEMPORARY FORMWORK.FOR SUBMITTALS OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

UPON REMOVAL OF TEMPORARY FORMWORK, ALL VOIDS AND HONEYCOMBS ON THE UNDERSIDE OF DECK SURFACE SHALL BE FILLED WITH THE SAME MATERIAL AS USED FOR THE PATCH, AND FINISHED TO CONFORM TO THE SURROUNDING CONCRETE SURFACE.

NO FORMWORK SHALL BE LEFT IN PLACE.







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DESIGN ENGINEER OF RECORD:

_ DATE : _

NOTES REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

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SUBSTRUCTURE R	REPAIR	QUANTI	ΓΥ ΤΑΒ	LE	
	QUANTITIES				
EPAIRS - END BENT 1 & 2	ESTI	MATE	ACT	-UAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
CAP	0	0			
CURTAIN WALL	0	0			
WINGWALL					
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
CAP	0	0			
CURTAIN WALL	0	0			
WINGWALL					
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT	
CAP		0			
CURTAIN WALL		0			
WINGWALL					
EPOXY COATING		AREA SF		AREA SF	
TOP OF CAP		100			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION

	PROJEC BRIDGE	HALIF	AX	.1042 C0 10055	13 UNTY
DocuSigned by William () Stall 6A2A92833F624 HILL 054816 6A2A92833F624 HILL 054816 07/08/2024		RTMENT C	RALEIGH	NSPORTA E REP/ 1 &	
		REVISIO)NS		SHEET NO.
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FINAL UNLESS ALL SIGNATURES COMPLETED	1	<u>.</u> 3 ද			total sheets 19

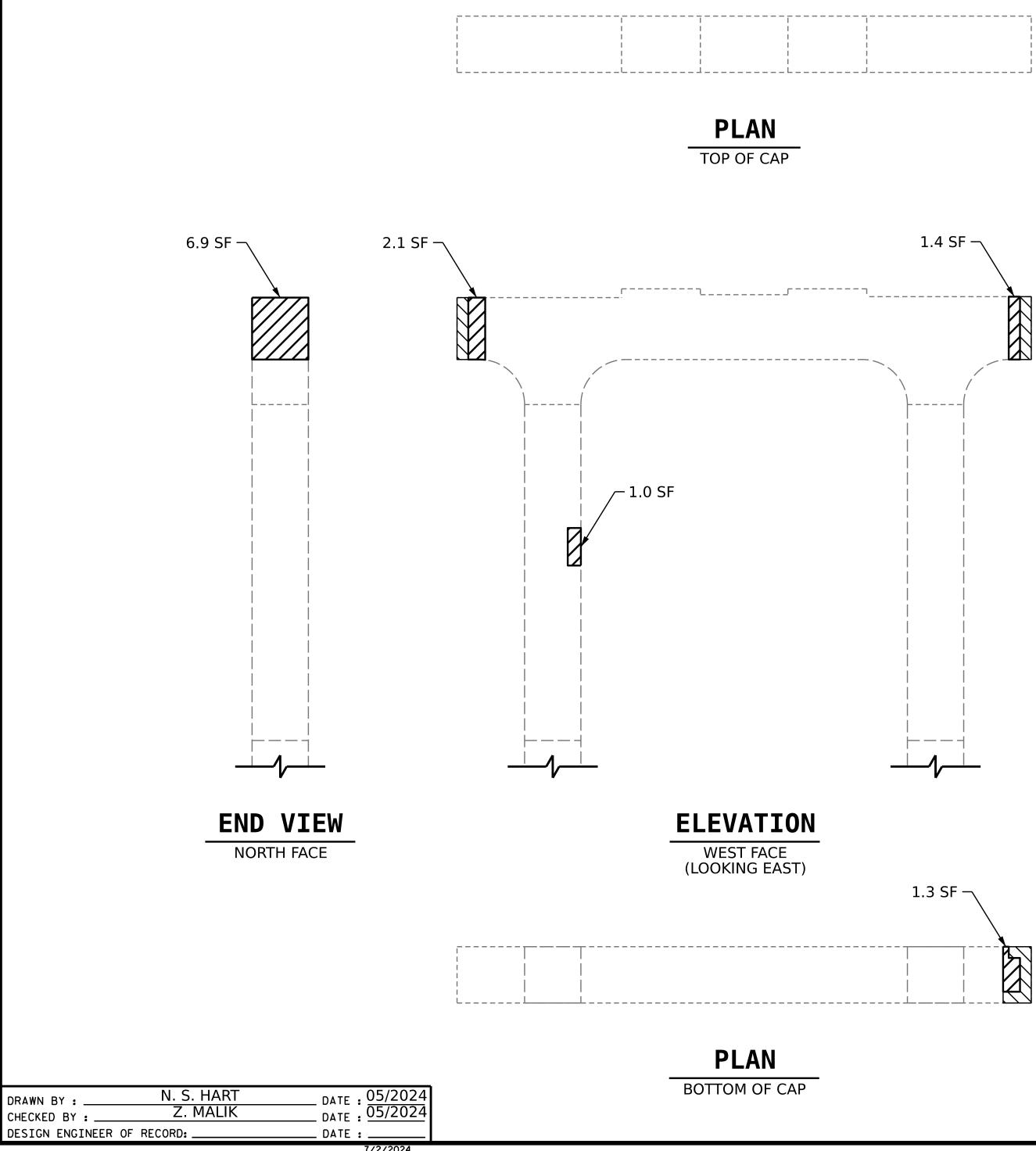


REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

SHEET.

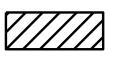


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FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS"

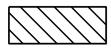


SHOTCRETE REPAIR AREA

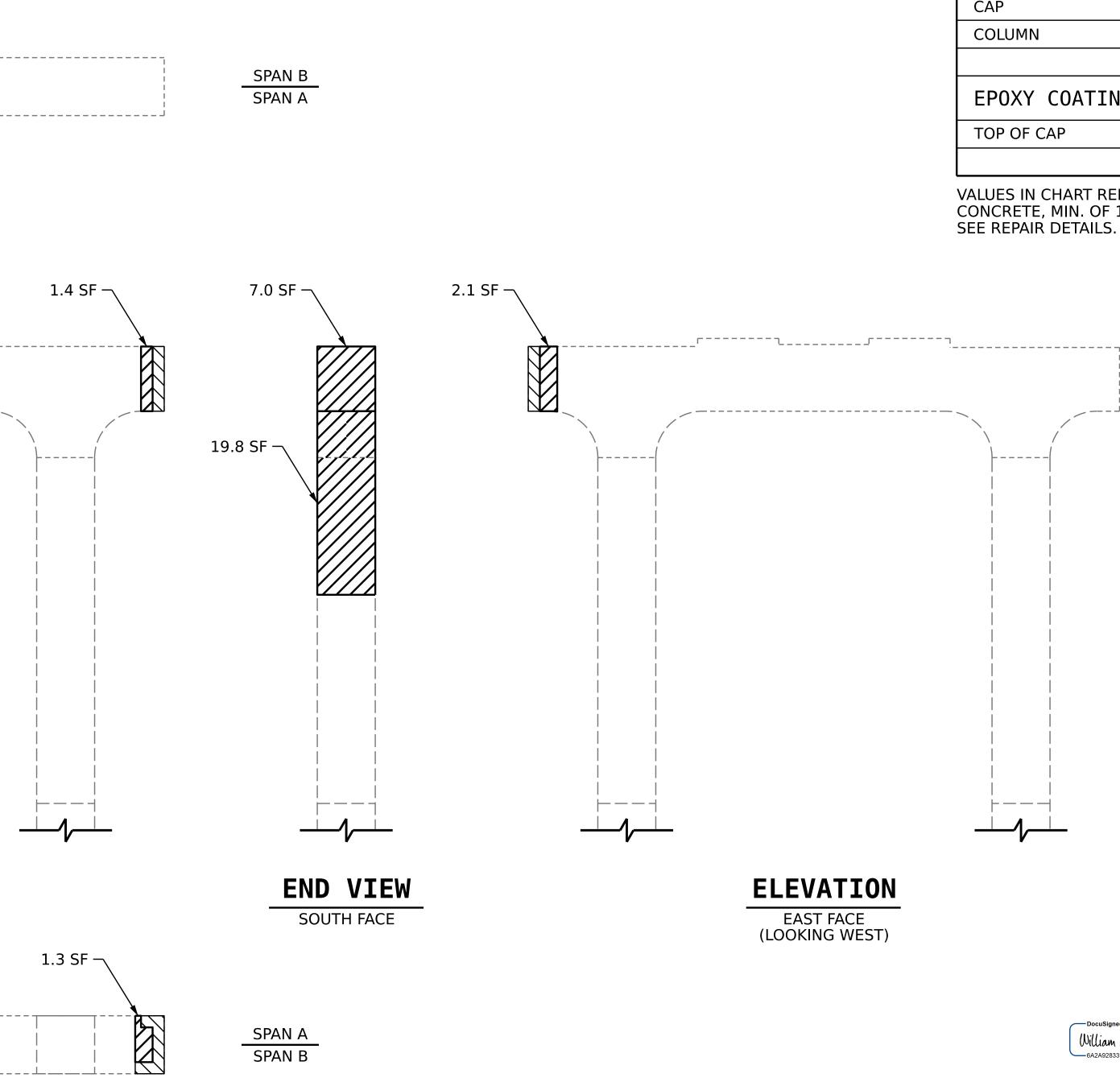


CONCRETE REPAIR AREA

PREVIOUSLY ACCOUNTED FOR AREA



EPOXY RESIN INJECTION



SUBSTRUCTURE R	EPAIR	QUANTI	ΤΥ ΤΑΒΙ	_E
QUANTITIES				
REPAIRS - BENT 1	ESTI	MATE	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	20.8	10.4		
COLUMN	20.8	10.4		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	0	0		
COLUMN	0	0		
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT
САР		0		
COLUMN		0		
EPOXY COATING		AREA SF		AREA SF
TOP OF CAP		59.8		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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DocuSigned by William () Smith 054816 6A2A92833F624 Milliam C. Smith 054816 Milliam C. Smith 054816		BSTRUC	RALEIGH	NSPORTA	
		REVISIO	NS		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE: NO.	BY:	DATE:	S1-15
FINAL UNLESS ALL	1	3			TOTAL SHEETS
SIGNATURES COMPLETED	2	4			19

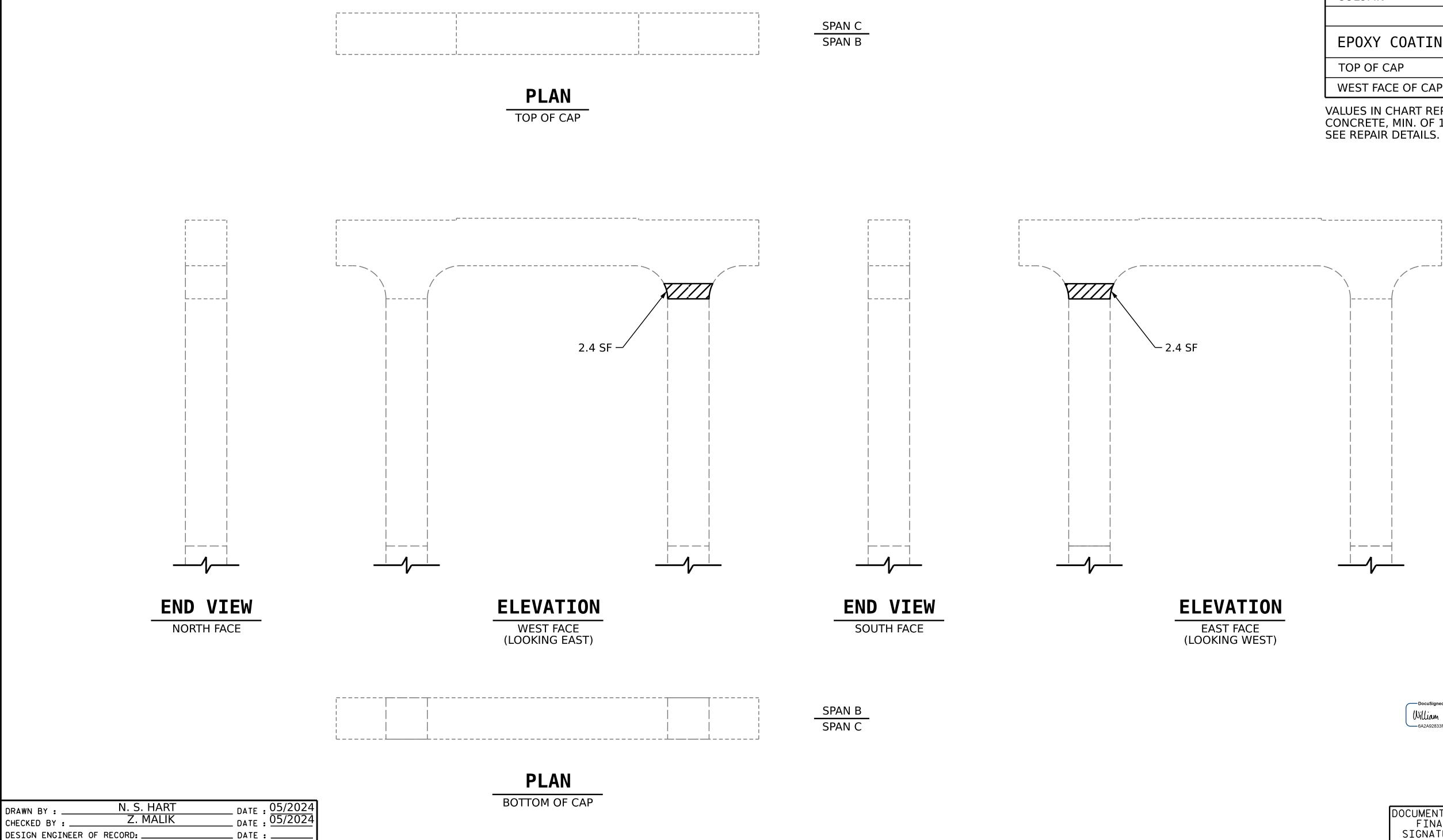


REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

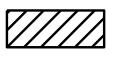
CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.



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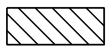
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SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION



				_
SUBSTRUCTURE R	EPAIR	QUANIL	ΙΥ ΙΑΒΙ	LE
QUANTITIES				
REPAIRS - BENT 2	ESTI	MATE	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	0	0		
COLUMN	4.8	2.4		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	0	0		
COLUMN	0	0		
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT
САР		0		
COLUMN		0		
EPOXY COATING		AREA SF		AREA SF
TOP OF CAP		59.8		
WEST FACE OF CAP		71.4		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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	PROJECT NO. <u>4B.104213</u> <u>HALIFAX</u> COUNTY BRIDGE NO. <u>410055</u>
DocuSigned by William () Smith 054816 6A2A92833F624 Milliam C. Smith 054816 6A2A92833F624 07/08/2024	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH SUBSTRUCTURE REPAIR BENT 2
	REVISIONS SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY: DATE: NO. BY: DATE: S1-16
FINAL UNLESS ALL	1 3 TOTAL SHEETS
SIGNATURES COMPLETED	2 4 19

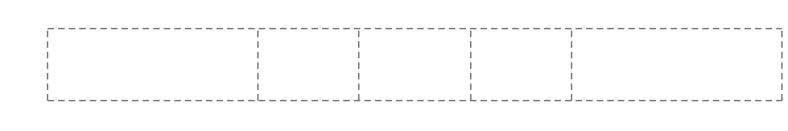


REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

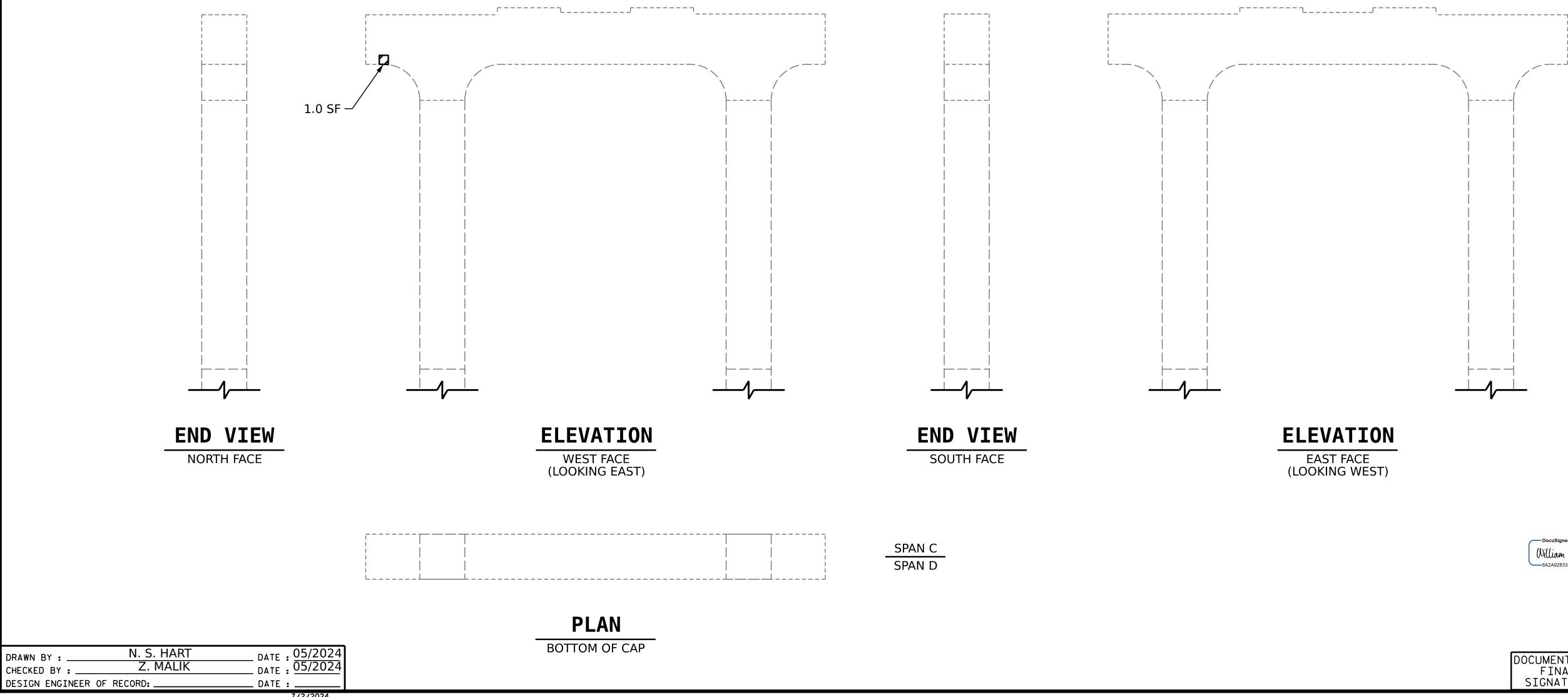
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.



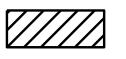
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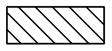
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SHOTCRETE REPAIR AREA

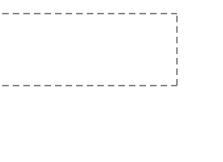


CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION



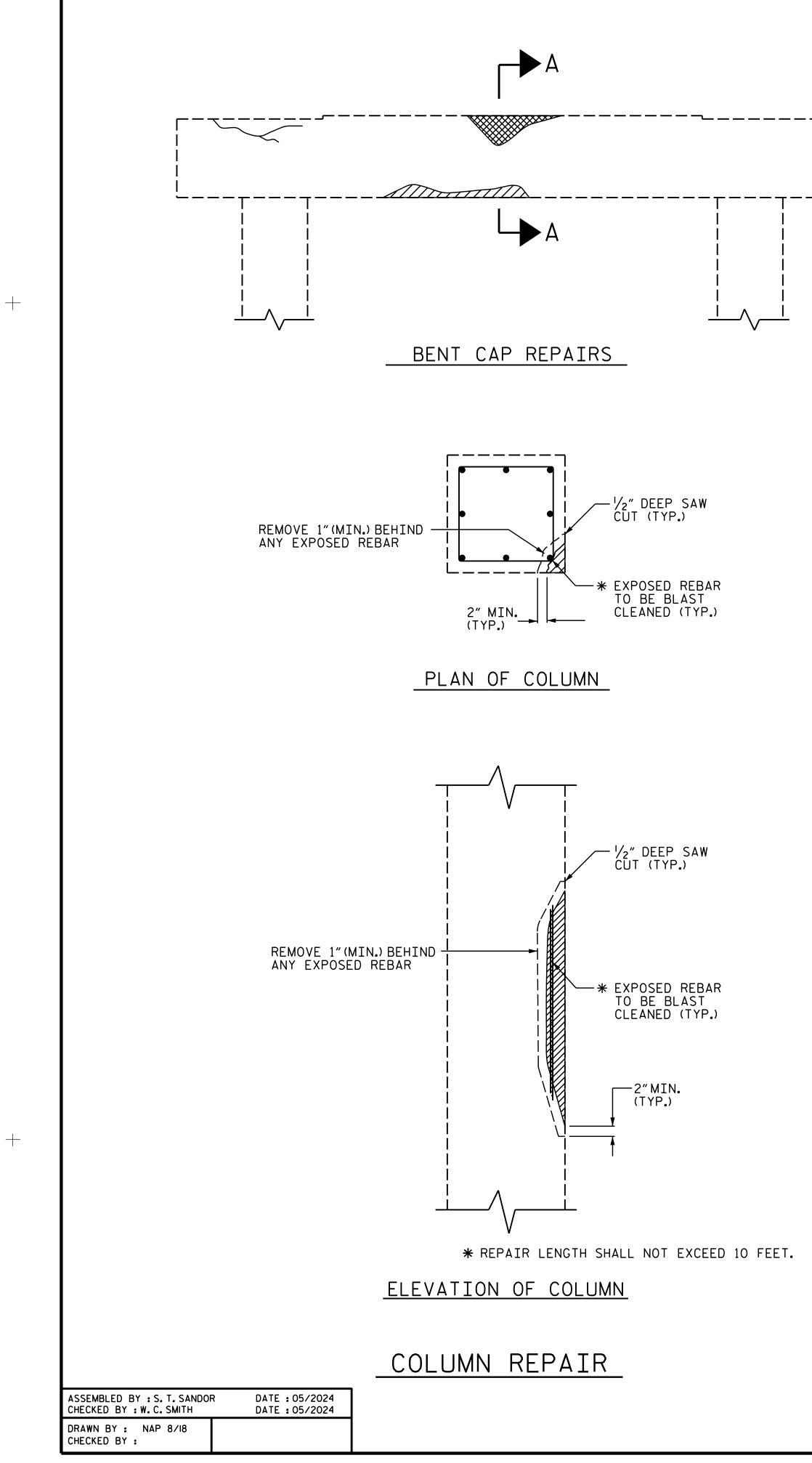
SPAN D SPAN C

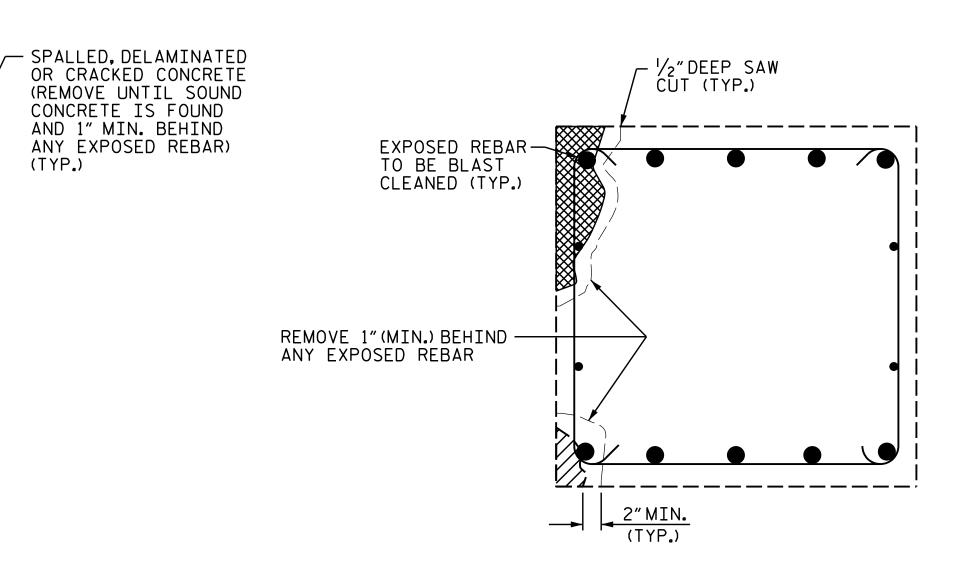
SUBSTRUCTURE REPAIR QUANTITY TABLE					
REPAIRS - BENT 3	ESTII	MATE	ACT	UAL	
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
САР	1.0	0.5			
COLUMN	0	0			
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
САР	0	0			
COLUMN	0	0			
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT	
САР		0			
COLUMN		0			
EPOXY COATING		AREA SF		AREA SF	
TOP OF CAP		59.8			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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	PROJEC BRIDGE	HALIF	AX	. 1042 CO 10055	13 UNTY
DocuSigned by William () Swith 054816 6A2A92833F6247 MMC: SHIT MMC: SHIT MMC		BSTRUC	RALEIGH	NSPORTA	
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SIGNATURES COMPLETED	2]		19





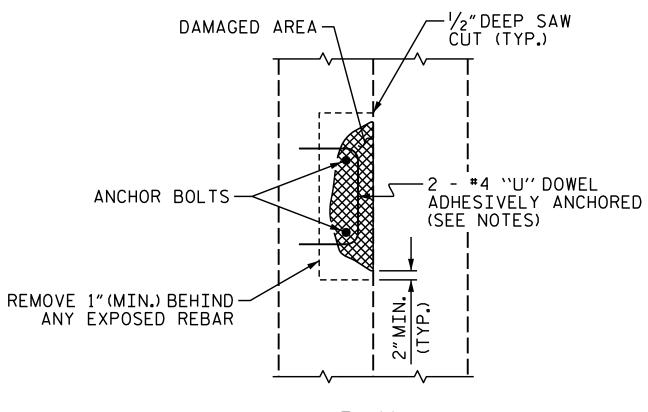
REPAIR KEY

SECTION A-A

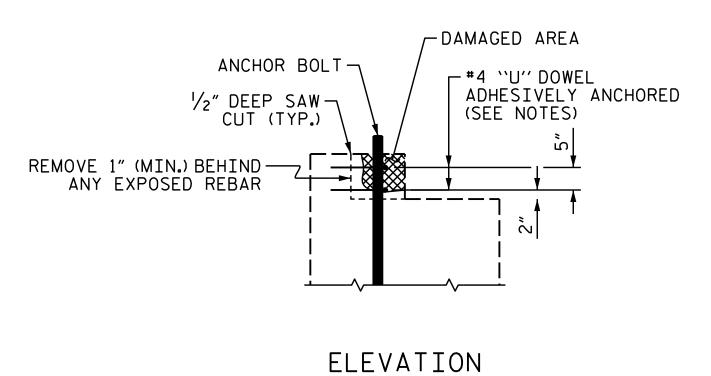
CAP REPAIR

SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA (FORM AND POUR)







SPLICE	LENGTH	TABLE	
BAR SIZE	MIN. SPLICE	E LENGTH	
# 4	2'-5	"	
# 5	3'-0	"	
#6	3'-7"		
#7	4'-2"		
#8	4'-9"		
#9	5'-4"		
# 10	6'-0"		
#11	6'-8"		



NOTES

TYPICAL BENT CAP REPAIRS ARE SHOWN. REPAIR DETAILS SIMILAR FOR END BENT CAPS AND STRUTS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

NO MORE THAN ONE-THIRD OF THE CAP OR COLUMN CIRCUMFERENCE SHALL BE REMOVED AT ONE TIME.SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN 30% OF A CAP OR COLUMN CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

SIMULTANEOUS REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP AND/OR COLUMN. BUT NO MORE THAN 3 OF THE CIRCUMFERENCE SHALL BE REMOVED AT ONE TIME. IF REMOVAL EXTENDS MORE THAN 11/2" BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING. ON COLUMNS AND PILES, NO MORE THAN 10 VERTICAL FEET MAY BE EXPOSED AT ONE TIME BEFORE PLACEMENT OF REPAIR CONCRETE.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

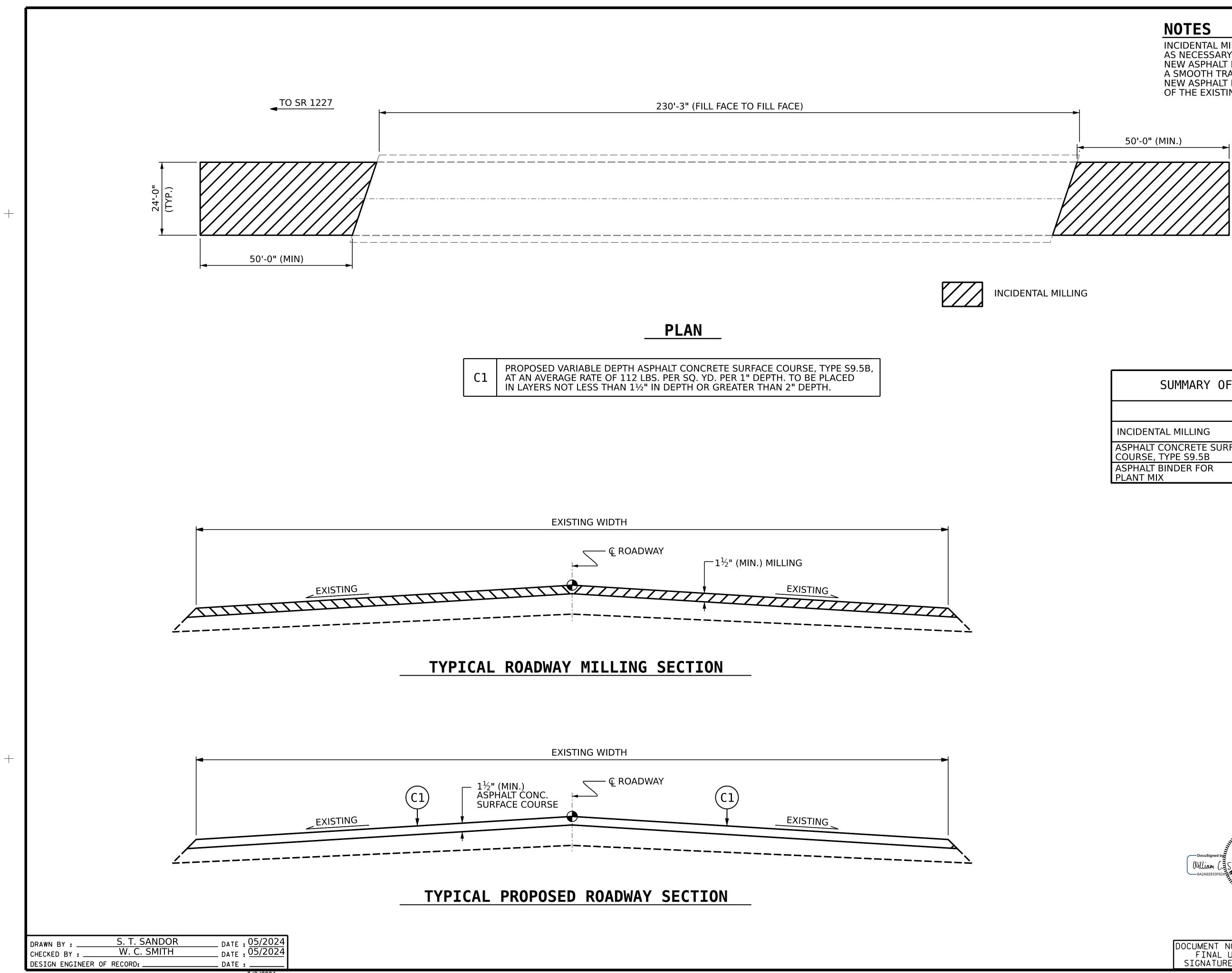
THE #4 ``U'' DOWELS ARE REQUIRED ONLY AROUND THE ANCHOR BOLTS. THE EXISTING REINFORCING STEEL IN THE PEDESTAL WALL SHALL BE CLEANED, STRAIGHTENED AND REMAIN IN PLACE.

FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

COAT ALL REPAIR SURFACE AREAS ON THE TOP OF CAPS, INCLUDING CHAMFERS, WITH EPOXY PROTECTIVE COATING, OVERLAPPING THE REPAIR AREA BY A MINIMUM OF 3" ON ALL POSSIBLE SIDES.

CLEAN ALL EXPOSED REINFORCING BARS AND PRESTRESSED STRANDS IN ACCORDANCE WITH APPROPRIATE SPECIAL PROVISIONS.FOR BARS WITH MORE THAN 10% SECTION LOSS. SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED. NOTE AND PROVIDE DETAILED DOCUMENTATION, INCLUDING LOCATION AND SEVERITY, OF ALL DAMAGE TO PRESTRESSED STRANDS THAT EXCEEDS 10% SECTION LOSS. IF FIVE OR MORE STRANDS ARE DAMAGED, NOTIFY THE ENGINEER PRIOR TO PLACEMENT OF REPAIR MATERIAL.

		ALIF	AX	<u>.1042:</u> co l 10055	JNTY
Docusigned by SEAL OS4816 William H. William C. String AZA92833F M. C. String M.		RTMENT S TYP AND	TANDAF	NSPORTA RD CAP	
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INCIDENTAL MILLING - EXISTING APPROACH ASPHALT PAVING TO BE MILLED AS NECESSARY TO ATTAIN MINIMUM 1½" DEPTH OF NEW ASPHALT PAVING. NEW ASPHALT PAVEMENT SHALL BE OF THICKNESS NECESSARY TO PROVIDE A SMOOTH TRANSITION BETWEEN THE ROADWAY AND THE BRIDGE DECK. THE NEW ASPHALT PAVEMENT THICKNESS MAY EXCEED 1½" DUE TO SETTLEMENT OF THE EXISTING APPROACH.

TO ENFIELD

SUMMARY OF QUANTITIES				
	ESTIMATE	ACTUAL		
IDENTAL MILLING	288.1 SY			
HALT CONCRETE SURFACE JRSE, TYPE S9.5B	30.0 TONS			
HALT BINDER FOR NT MIX	5.0 TONS			

	PROJEC <u> </u> BRIDGE	IALIF	AX	. 1042 CO 10055	13 OUNTY
DocuSigned by William (Hore SEAL 6A2A92833F624 07/08/2024	AI	PPROA	RALEIGH	NSPORTA ILLIN ROADW/	G
		REVIS	IONS		SHEET NO.
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SIGNATURES COMPLETED	2	0	4 		19

DESIGN DATA:

SPECIFICATIONS		AASHTO (CURRENT)
LIVE LOAD		SEE PLANS
IMPACT ALLOWANCE		SEE AASHTO
STRESS IN EXTREME STRUCTURAL STEEL	FIBER OF - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN.
	- AASHTO M270 GRADE 50W	27,000 LBS. PER SQ. IN.
	- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL	IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPR	RESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR		SEE AASHTO
STRUCTURAL TIMBER	- TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPE	ENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID P	RESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION IOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS, AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " \oslash Shear studs for the $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " \oslash STUDS FOR 4 - $\frac{3}{4}$ " \oslash STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " \oslash STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " \oslash studs based on the ratio of 3 - $\frac{7}{8}$ " \oslash STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADIOINING PIECES.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EOUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

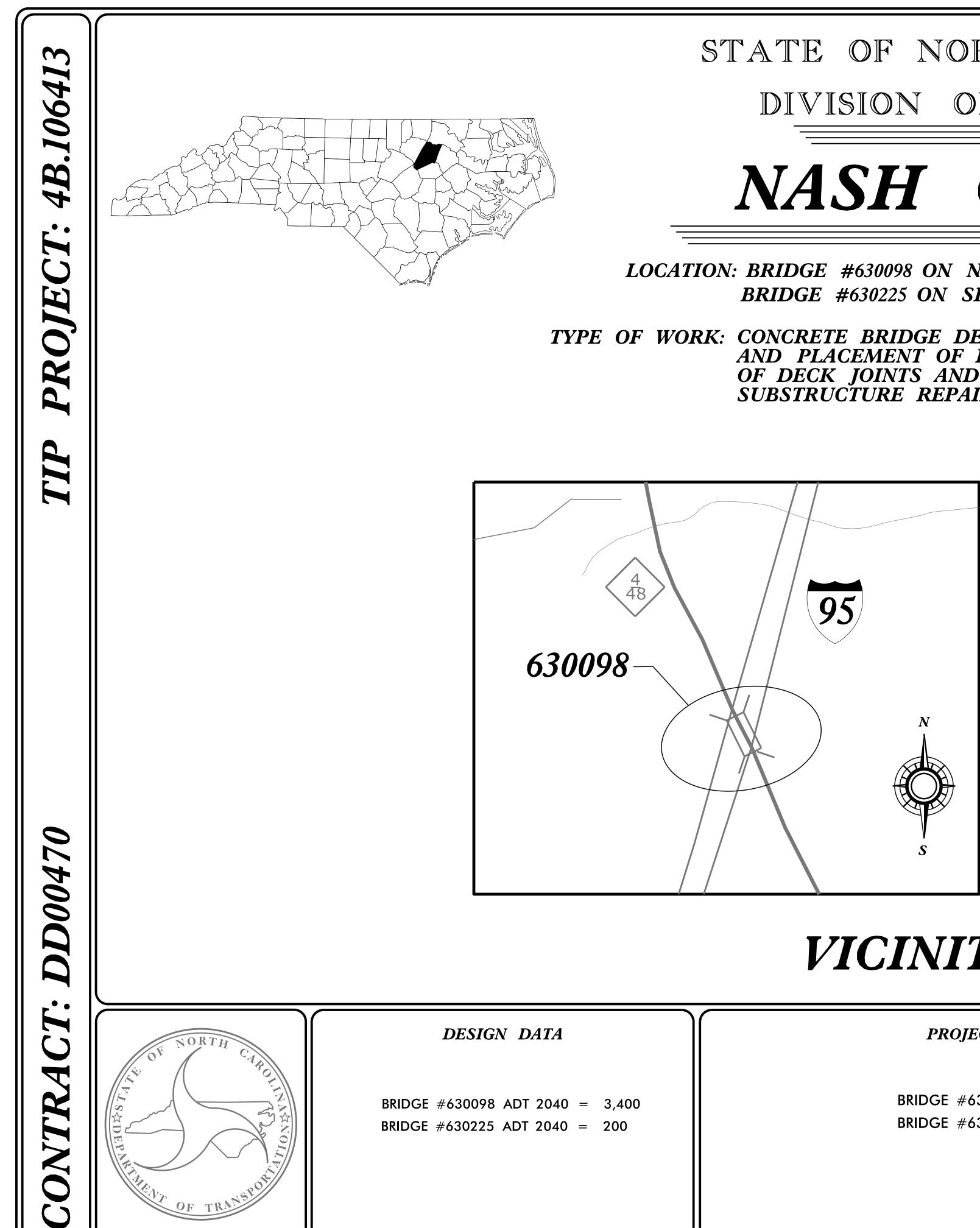
HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



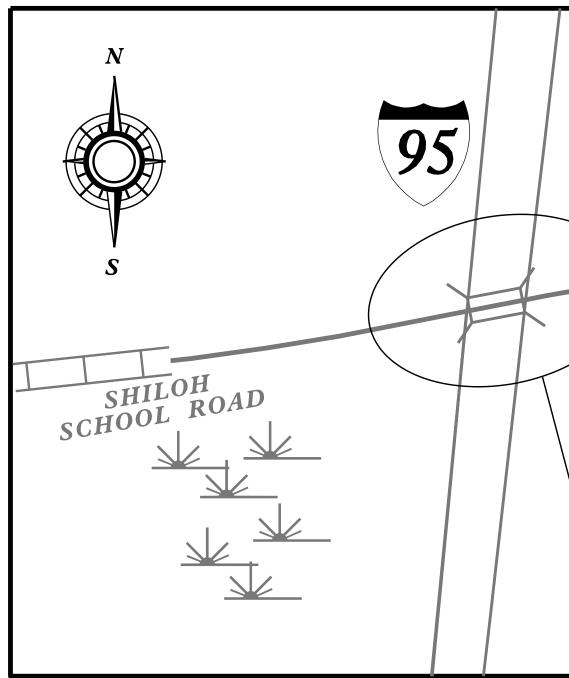
STATE OF NORTH CAROLINA

DIVISION OF HIGHWAYS

NASH COUNTY

LOCATION: BRIDGE #630098 ON NC 4/NC 48 OVER I-95 BRIDGE #630225 ON SR 1515 (SHILOH SCHOOL ROAD) OVER I-95

TYPE OF WORK: CONCRETE BRIDGE DECK REHABILITATION BY SCARIFICATION, HYDRO-DEMOLITION, AND PLACEMENT OF LATEX MODIFIED CONCRETE; DEMOLITION AND RECONSTRUCTION OF DECK JOINTS AND SEALS; PAINTING OF EXISTING BEARINGS WITH HRCSA; SUBSTRUCTURE REPAIRS WITH SHOTCRETE; EPOXY COATING; REPLACE APPROACH PAVEMENT



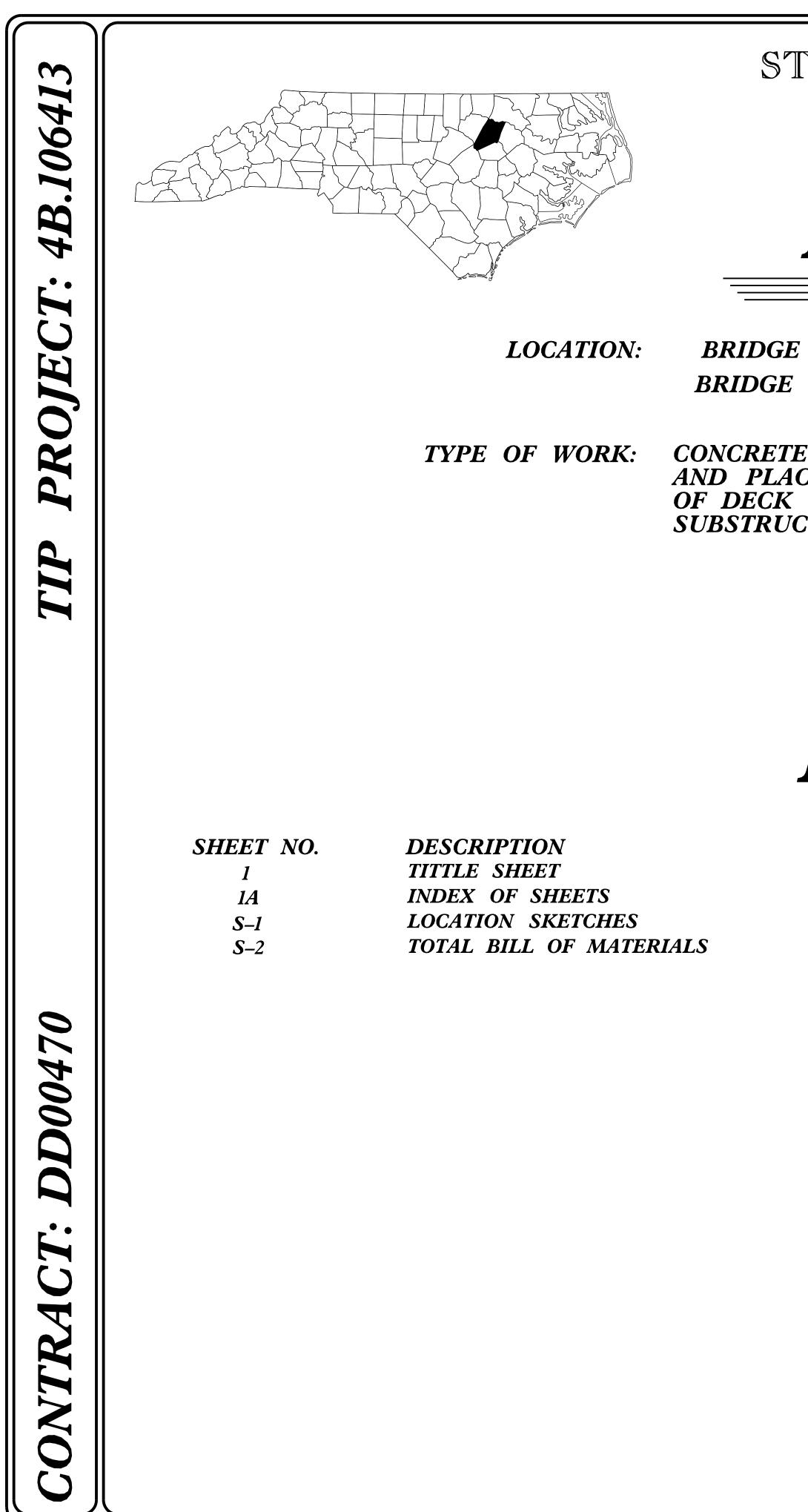
VICINITY MAP

PROJECT LENGTH BRIDGE #630098 = 0.06 MILES BRIDGE #630225 = 0.06 MILES LETTI

STATE S	TE STATE PROJECT REPERENCE NO. SHEET TOTAL NO. SHEETS				
N.C. 4B.106413					
STATE PROJ. NO. F. A. PROJ. NO. DESCRIPTION			TON		
4B.106413	_	P.E.			
4B.106413	_	CONST.			

<u>SR 1515.</u>	
<i>└</i> _630225	

Prepared in the Office of: DIVISION OF HIGHWAYS STRUCTURES MANAGEMENT UNIT 1000 BIRCH RIDGE DR. RALEIGH, N.C. 27610		
24 STANDARD SPECIFICATIONS	KRISTY ALFORD, PE	
NG DATE : EPTEMBER 24, 2024	PROJECT ENGINEER	
	ASTER G. ABRAHA, PE PROJECT DESIGN ENGINEER	



STATE OF NORTH CAROLINA

DIVISION OF HIGHWAYS

NASH COUNTY

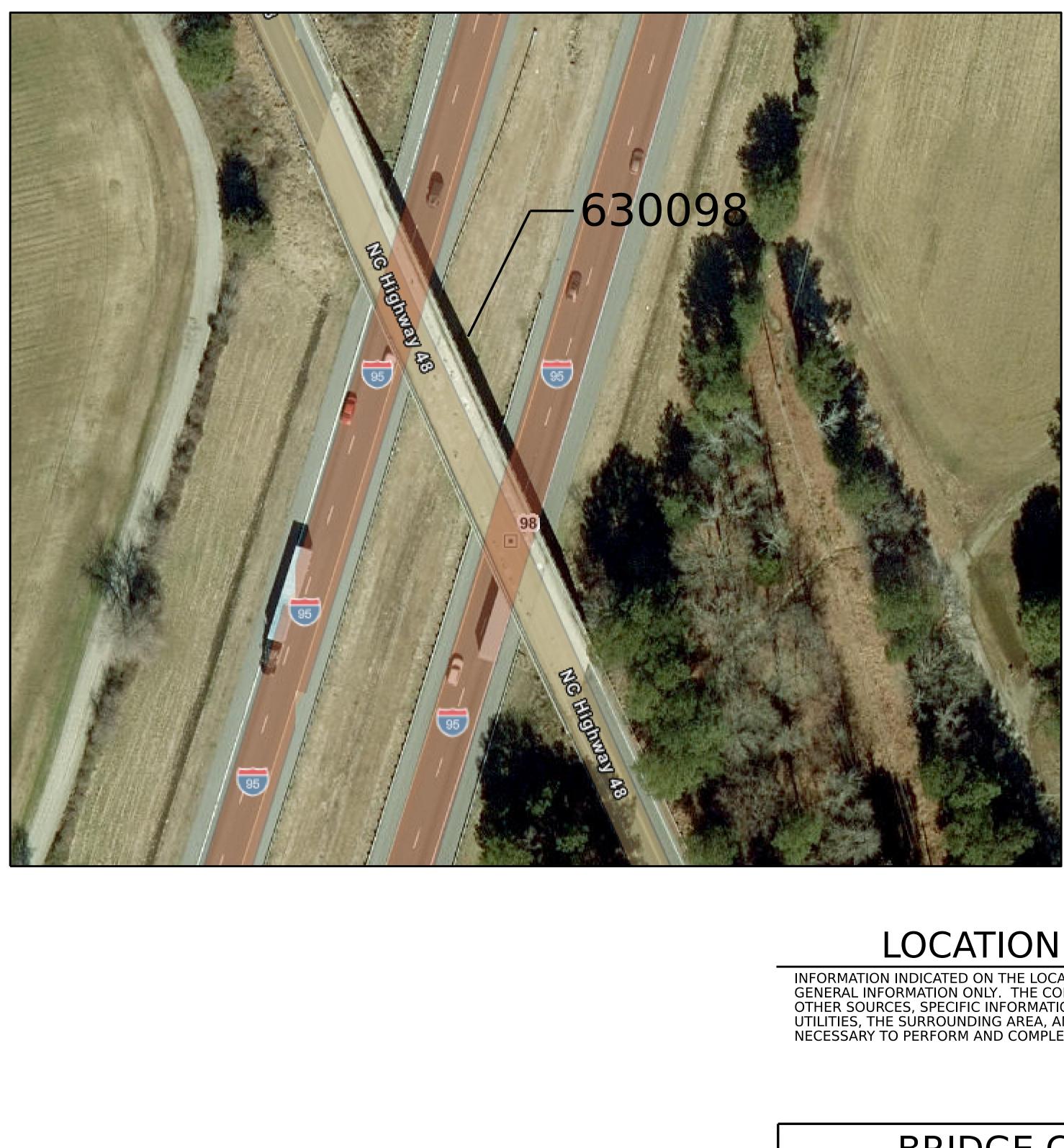
BRIDGE #630098 ON NC 4/NC 48 OVER I-95 BRIDGE #630225 ON SR 1515 (SHILOH SCHOOL ROAD) OVER I-95

CONCRETE BRIDGE DECK REHABILITATION BY SCARIFICATION, HYDRO-DEMOLITION, AND PLACEMENT OF LATEX MODIFIED CONCRETE; DEMOLITION AND RECONSTRUCTION OF DECK JOINTS AND SEALS; PAINTING OF EXISTING BEARINGS WITH HRCSA; SUBSTRUCTURE REPAIRS WITH SHOTCRETE; EPOXY COATING; REPLACE APPROACH PAVEMENT

INDEX OF SHEETS

SHEET NO. STRUCTURE S1–1 S1–2 S1–3 THRU S1–7 S1–8 THRU S1–12 S1–13 S1–14 S1–15	DESCRIPTION #630098 GENERAL DRAWING TYPICAL SECTION DECK SURFACE REPAIRS DECK UNDERSIDE REPAIRS JOINT REPAIR DETAILS DECK REPAIR DETAILS OVERHANG & DIAPHRAGM	SHEET NO. STRUCTURE S2–1 S2–2 S2–3 THRU S2–7 S2–8 THRU S2–12 S2–13 S2–14 S2–15	DESCRIPTION #630225 GENERAL DRAWING TYPICAL SECTION DECK SURFACE REPAIRS DECK UNDERSIDE REPAIRS JOINT REPAIR DETAILS DECK REPAIR DETAILS OVERHANG & DIAPHRAGM
SI-15 S1-16 S1-17 THRU S1-20	REPAIR DETAILS END BENT 1 & 2 BENTS	S2-15 S2-16 S2-17	REPAIR DETAILS BRIDGE JACKING DETAILS END BENT 1 & 2
S1-21	TYPICAL CAP AND COLUMN REPAIR DETAILS	S2–18 THRU S2–21 S2–22	BENTS TYPICAL CAP AND COLUMN REPAIR DETAILS
<i>S1–22</i>	APPROACH MILLING & TYPICAL ROADWAY SECTIONS	S2-23	APPROACH MILLING & TYPICAL ROADWAY SECTIONS
		SN	STANDARD NOTES

state N.C.	STATE PROJECT REPERENCE NO. SHEET NO. SHEET NO. SHEET SHEETS				
STATE PROJ. NO	STATE PROJ. NO. F. A. PROJ. NO. DESCRIPTIO		ION		
4B.10641	3	_	P.E.		
4B.10641	4B.106413 –		CONST.		



DRAWN BY :	Q. T. NGUYEN	DATE : 05/2024
CHECKED BY :	F. LEA	DATE : 05/2024

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LOCATION SKETCHES

INFORMATION INDICATED ON THE LOCATION SKETCH SHALL BE CONSIDERED GENERAL INFORMATION ONLY. THE CONTRACTOR SHALL CONFIRM, THROUGH OTHER SOURCES, SPECIFIC INFORMATION REGARDING THE BRIDGES, ROADWAYS, UTILITIES, THE SURROUNDING AREA, AND ANY OTHER ASPECTS THAT MAY BE NECESSARY TO PERFORM AND COMPLETE THE PROJECT.

BRIDGE COORDINATES			
BRIDGE No.	LONGITUDE		
630098	36°-05'-0.64″	77°-48′-31 . 97″	
630225	36°-05′-58.1″	77°-48′-20.19″	

	PROJEC	T NO.	4B	.1064	13
		NAS	Н	CC	DUNTY
	BRIDGE	E NO. 6	53009	8, 63	0225
NUMBER CAROLANT	DEPA	-	E OF NORTH CAR OF TRA RALEIGH	ROLINA NSPORTA	TION
DocuSigned by: William (. Smithen SEAL 6A2A92833F6241D	LOCATION SKETCHES				ES
07/08/2024					
		REVIS	SIONS		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO. BY:	DATE:	S-1
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GENERAL NOTES:

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THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT DUE TO THE NATURE OF PRESERVATION PROJECTS, THE EXTENT OF WORK CANNOT ALWAYS BE ACCURATELY DETERMINED PRIOR TO COMMENCEMENT OF WORK. REPAIR LOCATIONS AND ESTIMATES **OUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL** REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, 1 ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION THE REPAIRS.

EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT FOR DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN WHAT IS SH ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS.

WORK ON THE BRIDGE(S) SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW, EXCEPT WHERE THE CONTRACTOR'S PLAN USES PLATFORMS, NETS, SCREENS OR OTHER PROTECTIVE DEVICES TO CATCH THE MATERIAL. THE CONTRACTOR SHALL SUBMIT PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PERFORM ALL WORK WITH CARE SO THAT THE EXISTING STRUC WHICH IS TO REMAIN IN PLACE WILL NOT BE DAMAGED. IF THE CONTRACTOR DAMAGES PART OF THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE, THE DAMAGED AREA SHALL BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE ENGINEER AT NO ADDITIONAL COST TO THE DEPARTMENT.

ANY DAMAGE TO EXISTING REINFORCING STEEL, DURING CONTRACTOR'S OPERATIONS, SI BE REPAIRED AS DIRECTED BY THE ENGINEER AND PERFORMED AT NO ADDITIONAL COST THE DEPARTMENT.

FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE ELSEWHERE THE CONTRACT DOCUMENTS.

PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPRO COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR PAVEMENT MARKING SEE ELSEWHERE IN THE CONTRACT DOCUMENTS.

THE ELEVATIONS AND CLEARANCES SHOWN ON THE PLANS AT THE POINT OF MINIMUM VERTICAL CLEARANCE ARE FROM THE BEST INFORMATION AVAILABLE. PRIOR TO BEGINNING BRIDGE CONSTRUCTION, VERIFY THE ELEVATION(S) ON THE EXISTING PAVEMENT AND CHECK THE CLEARANCE. REPORT ANY VARIATIONS TO THE ENGINEER. ANY PLAN REVISIONS NECESSARY TO ACHIEVE THE REQUIRED MINIMUM VERTICAL CLEARANCE WILL BE PROVIDED BY THE DEPARTMENT.

	TOTAL BILL OF MATERIAL										
BRIDGE NO.	INCIDENTAL MILLING	ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B	ASPHALT BINDER FOR PLANT MIX	GROOVING BRIDGE FLOORS	POLLUTION CONTROL	CLASS SURFAC PREPARAT	E C	LATEX MODIFIED CONCRETE OVERLAY	PLACING & FINISHING LATEX MODIFIED CONCRETE OVERLAY	CONCRETE REPAIRS	SHOTCRETE REPAIRS
	SQ. YDS.	TONS	TONS	SQ. FT.	LUMP SUM	SQ. YI	D.	CU. YD.	SQ. YD.	CU. FT.	CU. FT.
630098	411.1	40	5	8,508.4	LUMP SUM	212.9		56.8	1,046.4	-	125.9
630225	285.7	30	5	6,333.3	LUMP SUM	83.0		41.1	797.3	10.3	61.4
TOTAL	696.8	70	10	14,841.7	LUMP SUM	295.9		97.9	1,843.7	10.3	187.3
BRIDGE NO.	PAINTING CONTAINMEN FOR BRIDGE NO		EPOXY COATING	ELASTOMER CONCRETE FOR PRESERVATIO		DEMC	DRO- DLITIOI RIDGE ECK		E BEARINGS W	XISTING /ITH HIGH LCIUM	TYPE II BRIDGE JACKING FOR BRIDGE NO
	LUMP SUM	LN. FT.	SQ. FT.	CU. FT.	SQ. F	SQ	. YD.	SQ. YE	D. EA.		EA.
630098	LUMP SUM	222.5	601.0	35.5	155.2	1,04	46.4	1,046.4	4 32		-
630225	LUMP SUM	135.1	332.5	21.0	92.8	79	7.3	797.3	32		5
TOTAL	LUMP SUM	357.6	933.5	56.5	248.0	1,8	43.7	1,843.7	7 64		5

)RAWN BY :			<u>. 05/2024</u>
CHECKED BY :	W. C. SMITH	DATE	06/2024

	FOR MAINTENANCE AND PROTECTION OF TRAFFI	C BENEATH PROPOSED ST
S OF THE	EXISTING JOINTS AND DECK DRAINS SHALL BE SE BRIDGE DECK. THE CONTRACTOR SHALL TAKE CA DRAINS IS CONTAINED. DRAINS IN SHOULDERS O	ARE THAT ANY CONSTRUC
TION OF	OF DEBRIS.	
	FOR BRIDGE JOINT DEMOLITION, SEE SPECIAL PR	OVISIONS.
	FOR CLEANING AND PAINTING EXISTING BEARING	GS WITH HRCSA, SEE SPEC
R ANY	FOR ELASTOMERIC CONCRETE FOR PRESERVATIO	N, SEE SPECIAL PROVISIC
SHOWN	FOR PAINTING CONTAINMENT AND POLLUTION CO SPECIAL PROVISION.	ONTROL, SEE "CLEANING /
,	FOR SHOTCRETE REPAIRS, SEE SPECIAL PROVISIO	DNS.
	FOR CONCRETE REPAIRS, SEE SPECIAL PROVISIO	NS.
DR	FOR VOLUMETRIC MIXER, SEE SPECIAL PROVISIO	NS.
IT	FOR BRIDGE JACKING, SEE SPECIAL PROVISIONS.	
	FOR ANCHOR BOLT REPAIRS, SEE SPECIAL PROVIS	SIONS.
JCTURE S ANY	FOR EPOXY RESIN INJECTION, SEE SPECIAL PROV	ISIONS.
A O	FOR SCARIYFYING BRIDGE DECK, CLASS II SURFA OF DECK, SEE "LMC OVERLAY SURFACE PREPARA	
SHALL ST TO	DURING CONSTRUCTION, APPROPRIATE MEASURI FLOW OR MIGRATE INTO ACTIVE TRAVEL LANES.	ES SHALL BE USED TO EN
RE IN	AT THE TIME OF PREPARATION OF THESE PLANS, REQUIRED. HOWEVER, IT MAY BE DETERMINED IN NECESSARY TO PROPERLY COMPLETE THE INTENI BE PREPARED TO PERFORM SUCH WORK IN A TIM	N THE FIELD THAT THE FO DED BRIDGE PRESERVATIO
OVAL A CE	EXTRA WORK AND SHALL BE ADDRESSED AS PER SPECIAPROVISIONS THAT OUTLINE REQUIREMENT THE PROJECT DOCUMENTS,BUT NO QUANTITIES H AS REQUIRED, IF EXTRA WORK IS ENCOUNTERED	R ARTICLE 104-7 OF THE S TS FOR THESE POTENTIAL HAVE BEEN LISTED. ACTU
	ITEM DESCRIPTION	UNIT
	1. CLASS III SURFACE PREPARATION	SQ. YDS.
	 EPOXY RESIN INJECTION ANCHOR BOLT REPAIR 	LN. FT. EA.
	4. VOLUMETRIC MIXER	LS

STRUCTURE, SEE SPECIAL PROVISIONS.

NING SURFACE PREPARATIONS OF THE JCTION DEBRIS THAT COLLECTS IN THE NES SHALL BE KEPT FREE AND CLEAR

ECIAL PROVISIONS.

SIONS.

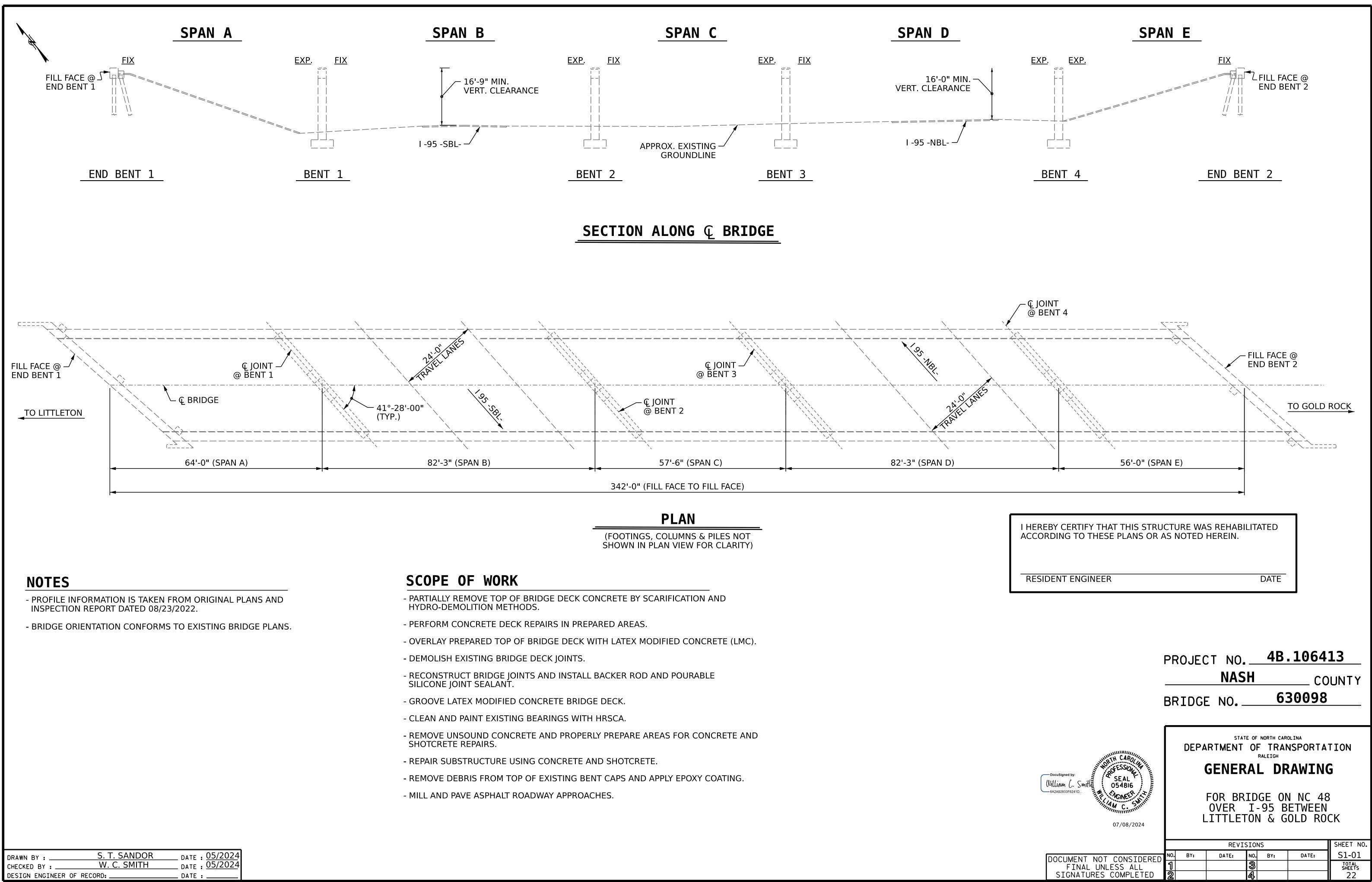
AND PAINTING EXISTING BEARINGS WITH HRSCA"

5 III SURFACE PREPARATION, AND HYDRO-DEMOLITION DN.

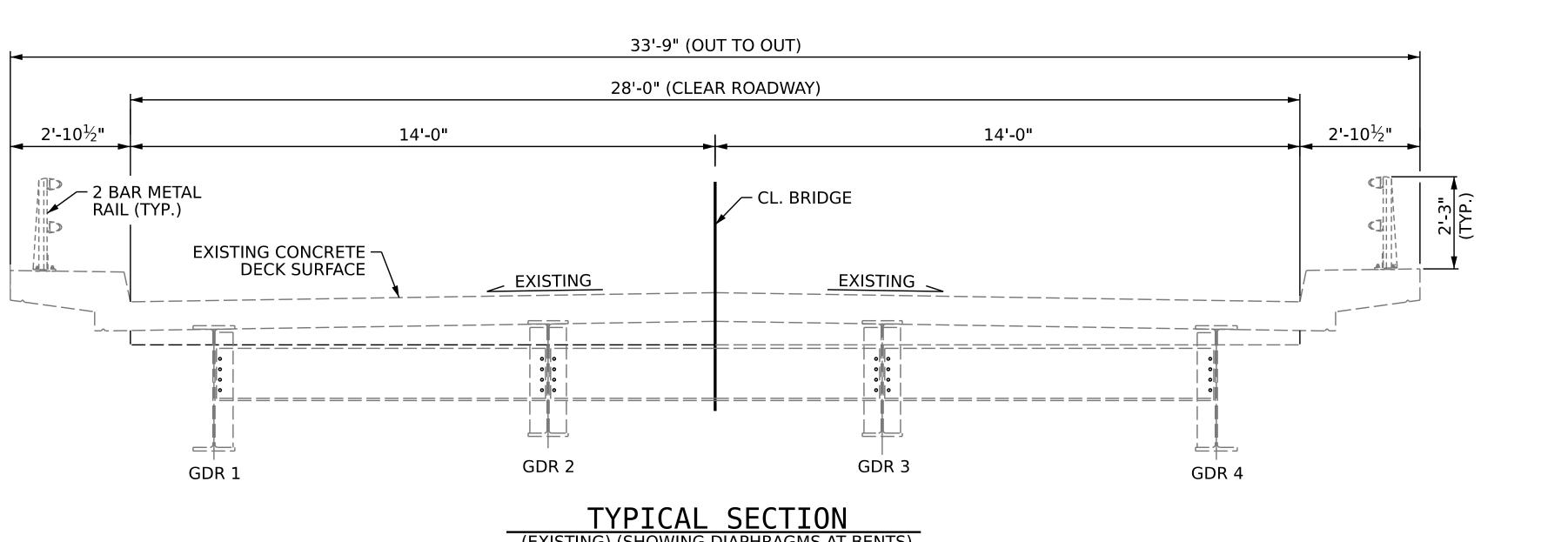
NSURE THAT HYDRO-DEMOLITION WATER DOES NOT

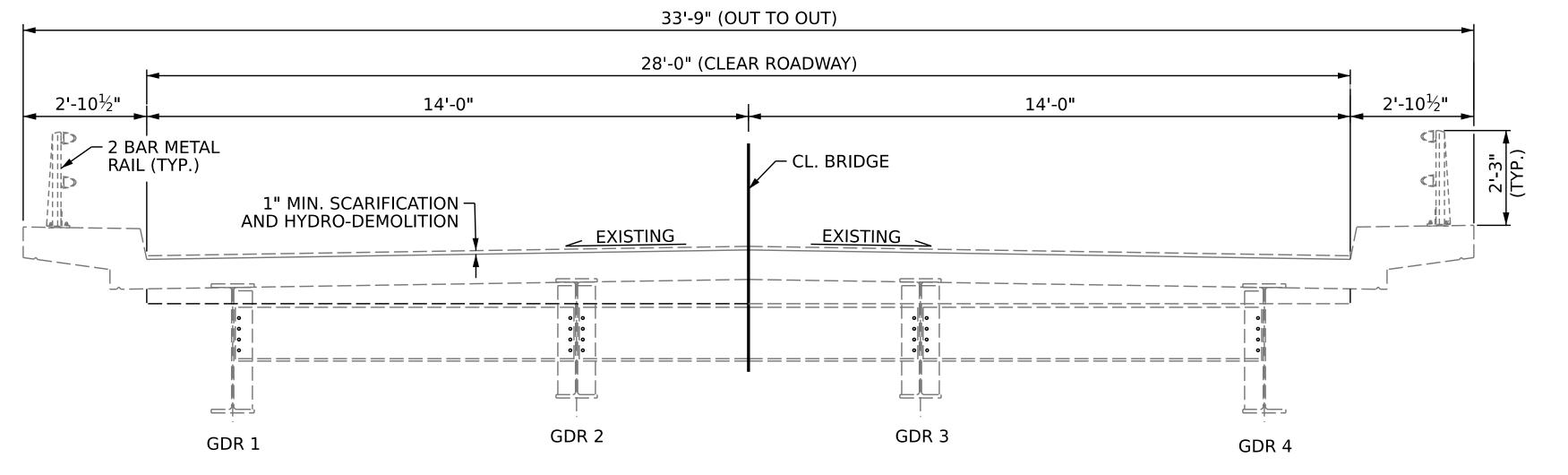
ED THAT THE FOLLOWING ITEM(S) LISTED WOULD BE OLLOWING ITEM(S) LISTED, OR OTHER WORK WILL BE FION/REHABILITATION WORK. THE CONTRACTOR SHALL RMINED IN THE FIELD. SUCH WORK SHALL BE CONSIDERED STANDARD SPECIFICATIONS. PROJECT AL ADDITIONAL WORK ITEMS HAVE BEEN PROVIDED IN UAL PAY ITEMS, QUANTITIES, AND COSTS WILL BE ESTABLISHED,

I	PROJEC	T NO NASH		.1064	13 UNTY
l	BRIDGE	E NO. 6	3009		
SUMPTH CAROLANT	DEPA	STATE (DF NORTH CARD DF TRAN RALEIGH		TION
DocuSigned by: William (Smither 6A2A92833F6241D BA2A92833F6241D	-	BILL OF			
07/08/2024					
		REVISI	ONS		SHEET NO.
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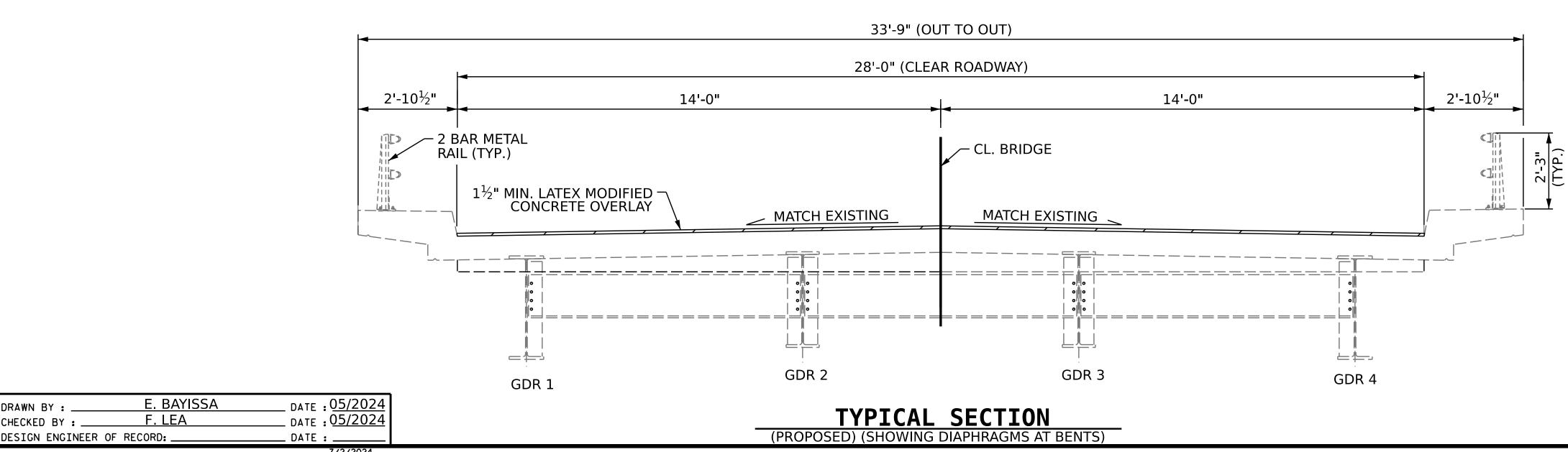


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CHECKED BY :	W.	C. SMITH	DATE	. 05/20
DESIGN ENGINEER	OF RECORD:		DATE	:









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(EXISTING) (SHOWING DIAPHRAGMS AT BENTS)

1" MIN.

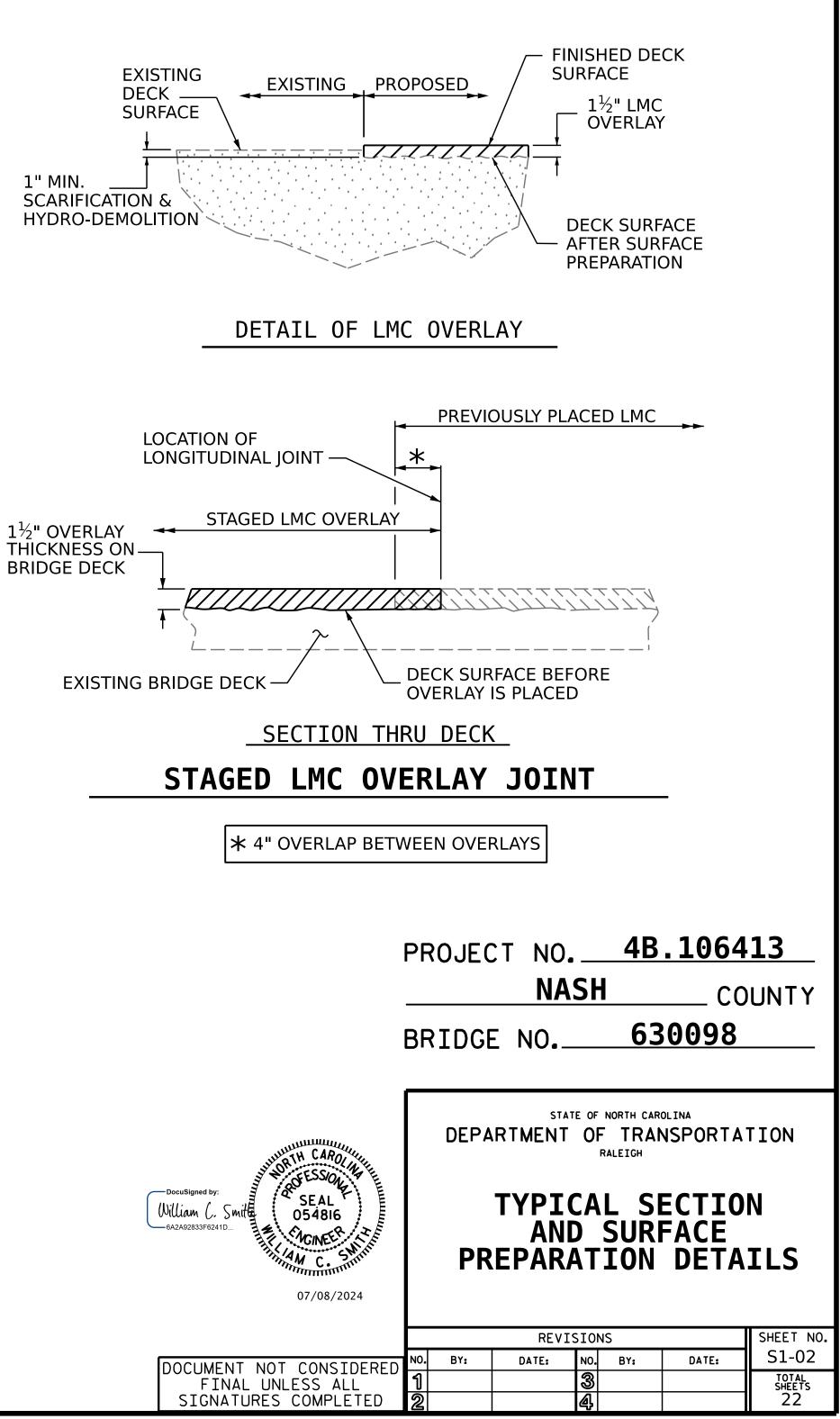


NOTES

FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF OVERLAY SURFACE PREPARATION AND LATEX MODIFIED CONCRETE (LMC) PLACEMENT, SEE ELSEWHERE IN THE CONTRACT DOCUMENTS.

WHEN PREPARING THE SURFACE FOR LMC OVERLAY ADJACENT TO THE PREVIOUSLY PLACED LMC STAGE, THE PREVIOUSLY PLACED LMC SHALL BE SAW-CUT TO THE FULL DEPTH OF THE LMC AT THE CENTERLINE OF THE BRIDGE AND ALL LMC IN THE 4" OVERLAP SHALL BE REMOVED WITH HAND TOOLS PRIOR TO PLACEMENT OF LMC IN THE SECOND STAGE.

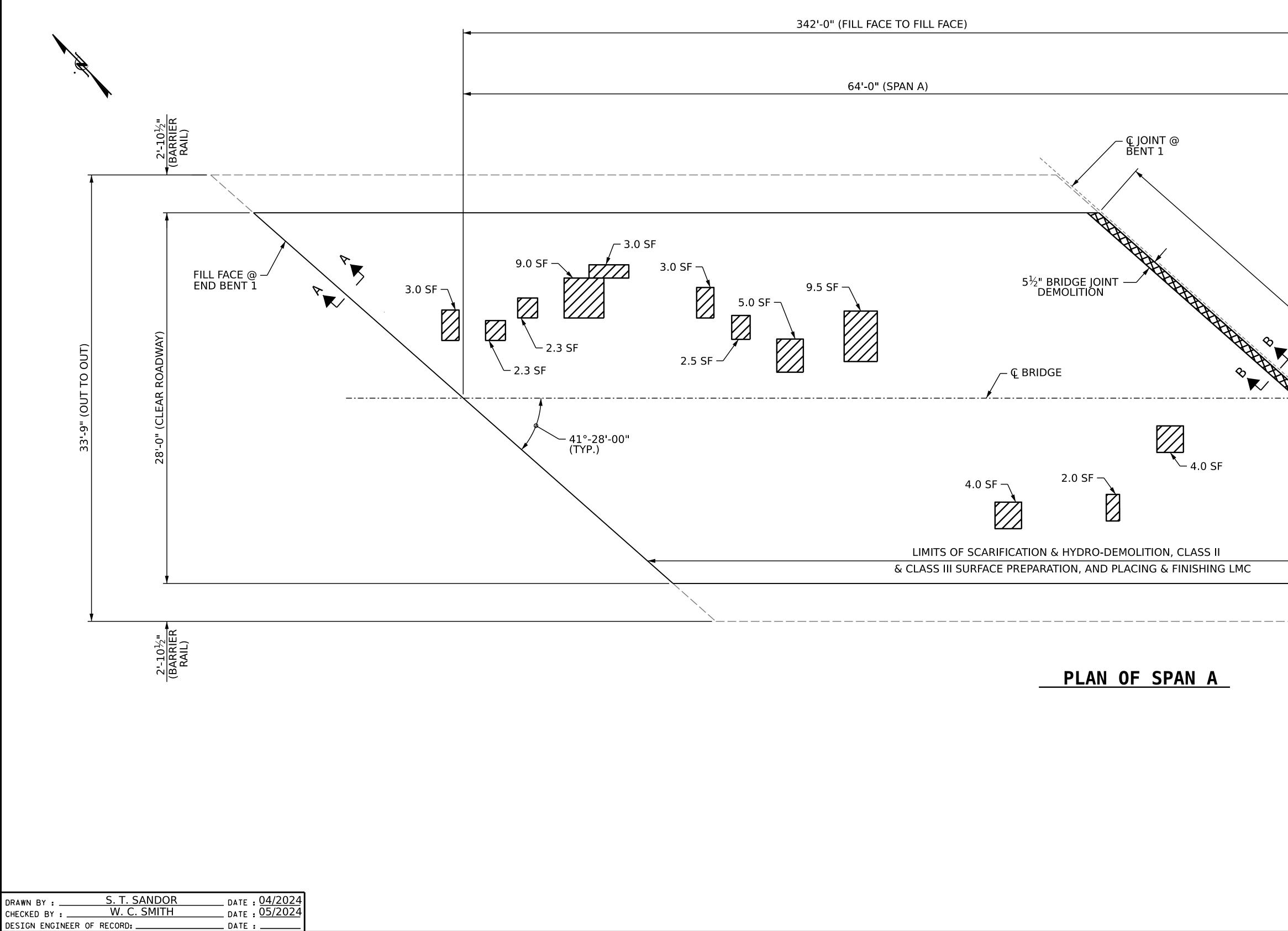
THE CONTRACTOR IS ALERTED TO THE FACT THAT THERE ARE MANY AREAS IN THE EXISTING BRIDGE DECK WHERE CONCRETE COVER OVER THE TOP MAT OF REINFORCING STEEL IS APPROXIMATELY $\frac{1}{2}$ ". APPROPRIATE CARE AND MEASURES SHALL BE TAKEN TO ENSURE THAT REINFORCING BARS ARE NOT DAMAGED DURING SURFACE PREPARATION OPERATIONS.



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FOR SECTIONS A-A AND B-B, SEE "JOINT DETAIL" SHEET S1-13. FOR DECK REPAIRS, SEE "DECK REPAIR DETAILS" SHEET S1-14.



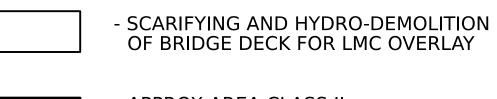
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DECK SURFACE REPAIR QUANTITY TABLE

DECK SURFACE REPAIR - SPAN A

	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	197.5 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	197.5 SQ. YDS.	
CLASS II SURFACE PREPARATION	5.5 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	9.8 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	197.5 SQ. YDS.	
GROOVING BRIDGE DECK	1591.7 SF	
BRIDGE JOINT DEMOLITION	19.4 SF	

QUANTITIES FOR LMC OVERLAY ARE BASED ON OVERLAY DEPTH PLUS AN ADDITIONAL $\frac{1}{4}$ " TO ACCOUNT FOR IRREGULARITIES IN HYDRO-DEMOLITION PROCESSES.





- APPROX AREA CLASS II SURFACE PREPARATION



⁴ SEAL ^{*} 054816

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William C. Smit

- BRIDGE JOINT DEMOLITION

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	ΓY

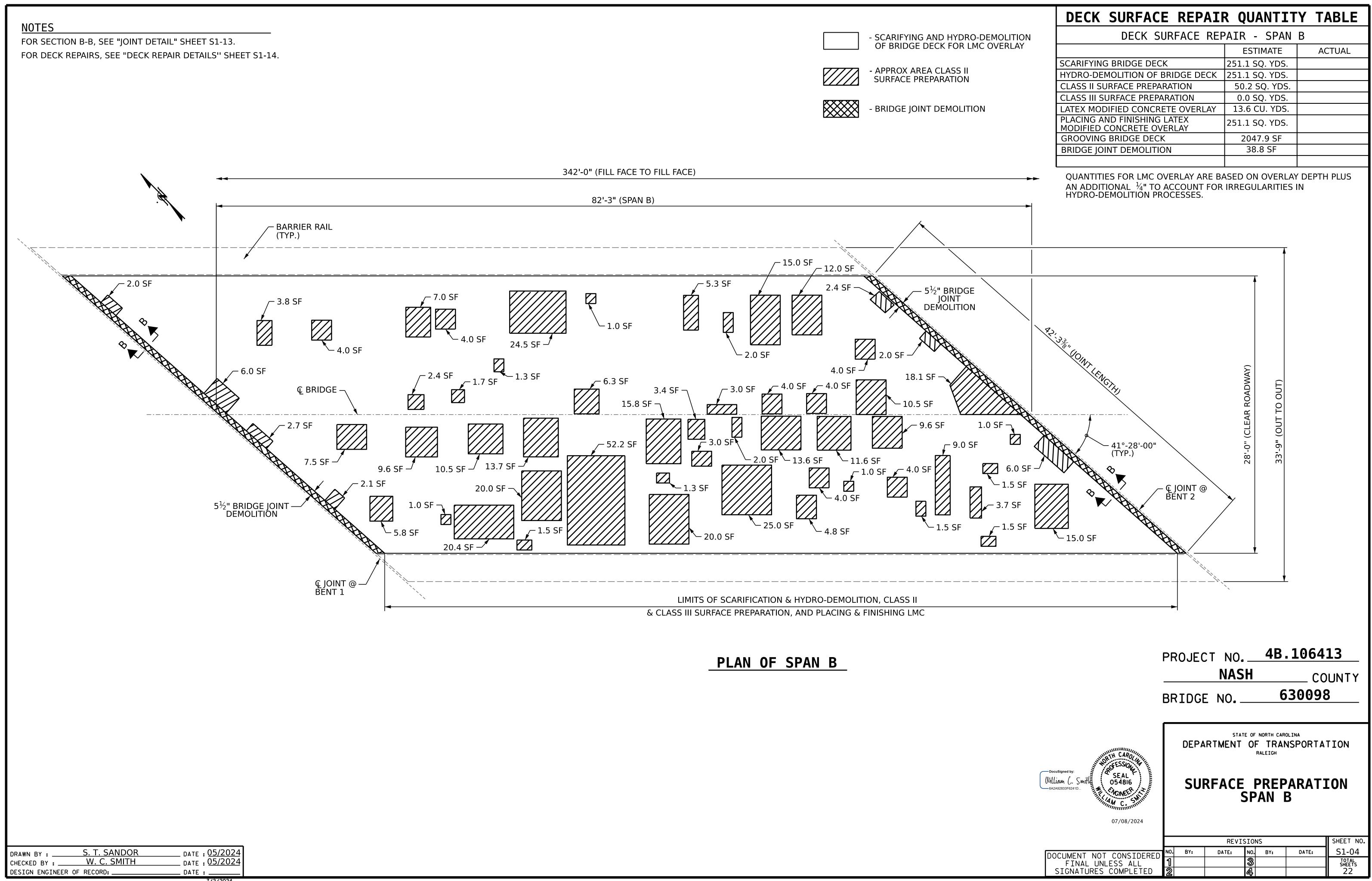
BRIDGE NO.____

630098

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SURFACE PREPARATION SPAN A

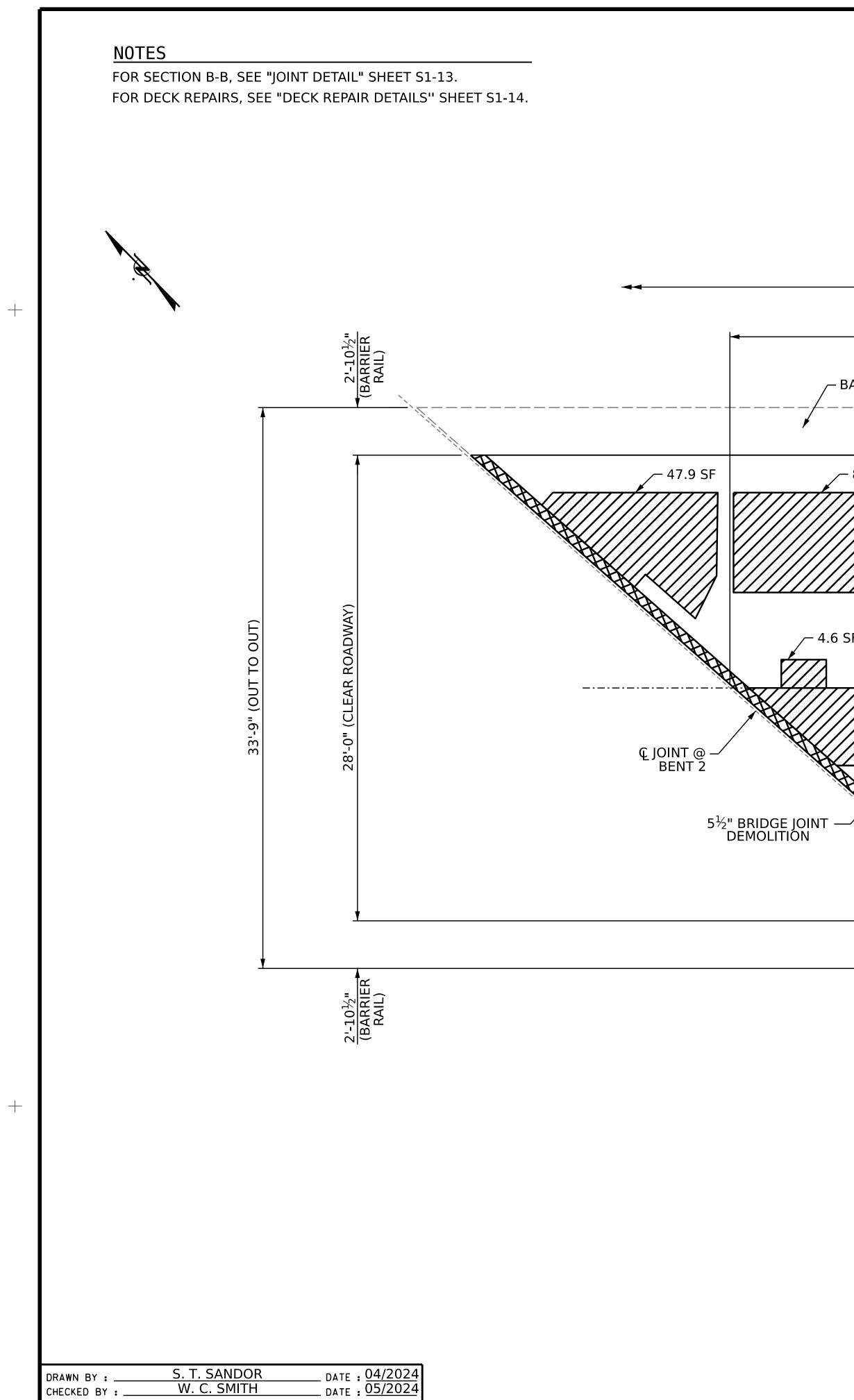
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		REVI	ISION	s		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO.	BY:	DATE:	S1-03
FINAL UNLESS ALL	1		3			TOTAL SHEETS
SIGNATURES COMPLETED	2		4			22









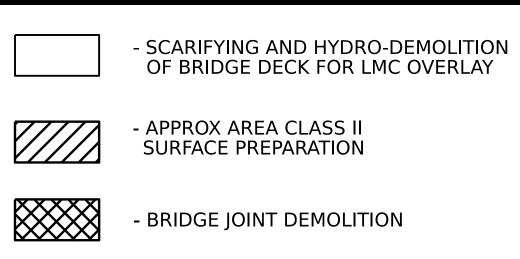


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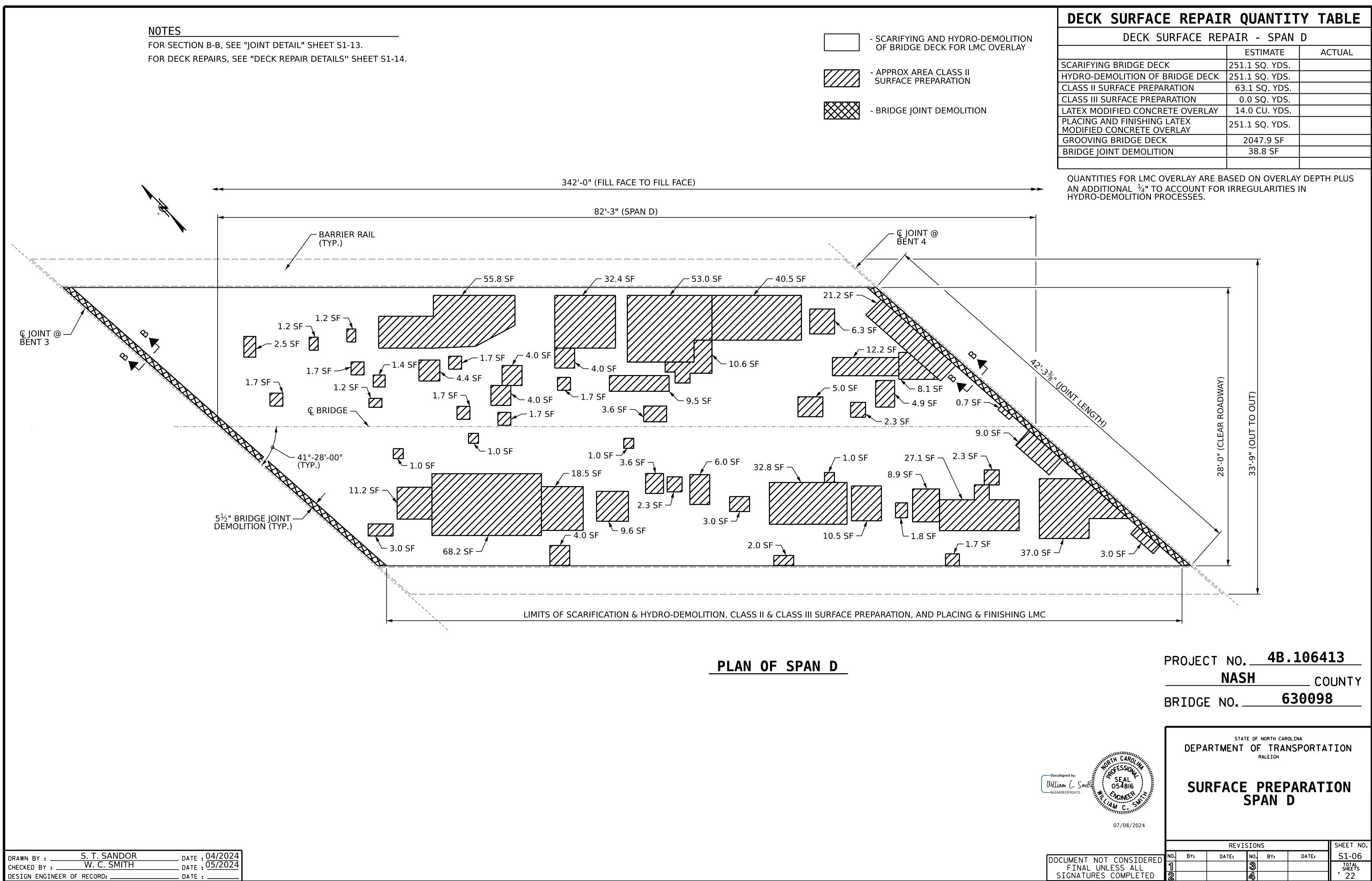


342'-0" (FILL FACE TO FILL FACE) 57'-6" (SPAN C) - Ç JOINT @ BENT 3 – BARRIER RAIL (TYP.) 6.0 SF -9.0 SF ∕-- 84.0 SF - 4.0 SF -14.7 SF 29.2 SF --5¹⁄2" BRIDGE JOINT DEMOLITION 5.0 SF --1.0 SF └─ 4.0 SF 27.8 SF – 58.8 SF /- 3.3 SF 1.0 SF 10.0 SF -1.2 SF € BRIDGE –∕ ∽ 3.0 SF <u>\</u>_12.2 SF /4.0 SF – 29.3 SF -11.2 SF 1.0 SF 6.0 SF –⁄ 5.8 SF 81.8 SF /- 32.6 SF 40.0 SF /- 5.5 SF 26.6 SF -_____ 19.9 SF -16.0 SF 13.5 SF 1.0 SF -∽ 4.2 SF LIMITS OF SCARIFICATION & HYDRO-DEMOLITION, CLASS

& CLASS III SURFACE PREPARATION, AND PLACING & FINISHING

PLAN OF SPAN C

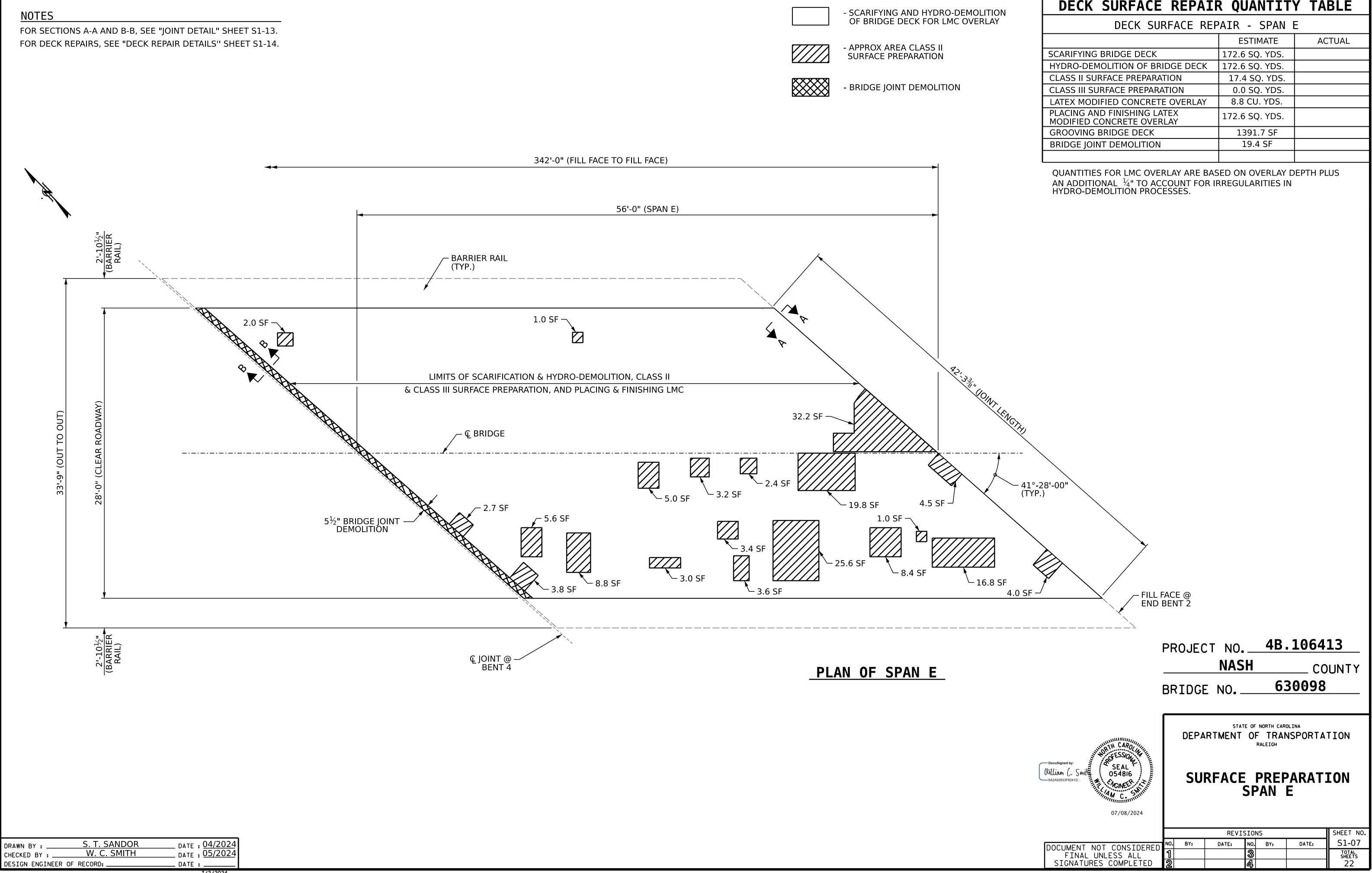
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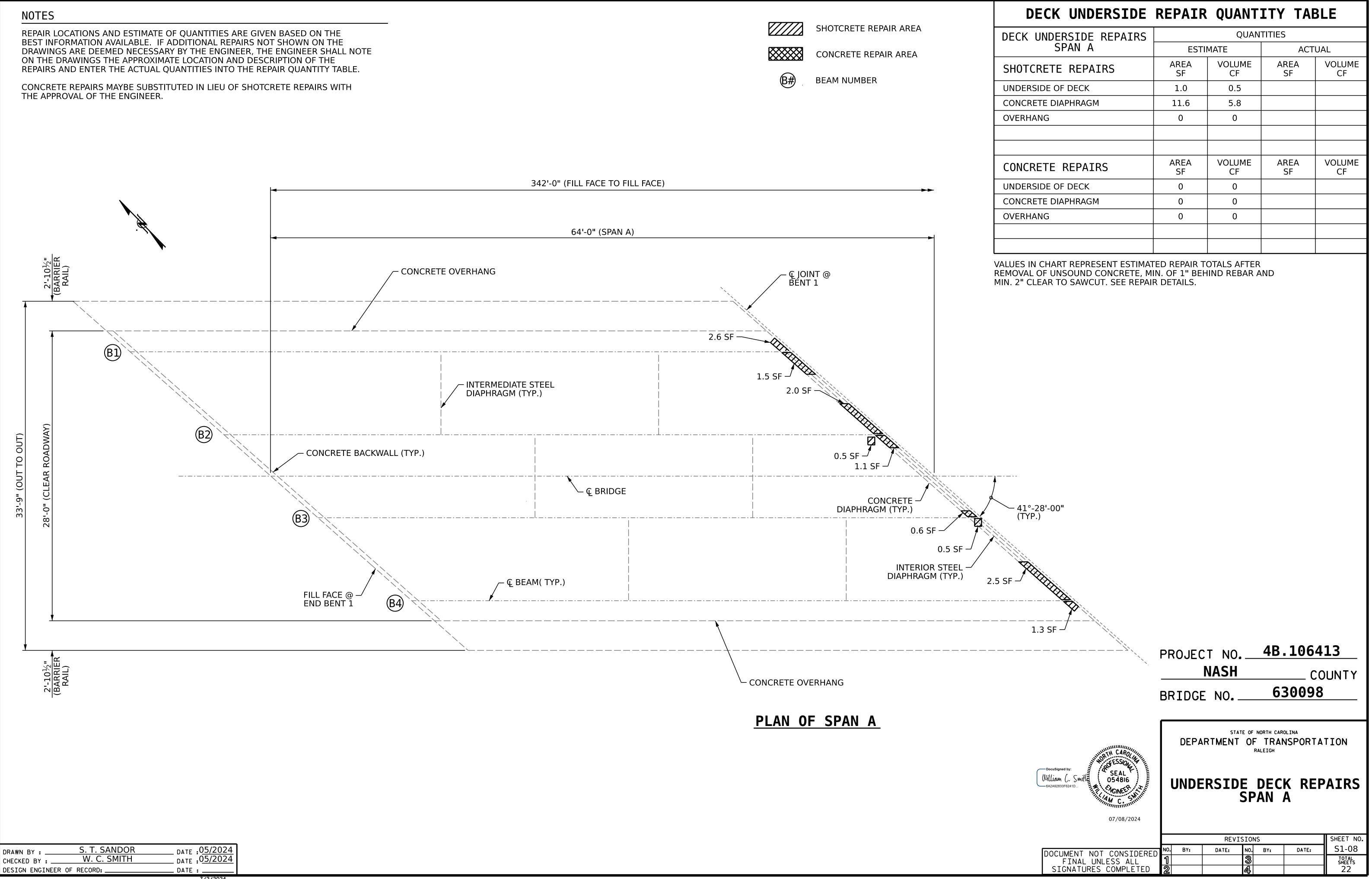
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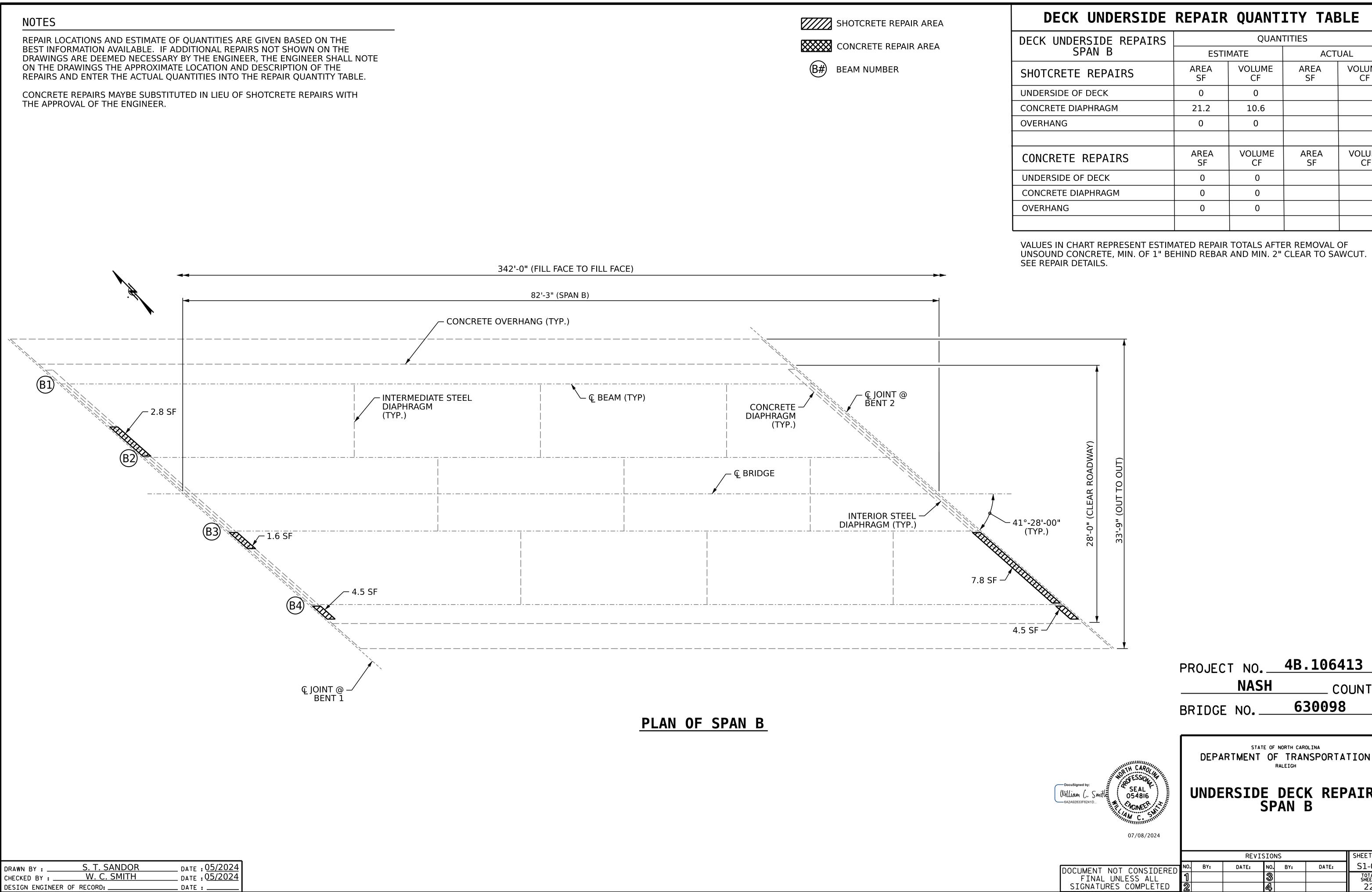




DECK SURFACE REPAIR	R QUANTIT	Y TABLE			
DECK SURFACE REPAIR - SPAN E					
	ESTIMATE	ACTUAL			
SCARIFYING BRIDGE DECK	172.6 SQ. YDS.				
HYDRO-DEMOLITION OF BRIDGE DECK	172.6 SQ. YDS.				
CLASS II SURFACE PREPARATION	17.4 SQ. YDS.				
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.				
LATEX MODIFIED CONCRETE OVERLAY	8.8 CU. YDS.				
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	172.6 SQ. YDS.				
GROOVING BRIDGE DECK	1391.7 SF				
BRIDGE JOINT DEMOLITION	19.4 SF				



DECK UNDERSIDE	REPAIR	QUANT	ITY TAE	BLE
DECK UNDERSIDE REPAIRS		QUAN	TITIES	
SPAN A	ESTIN	ИАТЕ	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	1.0	0.5		
CONCRETE DIAPHRAGM	11.6	5.8		
OVERHANG	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		



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DECK UNDERSIDE	REPAIR	QUANT	ΙΤΥ ΤΑΕ	BLE
DECK UNDERSIDE REPAIRS		QUAN	TITIES	
SPAN B	ESTIN	ИАТЕ	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	21.2	10.6		
OVERHANG	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		

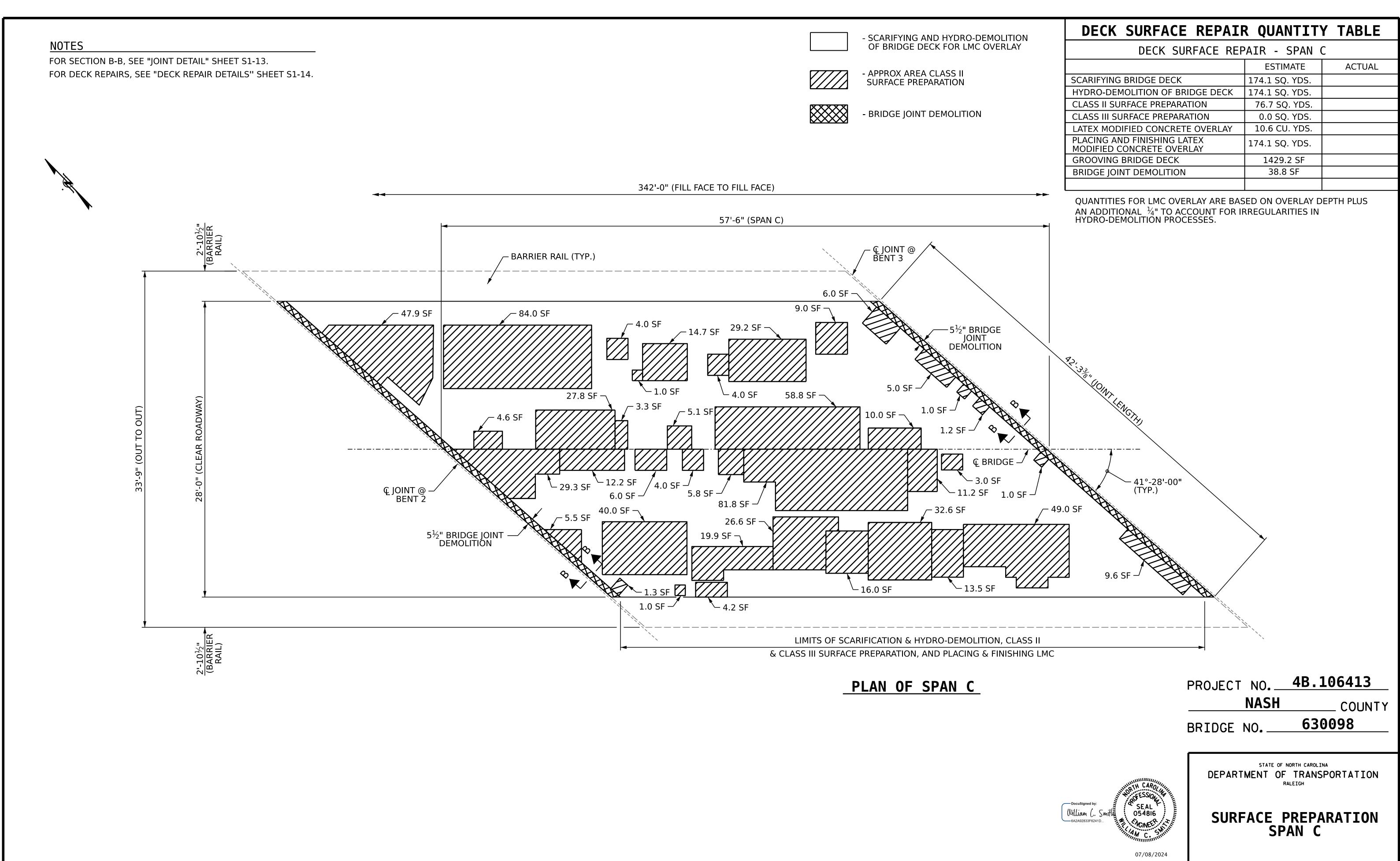
VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT.

054816 MCINEER MC . SMITHIN 07/08/2024	UNDERSIDE DECK REPAIRS SPAN B								
			SHEET NO.						
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ESS ALL	1			3			TOTAL SHEETS		
COMPLETED	2			4			22		

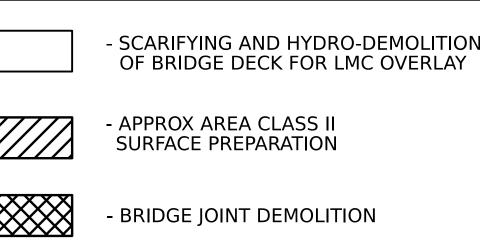
STATE OF NORTH CAROLINA

COUNTY

630098

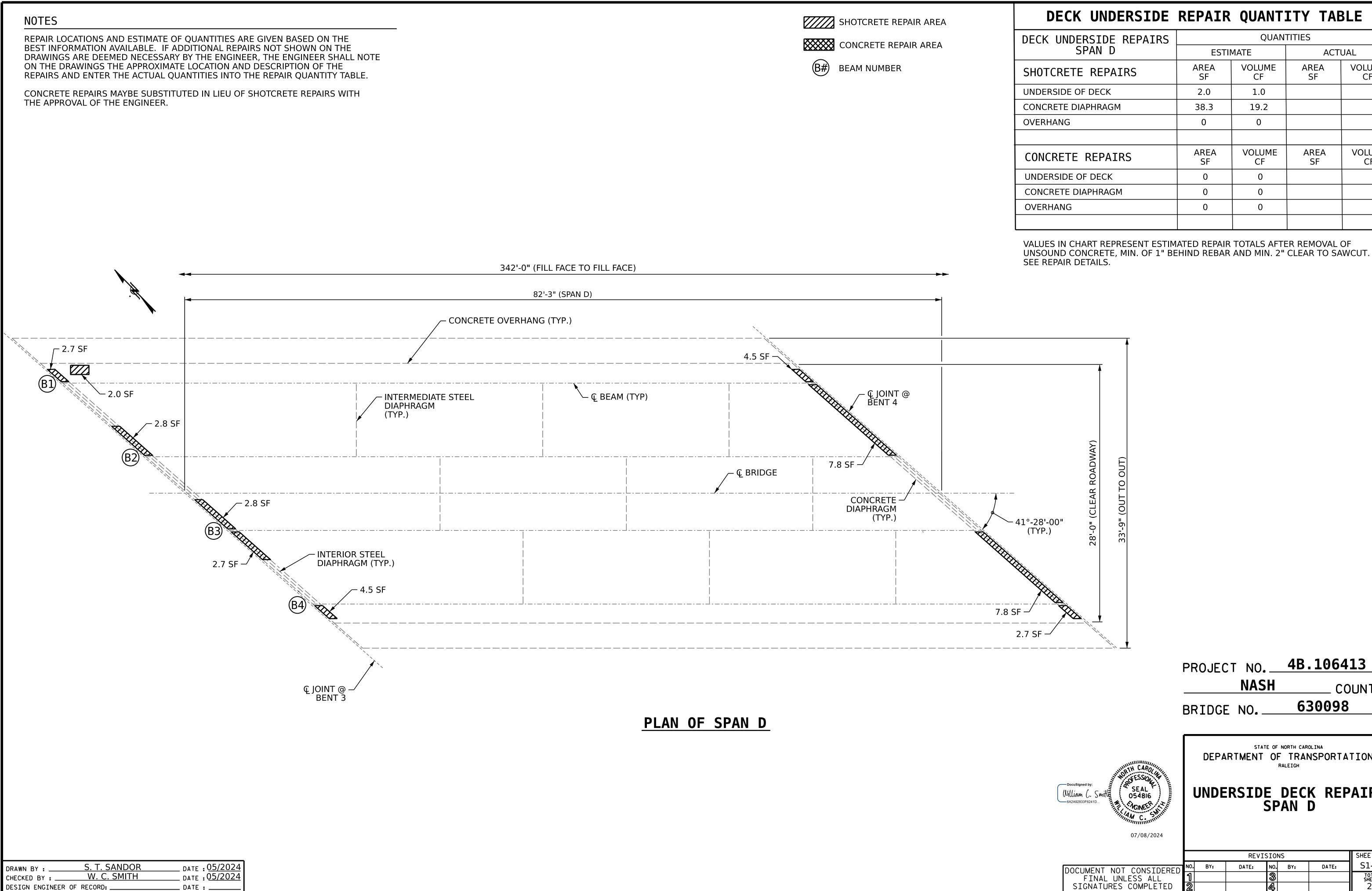


DRAWN BY :	S. T. SANDOR	DATE : 04/2024
CHECKED BY :	W. C. SMITH	DATE : 05/2024
DESIGN ENGINEER	R OF RECORD:	DATE :



SURFACE	PRE	PARATION
S	PAN	С

07/08/2024							
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FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			22



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DECK UNDERSIDE	REPAIR	QUANT	ΙΤΥ ΤΑΕ	BLE			
DECK UNDERSIDE REPAIRS	QUANTITIES						
SPAN D	ESTIN	ИАТЕ	ACT	UAL			
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
UNDERSIDE OF DECK	2.0	1.0					
CONCRETE DIAPHRAGM	38.3	19.2					
OVERHANG	0	0					
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF			
UNDERSIDE OF DECK	0	0					
CONCRETE DIAPHRAGM	0	0					
OVERHANG	0	0					

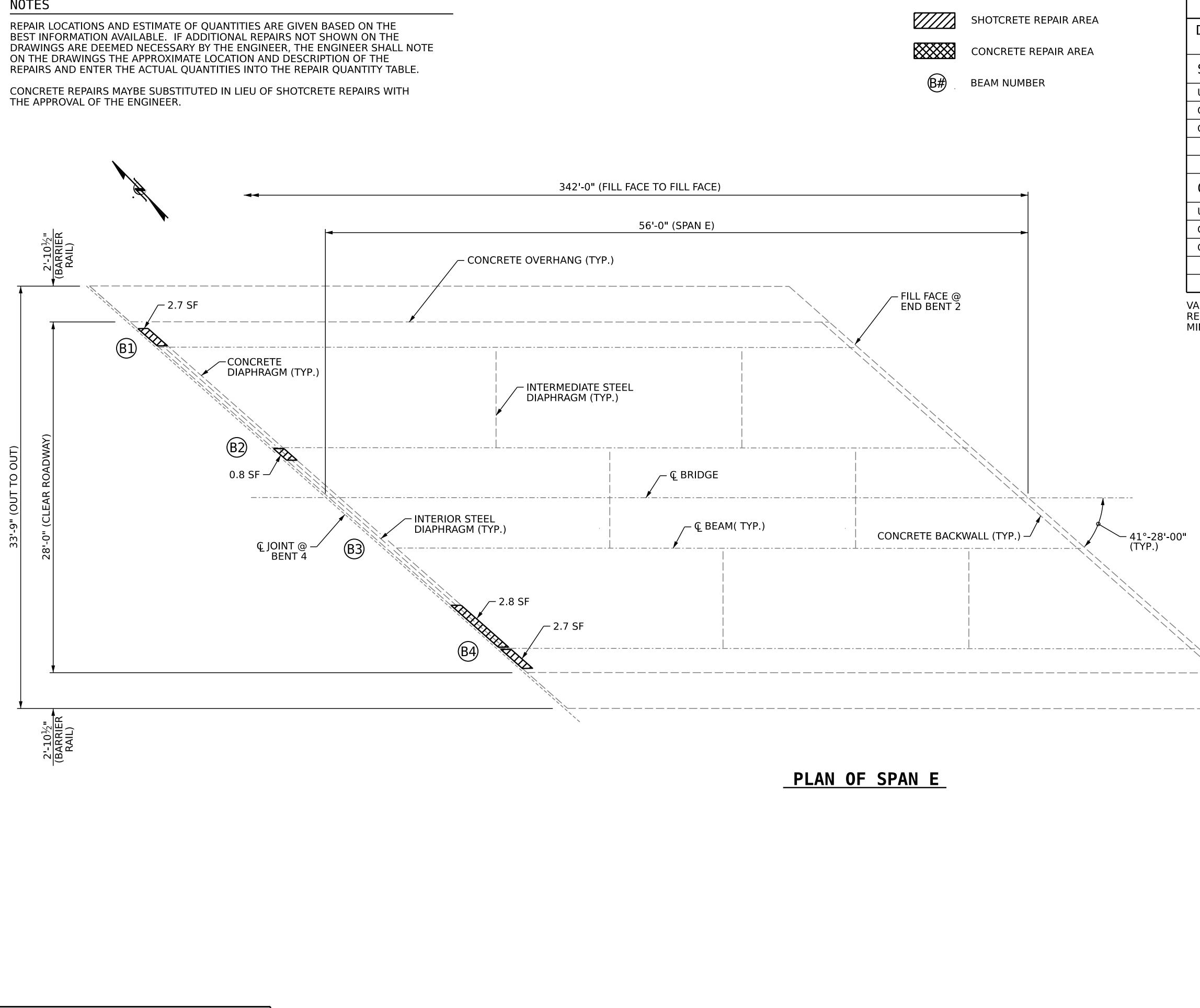
VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT.

NASH	COUNTY
BRIDGE NO	630098
DEPARTMENT OF	NORTH CAROLINA TRANSPORTATION ALEIGH
UNDERSIDE	DECK REPAIRS AN D

	_		DEVI	ISION	c		SHEET NO.
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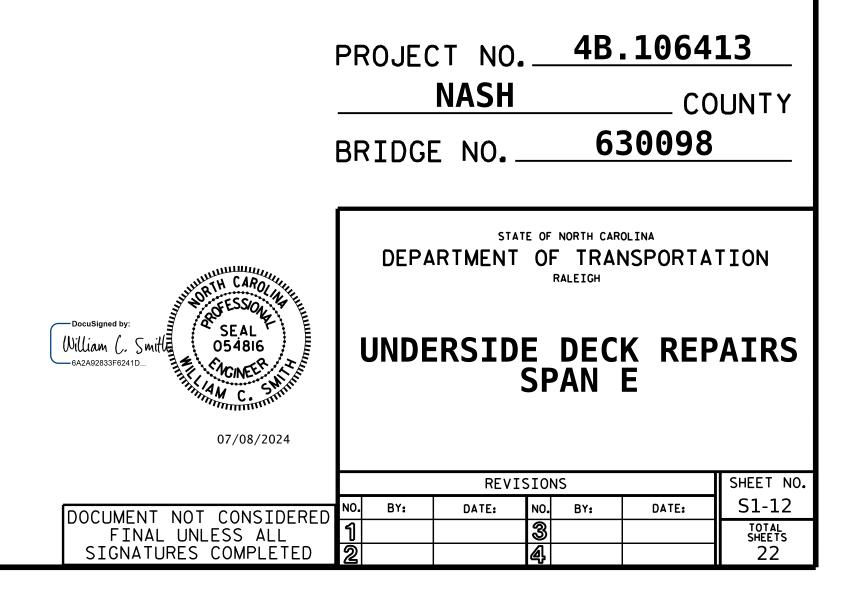


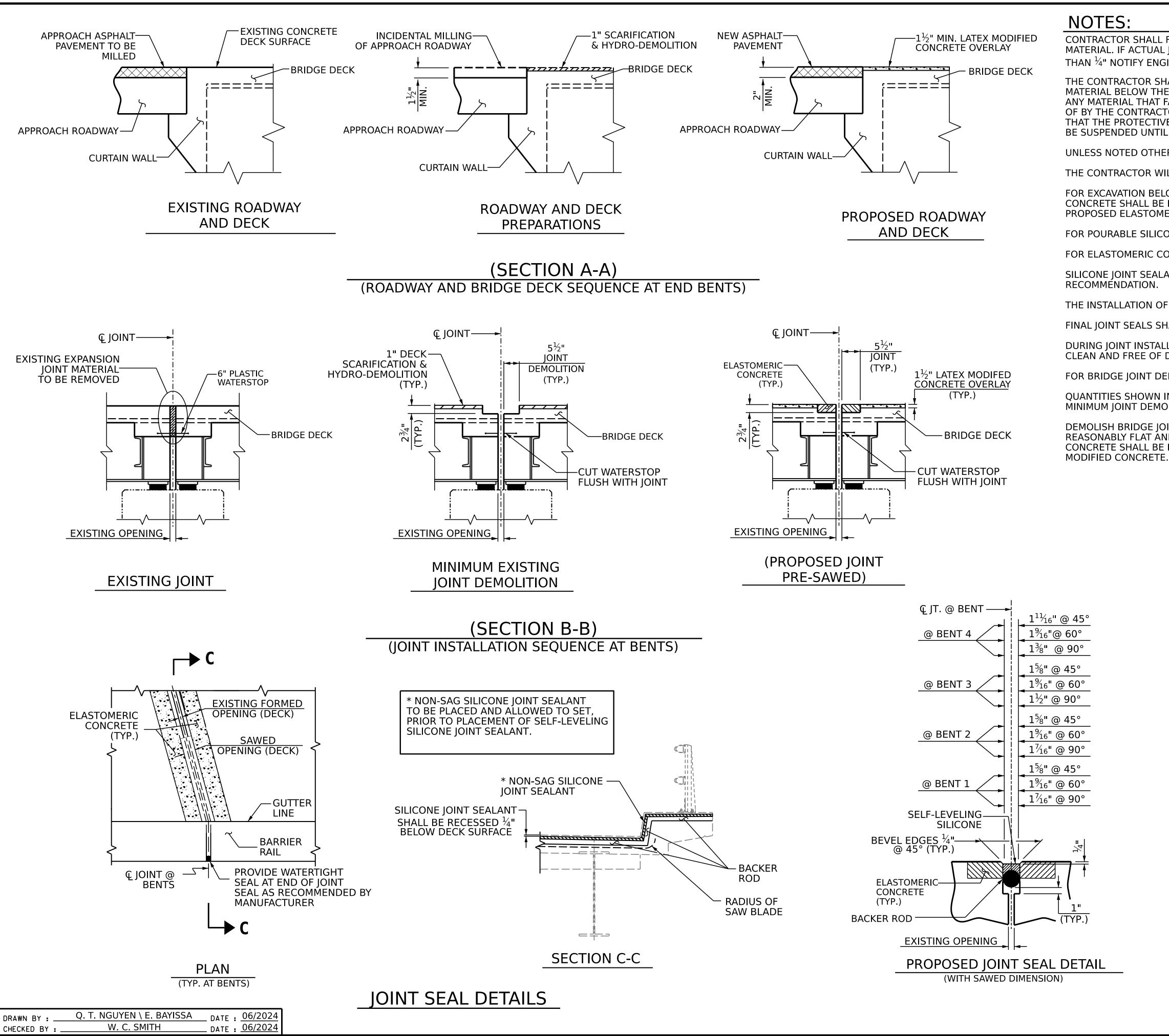
DRAWN BY :	S. T. SANDOR	DATE :05/2024
CHECKED BY :	W. C. SMITH	DATE :05/2024
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DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE				
DECK UNDERSIDE REPAIRS	QUANTITIES							
SPAN E	ESTI	MATE	ACT	UAL				
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF				
UNDERSIDE OF DECK	0	0						
CONCRETE DIAPHRAGM	9.0	4.5						
OVERHANG	0	0						
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF				
UNDERSIDE OF DECK	0	0						
CONCRETE DIAPHRAGM	0	0						
OVERHANG	0	0						

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.





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CONTRACTOR SHALL FIELD VERIFY THE EXISTING FORMED OPENING PRIOR TO OBTAINING JOINT MATERIAL. IF ACTUAL JOINT OPENINGS VARIES FROM THE OPENING INDICATED IN DETAIL MORE THAN $\frac{1}{4}$ " NOTIFY ENGINEER. REVISION TO THE JOINT SEAL SIZE MAY BE NECESSARY.

THE CONTRACTOR SHALL TAKE CARE DURING JOINT REHAB OPERATIONS NOT TO DROP ANY MATERIAL BELOW THE BRIDGE WITHOUT PROTECTIVE DEVICES BELOW TO CATCH THE MATERIAL. ANY MATERIAL THAT FALLS BELOW THE BRIDGE SHALL BE CONTAINED, REMOVED AND DISPOSED OF BY THE CONTRACTOR AT NO EXTRA COST TO THE DEPARTMENT. IF THE ENGINEER DETERMINES THAT THE PROTECTIVE DEVICES ARE NOT ADEQUATE OR NOT BEING EMPLOYED, THE WORK SHALL BE SUSPENDED UNTIL ADEQUATE PROTECTION IS PROVIDED.

UNLESS NOTED OTHERWISE RETAIN ALL EXISTING REINFORCING STEEL. CLEAN AND REPAIR AS NEEDED.

THE CONTRACTOR WILL NOT BE PERMITTED TO FORM THE JOINTS IN LIEU OF SAWING THE JOINT.

FOR EXCAVATION BELOW THE BOTTOM OF THE PLANNED JOINT DEMOLITION, APPROVED REPAIR CONCRETE SHALL BE PLACED IN THE EXCAVATED AREA TO THE ELEVATION AT BOTTOM OF THE PROPOSED ELASTOMERIC CONCRETE FOR PRESERVATION HEADERS SHOWN.

FOR POURABLE SILICONE JOINT SEALANT, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC CONCRETE FOR PRESERVATION, SEE SPECIAL PROVISIONS.

SILICONE IOINT SEALANT AND BACKER ROD SHALL BE INSTALLED AS PER MANUFACTURER'S

THE INSTALLATION OF JOINT SEAL SHALL BE WATERTIGHT.

FINAL JOINT SEALS SHALL NOT BE INSTALLED UNTIL THE OVERLAY IS COMPLETE.

DURING JOINT INSTALLATION PROCEDURE, THE JOINT AND SURROUNDING AREA SHALL BE KEPT CLEAN AND FREE OF DEBRIS.

FOR BRIDGE JOINT DEMOLITION, SEE SPECIAL PROVISIONS.

QUANTITIES SHOWN IN THE ELASTOMERIC CONCRETE FOR PRESERVATION ARE BASED ON THE MINIMUM JOINT DEMOLITION SHOWN.

DEMOLISH BRIDGE JOINT AREA SUCH THAT THE BOTTOM OF THE EXCAVATION SHALL BE REASONABLY FLAT AND LEVEL AND TO THE NECESSARY DEPTH. SUCH THAT ELASTOMERIC CONCRETE SHALL BE FOUNDED ON CONCRETE OR REPAIR CONCRETE SUBSTRATE, NOT LATEX

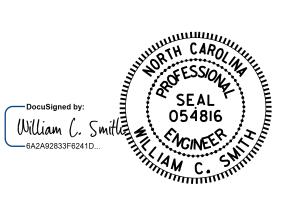
SUMMARY OF QUANTITIES						
	ESTIMATE	ACTUAL				
ELASTOMERIC CONCRETE FOR PRESERVATION	35.5 CF					
POURABLE SILICONE JOINT SEALANT	203.7 LF					

PROJECT NO. 48.106413

NASH COUNTY

630098 BRIDGE NO.

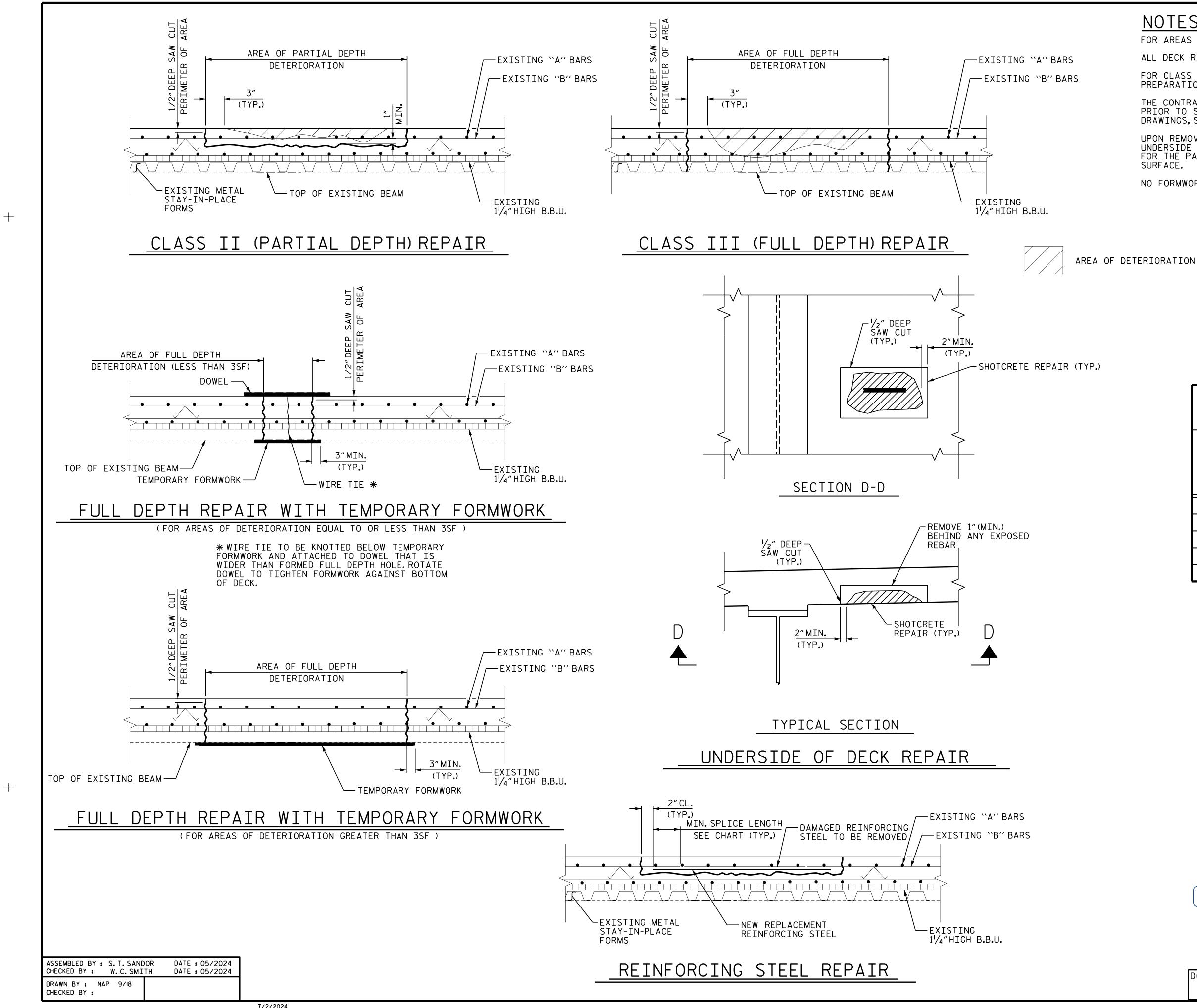
> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH



07/08/2024

JOINT REPAIR DETAILS

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DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S1-13
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			22



NOTES

FOR AREAS TO BE REPAIRED, SEE "PLAN OF SPAN" SHEETS.

ALL DECK REPAIRS SHALL BE COMPLETED PRIOR TO PLACEMENT OF OVERLAY.

FOR CLASS II AND CLASS III SURFACE PREPARATION, SEE "LMC OVERLAY SURFACE PREPARATIONS" SPECIAL PROVISION.

THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO STARTING WORK FOR TEMPORARY FORMWORK.FOR SUBMITTALS OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

UPON REMOVAL OF TEMPORARY FORMWORK, ALL VOIDS AND HONEYCOMBS ON THE UNDERSIDE OF DECK SURFACE SHALL BE FILLED WITH THE SAME MATERIAL AS USED FOR THE PATCH, AND FINISHED TO CONFORM TO THE SURROUNDING CONCRETE SURFACE.

NO FORMWORK SHALL BE LEFT IN PLACE.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS									
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	APPROACH ARAPET,	APPROAC	PARAPET AND BARRIER					
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL				
# 4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"				
* 5	2'-5"	2'-0"	2'-5″	2'-0"	3'-1"				
* 6	2'-10"	2′-5″	3'-7"	2'-5″	3′-8″				
# 7	4'-2"	2'-9"							
* 8	4′-9″	3'-2"							

	PROJECT NO.	4B.10	6413
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	BRIDGE NO.	63009	98
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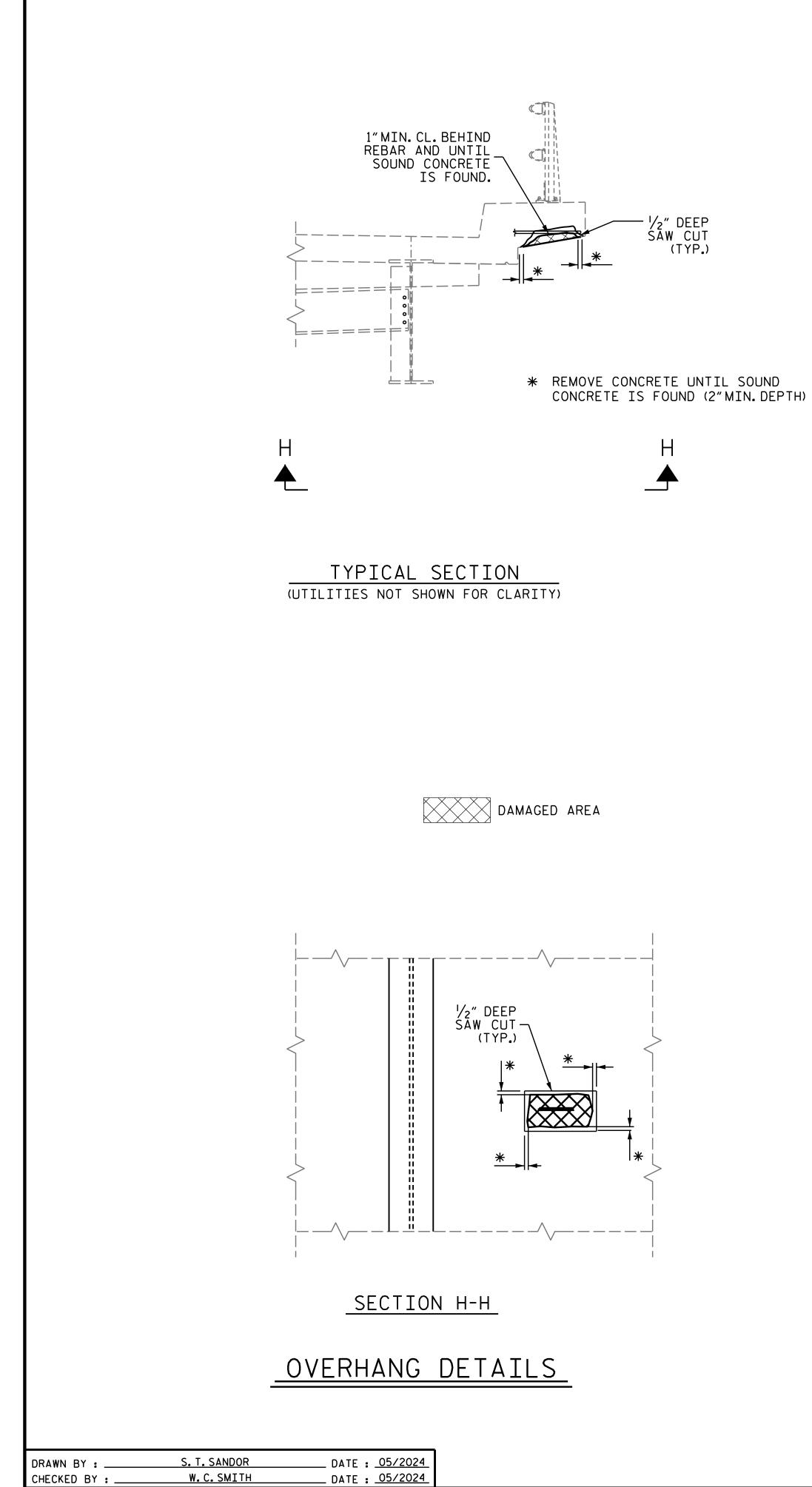
STANDARD

DECK REPAIR DETAILS

STH LARO OFESSION, لاً SEAL 0548I6 1 William C. Strike NCINEEP N

07/08/2024

		REVISIONS			SHEET NO.		
DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S1-14
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			4			22



THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

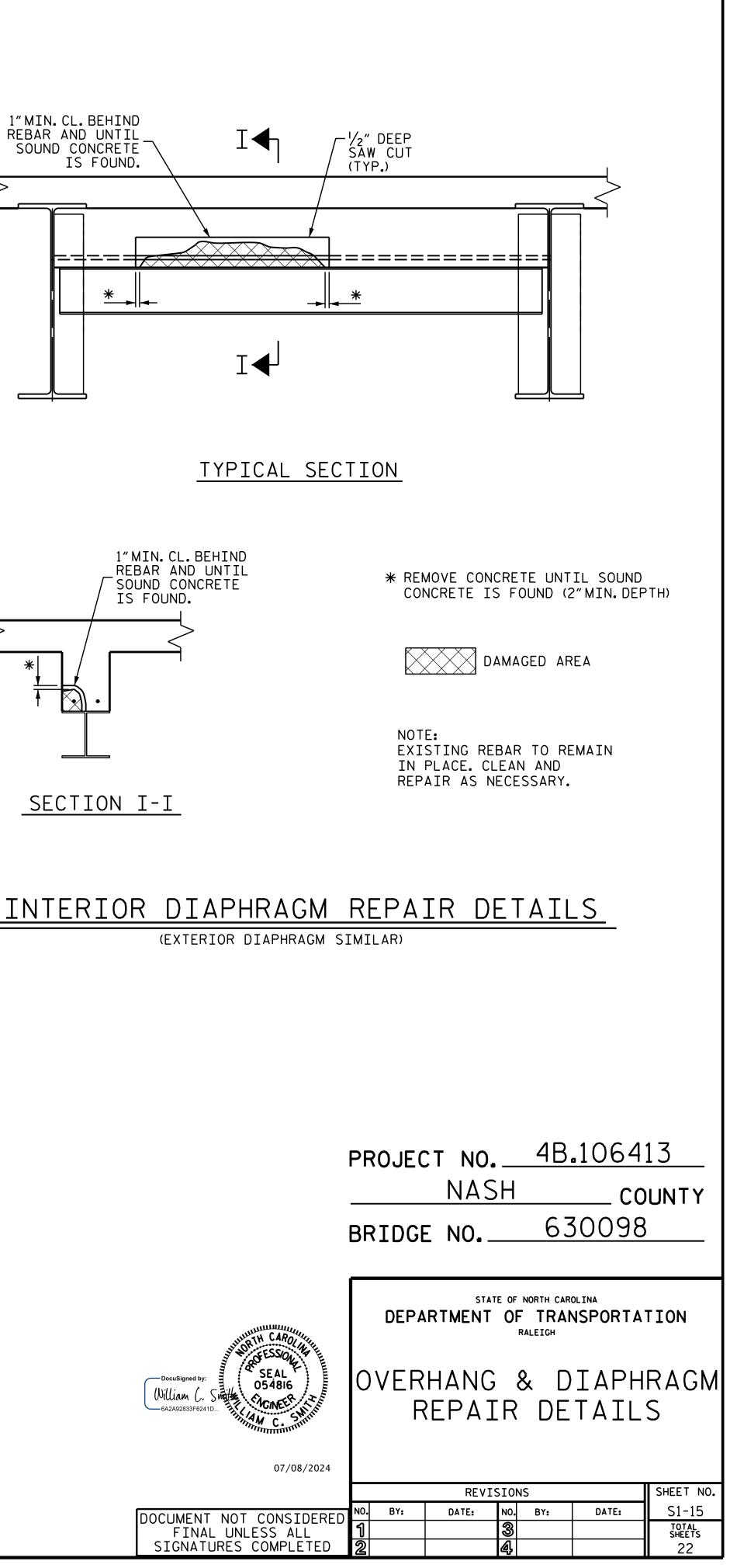
REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON the "DECK REPAIR DETAIL" SHEET.

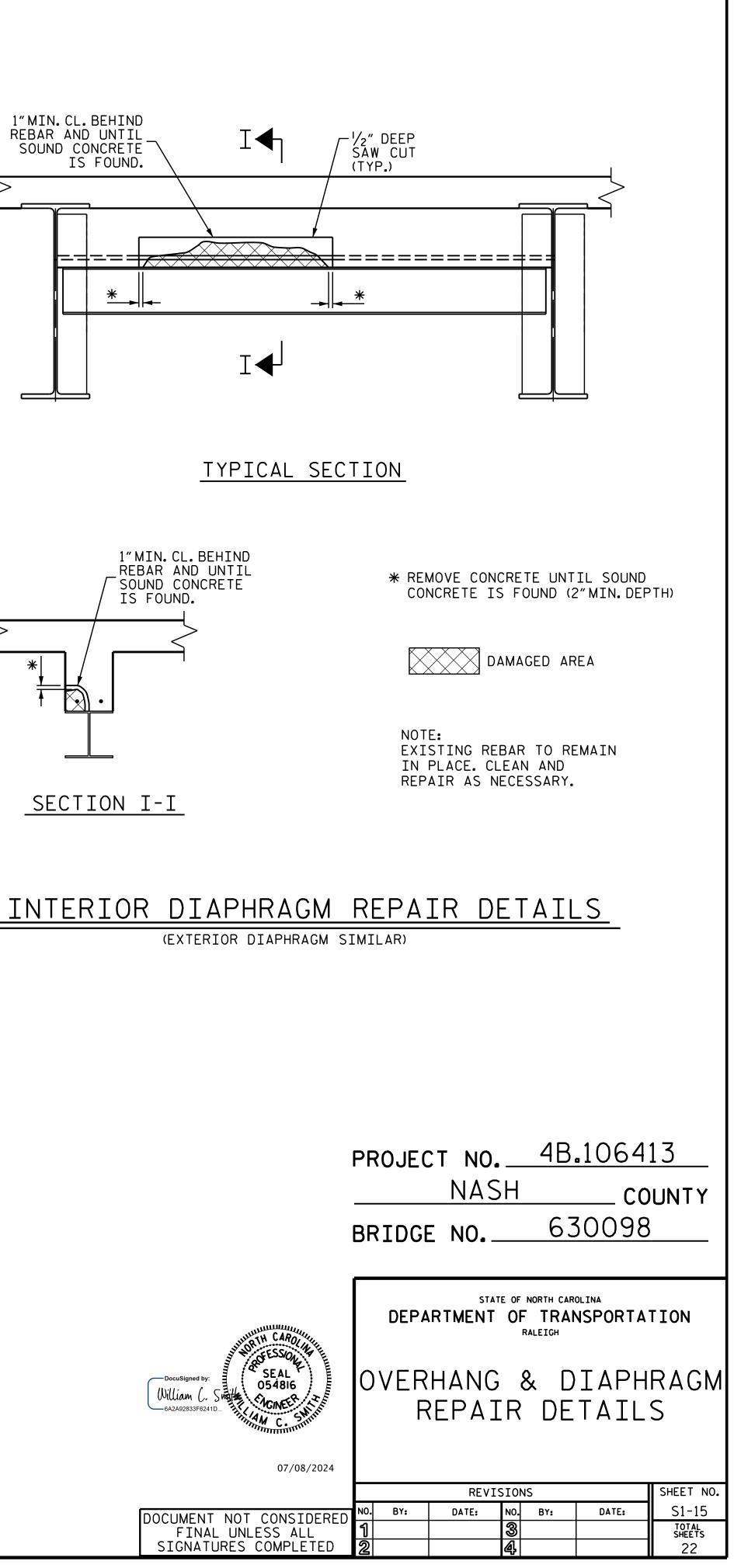
FOR AREAS TO BE REPAIRED, SEE "UNDERSIDE DECK REPAIRS" SHEETS.

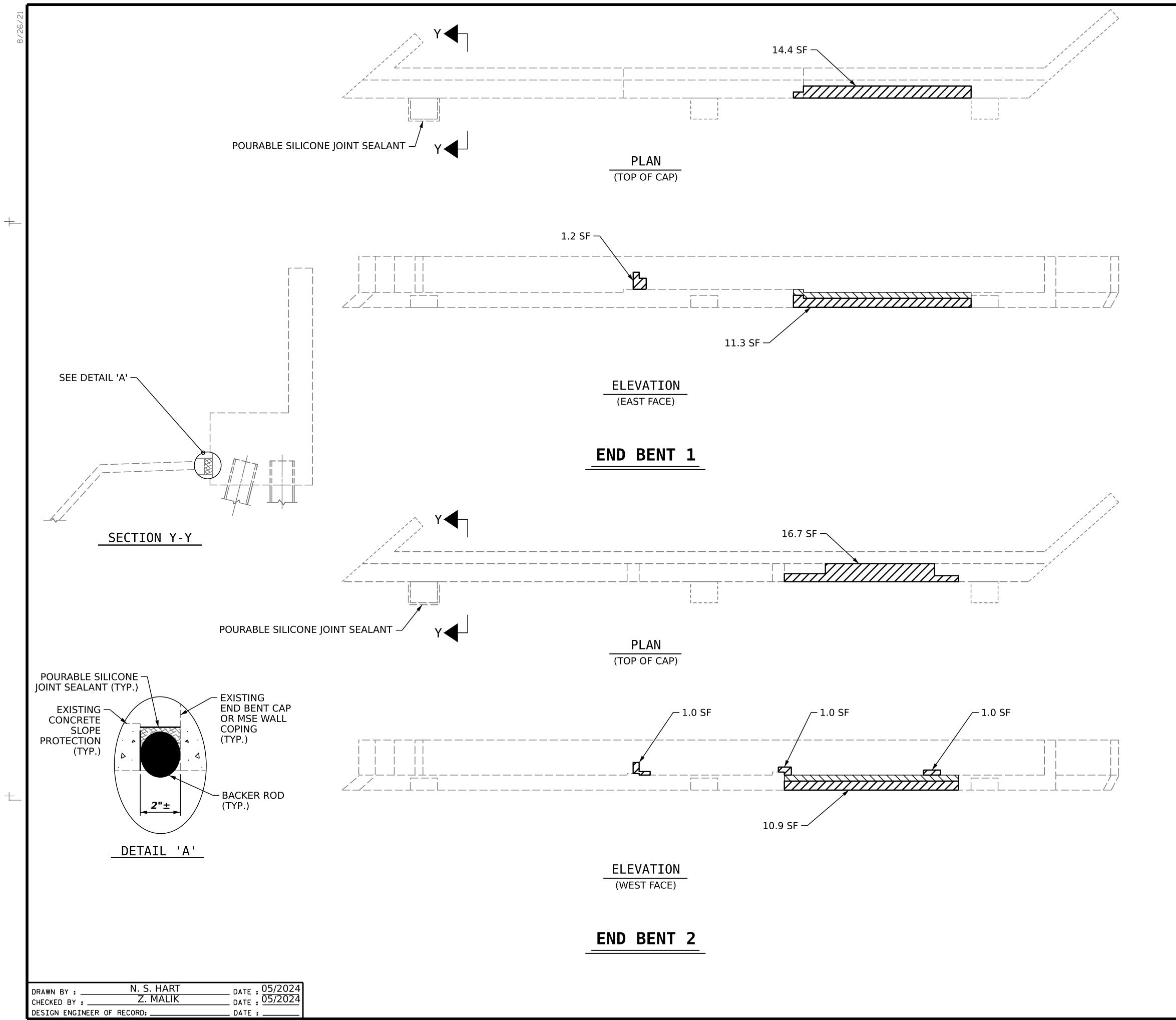
THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO STARTING WORK FOR TEMPORARY FORMWORK.FOR SUBMITTALS OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

UPON REMOVAL OF TEMPORARY FORMWORK, ALL VOIDS AND HONEYCOMBS ON THE UNDERSIDE OF DECK SURFACE SHALL BE FILLED WITH THE SAME MATERIAL AS USED FOR THE PATCH, AND FINISHED TO CONFORM TO THE SURROUNDING CONCRETE SURFACE.

NO FORMWORK SHALL BE LEFT IN PLACE.

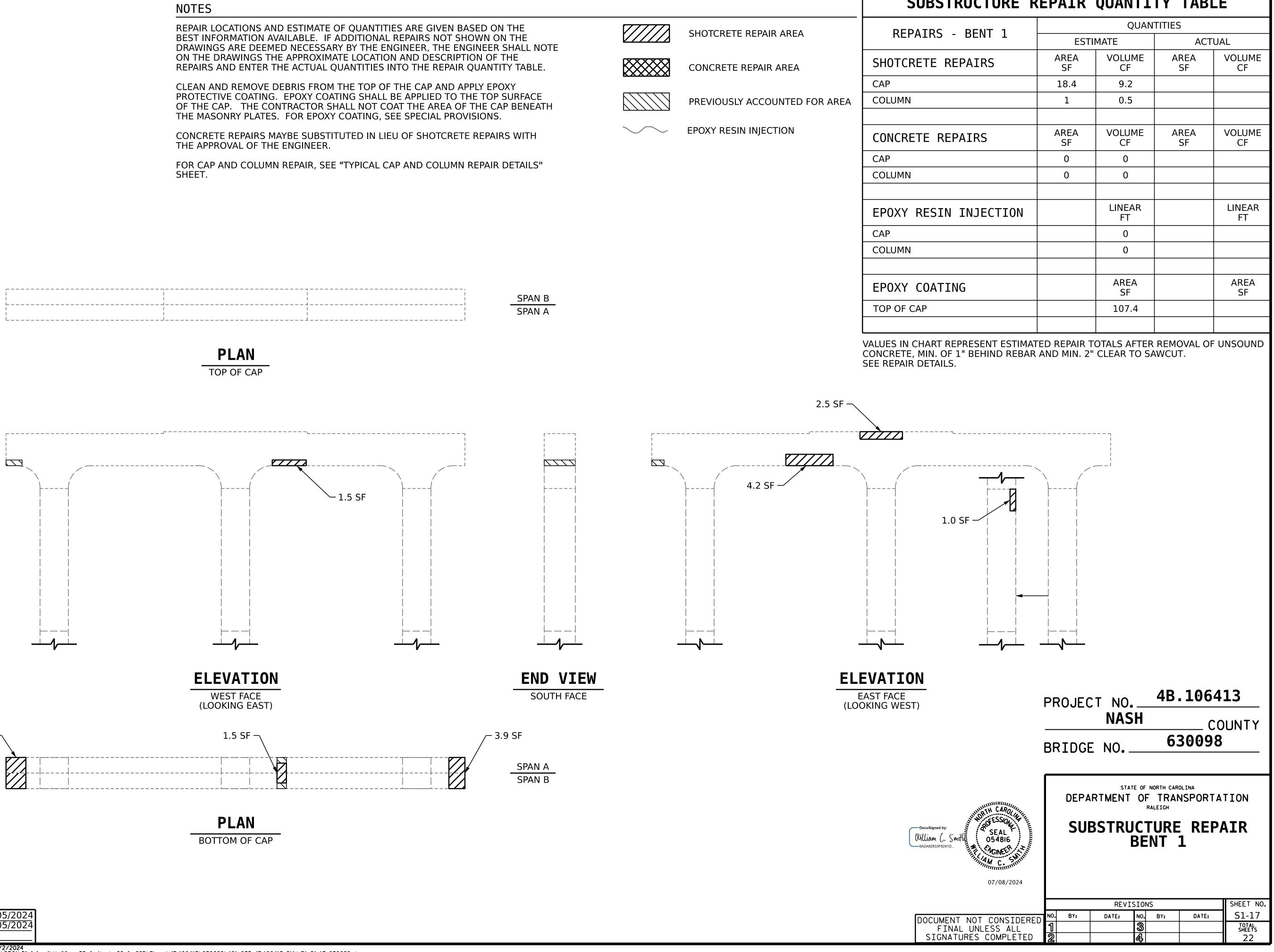




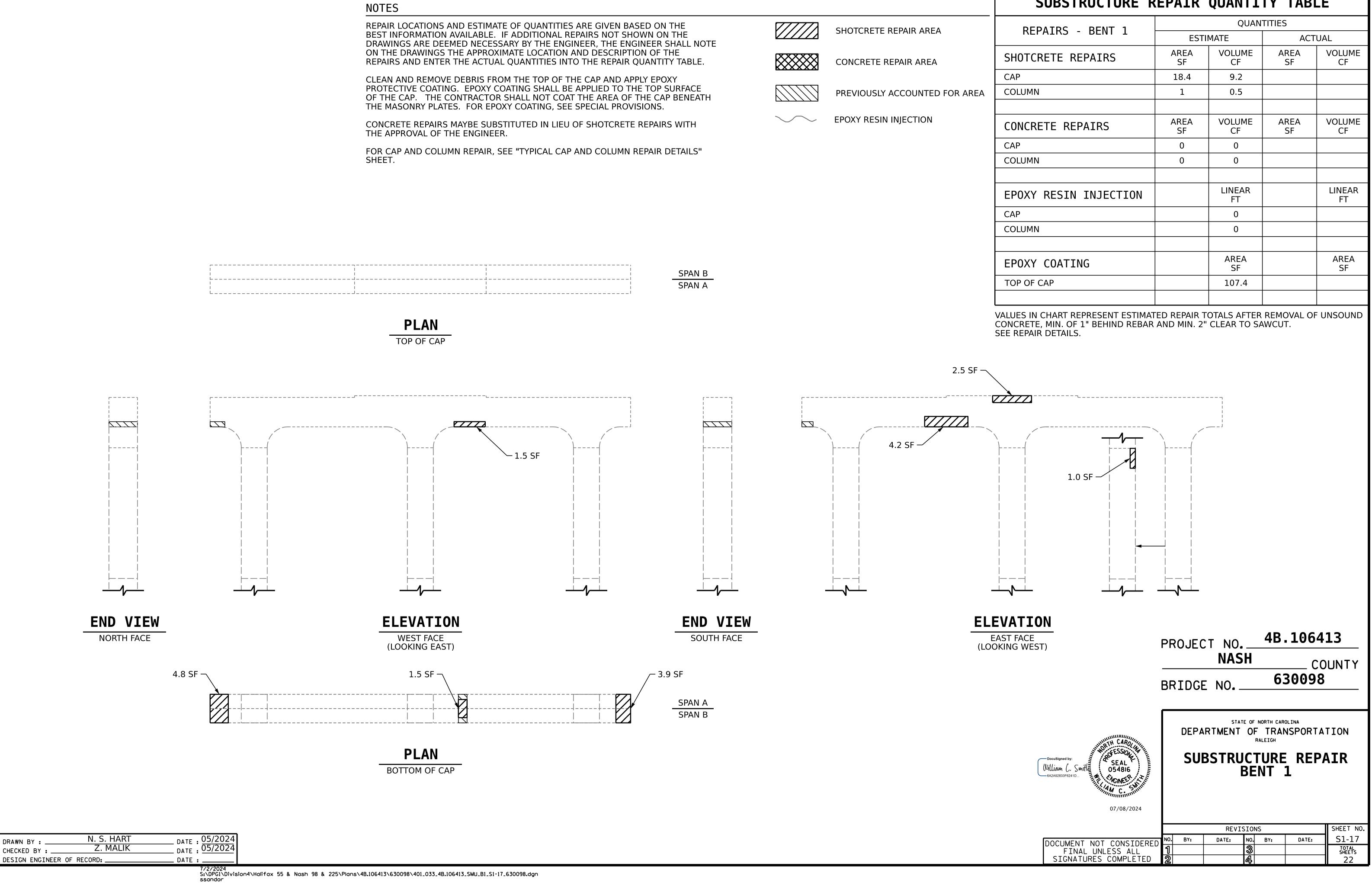


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SU	IBSTRUCTURE R	REPAIR	QUANTI	ΓΥ ΤΑΒ	LE	
END	BENT 1 & 2		QUAN	TITIES		
		EST	IMATE	ACT	ΓUAL	
SHOTCRE	TE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
САР		53.3	26.7			
CURTAIN W	ALL	4.2	2.1			
WINGWALL						
CONCRET	E REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
САР		0	0			
CURTAIN W	ALL	0	0			
WINGWALL						
EPOXY R	ESIN INJECTION		LINEAR FT		LINEAR FT	
САР			0			
CURTAIN W	ALL		0			
WINGWALL						
EPOXY C	OATING		AREA SF		AREA SF	
TOP OF CAI	p		171.4			
POURABL	E SILICONE JT. S	EALANT	LINEAR		LINEAR	
JOINT			FT 11.5		FT	
_	IART REPRESENT ESTIMAT					
REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE. CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS. CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER. SHOTCRETE REPAIR AREA OCONCRETE REPAIR AREA PREVIOUSLY ACCOUNTED FOR AREA						
		PROJEC	T NO.	4B.106	6413	
			NASH		COUNTY	
				63009		
		BRIDGE	NO			
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION						
RALEIGH RALEIGH						
Uilliam (. Smither SEAL 054816 MONEER STRUCTURE REPAIR SUBSTRUCTURE REPAIR END BENT 1 & END BENT 2						
	07/08/2024					
ما	OCUMENT NOT CONSIDERE	D NO. BY:		BY: DATE:	0 0	
U	FINAL UNLESS ALL	1	3		TOTAL SHEETS	







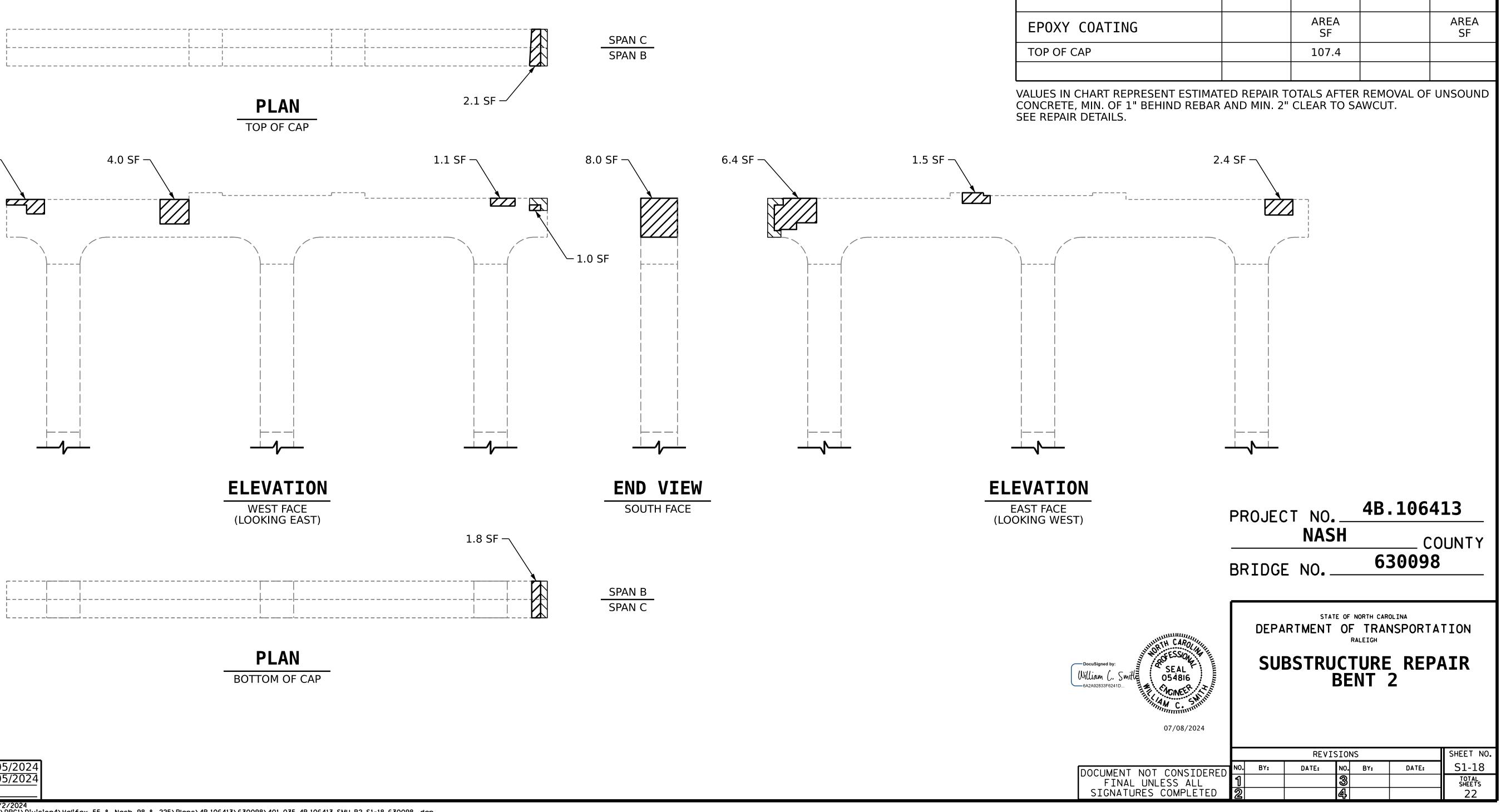
SUBSTRUCTURE REPAIR QUANTITY TABLE						
REPAIRS - BENT 1	QUANTITIES					
REFAIRS - DENT I	ESTI	MATE	ACT	UAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
САР	18.4	9.2				
COLUMN	1	0.5				
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
САР	0	0				
COLUMN	0	0				
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT		
САР		0				
COLUMN		0				
EPOXY COATING		AREA SF		AREA SF		
TOP OF CAP		107.4				

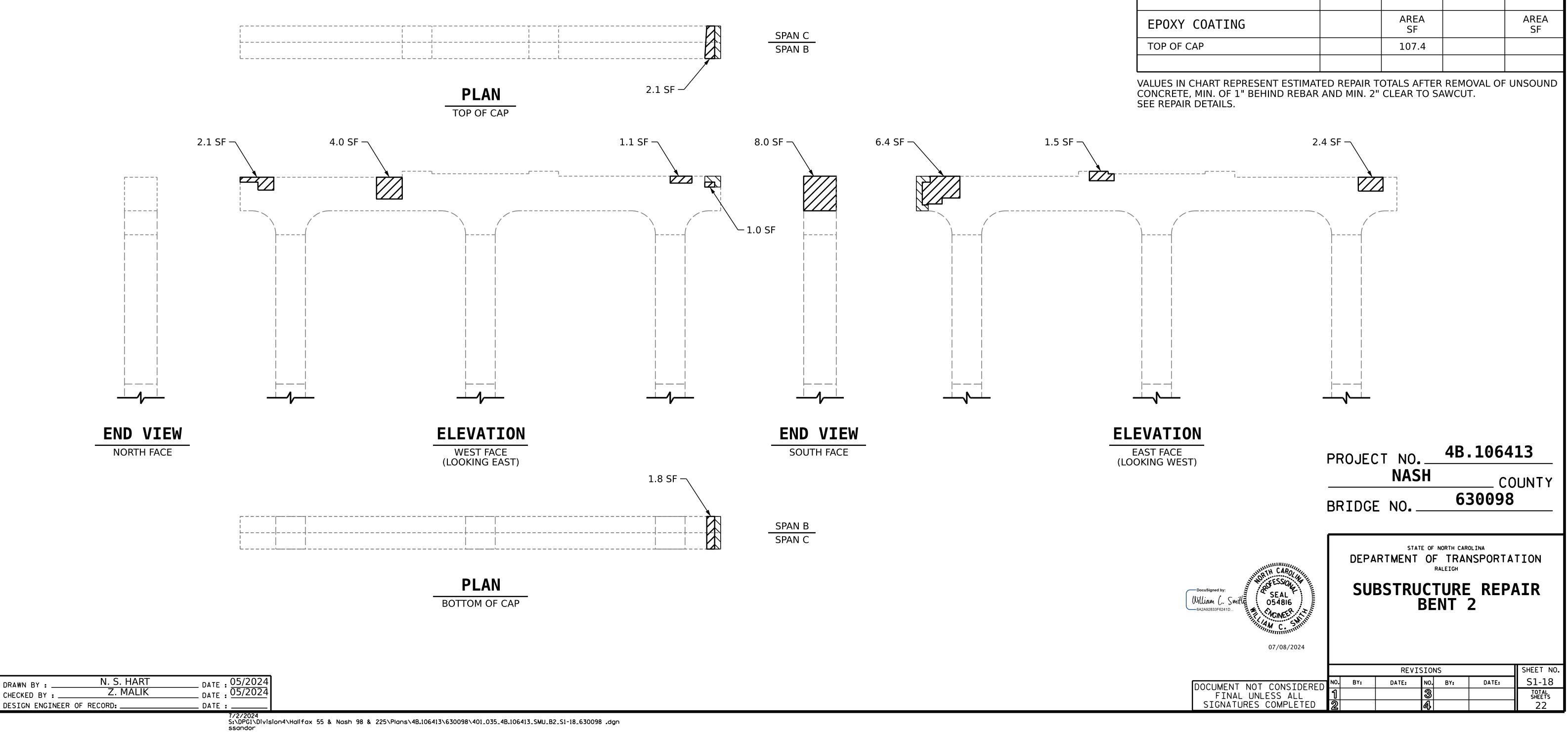
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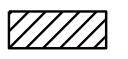
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.





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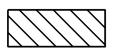
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SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION

SUBSTRUCTURE REPAIR QUANTITY TABLE						
REPAIRS - BENT 2	QUANTITIES					
REFAIRS - DENT Z	ESTII	MATE	ACT	UAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
	30.4	15.2				
COLUMN	0	0				
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
	0	0				
COLUMN	0	0				
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT		
САР		0				
COLUMN		0				
EPOXY COATING		AREA SF		AREA SF		
TOP OF CAP		107.4				

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SHEET.

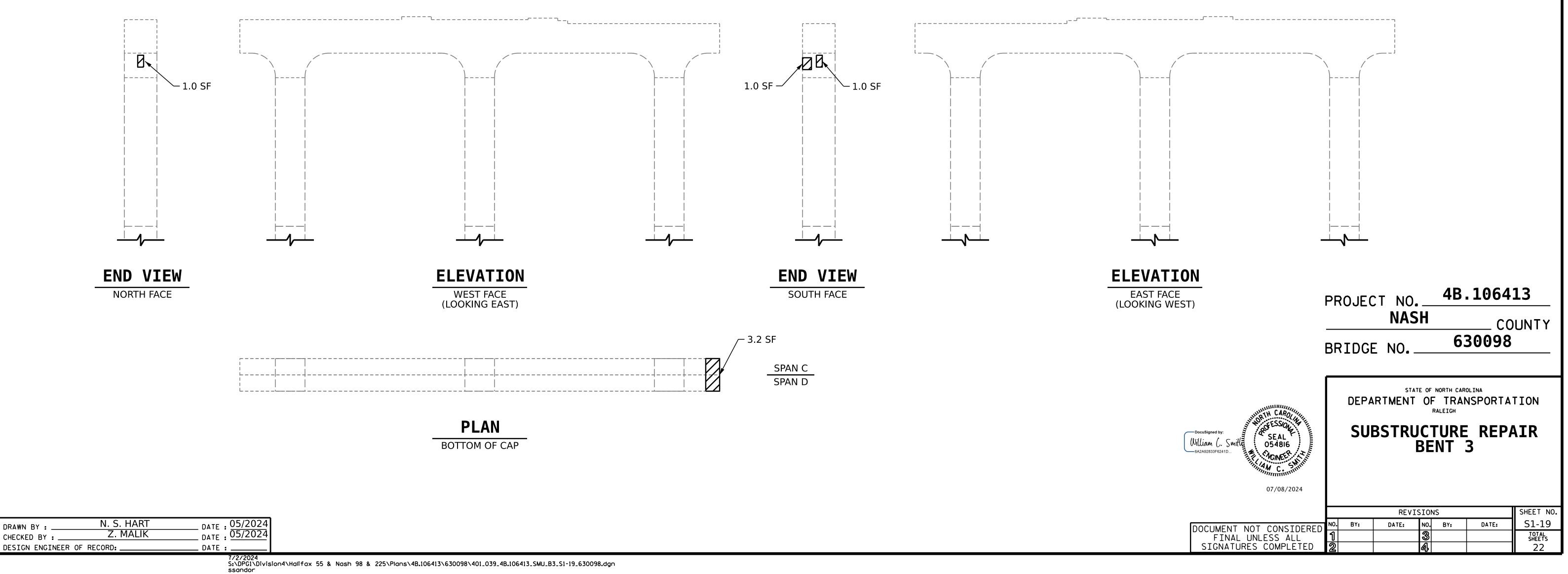


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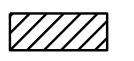
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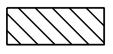
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS"



SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION





SUBSTRUCTURE REPAIR QUANTITY TABLE						
REPAIRS - BENT 3	QUANTITIES					
REPAIRS - DENI S	ESTI	MATE	ACT	UAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
САР	3.2	1.6				
COLUMN	3.0	1.5				
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
САР	0	0				
COLUMN	0	0				
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT		
САР		0				
COLUMN		0				
EPOXY COATING		AREA SF		AREA SF		
TOP OF CAP		107.4				

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

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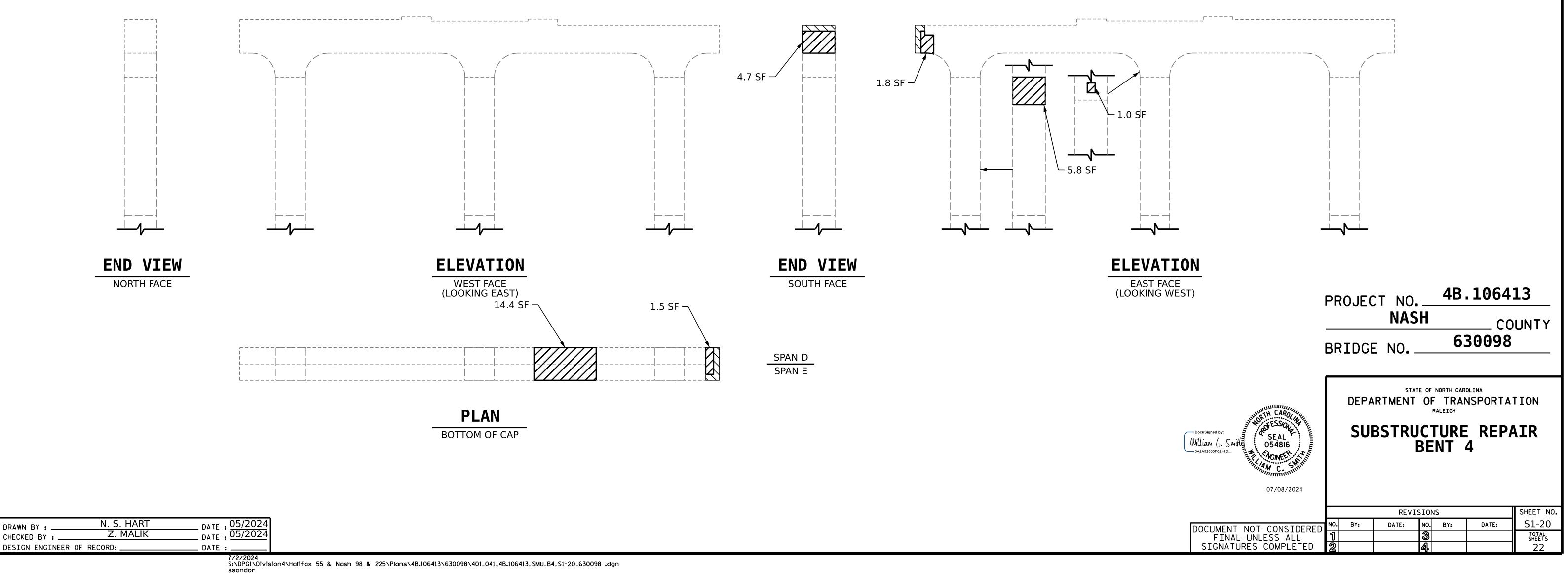
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS" SHEET.

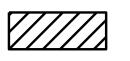


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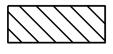




SHOTCRETE REPAIR AREA

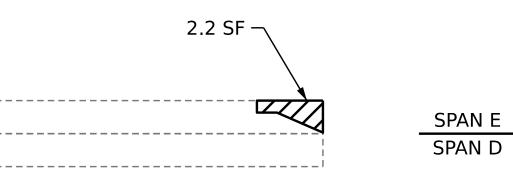


CONCRETE REPAIR AREA



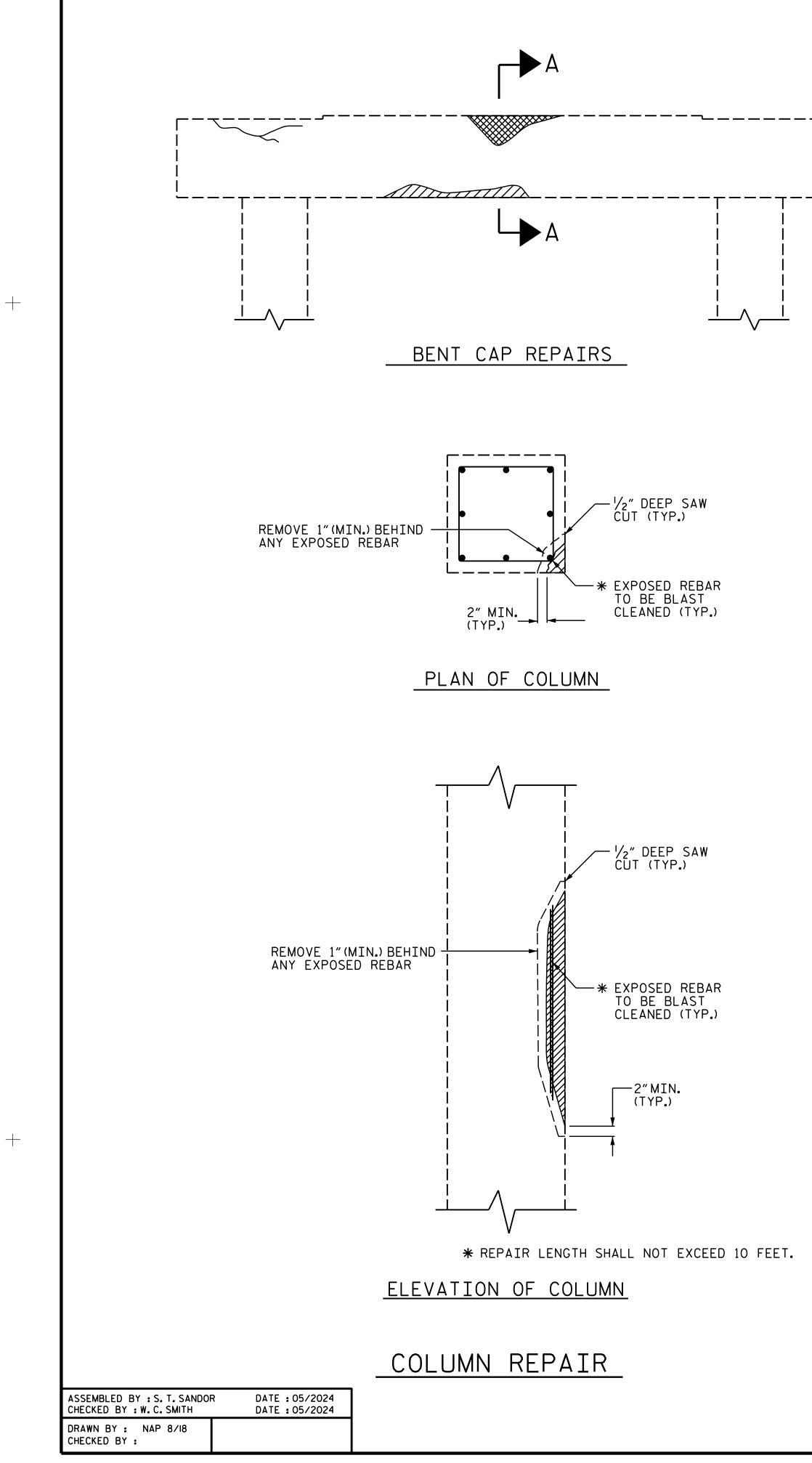
PREVIOUSLY ACCOUNTED FOR AREA

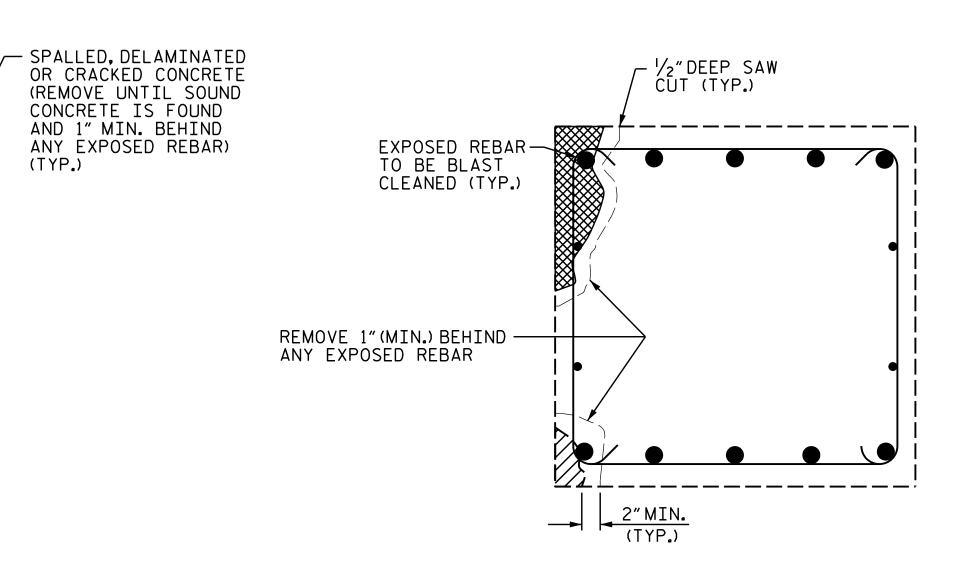
EPOXY RESIN INJECTION



SUBSTRUCTURE REPAIR QUANTITY TABLE						
REPAIRS - BENT 4	QUANTITIES					
REFAIRS - DENI 4	ESTI	MATE	ACT	UAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
САР	24.6	12.3				
COLUMN	6.8	3.4				
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF		
САР	0	0				
COLUMN	0	0				
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT		
САР		0				
COLUMN		0				
EPOXY COATING		AREA SF		AREA SF		
TOP OF CAP		107.4				

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.





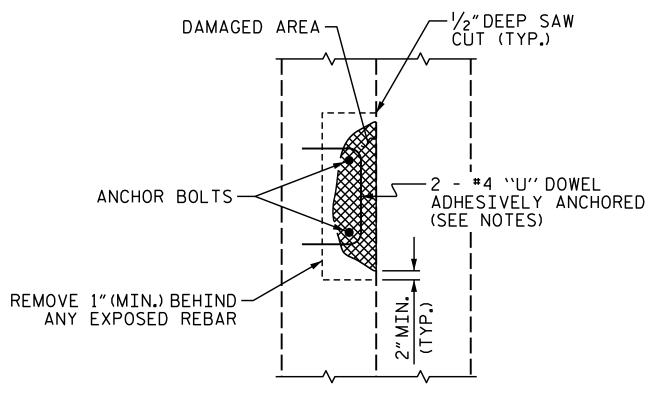
REPAIR KEY

SECTION A-A

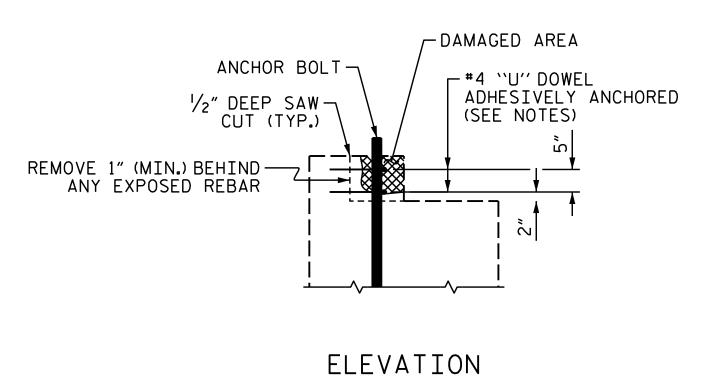
CAP REPAIR

SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA (FORM AND POUR)







SPLICE	LENGTH TABLE			
BAR SIZE	MIN. SPLICE LENGTH			
# 4	2'-5″			
# 5	3'-0"			
# 6	3'-7"			
# 7	4'-2"			
# 8	4'-9"			
#9	5'-4"			
# 10	6'-0"			
#11	6'-8"			



NOTES

TYPICAL BENT CAP REPAIRS ARE SHOWN.REPAIR DETAILS SIMILAR FOR END BENT CAPS AND STRUTS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1"BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

NO MORE THAN ONE-THIRD OF THE CAP OR COLUMN CIRCUMFERENCE SHALL BE REMOVED AT ONE TIME.SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN 30% OF A CAP OR COLUMN CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

SIMULTANEOUS REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP AND/OR COLUMN, BUT NO MORE THAN ¹/₃ OF THE CIRCUMFERENCE SHALL BE REMOVED AT ONE TIME. IF REMOVAL EXTENDS MORE THAN 1¹/₂" BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING. ON COLUMNS AND PILES, NO MORE THAN 10 VERTICAL FEET MAY BE EXPOSED AT ONE TIME BEFORE PLACEMENT OF REPAIR CONCRETE.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

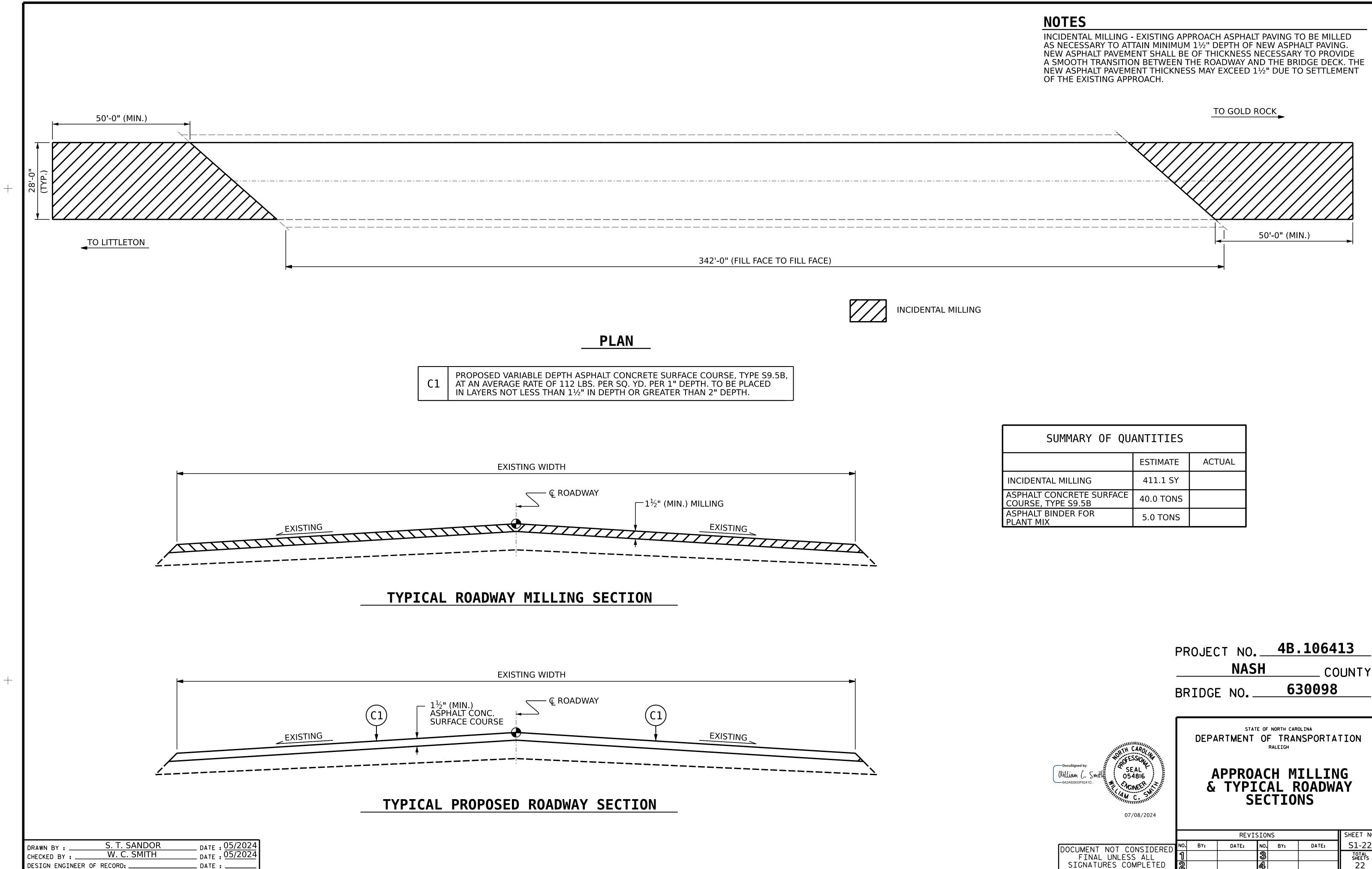
THE #4 ``U'' DOWELS ARE REQUIRED ONLY AROUND THE ANCHOR BOLTS. THE EXISTING REINFORCING STEEL IN THE PEDESTAL WALL SHALL BE CLEANED, STRAIGHTENED AND REMAIN IN PLACE.

FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

COAT ALL REPAIR SURFACE AREAS ON THE TOP OF CAPS, INCLUDING CHAMFERS, WITH EPOXY PROTECTIVE COATING, OVERLAPPING THE REPAIR AREA BY A MINIMUM OF 3" ON ALL POSSIBLE SIDES.

CLEAN ALL EXPOSED REINFORCING BARS AND PRESTRESSED STRANDS IN ACCORDANCE WITH APPROPRIATE SPECIAL PROVISIONS.FOR BARS WITH MORE THAN 10% SECTION LOSS, SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED.NOTE AND PROVIDE DETAILED DOCUMENTATION, INCLUDING LOCATION AND SEVERITY, OF ALL DAMAGE TO PRESTRESSED STRANDS THAT EXCEEDS 10% SECTION LOSS. IF FIVE OR MORE STRANDS ARE DAMAGED, NOTIFY THE ENGINEER PRIOR TO PLACEMENT OF REPAIR MATERIAL.

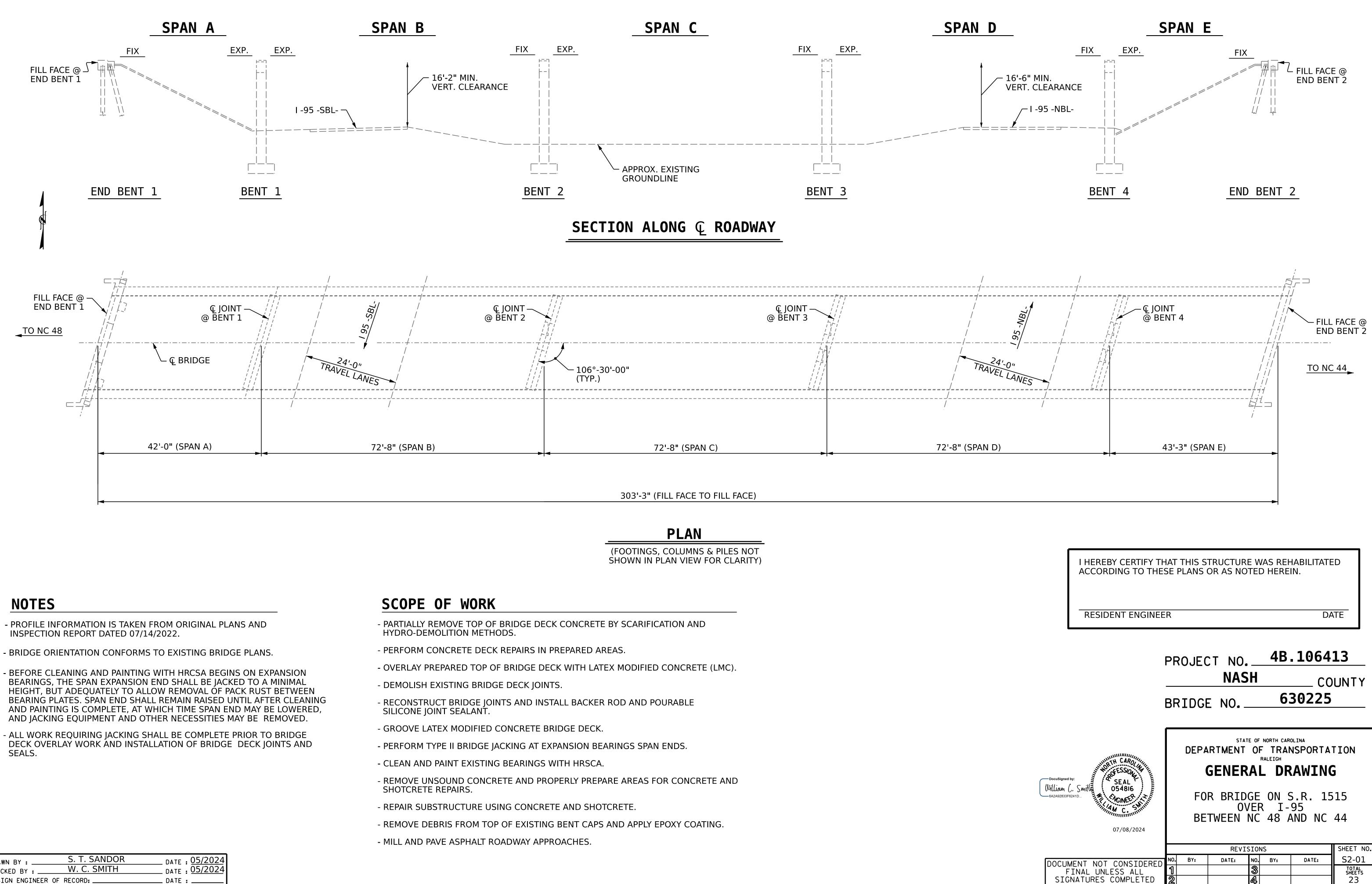
	PROJECT NO <u>NASH</u> BRIDGE NO	4B.10641 col 630098	JNTY
Docusigned by: William (Shirthing SEAL BAZA92833F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA9283F6241D. BAZA928283F6241D. BAZA9283F6241D. BAZA92877 BAZA928777 BAZA977777777777777777777777777777777777	DEPARTMENT OF STA TYPI(AND	F NORTH CAROLINA F TRANSPORTAT RALEIGH ANDARD CAL CAP COLUMN COLUMN DETAILS	
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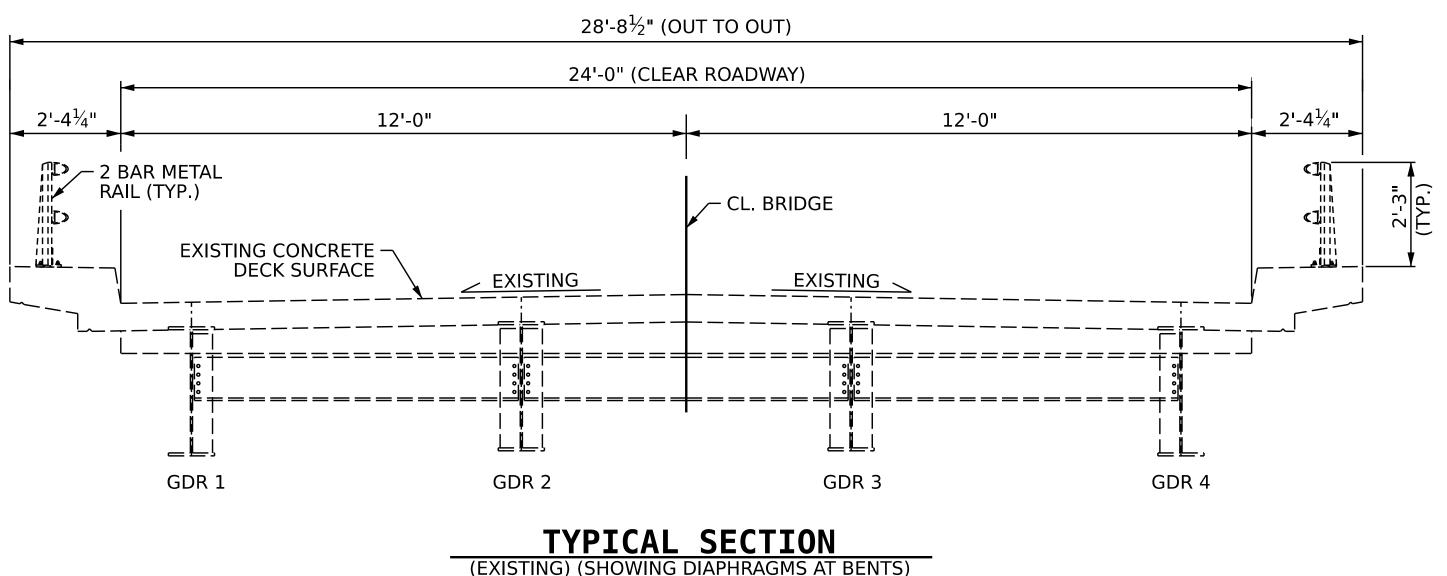
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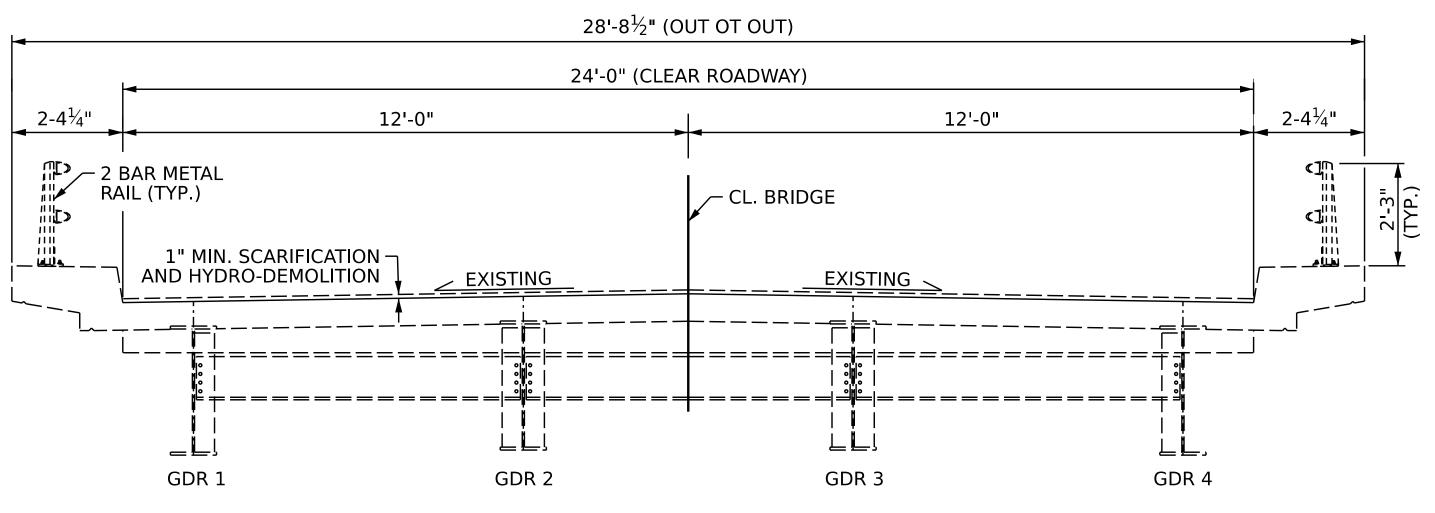
SUMMARY OF QUANTITIES				
	ESTIMATE	ACTUAL		
CIDENTAL MILLING	411.1 SY			
PHALT CONCRETE SURFACE JRSE, TYPE S9.5B	40.0 TONS			
PHALT BINDER FOR NT MIX	5.0 TONS			

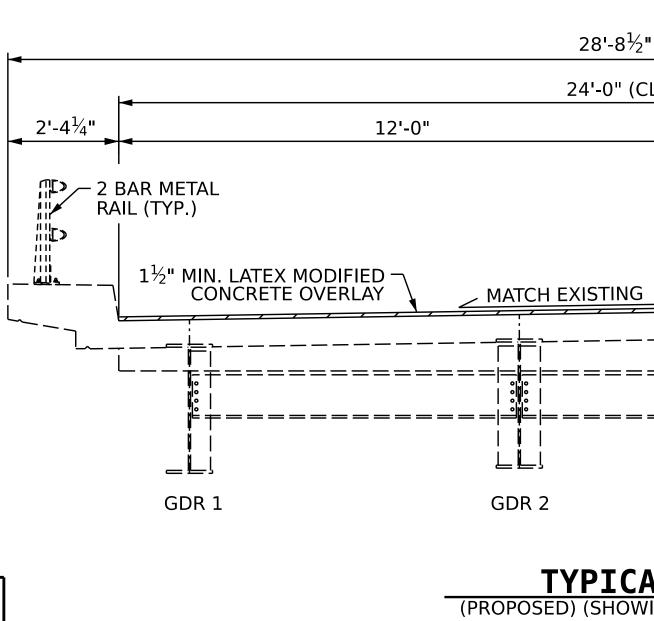
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	BRIDG			<u> </u>	
WIND CAROLANTE	DEPA	state RTMENT (OF NORTH CAR OF TRAI RALEIGH		TION
DocuSigned by: William (. Smither 6A2A92833F6241D	_	APPROA TYPIC SE		ROADW	
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DRAWN BY : _	S. T. SANDOR	DATE : 05/2024
CHECKED BY :	W. C. SMITH	DATE : 05/2024
DESIGN ENGIN	EER OF RECORD:	DATE :
	CHECKED BY :	







DRAWN BY :	E. BAYISSA	DATE : 05/2024
CHECKED BY :	F. LEA	DATE : <u>05/2024</u>
DESIGN ENGINEER	OF RECORD:	DATE :

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1" MIN. SCARIFICATION & HYDRO-DEMOLITION

TYPICAL SECTION (DECK PREPARATION) (SHOWING DIAPHRAGMS AT BENTS)

> 28'-8¹/₂" (OUT TO OUT) 24'-0" (CLEAR ROADWAY) 2'-4¼" 12'-0" Cline
> Cline – CL. BRIDGE 2'-3" (ТҮР.) MATCH EXISTING ╘╹ GDR 3 GDR 4

TYPICAL SECTION

(PROPOSED) (SHOWING DIAPHRAGMS AT BENTS)

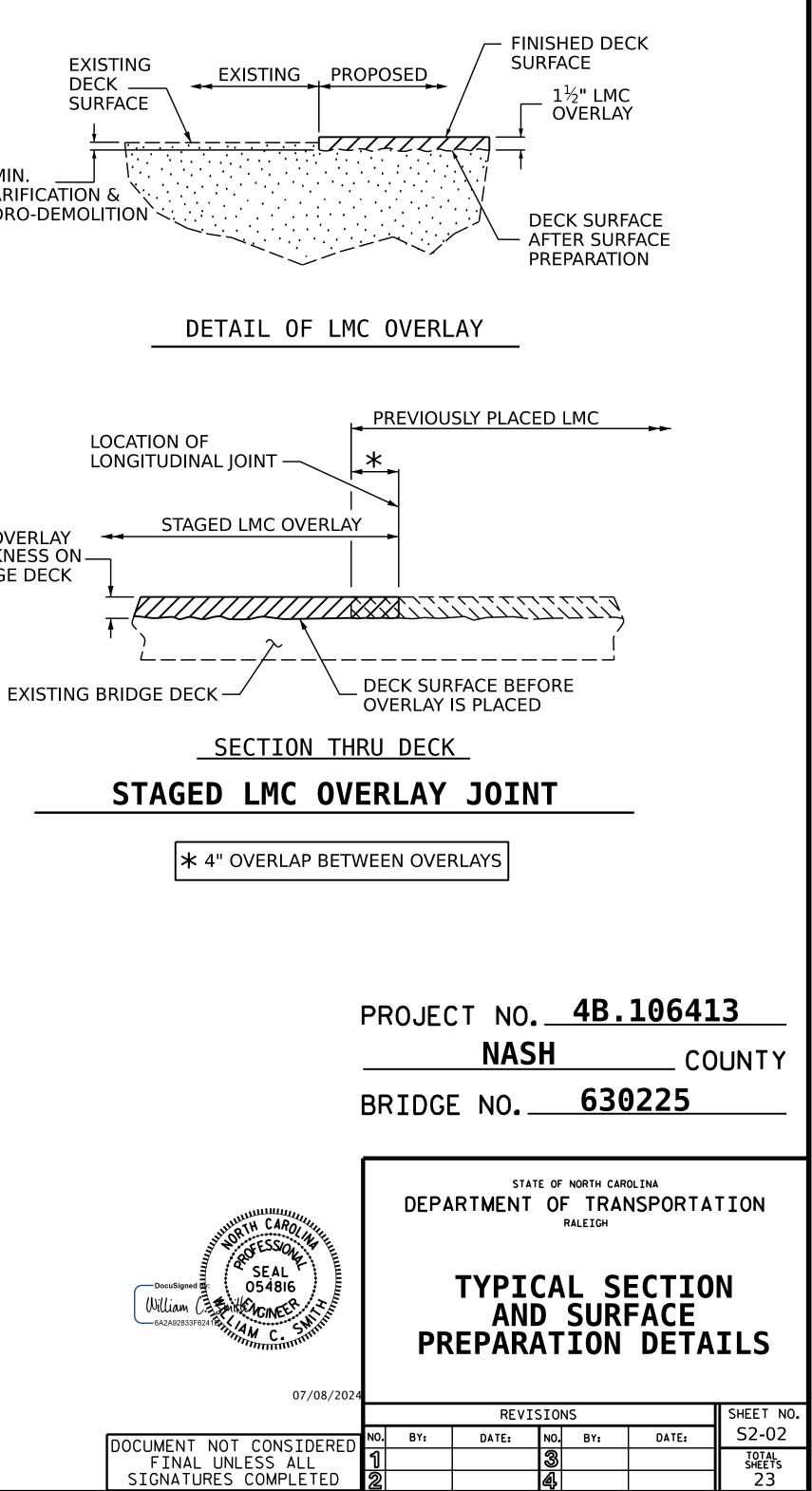
$1\frac{1}{2}$ " OVERLAY THICKNESS ON ____ **BRIDGE DECK**

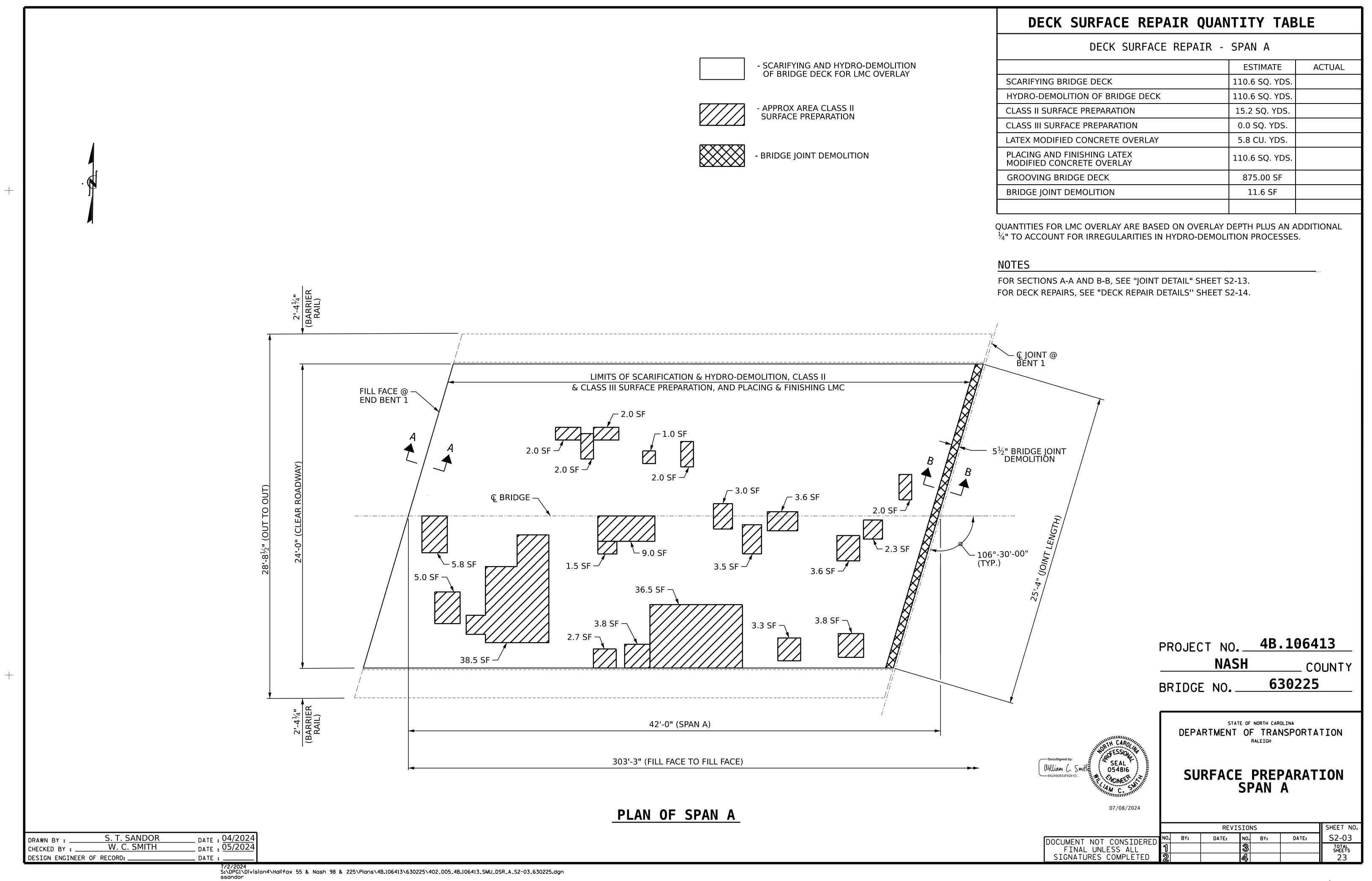
NOTES

THE CONTRACTOR IS ALERTED TO THE FACT THAT THERE ARE MANY AREAS IN THE EXISTING BRIDGE DECK WHERE CONCRETE COVER OVER THE TOP MAT OF REINFORCING STEEL IS APPROXIMATELY $\frac{1}{2}$ ". APPROPRIATE CARE AND MEASURES SHALL BE TAKEN TO ENSURE THAT REINFORCING BARS ARE NOT DAMAGED DURING SURFACE PREPARATION OPERATIONS.

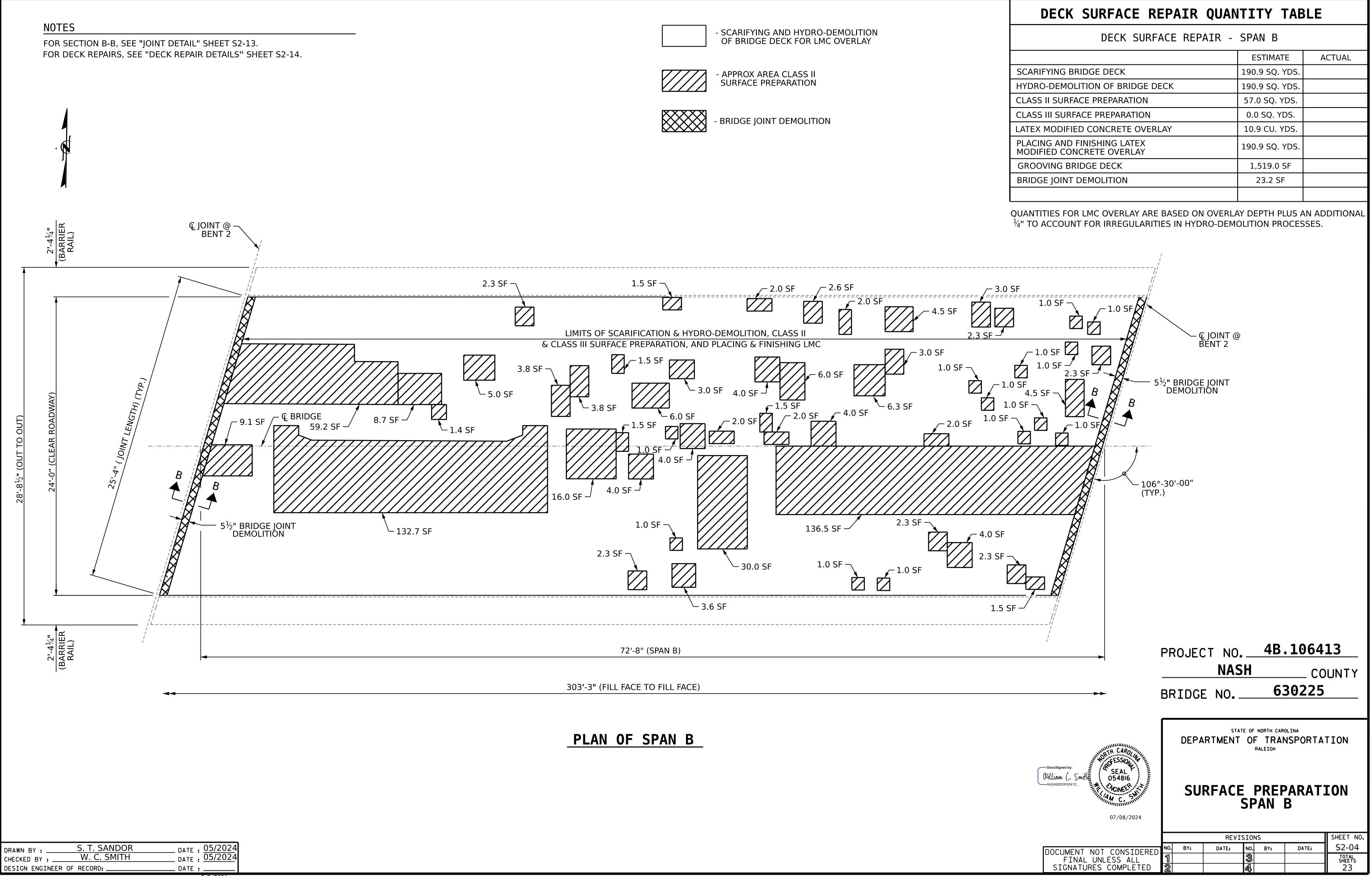
FOR LANE WIDTHS, SEQUENCING AND OTHER TRAFFIC CONTROL MEASURES FOR STAGING OF OVERLAY SURFACE PREPARATION AND LATEX MODIFIED CONCRETE (LMC) PLACEMENT, SEE ELSEWHERE IN THE CONTRACT DOCUMENTS.

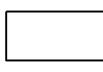
WHEN PREPARING THE SURFACE FOR LMC OVERLAY ADJACENT TO THE PREVIOUSLY PLACED LMC STAGE, THE PREVIOUSLY PLACED LMC SHALL BE SAW-CUT TO THE FULL DEPTH OF THE LMC AT THE CENTERLINE OF THE BRIDGE AND ALL LMC IN THE 4" OVERLAP SHALL BE REMOVED WITH HAND TOOLS PRIOR TO PLACEMENT OF LMC IN THE SECOND STAGE.

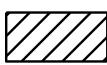


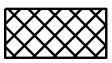


	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	110.6 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	110.6 SQ. YDS.	
CLASS II SURFACE PREPARATION	15.2 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	5.8 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	110.6 SQ. YDS.	
GROOVING BRIDGE DECK	875.00 SF	
BRIDGE JOINT DEMOLITION	11.6 SF	

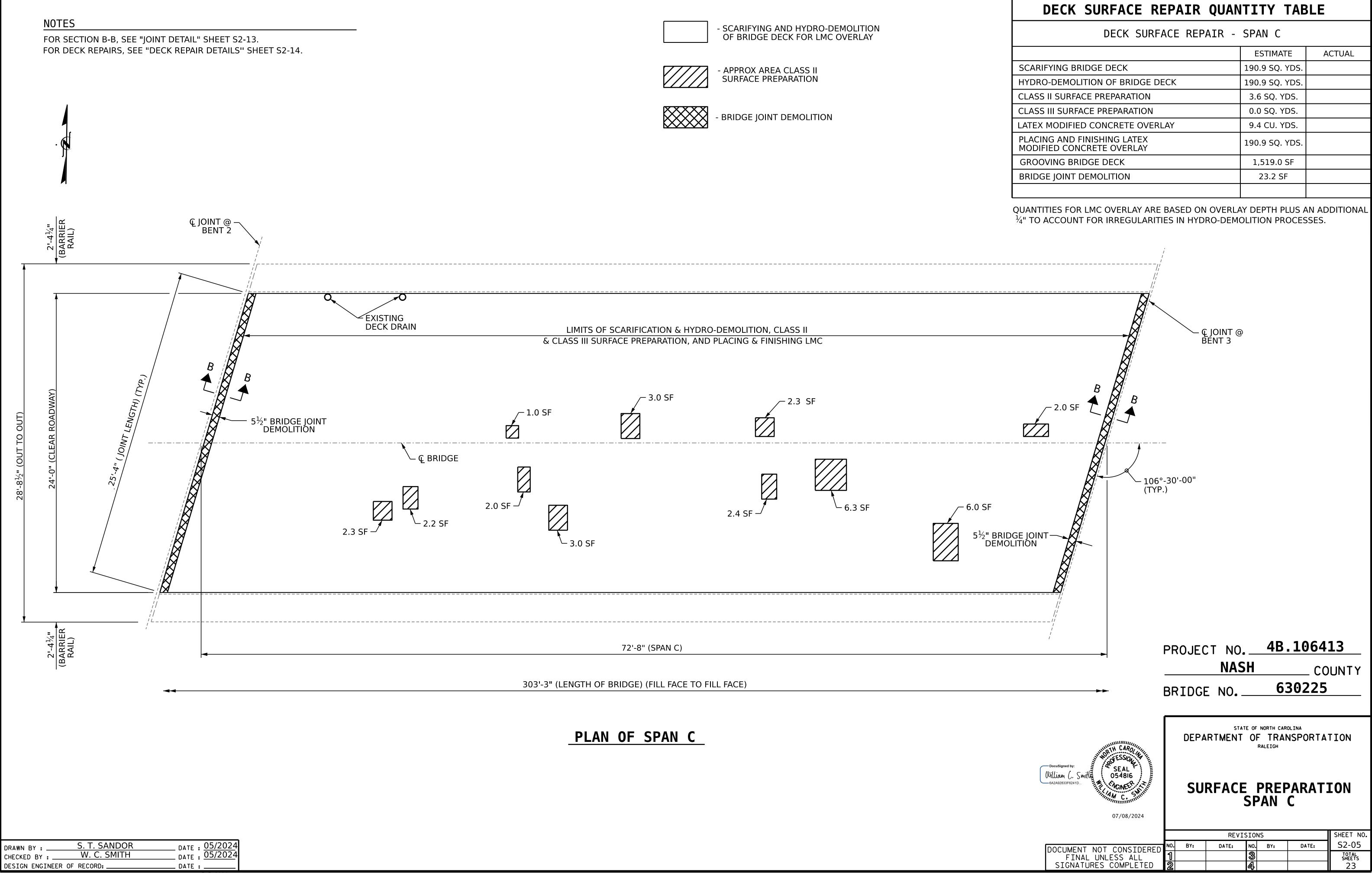


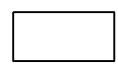


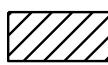


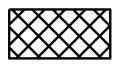


	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	190.9 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	190.9 SQ. YDS.	
CLASS II SURFACE PREPARATION	57.0 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	10.9 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	190.9 SQ. YDS.	
GROOVING BRIDGE DECK	1,519.0 SF	
BRIDGE JOINT DEMOLITION	23.2 SF	

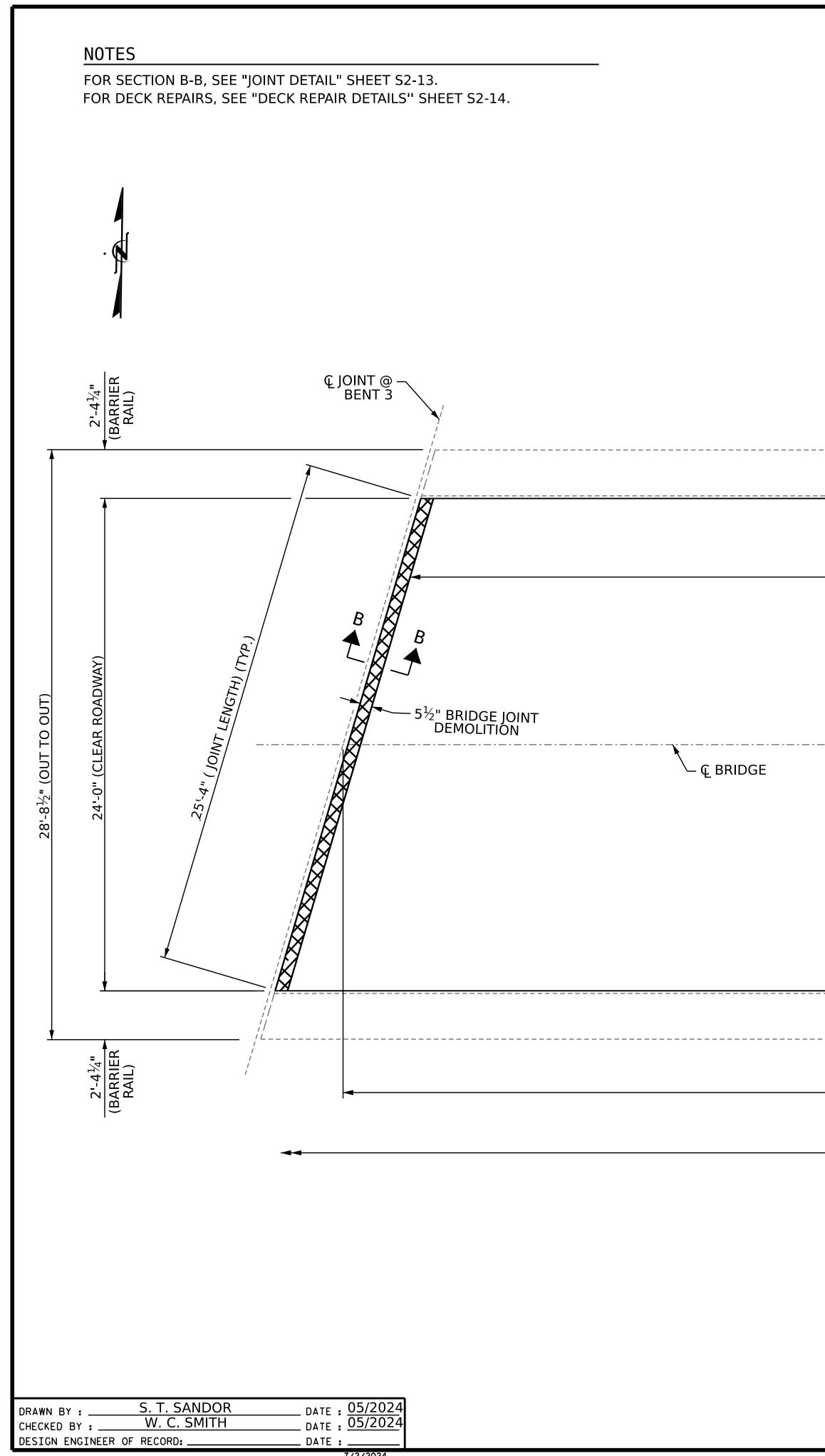




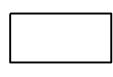




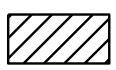
	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	190.9 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	190.9 SQ. YDS.	
CLASS II SURFACE PREPARATION	3.6 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	9.4 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	190.9 SQ. YDS.	
GROOVING BRIDGE DECK	1,519.0 SF	
BRIDGE JOINT DEMOLITION	23.2 SF	



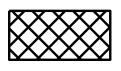
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- SCARIFYING AND HYDRO-DEMOLITION OF BRIDGE DECK FOR LMC OVERLAY



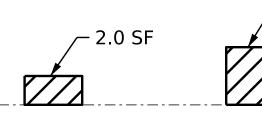
- APPROX AREA CLASS II SURFACE PREPARATION



BRIDGE JOINT DEMOLITION

LIMITS OF SCARIFICATION & HYDRO-DEMOLITION, CLASS II & CLASS III SURFACE PREPARATION, AND PLACING & FINISHING LMC

- 3.0 SF



5¹⁄2" BRIDGE JOINT DEMOLITION

PLAN OF SPAN D

303'-3" (LENGTH OF BRIDGE) (FILL FACE TO FILL FACE)

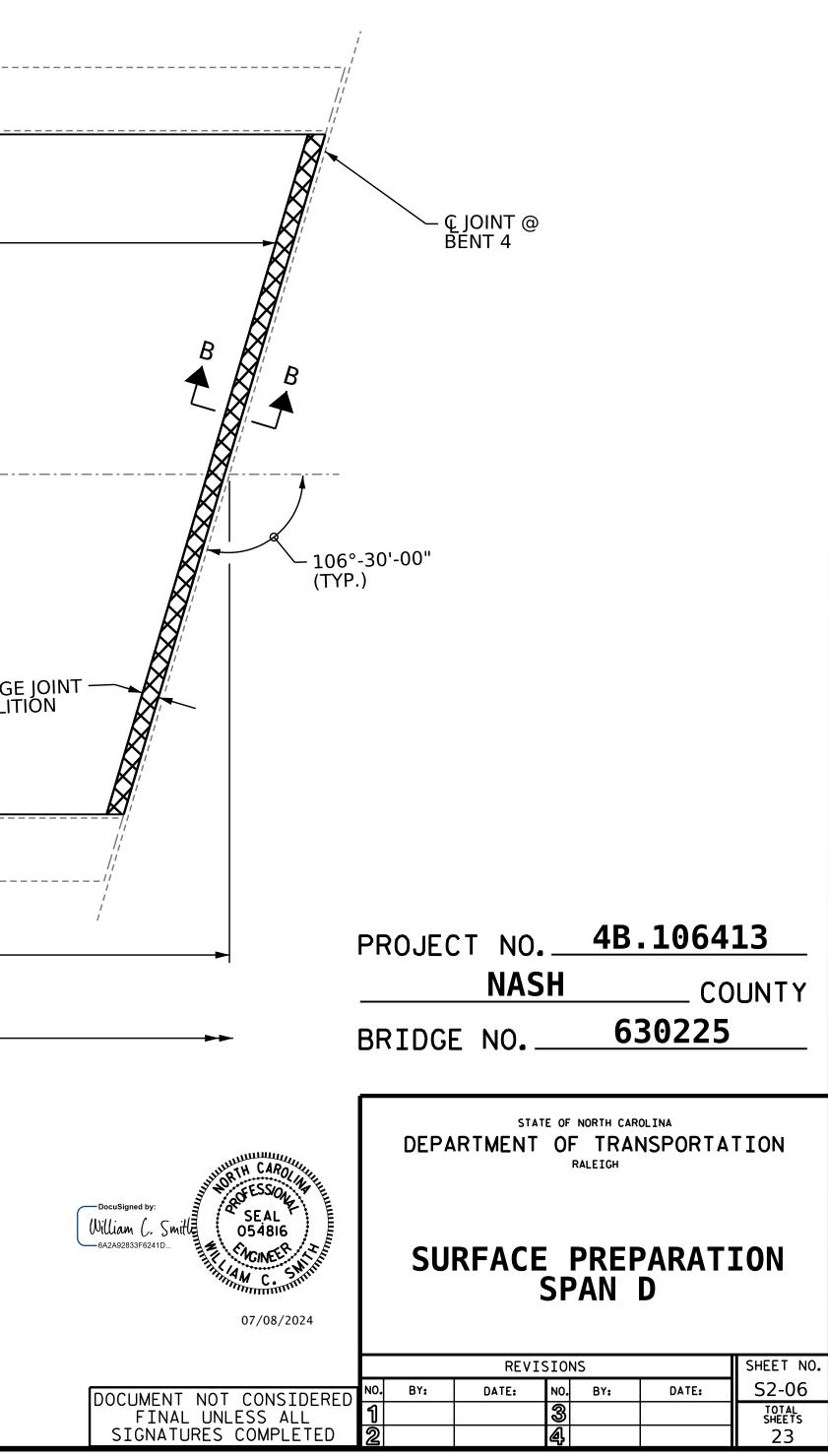
72'-8" (SPAN D)

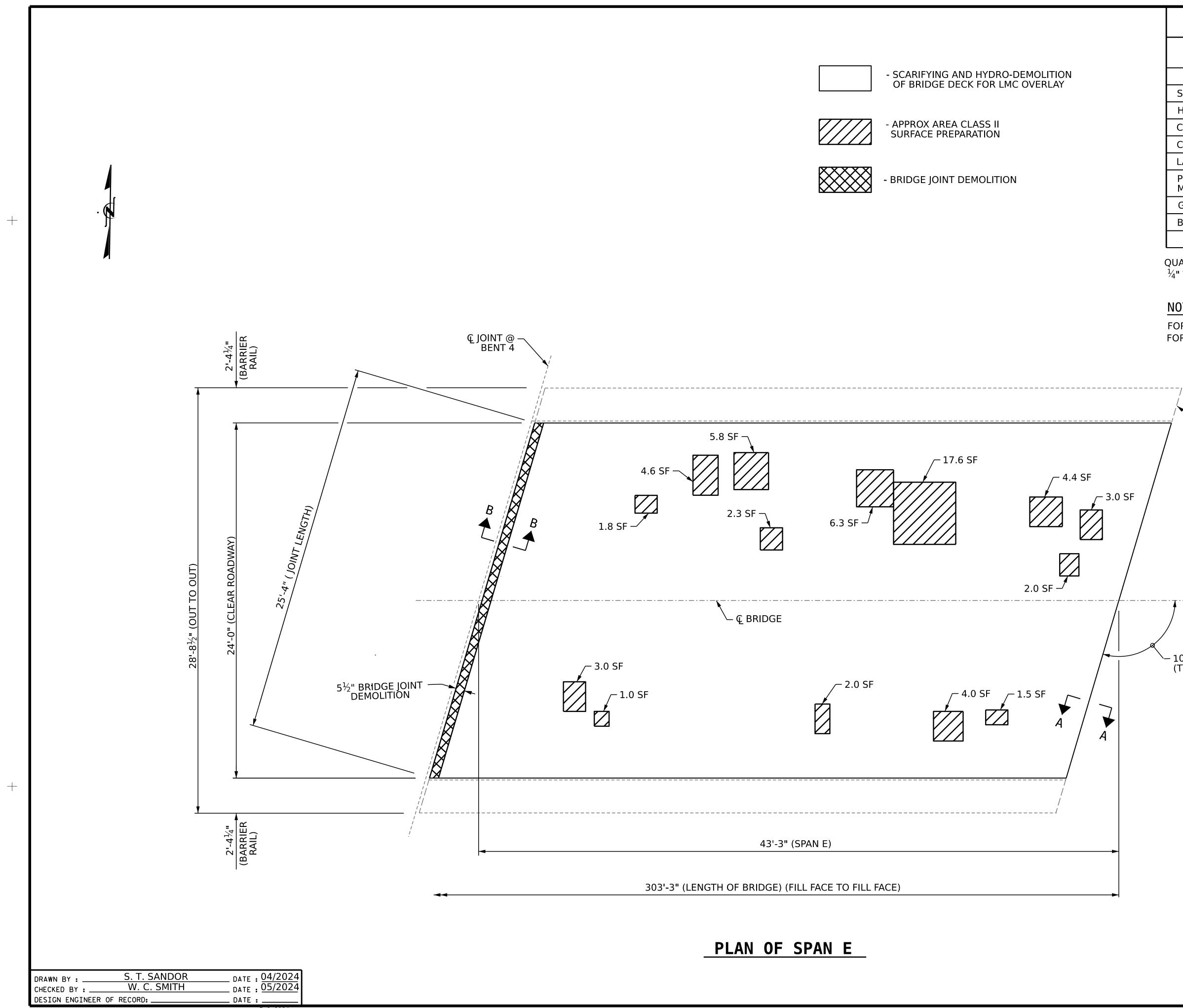
DECK SURFACE REPAIR QUANTITY TABLE

DECK SURFACE REPAIR - SPAN D

	ЕСТИЛАТЕ	
	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	190.9 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	190.9 SQ. YDS.	
CLASS II SURFACE PREPARATION	0.6 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	9.3 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	190.9 SQ. YDS.	
GROOVING BRIDGE DECK	1,519.0 SF	
BRIDGE JOINT DEMOLITION	23.2 SF	

QUANTITIES FOR LMC OVERLAY ARE BASED ON OVERLAY DEPTH PLUS AN ADDITIONAL $\frac{1}{4}$ " TO ACCOUNT FOR IRREGULARITIES IN HYDRO-DEMOLITION PROCESSES.





DECK SURFACE REPAIR QUANTITY TABLE

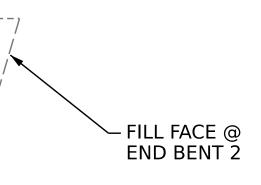
DECK SURFACE REPAIR - SPAN E

	ESTIMATE	ACTUAL
SCARIFYING BRIDGE DECK	114.0 SQ. YDS.	
HYDRO-DEMOLITION OF BRIDGE DECK	114.0 SQ. YDS.	
CLASS II SURFACE PREPARATION	6.6 SQ. YDS.	
CLASS III SURFACE PREPARATION	0.0 SQ. YDS.	
LATEX MODIFIED CONCRETE OVERLAY	5.7 CU. YDS.	
PLACING AND FINISHING LATEX MODIFIED CONCRETE OVERLAY	114.0 SQ. YDS.	
GROOVING BRIDGE DECK	901.3 SF	
BRIDGE JOINT DEMOLITION	11.6 SF	

QUANTITIES FOR LMC OVERLAY ARE BASED ON OVERLAY DEPTH PLUS AN ADDITIONAL $\frac{1}{4}$ " TO ACCOUNT FOR IRREGULARITIES IN HYDRO-DEMOLITION PROCESSES.

NOTES

FOR SECTIONS A-A AND B-B, SEE "JOINT DETAIL" SHEET S2-13. FOR DECK REPAIRS, SEE "DECK REPAIR DETAILS" SHEET S2-14.



- 106°-30'-00" (TYP.)

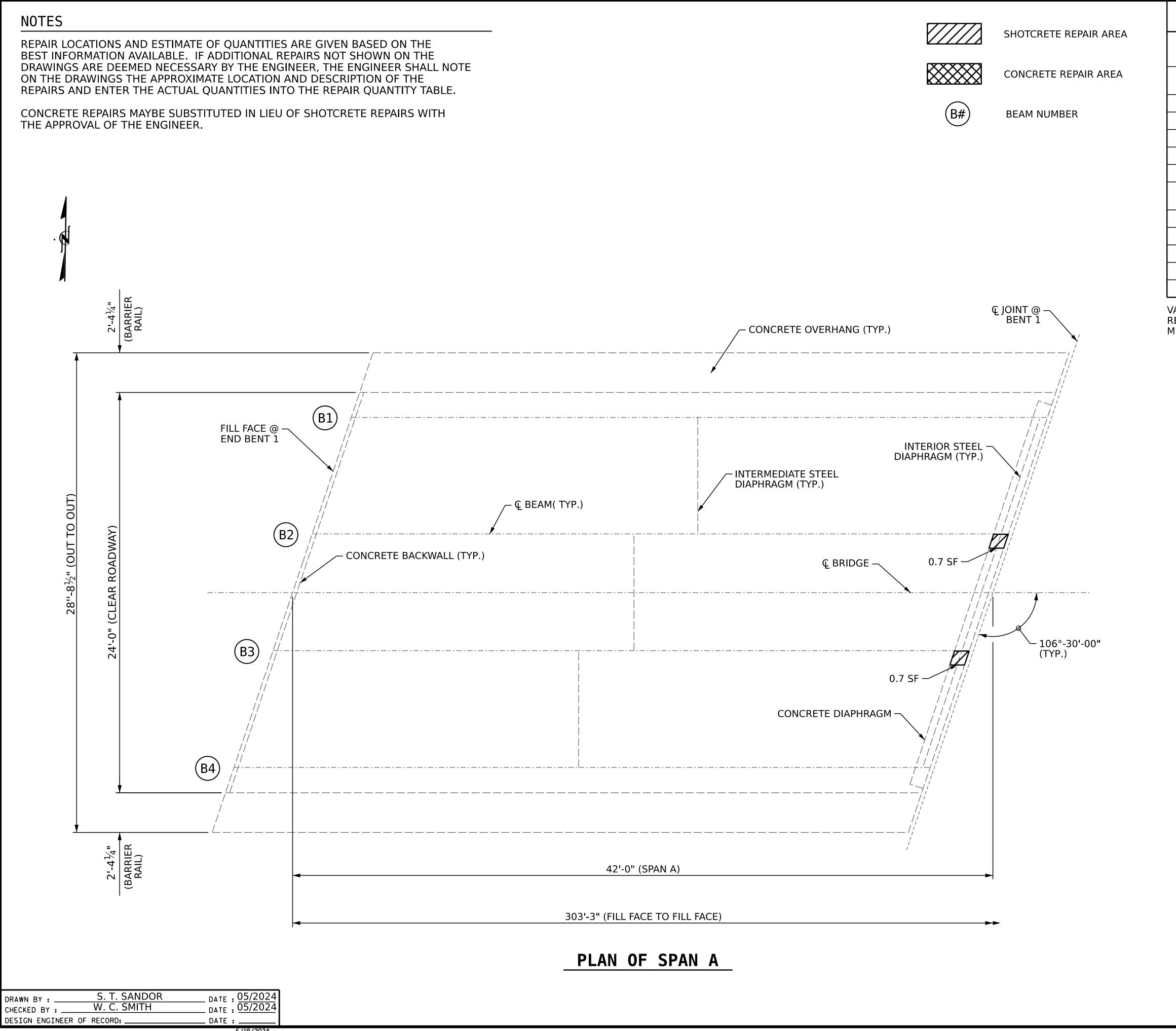
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FINAL UNLESS ALL SIGNATURES COMPLETED	2				sheets 23

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ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

THE APPROVAL OF THE ENGINEER.



6/19/2024 S:\DPG1\Division4\Halifax 55 & Nash 98 & 225\Plans\4B.106413\630225\402_015_4B.106413_SMU_DUR_A_S2-08_630225 .dgn ssandor

DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE
DECK UNDERSIDE REPAIRS		QUAN	TITIES	
SPAN A	ESTI	MATE	АСТ	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	1.4	0.7		
OVERHANG	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

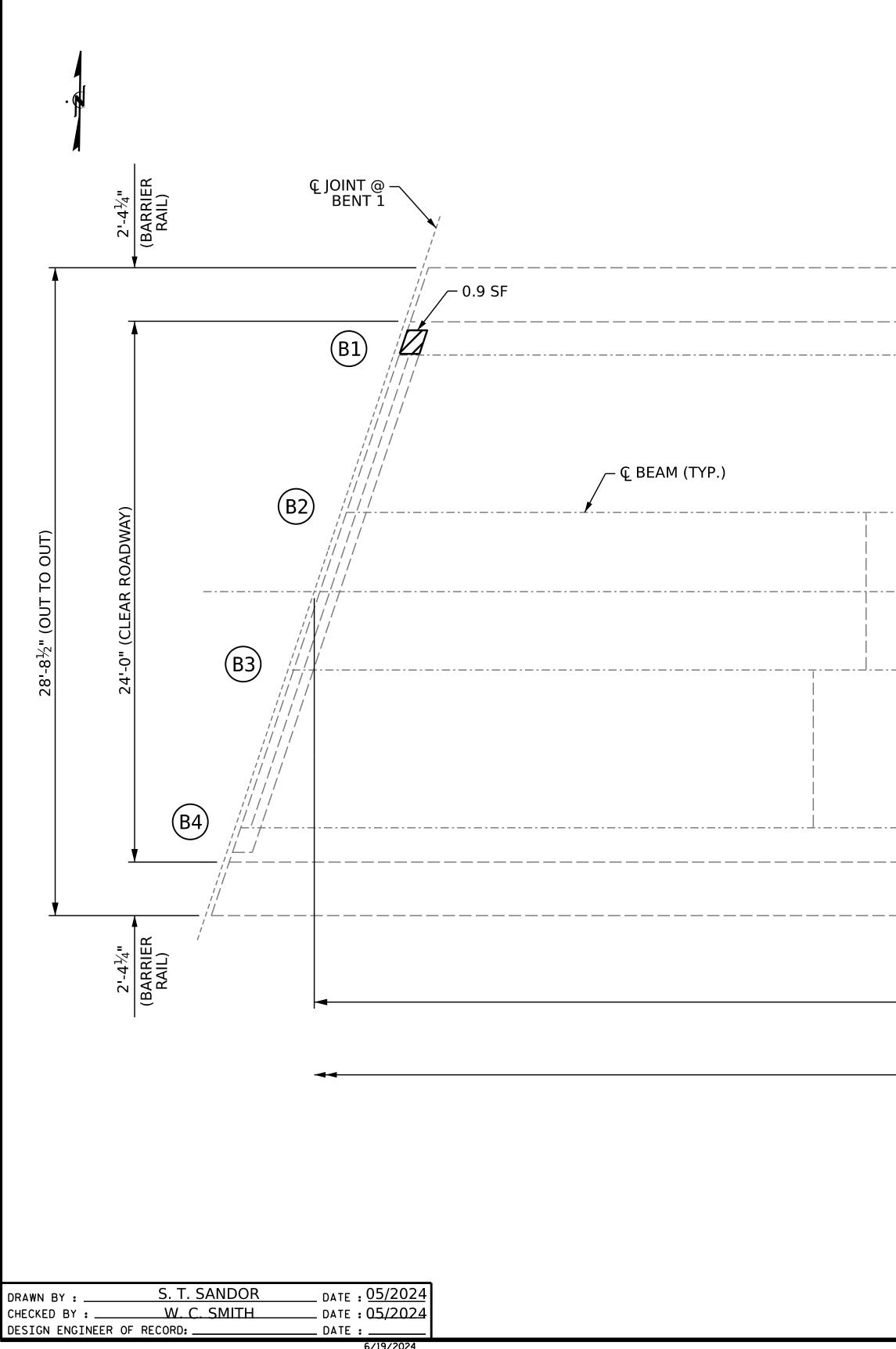
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		NASH		CO	UNTY
	BRIDGE	NO	63	30225	
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REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.



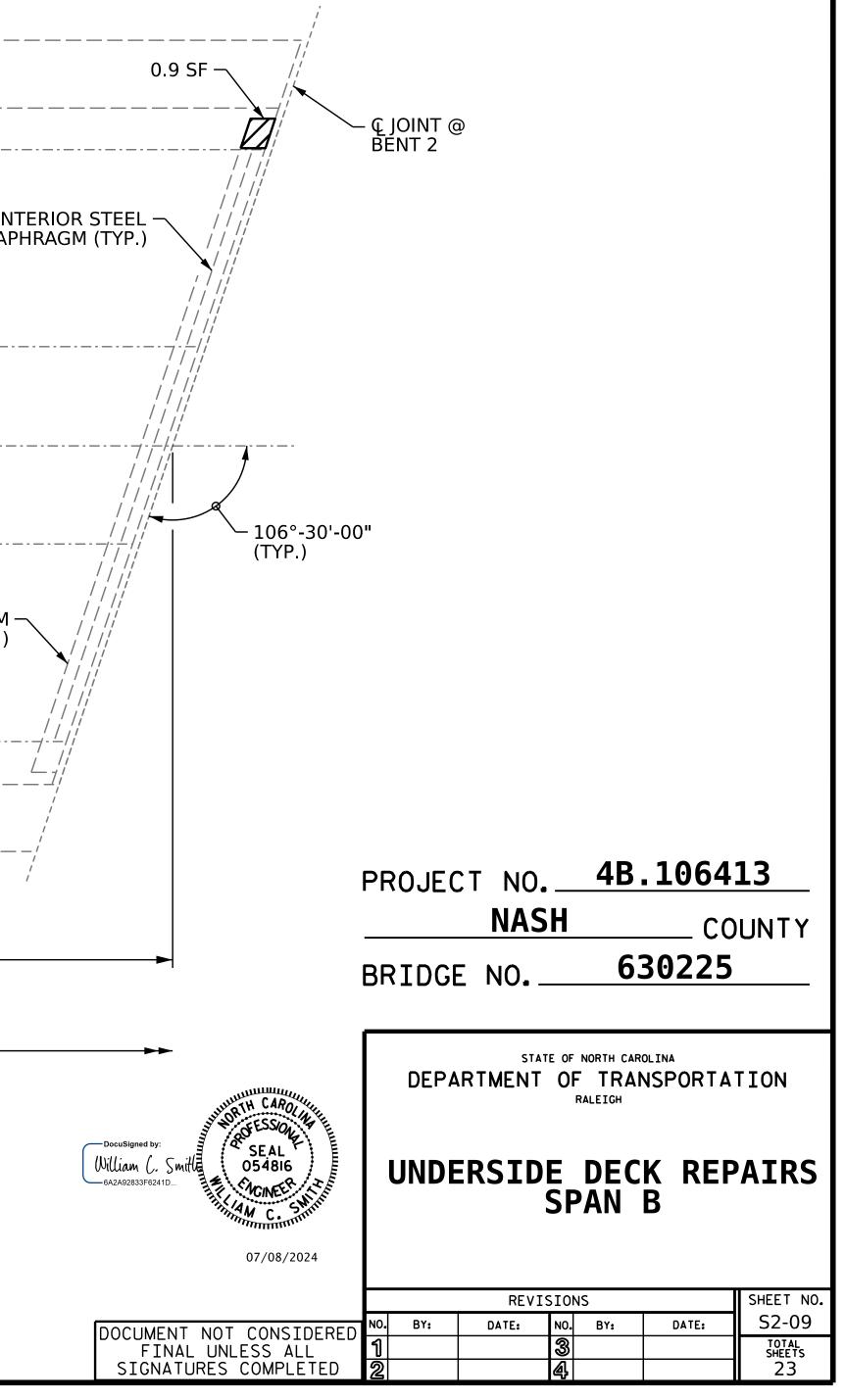
	SHOTCRETE REPAIR AREA	
K	 CONCRETE REPAIR AREA	
54) BEAM NUMBER	
)	
		(
		VAI CO DE
		IN DIAF
r 	 	
	 CONCRETE DIA	PHRAGM
		(TYP.)
i	 	
		CONCRETE REPAIR AREA

303'-3" (FILL FACE TO FILL FACE)

PLAN OF SPAN B

DECK UNDERSIDE	REPAIR	QUANT	ITY TAE	BLE
DECK UNDERSIDE REPAIRS		QUAN	TITIES	
SPAN A	ESTI	ИАТЕ	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	1.8	0.9		
OVERHANG	0	0		
	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		
	0	0		

ALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND ONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR ETAILS.



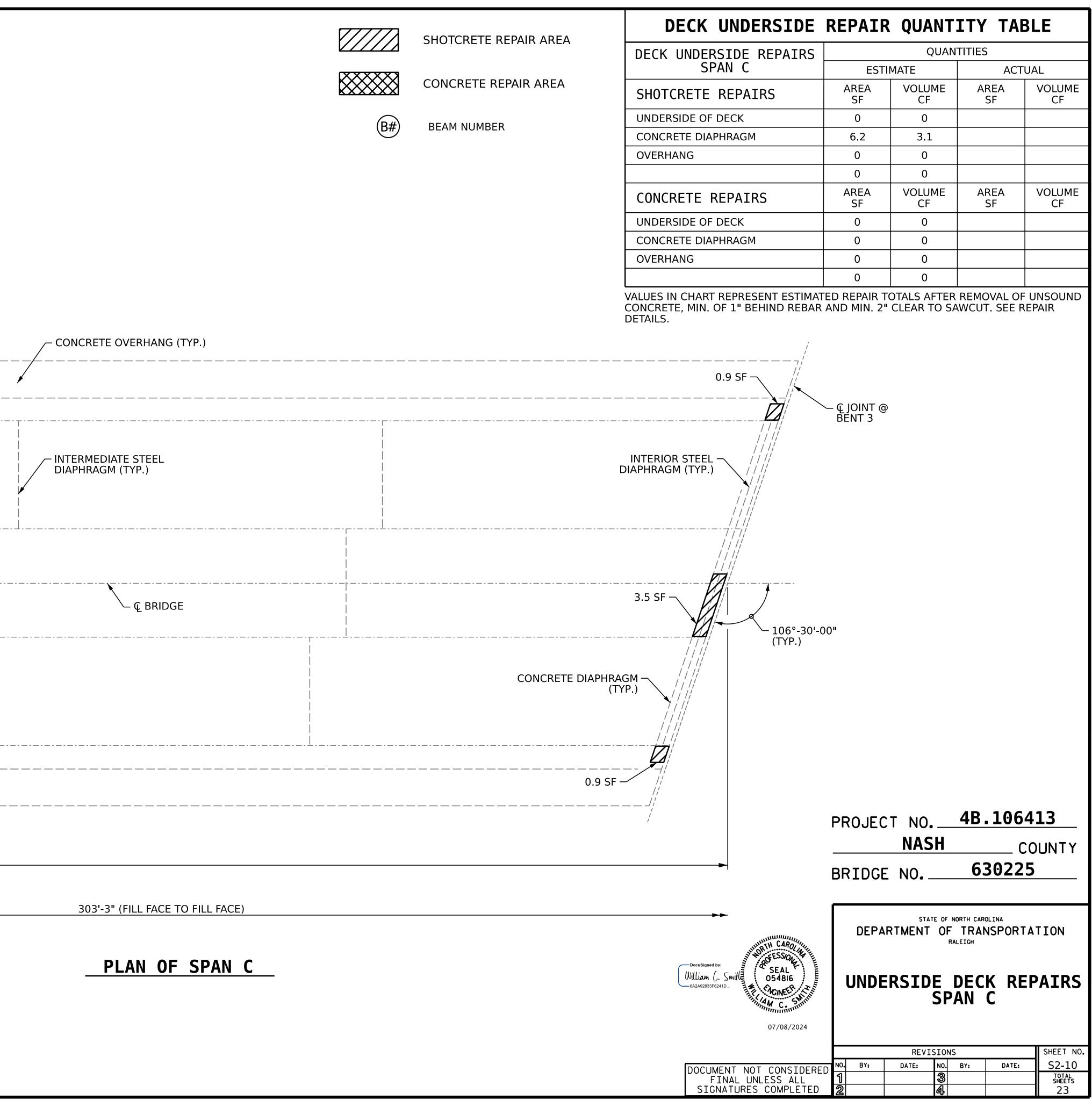
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DESIGN ENGINEER OF RECORD: .

. DATE : _

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE. CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER. € JOINT @ -BENT 2 ∕-- 0.9 SF (B1)- 🧲 BEAM (TYP.) (B2) ROADWAY Ц J **B**3 4 (B4) 2'-4¹/₄" ARRIER RAIL) S. T. SANDOR _ DATE : 05/2024 DRAWN BY : W. C. SMITH _ DATE : 05/2024 CHECKED BY : ___



DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE
DECK UNDERSIDE REPAIRS		QUAN	TITIES	
SPAN C	ESTIN	ИАТЕ	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	6.2	3.1		
OVERHANG	0	0		
	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
UNDERSIDE OF DECK	0	0		
CONCRETE DIAPHRAGM	0	0		
OVERHANG	0	0		
	0	0		

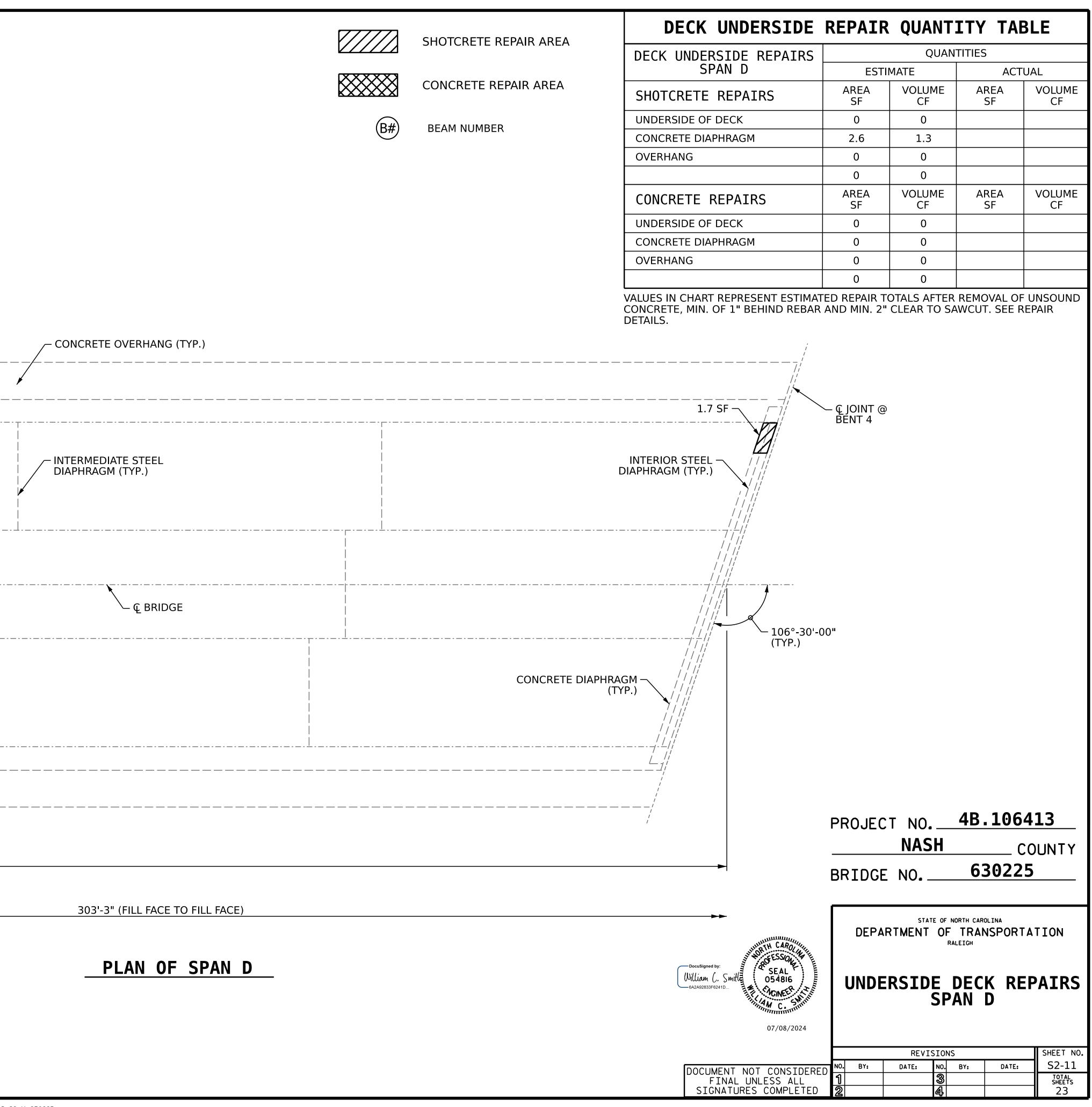
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DESIGN ENGINEER OF RECORD:

. DATE : _

REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE. CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER. € JOINT @ -BENT 3 ∕-- 0.9 SF (B1)- 🧲 BEAM (TYP.) (B2) ROADWAY К **B**3 4 (B4) 2'-4¹/₄" ARRIER RAIL) S. T. SANDOR _ DATE : 05/2024 DRAWN BY : W. C. SMITH _ DATE : 05/2024 CHECKED BY : ___



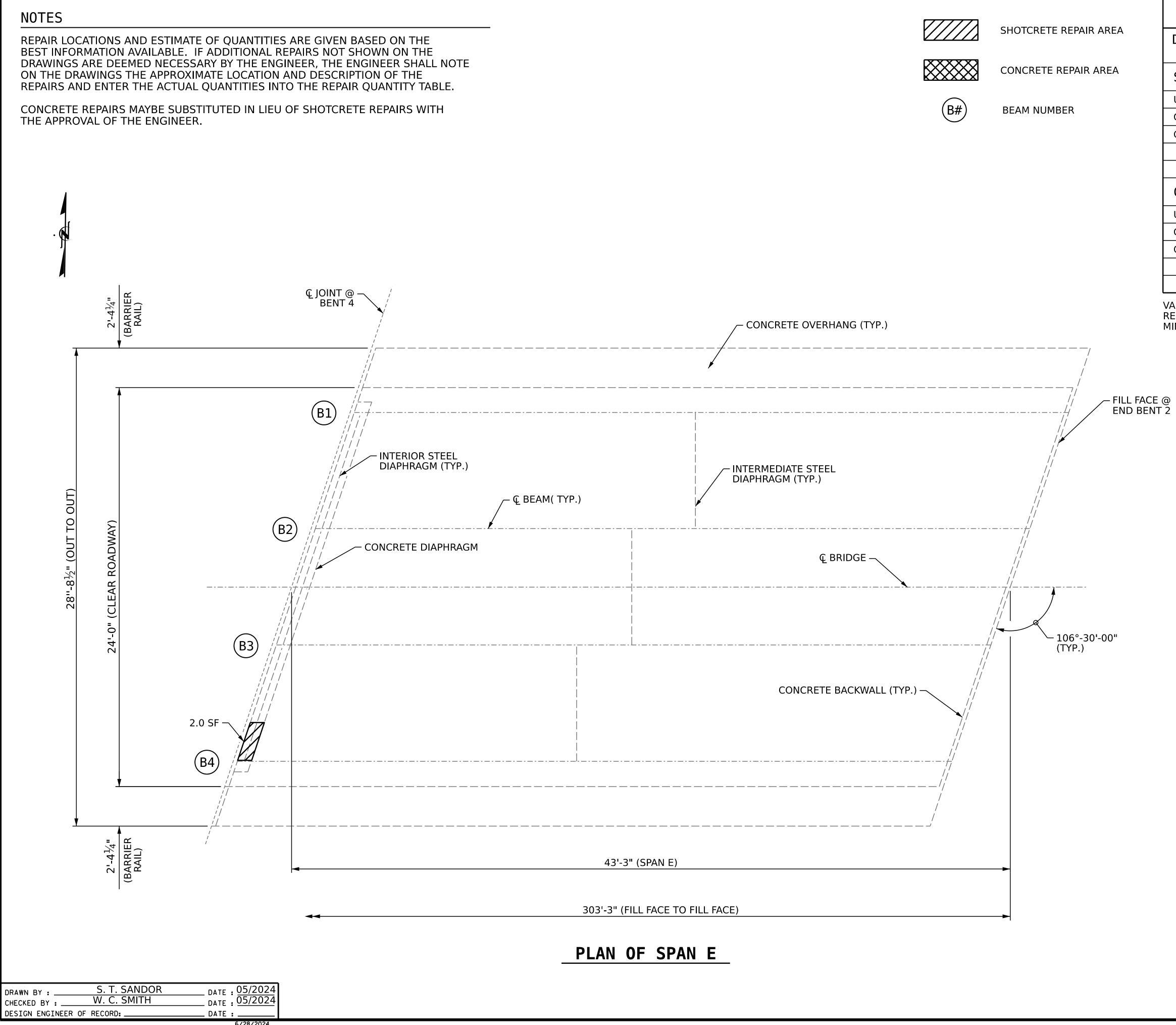
REPAIR	QUANT	ITY TA	BLE		
QUANTITIES					
ESTIN	1ATE	ACTUAL			
AREA SF	VOLUME CF	AREA SF	VOLUME CF		
0	0				
2.6	1.3				
0	0				
0	0				
AREA SF	VOLUME AREA CF SF		VOLUME CF		
0	0				
0	0				
0	0				
0	0				
	ESTIN AREA SF 0 2.6 0 2.6 0 0 AREA SF 0 0 0 0 0 0	QUAN ESTIMATE AREA SF VOLUME CF 0 0 2.6 1.3 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ESTIMATEACTAREA SFVOLUME CFAREA SF0002.61.30000000AREA SFVOLUME CFAREA SF000000000000000000000000		

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ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

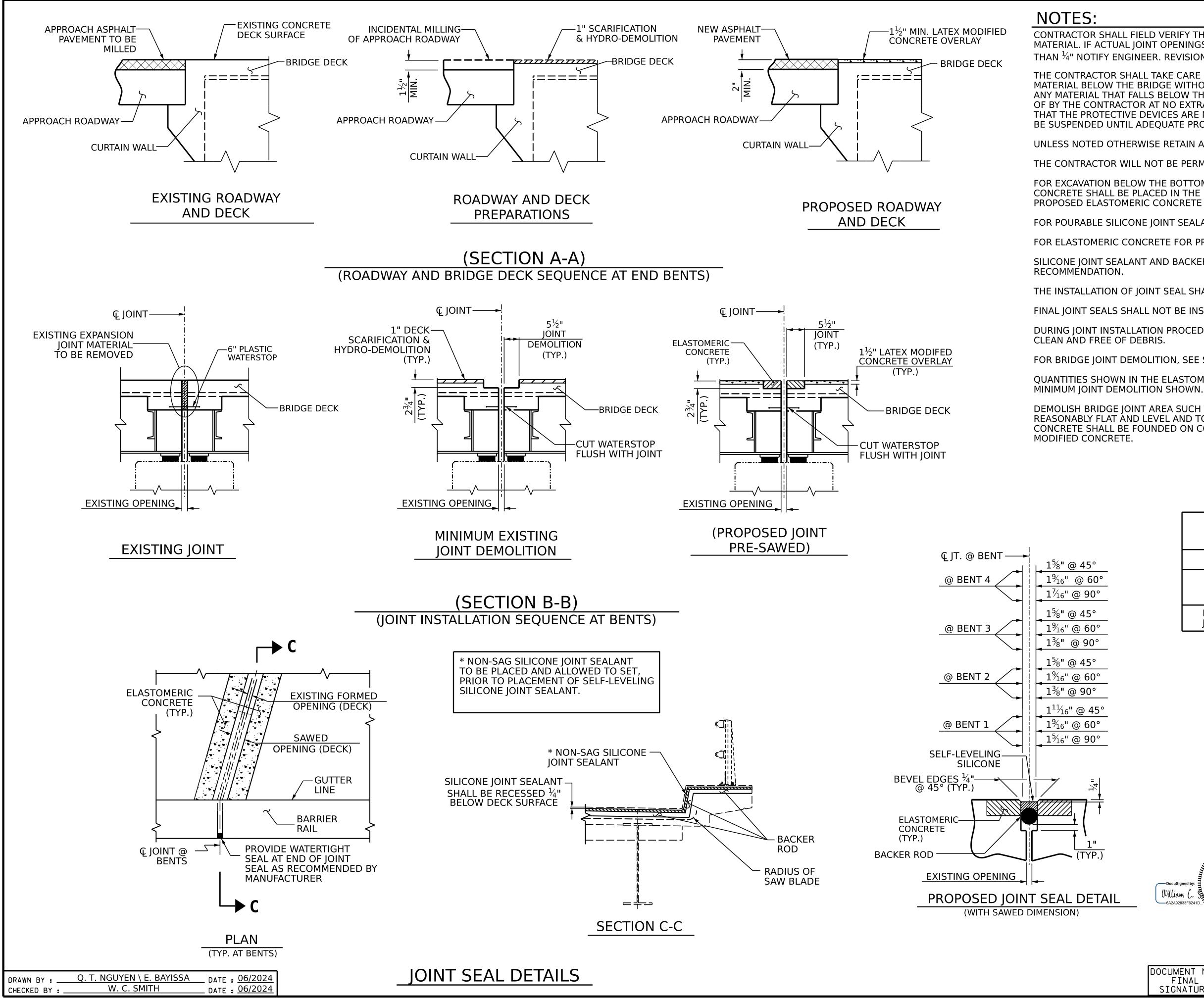


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DECK UNDERSIDE	REPAIR	QUANT	ITY TA	BLE	
DECK UNDERSIDE REPAIRS	QUANTITIES				
SPAN E	ESTI	МАТЕ	ACTUAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
UNDERSIDE OF DECK	0	0			
CONCRETE DIAPHRAGM	2.0	1.0			
OVERHANG	0	0			
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
UNDERSIDE OF DECK	0	0			
CONCRETE DIAPHRAGM	0	0			
OVERHANG	0	0			

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

	PROJECT NO. 48.10	6413			
	NASH	_ COUNTY			
	BRIDGE NO. 630	225			
DocuSigned by:	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH UNDERSIDE DECK REPAIL				
DocuSigned by: William C. Smith 6A2A92833F6241D MGINEER MGINEER 07/08/2024					
	REVISIONS				
DOCUMENT NOT CONSIDERED		DATE: S2-12			
FINAL UNLESS ALL SIGNATURES COMPLETED	1 3 2 4	shee ts 23			



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CONTRACTOR SHALL FIELD VERIFY THE EXISTING FORMED OPENING PRIOR TO OBTAINING JOINT MATERIAL. IF ACTUAL JOINT OPENINGS VARIES FROM THE OPENING INDICATED IN DETAIL MORE THAN $\frac{1}{4}$ " NOTIFY ENGINEER. REVISION TO THE JOINT SEAL SIZE MAY BE NECESSARY.

THE CONTRACTOR SHALL TAKE CARE DURING JOINT REHAB OPERATIONS NOT TO DROP ANY MATERIAL BELOW THE BRIDGE WITHOUT PROTECTIVE DEVICES BELOW TO CATCH THE MATERIAL. ANY MATERIAL THAT FALLS BELOW THE BRIDGE SHALL BE CONTAINED, REMOVED AND DISPOSED OF BY THE CONTRACTOR AT NO EXTRA COST TO THE DEPARTMENT. IF THE ENGINEER DETERMINES THAT THE PROTECTIVE DEVICES ARE NOT ADEQUATE OR NOT BEING EMPLOYED, THE WORK SHALL BE SUSPENDED UNTIL ADEQUATE PROTECTION IS PROVIDED.

UNLESS NOTED OTHERWISE RETAIN ALL EXISTING REINFORCING STEEL. CLEAN AND REPAIR AS NEEDED.

THE CONTRACTOR WILL NOT BE PERMITTED TO FORM THE JOINTS IN LIEU OF SAWING THE JOINT.

FOR EXCAVATION BELOW THE BOTTOM OF THE PLANNED JOINT DEMOLITION, APPROVED REPAIR CONCRETE SHALL BE PLACED IN THE EXCAVATED AREA TO THE ELEVATION AT BOTTOM OF THE PROPOSED ELASTOMERIC CONCRETE FOR PRESERVATION HEADERS SHOWN.

FOR POURABLE SILICONE JOINT SEALANT, SEE SPECIAL PROVISIONS.

FOR ELASTOMERIC CONCRETE FOR PRESERVATION, SEE SPECIAL PROVISIONS.

SILICONE JOINT SEALANT AND BACKER ROD SHALL BE INSTALLED AS PER MANUFACTURER'S

THE INSTALLATION OF JOINT SEAL SHALL BE WATERTIGHT.

FINAL JOINT SEALS SHALL NOT BE INSTALLED UNTIL THE OVERLAY IS COMPLETE.

DURING JOINT INSTALLATION PROCEDURE, THE JOINT AND SURROUNDING AREA SHALL BE KEPT

FOR BRIDGE JOINT DEMOLITION, SEE SPECIAL PROVISIONS.

QUANTITIES SHOWN IN THE ELASTOMERIC CONCRETE FOR PRESERVATION ARE BASED ON THE

DEMOLISH BRIDGE JOINT AREA SUCH THAT THE BOTTOM OF THE EXCAVATION SHALL BE REASONABLY FLAT AND LEVEL AND TO THE NECESSARY DEPTH. SUCH THAT ELASTOMERIC CONCRETE SHALL BE FOUNDED ON CONCRETE OR REPAIR CONCRETE SUBSTRATE, NOT LATEX

SUMMARY OF QUANTITIES				
	ESTIMATE	ACTUAL		
ELASTOMERIC CONCRETE FOR PRESERVATION	21.0 CF			
POURABLE SILICONE JOINT SEALANT	119.9 LF			

PROJECT NO	4B.106413
NASH	COUNTY

630225 BRIDGE NO.

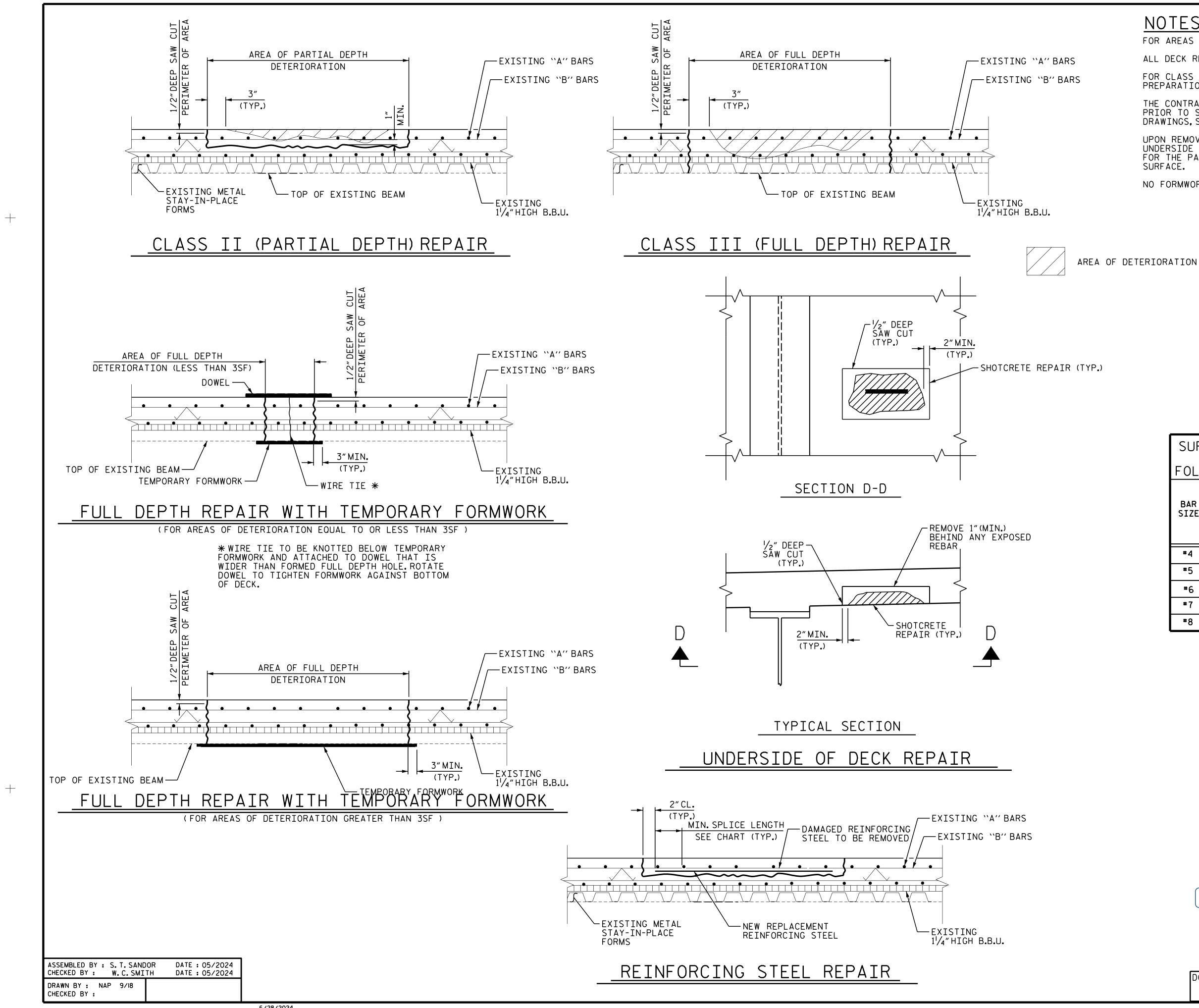
> STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH





JOINT REPAIR DETAILS

07/08/2024							
		REVISIONS					SHEET NO.
DOCUMENT NOT CONSIDERED	NO.	BY:	DATE:	NO.	BY:	DATE:	S2-13
FINAL UNLESS ALL	1			3			TOTAL SHEETS
SIGNATURES COMPLETED	2			ፈ ን			23



FOR AREAS TO BE REPAIRED, SEE "PLAN OF SPAN" SHEETS.

ALL DECK REPAIRS SHALL BE COMPLETED PRIOR TO PLACEMENT OF OVERLAY.

FOR CLASS II AND CLASS III SURFACE PREPARATION, SEE "LMC OVERLAY SURFACE PREPARATIONS" SPECIAL PROVISION.

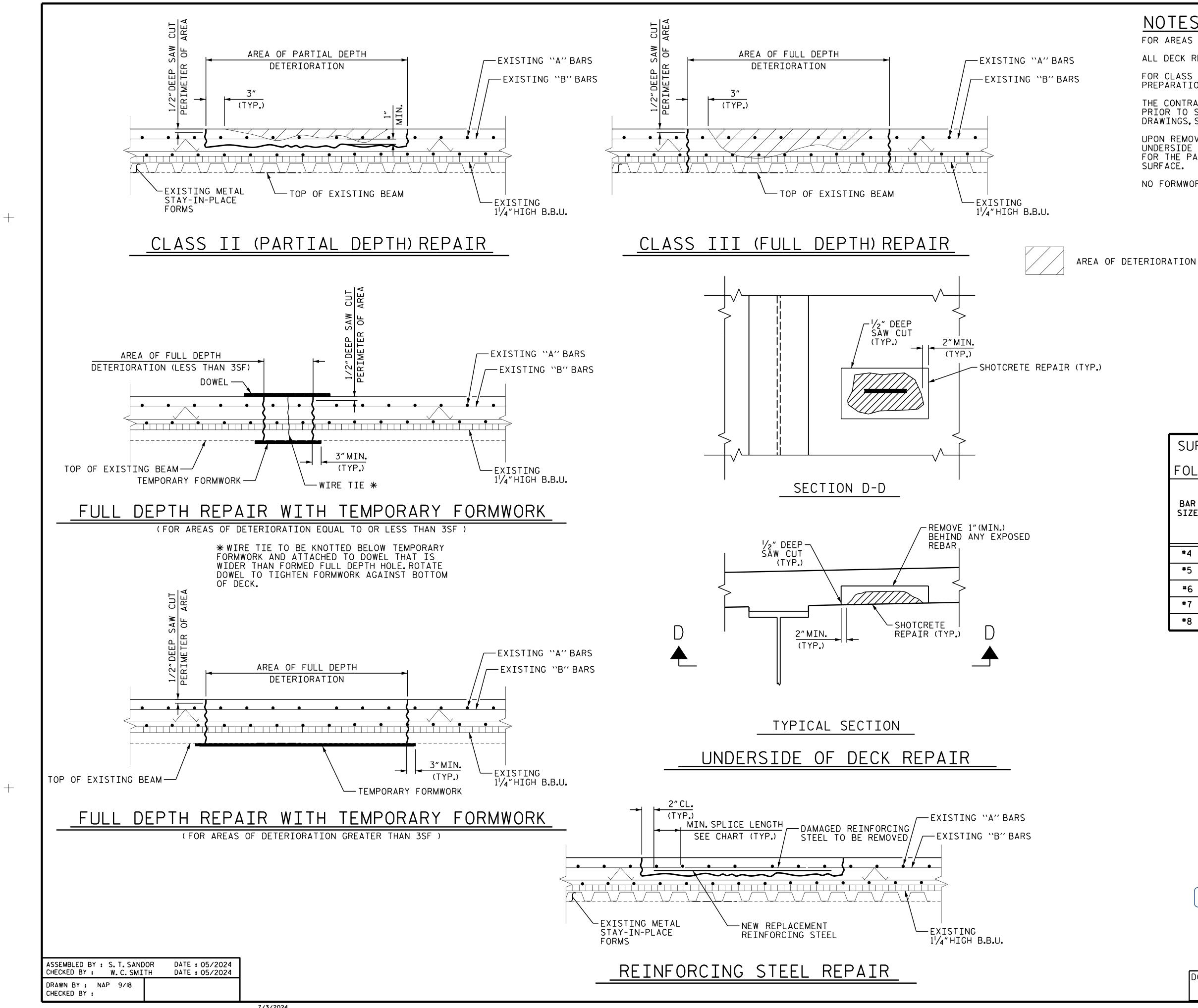
THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO STARTING WORK FOR TEMPORARY FORMWORK.FOR SUBMITTALS OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

UPON REMOVAL OF TEMPORARY FORMWORK, ALL VOIDS AND HONEYCOMBS ON THE UNDERSIDE OF DECK SURFACE SHALL BE FILLED WITH THE SAME MATERIAL AS USED FOR THE PATCH, AND FINISHED TO CONFORM TO THE SURROUNDING CONCRETE SURFACE.

NO FORMWORK SHALL BE LEFT IN PLACE.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS								
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	APPROACH	APPROACH SLABS		PARAPET AND BARRIER			
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL			
# 4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"			
* 5	2'-5″	2'-0"	2'-5″	2'-0"	3'-1"			
* 6	2'-10"	2′-5″	3′-7″	2'-5"	3′-8″			
* 7	4'-2"	2'-9"						
* 8	4'-9"	3'-2″						

	PROJEC BRIDGE	NAS			<u>.1064</u> co <u>)225</u>	<u>13</u> UNTY
DocuSigned by: William (6A2A92833F6241D. DOCUSIGNED MULLIAM (MOREFERMINING M		rtment S	OF T A	NDAF	NSPORTA	
		REVIS	SION	S		SHEET NO.
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SIGNATURES COMPLETED	2		4			23



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NOTES

FOR AREAS TO BE REPAIRED, SEE "PLAN OF SPAN" SHEETS.

ALL DECK REPAIRS SHALL BE COMPLETED PRIOR TO PLACEMENT OF OVERLAY.

FOR CLASS II AND CLASS III SURFACE PREPARATION, SEE "LMC OVERLAY SURFACE PREPARATIONS" SPECIAL PROVISION.

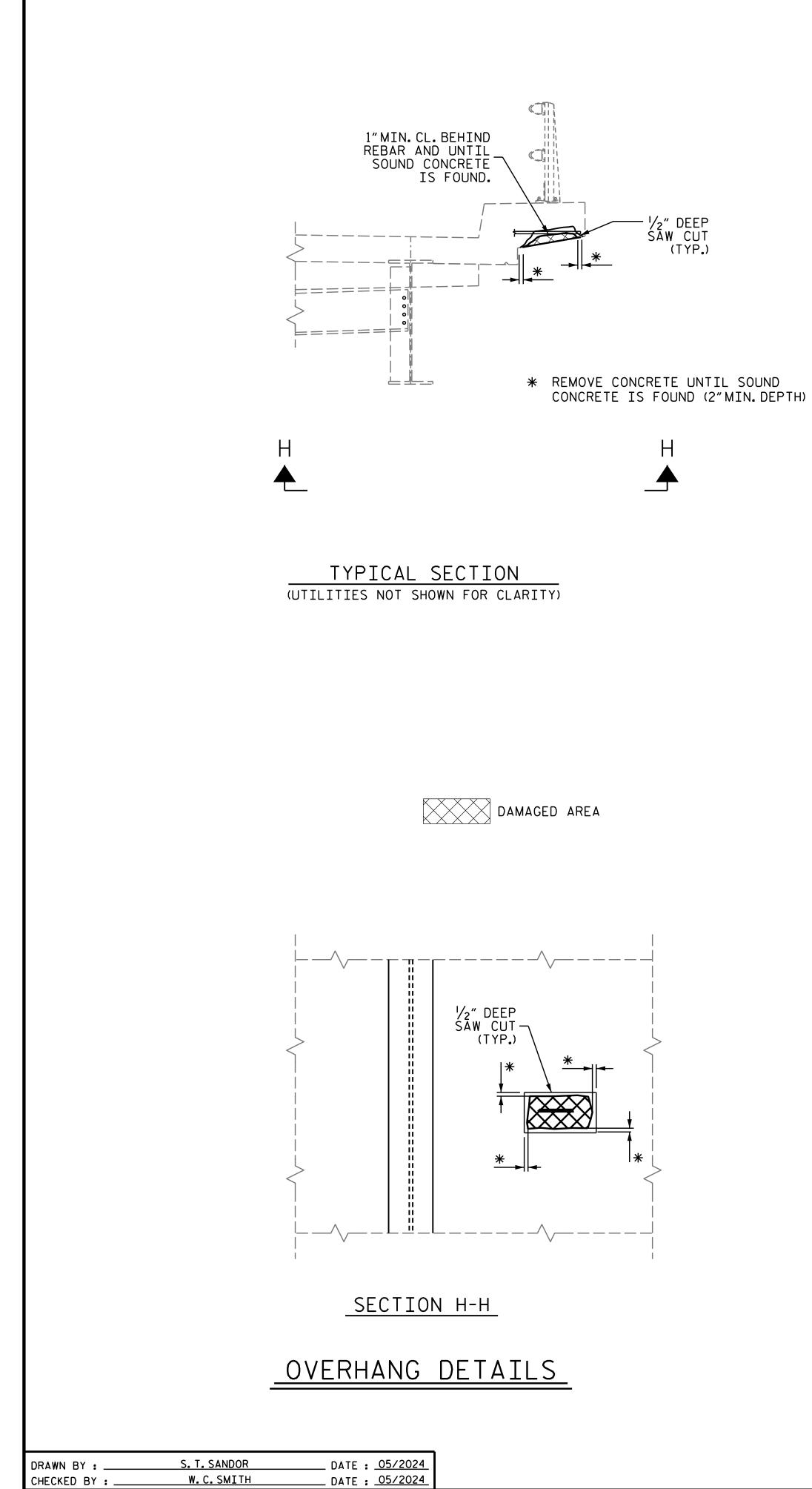
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NO FORMWORK SHALL BE LEFT IN PLACE.

SUPERSTRUCTURE REINFORCING STEEL LENGTHS ARE BASED ON THE FOLLOWING MINIMUM SPLICE LENGTHS								
BAR SIZE	SUPERST EXCEPT A SLABS, P AND BARR	APPROACH	APPROACH SLABS		PARAPET AND BARRIER			
	EPOXY COATED	UNCOATED	EPOXY COATED	UNCOATED	RAIL			
# 4	1'-11"	1'-7"	1'-11"	1'-7"	2'-6"			
* 5	2'-5″	2'-0"	2'-5″	2'-0"	3'-1"			
* 6	2'-10"	2′-5″	3′-7″	2'-5"	3′-8″			
* 7	4'-2"	2'-9"						
* 8	4'-9"	3'-2″						

	PROJEC BRIDGE	NAS			<u>.1064</u> co <u>)225</u>	<u>13</u> UNTY
DocuSigned by: William (6A2A92833F6241D. DOCUSIGNED MULLIAM (MOREFERMINING M		rtment S	OF T A	NDAF	NSPORTA	
		REVIS	SION	S		SHEET NO.
DOCUMENT NOT CONSIDERED	NO. BY:	DATE:	NO.	BY:	DATE:	S2-14
FINAL UNLESS ALL	1		3			TOTAL SHEETS
SIGNATURES COMPLETED	2		4			23



THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

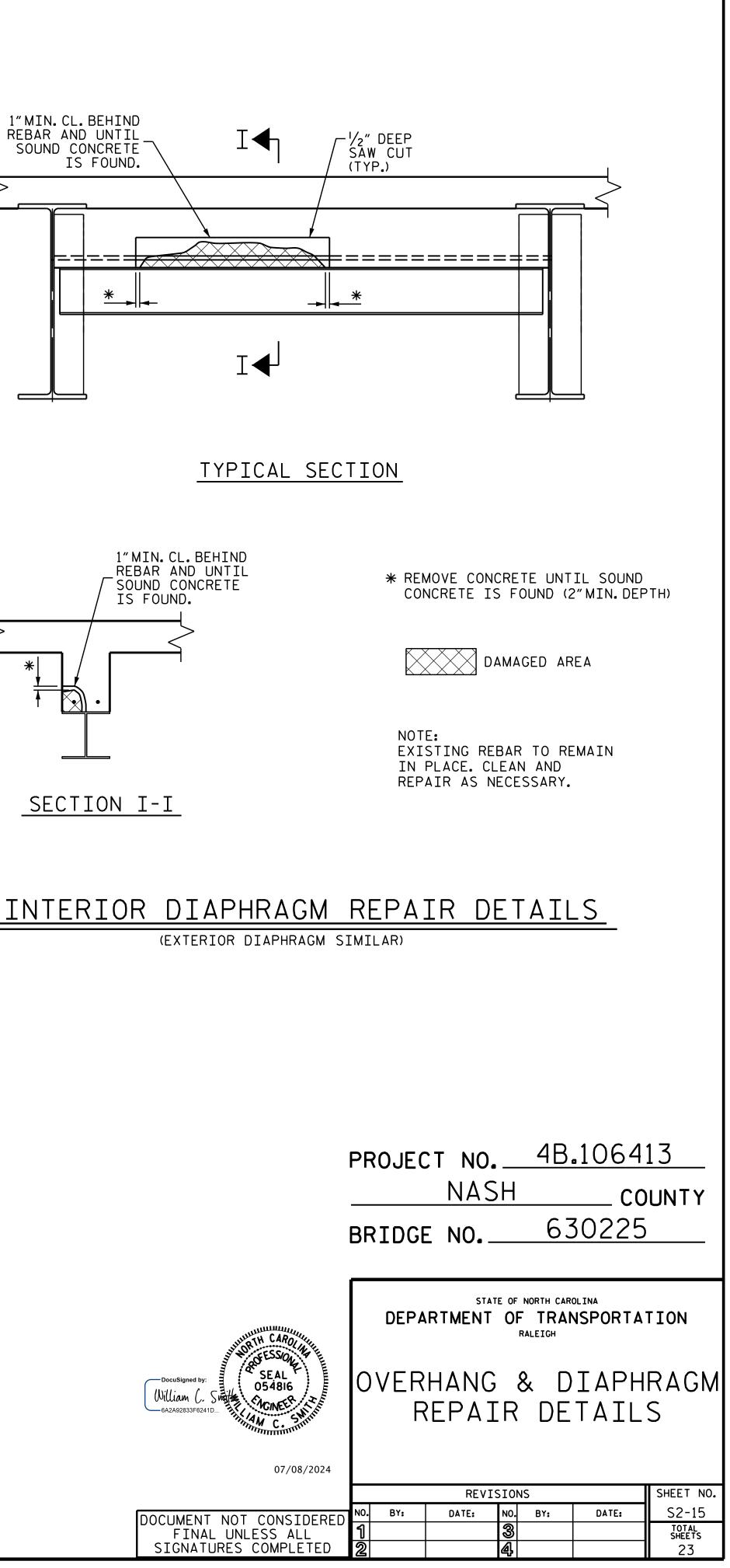
REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON the "DECK REPAIR DETAILD" SHEET.

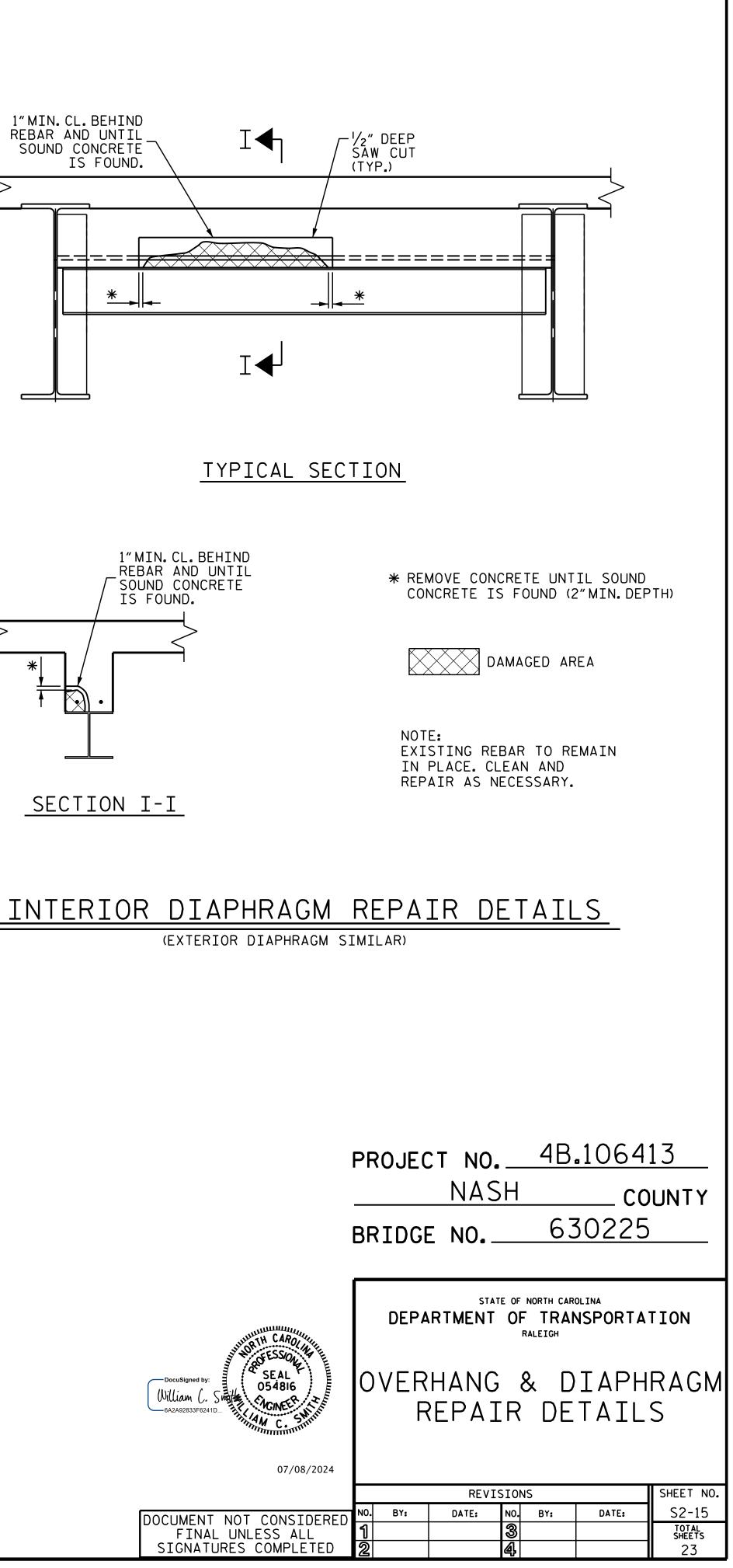
FOR AREAS TO BE REPAIRED, SEE "UNDERSIDE DECK REPAIRS" SHEETS.

THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO STARTING WORK FOR TEMPORARY FORMWORK.FOR SUBMITTALS OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

UPON REMOVAL OF TEMPORARY FORMWORK, ALL VOIDS AND HONEYCOMBS ON THE UNDERSIDE OF DECK SURFACE SHALL BE FILLED WITH THE SAME MATERIAL AS USED FOR THE PATCH, AND FINISHED TO CONFORM TO THE SURROUNDING CONCRETE SURFACE.

NO FORMWORK SHALL BE LEFT IN PLACE.

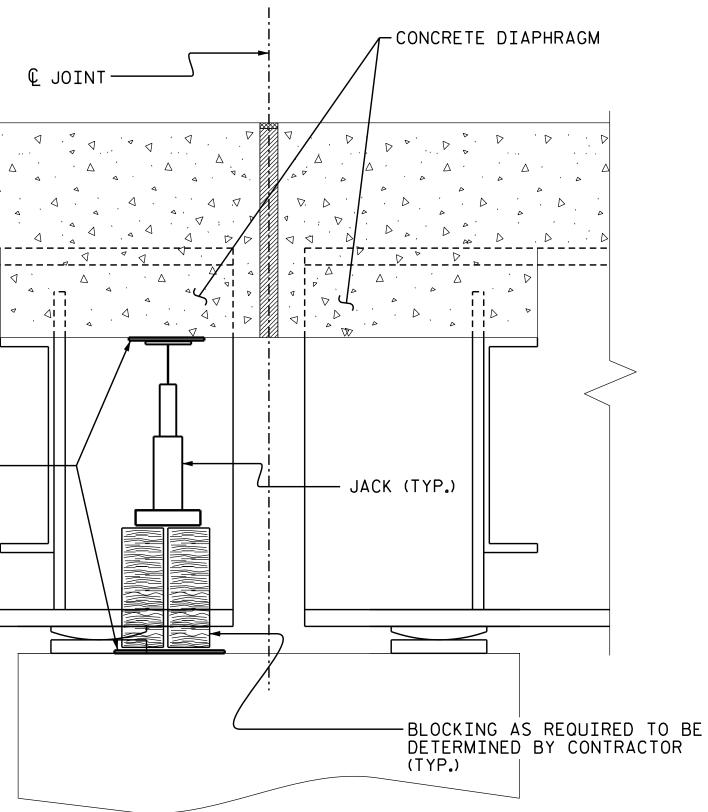




			4
		1"(MIN.) THIC	
		PLATES NE DISTRIBUTE T (REQUIRED AT JACK AND BE	FDFD TO
ASSEMBLED BY : S. T. SANDOR CHECKED BY : W. C. SMITH	DATE : 05/2024 DATE : 05/2024		

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SECTION THRU DIAPHRAGM

BRIDGE JACKING TABLE							
LOCATION	SPAN	BEAM(S)	BRIDGE JACKING TYPE	DEAD LOAD (DC + DW) (KIPS)			
BENT 1	А	1, 2, 3, 4	TYPE II				
BENT 1	В	1, 2, 3, 4	TYPE II				
BENT 2	С	1,2,3,4	TYPE II				
BENT 3	D	1, 2, 3, 4	TYPE II				
BENT 4	E	1,2,3,4	TYPE II				

BRIDGE JACKING NOTES:

THIS DETAIL IS A GENERIC EXAMPLE OF A JACKING SCHEME AND DOES NOT NECESSARILY REPRESENT SPECIFIC CONDITIONS AT A PARTICULAR BRIDGE. ACTUAL BRIDGE GEOMETRIES, DIMENSIONS, AND CONDITIONS MAY DIFFER FROM THIS DETAIL. PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL INVESTIGATE THE BRIDGES ON THE PROJECT AND DEVELOP A JACKING PLAN TO BE SUBMITTED FOR REVIEW AND APPROVAL. SEE BRIDGE JACKING SPECIAL PROVISION.

PRIOR TO BRIDGE JACKING OPERATIONS. THE ENGINEER AND CONTRACTOR SHALL INSPECT THE STRUCTURE FOR ANY NOTABLE DEFECTS TO THE PRIMARY AND SECONDARY STRUCTURAL MEMBERS. ALL NOTABLE DEFECTS SHALL BE DOCUMENTED AND REPORTED TO THE AREA BRIDGE MAINTENANCE ENGINEER PRIOR TO COMMENCEMENT OF ANY BRIDGE JACKING. THE CONTRACTOR SHALL PROVIDE SAFE AND SUFFICIENT ACCESS TO ALL STRUCTURAL MEMBERS FOR THE ENGINEER TO ESTABLISH PROPER DOCUMENTATION.

PRIOR TO JACKING, THE CONTRACTOR SHALL ENSURE THERE ARE NO OBSTACLES PREVENTING THE BEAM FROM BEING LIFTED.

THE BEAM SHALL BE LIFTED ENOUGH THAT THE BEAM CLEARS THE BEARINGS AND ALL LOAD IS SUPPORTED BY THE JACKS. AFTER JACKING IS COMPLETE, THE CONTRACTOR SHALL PROVIDE FOR A METHOD TO REMOVE THE JACKS AND SUPPORT THE BEAM FOR DEAD AND LIVE LOAD DURING THE REPAIR OPERATIONS. IF THE JACKS REMAIN IN PLACE DURING THE ENTIRE JACKING AND REPAIR OPERATION, THEY SHALL HAVE MECHANICAL LOCK OFF CAPABILITIES.

IF, DURING THE JACKING PROCESS, OR WHILE THE BEAM IS BEING SUPPORTED. THE BEAM SHIFTS FROM ITS ORIGINAL POSITION, ALL WORK SHALL CEASE AND THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY.

BEARINGS ADJACENT TO THE BEAM BEING JACKED MAY BE LOOSENED TO DECREASE THE RESISTANCE OF THE DECK SLAB DURING JACKING. ALL BEARINGS LOOSENED SHALL BE TIGHTENED BACK AFTER REPAIR OPERATIONS ARE COMPLETED AND THE JACKS AND BLOCKING HAVE BEEN REMOVED.

THE MAXIMUM DIFFERENTIAL BETWEEN ADJACENT BEAMS THAT ARE BEING JACKED IS 1/8".

LOADS PROVIDED IN THE "BRIDGE JACKING TABLE" ARE SHOWN FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR'S ENGINEER SHALL DETERMINE THE EXPECTED LOADS TO BE LIFTED DURING THE BRIDGE JACKING OPERATIONS.

THE CONTRACTOR SHALL SUBMIT WORKING DRAWINGS AND CALCULATIONS OF THE JACKING PROCEDURE(S) SEALED BY A PROFESSIONAL ENGINEER IN THE STATE OF NORTH CAROLINA TO THE ENGINEER FOR APPROVAL PRIOR TO BRIDGE JACKING OPERATIONS.

FOR TYPE II BRIDGE JACKING, SEE SPECIAL PROVISIONS.

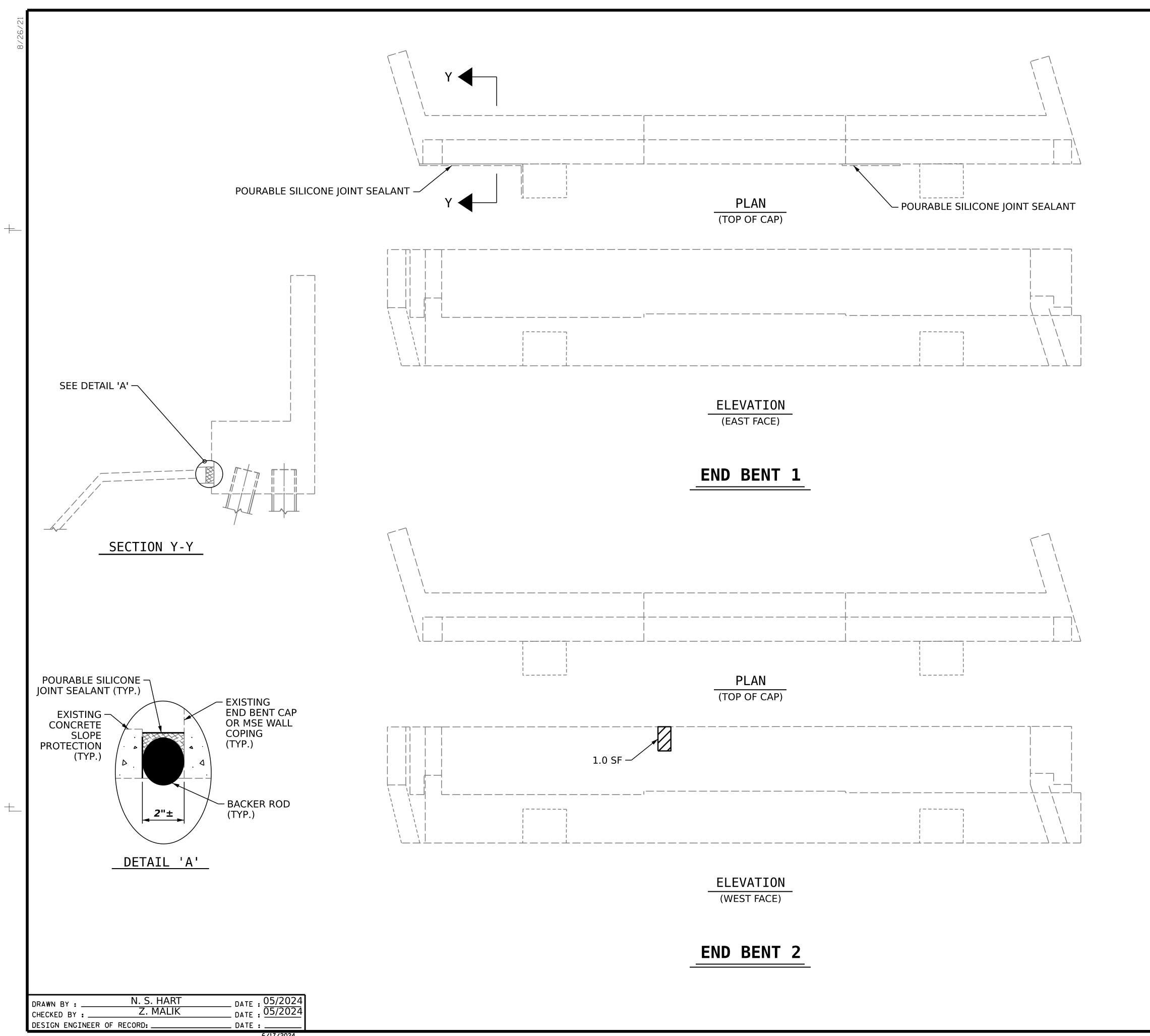
FOR WORKING DRAWING SUBMITTALS, SEE SPECIAL PROVISIONS.

ANY STEEL THAT HAS BEEN WELDED TO THE EXISTING STRUCTURE SHALL REMAIN IN PLACE.

TYPE II BRIDGE JACKING SHALL BE DONE WITH A HYDRAULIC JACKING SYSTEM THAT LIFTS EACH BEAM ALONG ENTIRE SPAN END WITH EQUAL FORCE AND AT EQUAL RATE.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPAIRING ANY DAMAGE CAUSED TO THE EXISTING STRUCTURE BY BRIDGE JACKING OPERATIONS AT NO ADDITIONAL COST TO THE DEPARTMENT.

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WINN CAROLANT	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						
DocuSigned by: William (. Smither 6A2A92833F6241D	STANDARD BRIDGE JACKING DETAILS						
07/08/2024							
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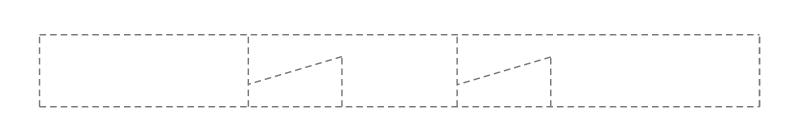
SUBSTRUCTURE R	REPAIR	QUANTI	TY TABI	LE
REPAIRS - END BENT 1 & 2			TITIES	
	ESTI AREA	MATE VOLUME	ACT AREA	UAL VOLUME
SHOTCRETE REPAIRS	SF	CF	SF	CF
CAP CURTAIN WALL	0	0		
WINGWALL	1.0	0.5		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	0	0		
CURTAIN WALL WINGWALL	0	0		
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT
САР		0		
		0		
		AREA		AREA
EPOXY COATING TOP OF CAP		SF 98.5		SF
POURABLE SILICONE JT. S	EALANT	LINEAR		LINEAR FT
JOINT		10.1		
VALUES IN CHART REPRESENT ESTIMAT				
REMOVAL OF UNSOUND CONCRETE, M MIN. 2" CLEAR TO SAWCUT. SEE REPAI		HIND REBAR A	ND	
NOTES				
REPAIR LOCATIONS AND ESTIMATE OF O BEST INFORMATION AVAILABLE. IF ADD DRAWINGS ARE DEEMED NECESSARY E ON THE DRAWINGS THE APPROXIMATE REPAIRS AND ENTER THE ACTUAL QUA CLEAN AND REMOVE DEBRIS FROM TH PROTECTIVE COATING. EPOXY COATING OF THE CAP. THE CONTRACTOR SHAL	DITIONAL REP BY THE ENGIN LOCATION A NTITIES INTC E TOP OF TH G SHALL BE A L NOT COAT	PAIRS NOT SH NEER, THE ENO ND DESCRIPT O THE REPAIR E CAP AND AF APPLIED TO TI THE AREA OF	OWN ON THE GINEER SHAL ION OF THE QUANTITY TA PPLY EPOXY HE TOP SURF/ THE CAP BEN	L NOTE BLE. ACE
THE MASONRY PLATES. FOR EPOXY CC CONCRETE REPAIRS MAYBE SUBSTITUT THE APPROVAL OF THE ENGINEER.				ТН
SHOTCRETE REPAIR ARE	A			
CONCRETE REPAIR AREA	A			
PREVIOUSLY ACCOUNTER	D FOR AREA			
EPOXY RESIN INJECTION				
	PROJEC	NASH	4B.106	COUNTY
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REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

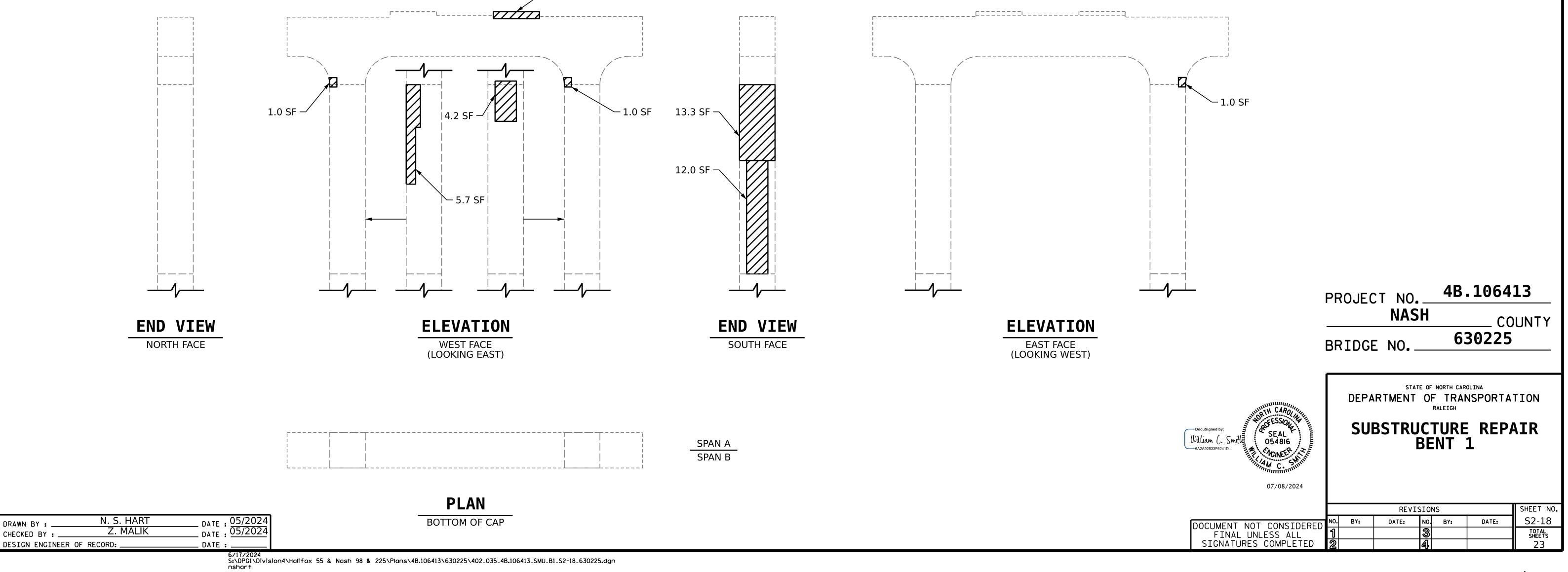
CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

SHEET.



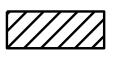




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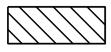
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS"



SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION

SPAN B SPAN A



SUBSTRUCTURE R	EPAIR	QUANTI	TY TAB	LE
		QUAN	TITIES	
REPAIRS - BENT 1	ESTI	MATE	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	1.6	0.8		
COLUMN	38.2	19.1		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
CAP	0	0		
COLUMN	0	0		
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT
CAP		0		
COLUMN		0		
EPOXY COATING		AREA SF		AREA SF
TOP OF CAP		58.5		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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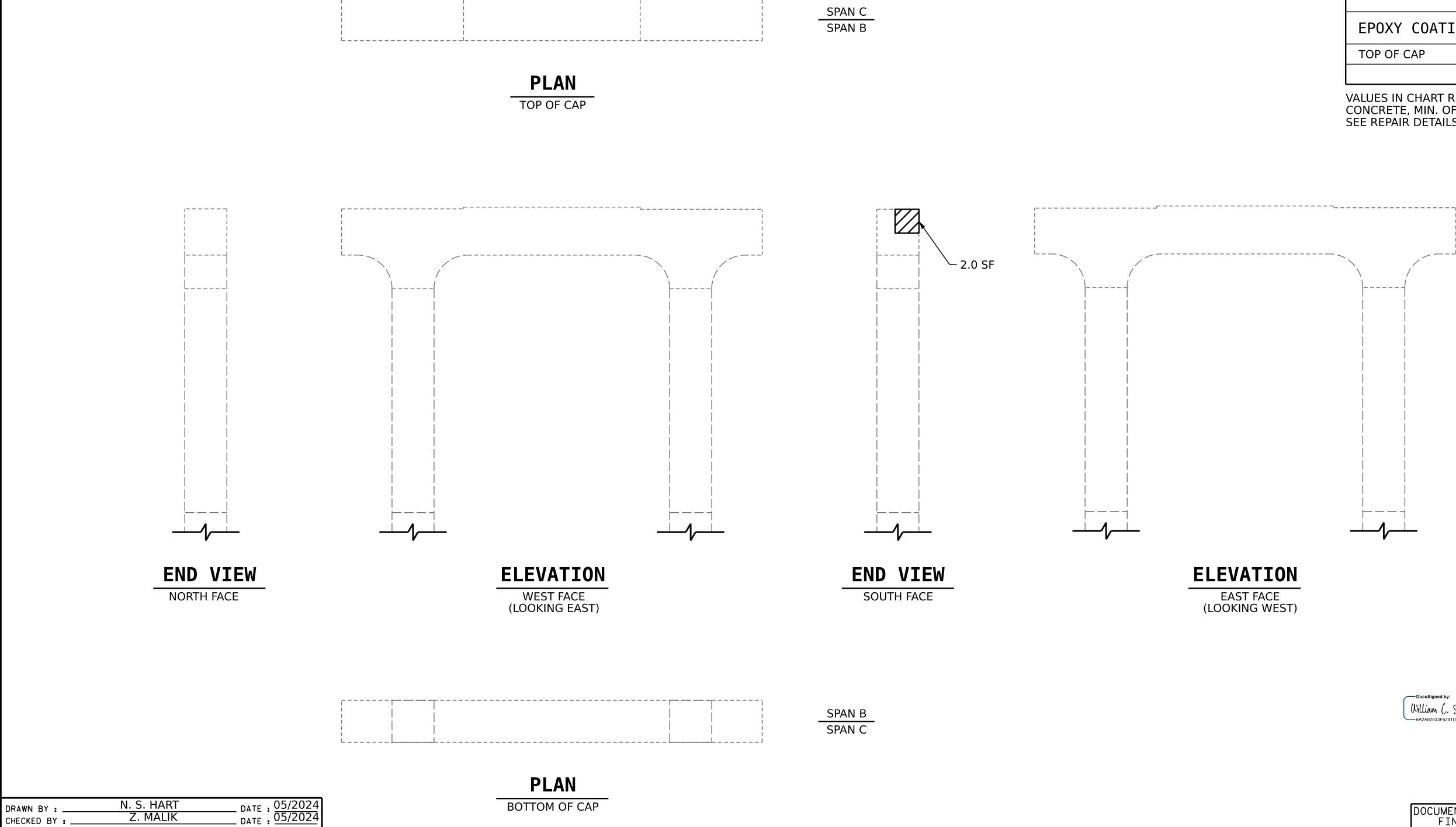
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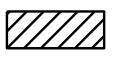
CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

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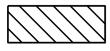
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS"



SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA



SUBSTRUCTURE R	EPAIR	QUANTI	ΤΥ ΤΑΒΙ	_E
REPAIRS - BENT 2		QUAN	TITIES	
REFAIRS - DENT Z	ESTI	MATE	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	2.0	1.0		
COLUMN	0	0		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	0	0		
COLUMN	0	0		
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT
САР		0		
COLUMN		0		
EPOXY COATING		AREA SF		AREA SF
TOP OF CAP		58.5		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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REPAIR LOCATIONS AND ESTIMATE OF QUANTITIES ARE GIVEN BASED ON THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS AND ENTER THE ACTUAL QUANTITIES INTO THE REPAIR QUANTITY TABLE.

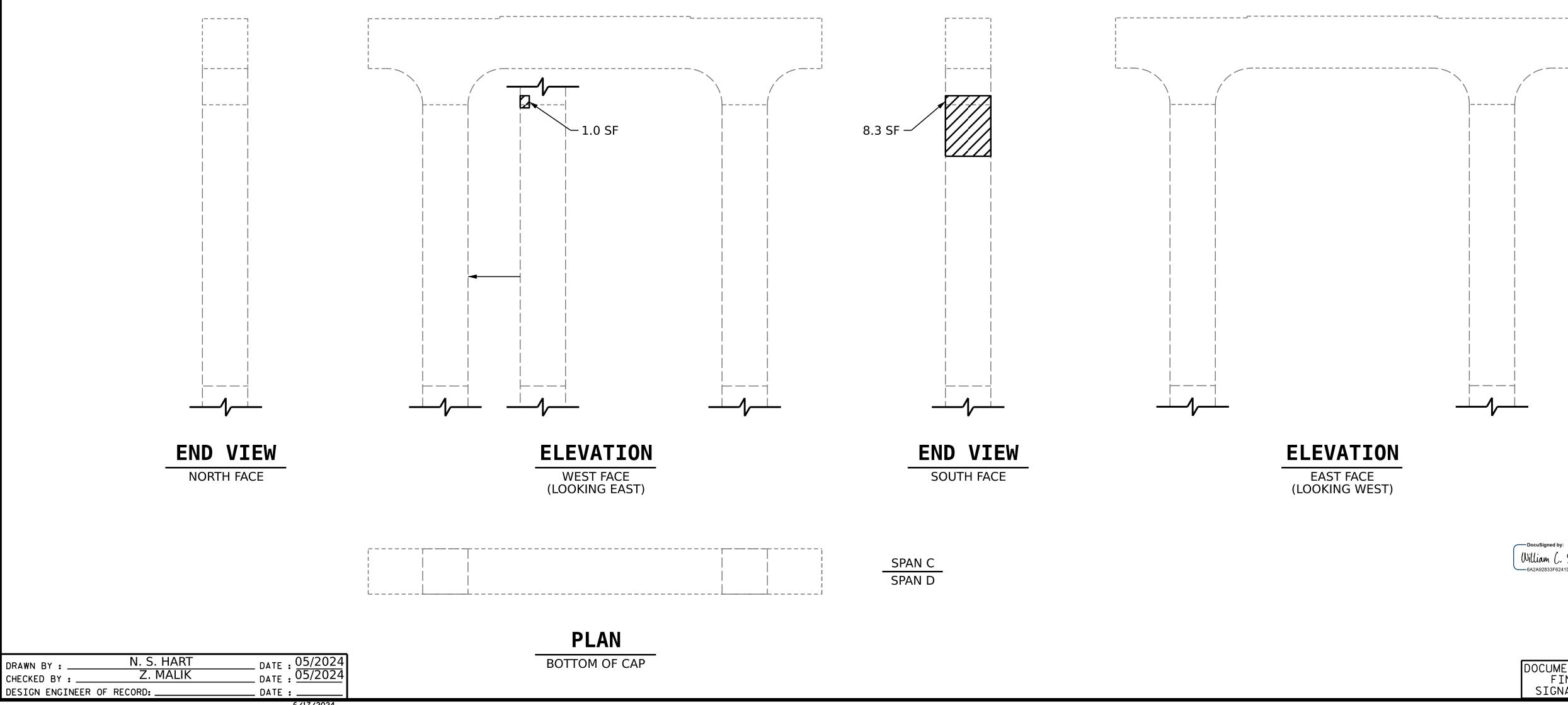
CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

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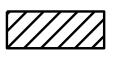


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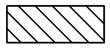
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS"



SHOTCRETE REPAIR AREA



CONCRETE REPAIR AREA



PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION

SPAN D SPAN C

SUBSTRUCTURE R	EPAIR	QUANTI	ΤΥ ΤΑΒΙ	E
REPAIRS - BENT 3		QUAN ⁻	TITIES	
REPAIRS - DENI S	ESTI	MATE	ACT	UAL
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	0	0		
COLUMN	9.3	4.7		
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF
САР	0	0		
COLUMN	0	0		
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT
САР		0		
COLUMN		0		
EPOXY COATING		AREA SF		AREA SF
TOP OF CAP		58.5		

VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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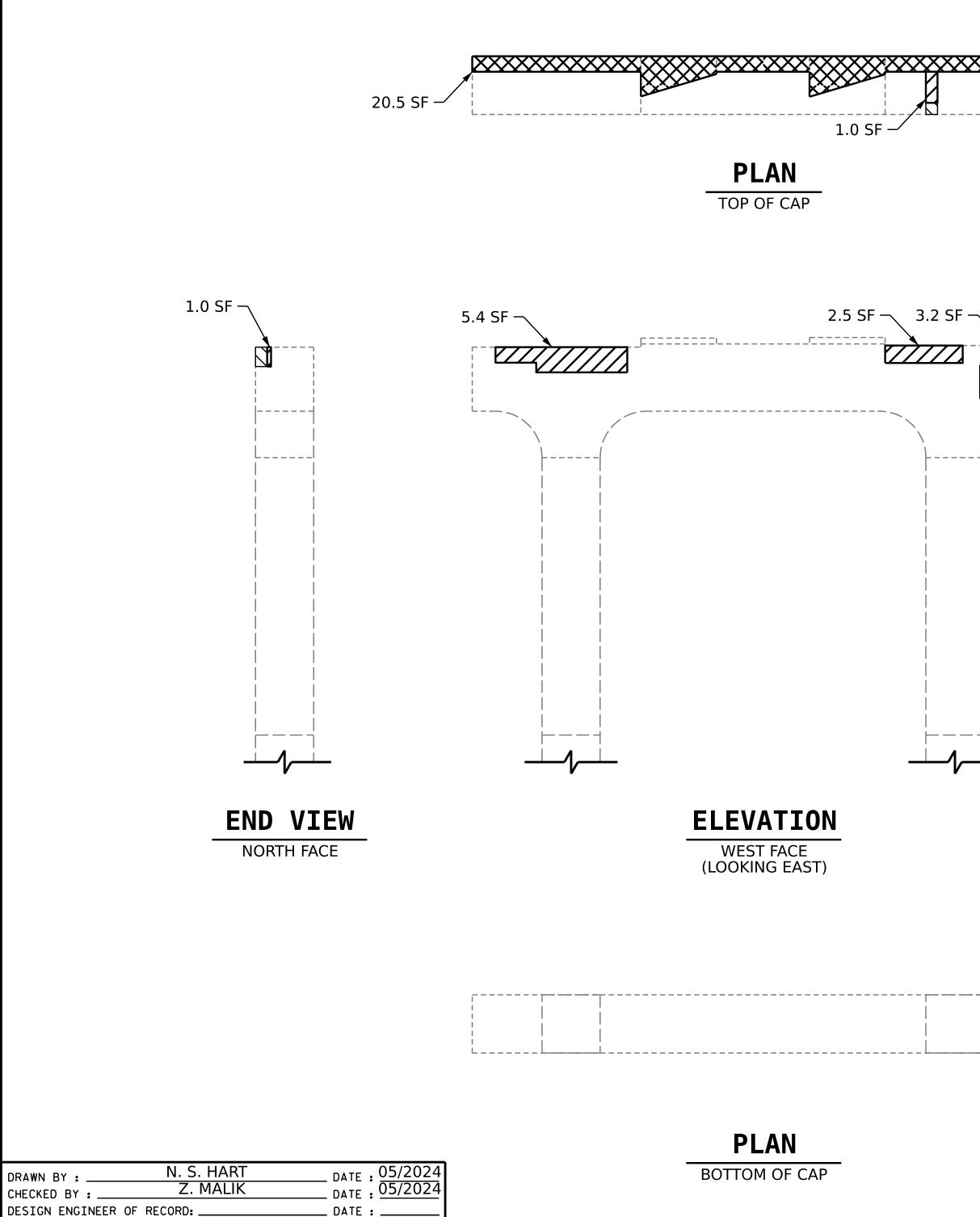


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CLEAN AND REMOVE DEBRIS FROM THE TOP OF THE CAP AND APPLY EPOXY PROTECTIVE COATING. EPOXY COATING SHALL BE APPLIED TO THE TOP SURFACE OF THE CAP. THE CONTRACTOR SHALL NOT COAT THE AREA OF THE CAP BENEATH THE MASONRY PLATES. FOR EPOXY COATING, SEE SPECIAL PROVISIONS.

CONCRETE REPAIRS MAYBE SUBSTITUTED IN LIEU OF SHOTCRETE REPAIRS WITH THE APPROVAL OF THE ENGINEER.

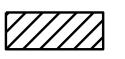
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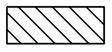
FOR CAP AND COLUMN REPAIR, SEE "TYPICAL CAP AND COLUMN REPAIR DETAILS"



SHOTCRETE REPAIR AREA

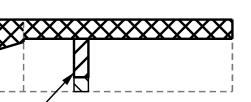


CONCRETE REPAIR AREA

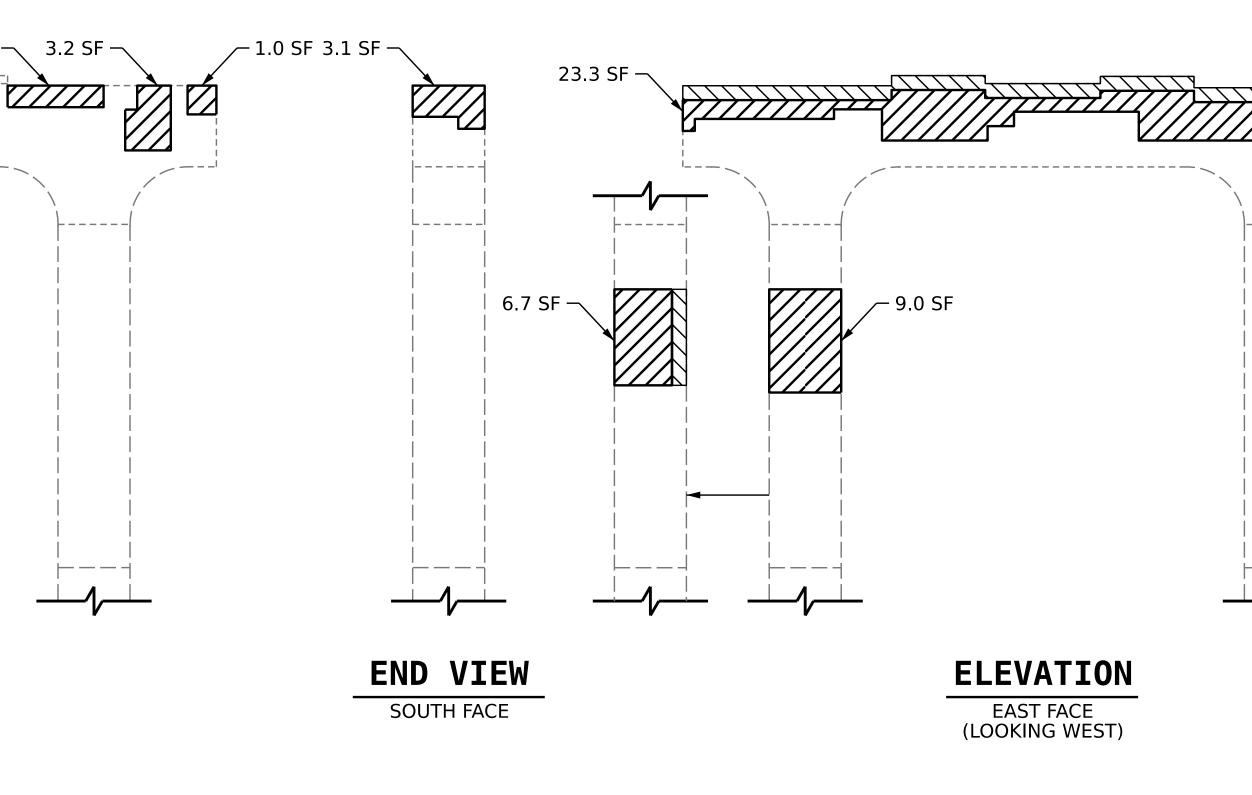


PREVIOUSLY ACCOUNTED FOR AREA

EPOXY RESIN INJECTION \sim



SPAN E SPAN D



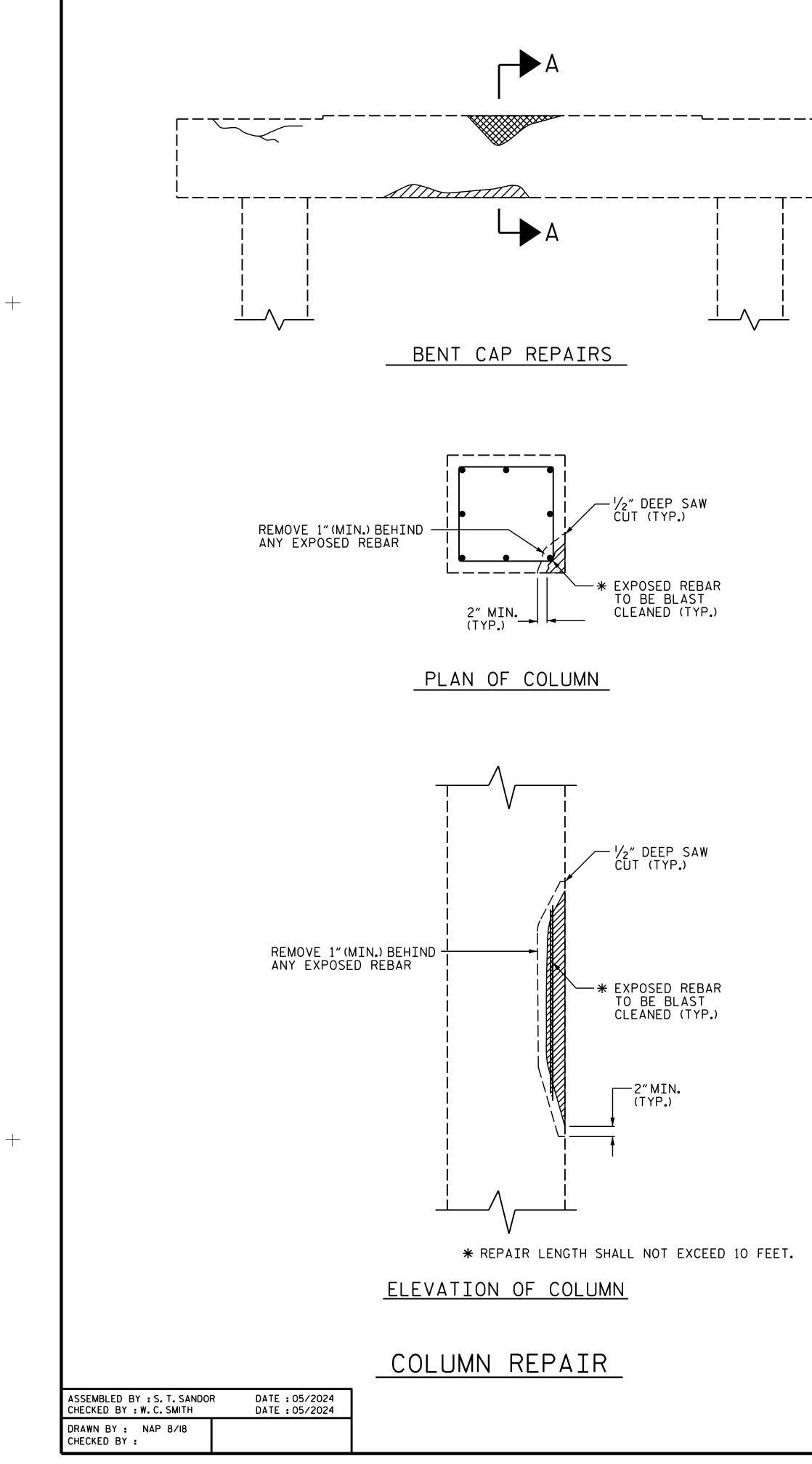


SUBSTRUCTURE REPAIR QUANTITY TABLE					
REPAIRS - BENT 4	QUANTITIES				
REFAIRS - DENI 4	ESTIMATE		ACTUAL		
SHOTCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
САР	40.5	20.3			
COLUMN	15.7	7.9			
CONCRETE REPAIRS	AREA SF	VOLUME CF	AREA SF	VOLUME CF	
САР	20.5	10.3			
COLUMN	0	0			
EPOXY RESIN INJECTION		LINEAR FT		LINEAR FT	
САР		0			
COLUMN		0			
EPOXY COATING		AREA SF		AREA SF	
TOP OF CAP		58.5			

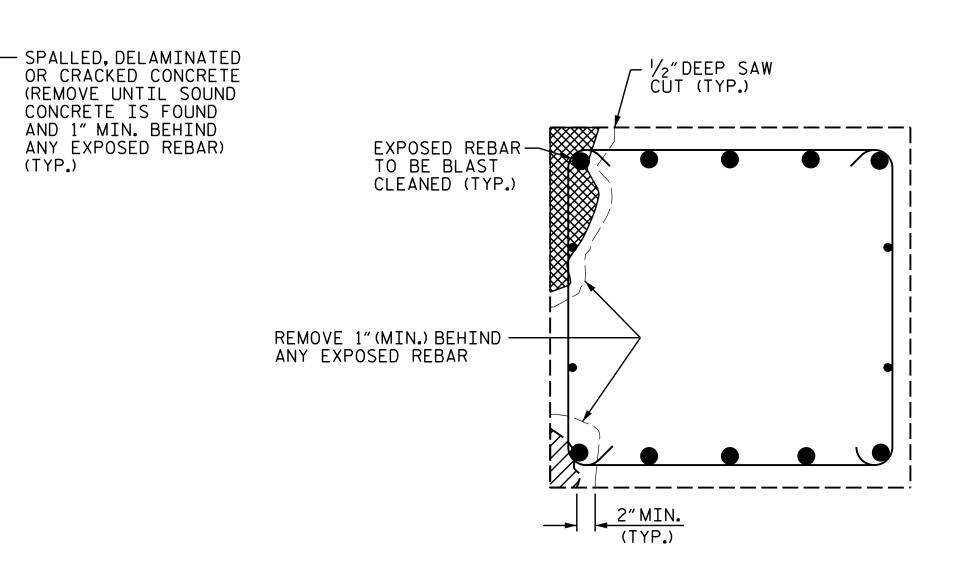
VALUES IN CHART REPRESENT ESTIMATED REPAIR TOTALS AFTER REMOVAL OF UNSOUND CONCRETE, MIN. OF 1" BEHIND REBAR AND MIN. 2" CLEAR TO SAWCUT. SEE REPAIR DETAILS.

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	PROJECT NO. NAS BRIDGE NO	H	1064 CO 0225	13 UNTY
DocuSigned by: William (. Smith 6A2A92833F6241D 07/08/2024	DEPARTMENT SUBSTRU	RALEIGH	SPORTAT	
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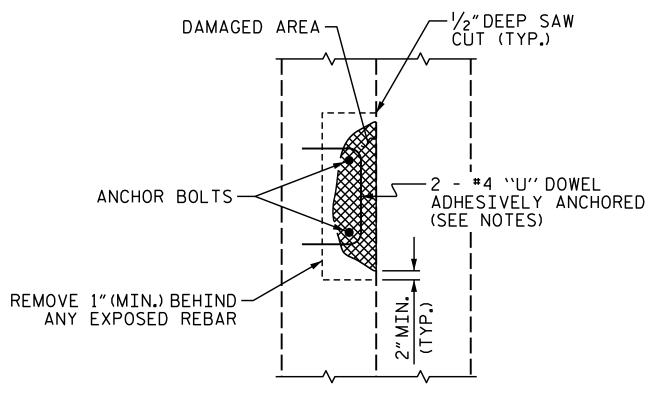
REPAIR KEY

SECTION A-A

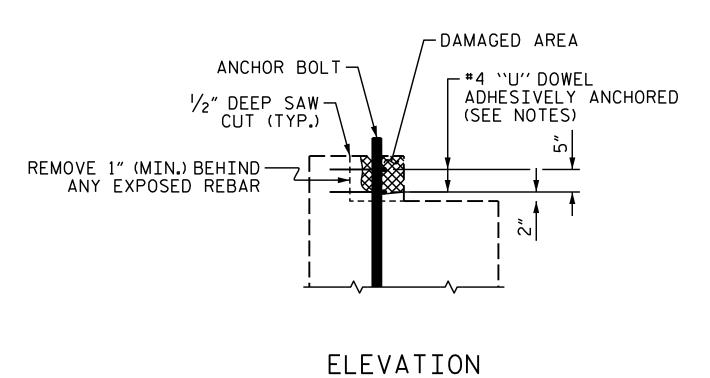
CAP REPAIR

SHOTCRETE REPAIR AREA

CONCRETE REPAIR AREA (FORM AND POUR)







PEDESTAL WALL REPAIR

SPLICE	LENGTH TABL	E	
BAR SIZE	MIN. SPLICE LENGTH		
# 4	2'-5"		
# 5	3'-0"		
# 6	3'-7"		
# 7	4'-2"		
#8	4'-9"		
# 9	5′-4″		
# 10	6'-0"		
#11	6'-8"		

NOTES

TYPICAL BENT CAP REPAIRS ARE SHOWN. REPAIR DETAILS SIMILAR FOR END BENT CAPS AND STRUTS.

THE METHOD USED TO DELINEATE THE AREAS OF UNSOUND CONCRETE TO BE REPAIRED SHALL NOT PERMANENTLY MARK THE CONCRETE, LEAVE ANY RESIDUE AFTER REMOVAL OR REQUIRE HARSH CHEMICALS TO REMOVE.

THE CONTRACTOR SHALL REMOVE THE DETERIORATED CONCRETE IN ACCORDANCE WITH THE GUIDELINES SET IN THESE NOTES, IN THE SPECIAL PROVISIONS AND THE STANDARD SPECIFICATIONS.

REMOVE UNSOUND CONCRETE TO THE EXTENT NECESSARY, MINIMUM OF 1" BEHIND REBAR AND MINIMUM OF 2" CLEARANCE TO SAWCUT.

NO MORE THAN ONE-THIRD OF THE CAP OR COLUMN CIRCUMFERENCE SHALL BE REMOVED AT ONE TIME.SHOULD IT BECOME NECESSARY TO REMOVE MORE THAN 30% OF A CAP OR COLUMN CROSS SECTIONAL AREA, NOTIFY THE ENGINEER PRIOR TO PROCEEDING.

SIMULTANEOUS REMOVAL OF UNSOUND CONCRETE MAY BE PERMITTED ON MORE THAN ONE FACE OF A CAP AND/OR COLUMN. BUT NO MORE THAN 3 OF THE CIRCUMFERENCE SHALL BE REMOVED AT ONE TIME. IF REMOVAL EXTENDS MORE THAN 11/2" BEHIND THE MAIN REINFORCING BARS, NOTIFY THE ENGINEER PRIOR TO PROCEEDING. ON COLUMNS AND PILES, NO MORE THAN 10 VERTICAL FEET MAY BE EXPOSED AT ONE TIME BEFORE PLACEMENT OF REPAIR CONCRETE.

REINFORCING STEEL WHICH IS DETERMINED BY THE ENGINEER TO BE REPLACED, SHALL BE REMOVED TO A POINT WHERE IT IS SOUND. THE PATCH SHALL EXTEND A SUFFICIENT DISTANCE BEYOND THIS POINT TO DEVELOP A SPLICE LENGTH SPECIFIED IN THE TABLE ON THIS SHEET.

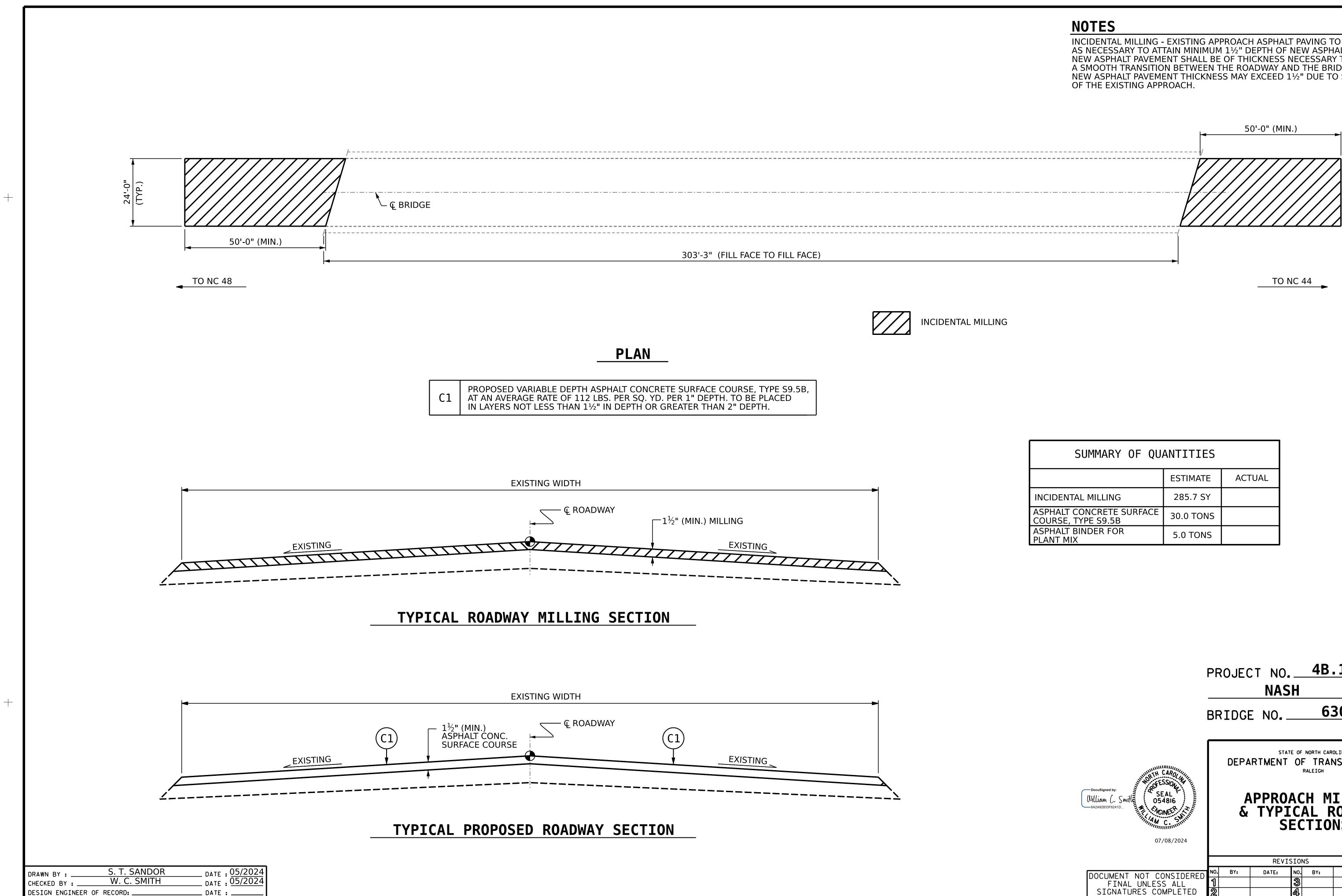
THE #4 ``U'' DOWELS ARE REQUIRED ONLY AROUND THE ANCHOR BOLTS. THE EXISTING REINFORCING STEEL IN THE PEDESTAL WALL SHALL BE CLEANED, STRAIGHTENED AND REMAIN IN PLACE.

FOR ADHESIVELY ANCHORED ANCHOR BOLTS OR DOWELS, SEE STANDARD SPECIFICATIONS.

COAT ALL REPAIR SURFACE AREAS ON THE TOP OF CAPS, INCLUDING CHAMFERS, WITH EPOXY PROTECTIVE COATING, OVERLAPPING THE REPAIR AREA BY A MINIMUM OF 3" ON ALL POSSIBLE SIDES.

CLEAN ALL EXPOSED REINFORCING BARS AND PRESTRESSED STRANDS IN ACCORDANCE WITH APPROPRIATE SPECIAL PROVISIONS.FOR BARS WITH MORE THAN 10% SECTION LOSS. SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED. NOTE AND PROVIDE DETAILED DOCUMENTATION, INCLUDING LOCATION AND SEVERITY, OF ALL DAMAGE TO PRESTRESSED STRANDS THAT EXCEEDS 10% SECTION LOSS. IF FIVE OR MORE STRANDS ARE DAMAGED, NOTIFY THE ENGINEER PRIOR TO PLACEMENT OF REPAIR MATERIAL.

	PROJECT BRIDGE	NASH		<u>.1064</u> co l 30225	JNTY
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INCIDENTAL MILLING - EXISTING APPROACH ASPHALT PAVING TO BE MILLED AS NECESSARY TO ATTAIN MINIMUM 1½" DEPTH OF NEW ASPHALT PAVING. NEW ASPHALT PAVEMENT SHALL BE OF THICKNESS NECESSARY TO PROVIDE A SMOOTH TRANSITION BETWEEN THE ROADWAY AND THE BRIDGE DECK. THE NEW ASPHALT PAVEMENT THICKNESS MAY EXCEED 1½" DUE TO SETTLEMENT

SUMMARY OF QUANTITIES			
	ESTIMATE	ACTUAL	
CIDENTAL MILLING	285.7 SY		
PHALT CONCRETE SURFACE JRSE, TYPE S9.5B	30.0 TONS		
PHALT BINDER FOR NT MIX	5.0 TONS		

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		NASH		CO	UNTY
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DESIGN DATA:

SPECIFICATIONS		AASHTO (CURRENT)
LIVE LOAD		SEE PLANS
IMPACT ALLOWANCE		SEE AASHTO
STRESS IN EXTREME STRUCTURAL STEEL	FIBER OF - AASHTO M270 GRADE 36	20,000 LBS. PER SQ. IN.
	- AASHTO M270 GRADE 50W	27,000 LBS. PER SQ. IN.
	- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL	IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPR	RESSION	1,200 LBS. PER SQ. IN.
CONCRETE IN SHEAR		SEE AASHTO
STRUCTURAL TIMBER	- TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS. PER SQ. IN.
COMPRESSION PERPE	ENDICULAR TO GRAIN OF TIMBER	375 LBS. PER SQ. IN.
EQUIVALENT FLUID P	RESSURE OF EARTH	30 LBS. PER CU. FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES. ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $1\frac{1}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION IOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS, AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

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DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " \oslash Shear studs for the $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ " \oslash STUDS FOR 4 - $\frac{3}{4}$ " \oslash STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " \oslash STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " \varnothing studs based on the ratio of 3 - $\frac{7}{8}$ " \varnothing STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADIOINING PIECES.

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EOUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.