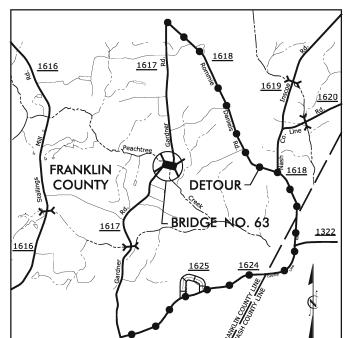
-5105O **PROIEC**

45351.

See Sheet 1-A For Index of Sheets

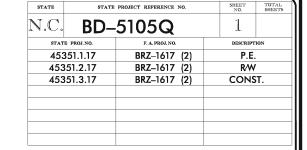


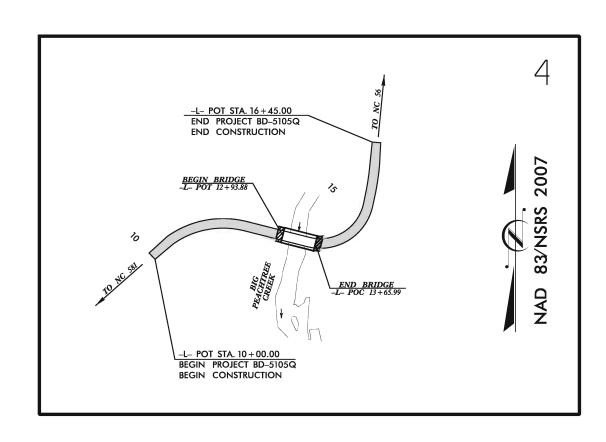
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

FRANKLIN COUNTY

LOCATION: BRIDGE NO. 63 ON SR 1617 (GARDNER RD.) OVER BIG PEACHTREE CREEK

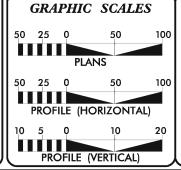
TYPE OF WORK: GRADING, DRAINAGE, PAVING & STRUCTURE





CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III

DESIGN EXCEPTION REQUIRED FOR HORIZONTAL & VERTICAL CURVES



DESIGN DATA

ADT 2011 = 240ADT 2031 = 480DHV = 10 %D = 50 %T = 6 % *

V = 55 MPH* TTST = NA DUAL NAFUNC CLASS = LOCAL

SUB-REGIONAL TIER

PROJECT LENGTH

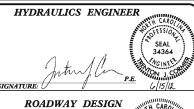
LENGTH ROADWAY STATE PROJECT BD-5105Q = 0.108 MILES LENGTH STRUCTURES STATE PROJECT BD-5105Q = 0.014 MILES TOTAL LENGTH STATE PROJECT BD-5105Q = 0.122 MILES

DEPARTMENT OF TRANSPORTATION IN THE OFFICE OF: 2012 STANDARD SPECIFICATIONS RIGHT OF WAY DATE: DENNIS MORY, PE NOVEMBER 2011 LETTING DATE:

REPARED FOR THE NORTH CAROLINA

JULY 2012

HENRY BARE





DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA



STATE HIGHWAY DESIGN ENGINEER

GENERAL NOTES

GENERAL NOTES:

2012 SPECIFICATIONS EFFECTIVE: 01-17-12

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

END BENTS:

THE SURVEYOR SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTIONS PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY CONTRACT.

PROJECT REFERENCE NO. SHEET NO. BD-5/05Q I-A



Florence & Hutcheson

CONSULTING ENGINEERS
5121 Kingdom Wey, Suite 100 Raleigh, NC 27607
NC Liconse No: F-0
ROADWAY DESIGN

ENGINEER
CAROLINIA
CAROLIN

ROADWAY STANDARD DRAWINGS

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch - N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

SID.NO.	ITILE
DIVISION	2 - EARTHWORK
200.03	Method of Clearing - Method III
225.02	Guide for Grading Subgrade - Secondary and Local
225.04	Method of Obtaining Superelevation - Two Lane Pavement
DIVISION	3 - PIPE CULVERTS
300.01	Method of Pipe Installation
DIVISION	4 - MAJOR STRUCTURES
422.11	Bridge Approach Fills - Sub Regional Tier
DIVISION	5 - SUBGRADE, BASES AND SHOULDERS
560.01	Method of Shoulder Construction - High Side of Superelevated Curve - Method I
DIVISION	8 - INCIDENTALS
806.01	Concrete Right-of-Way Marker
840.20	Frames and Wide Slot Flat Grates
840.46	Traffic Bearing Precast Drainage Structure
846.01	Concrete Curb, Gutter and Curb & Gutter
846.04	Drop Inlet Installation in Shoulder Berm Gutter
862.01	Guardrail Placement
862.02	Guardrail Installation
862.03	Structure Anchor Units
876.02	Guide for Rip Rap at Pipe Outlets
876.04	Drainage Ditches with Class 'B' Rip Rap

INDEX OF SHEETS

INDEX OF SHE	
SHEET NUMBER	SHEET
1	TITLE SHEET
1-A	INDEX OF SHEETS GENERAL NOTES LIST OF STANDARD DRAWINGS
1-B	CONVENTIONAL SYMBOLS
3	DRAINAGE SUMMARY GUARDRAIL SUMMARY SUMMARY OF EARTHWORK SUMMARY OF PAVEMENT REMOVAL SUMMARY OF SHOULDER BERM GUTTER
4	PLAN & PROFILE TYPICAL SECTIONS & PAVEMENT SCHEDULE PARCEL INDEX DRAINAGE DITCH DETAILS
TCP-1 THRU TCP-2	TRAFFIC CONTROL PLANS
EC-1 THRU EC-5	EROSION CONTROL PLANS
RF-1	REFORESTATION DETAIL SHEET
X-1 THRU X-3	CROSS-SECTIONS
S-1 THRU S-17	STRUCTURE PLANS
SN	STANDARD NOTES

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

CONVENTIONAL PLAN SHEET SYMBOLS

BOUNDARIES AND PROPERTY	·•	RAILROADS:				Water Manhole	(W)
	•	Standard Gauge	CSX TRANSPORTATION			Water Meter	0
State Line		RR Signal Milepost	0			Water Valve	\otimes
County Line		Switch —	MILEPOST 35	EXISTING STRUCTURES:		Water Hydrant	•\$
Township Line		RR Abandoned	SWITCH	MAJOR:		Recorded U/G Water Line	
City Line				Bridge, Tunnel or Box Culvert	CONC	Designated U/G Water Line (S.U.E.*)	
Reservation Line		RR Dismantled		Bridge Wing Wall, Head Wall and End Wall) conc ww [Above Ground Water Line	A/G Water
Property Line		RIGHT OF WAY:	•	MINOR:			
Existing Iron Pin	EIP	Baseline Control Point	•	Head and End Wall	COVIC HIM	TV:	
Property Corner	×	Existing Right of Way Marker	\triangle	Pipe Culvert	CONC HIS	TV Satellite Dish	K
Property Monument	ECM	Existing Right of Way Line		·		TV Pedestal	2
Parcel/Sequence Number	 @	Proposed Right of Way Line		Footbridge 2	Псв		
Existing Fence Line	××-	Proposed Right of Way Line with		Drainage Box: Catch Basin, DI or JB	СВ	TV Tower	\otimes
Proposed Woven Wire Fence		Iron Pin and Cap Marker	w –	Paved Ditch Gutter		U/G TV Cable Hand Hole	Head
Proposed Chain Link Fence		Proposed Right of Way Line with Concrete or Granite Marker		Storm Sewer Manhole —————	(S)	Recorded U/G TV Cable	
Proposed Barbed Wire Fence	→	Existing Control of Access	(Ş)	Storm Sewer		Designated U/G TV Cable (S.U.E.*)	Iv
Existing Wetland Boundary		Proposed Control of Access	(8)			Recorded U/G Fiber Optic Cable	
Proposed Wetland Boundary	-9/ 3	Existing Easement Line		UTILITIES:		Designated U/G Fiber Optic Cable (S.U.E.*)—	
	5.0	-		POWER:			
Existing Endangered Animal Boundary	2,0	Proposed Temporary Construction Easement -	E	Existing Power Pole	•	GAS:	
Existing Endangered Plant Boundary		Proposed Temporary Drainage Easement	TDE	Proposed Power Pole	6	Gas Valve	\Diamond
BUILDINGS AND OTHER CUL	TURE:	Proposed Permanent Drainage Easement	PDE	Existing Joint Use Pole		Gas Meter	\Diamond
Gas Pump Vent or U/G Tank Cap	_ 0	Proposed Permanent Drainage / Utility Easemen	tDUE	Proposed Joint Use Pole	- ċ -	Recorded U/G Gas Line	G
Sign -	<u></u>	Proposed Permanent Utility Easement	PUE	Power Manhole	P	Designated U/G Gas Line (S.U.E.*)	
Well -		Proposed Temporary Utility Easement	TUE	Power Line Tower		Above Ground Gas Line	A/G Gos
Small Mine	→	Proposed Permanent Easement with Iron Pin and Cap Marker		Power Transformer	M		
Foundation		ROADS AND RELATED FEATURE	2 5 .	U/G Power Cable Hand Hole	Hid	SANITARY SEWER:	
Area Outline	_	Existing Edge of Pavement		H–Frame Pole	•—•	Sanitary Sewer Manhole	(A)
Cemetery	— [†]	Existing Curb		Recorded U/G Power Line		Sanitary Sewer Cleanout	(
Building —		-	С			U/G Sanitary Sewer Line —	
School	_ —	Proposed Slope Stakes Cut	F	Designated U/G Power Line (S.U.E.*)		Above Ground Sanitary Sewer	
Church		Proposed Slope Stakes Fill				•	A/G Sanitary Sew
		Proposed Wheel Chair Ramp	(WCR)	TELEPHONE:		Recorded SS Forced Main Line	F\$S
Dam -		Existing Metal Guardrail		Existing Telephone Pole		Designated SS Forced Main Line (S.U.E.*) —	— — — FSS — —
HYDROLOGY:		Proposed Guardrail		Proposed Telephone Pole	-0-		
Stream or Body of Water -		Existing Cable Guiderail		Telephone Manhole	①	MISCELLANEOUS:	
Hydro, Pool or Reservoir	- [Proposed Cable Guiderail	_ 0 0 0 0	Telephone Booth	3	Utility Pole	•
Jurisdictional Stream		Equality Symbol	②	Telephone Pedestal	T	Utility Pole with Base ———————	
Buffer Zone 1	BZ 1 —	Pavement Removal		Telephone Cell Tower	, ā ,	Utility Located Object ——————	\odot
Buffer Zone 2	BZ 2	VEGETATION:		U/G Telephone Cable Hand Hole	HE	Utility Traffic Signal Box	5
Flow Arrow	~	Single Tree	ඪ	Recorded U/G Telephone Cable		Utility Unknown U/G Line	
Disappearing Stream —	>	Single Shrub	¢	Designated U/G Telephone Cable (S.U.E.*)	=	U/G Tank; Water, Gas, Oil	
Spring —		Hedge =		Recorded U/G Telephone Conduit		A/G Tank; Water, Gas, Oil	
Wetland —	*	Woods Line		Designated U/G Telephone Conduit (S.U.E.*)		U/G Test Hole (S.U.E.*)	•
Proposed Lateral, Tail, Head Ditch	- 	Orchard =	6 6 6 6 6 6 6 6	Recorded U/G Fiber Optics Cable		Abandoned According to Utility Records —	AATUR
False Sump	- For -	Vineyard	ω ω ω ω	Designated U/G Fiber Optics Cable (S.U.E.*)		End of Information	E.O.I.

SUMMARY OF EARTHWORK

STATION	STATION	UNCL. EXCAY.	EMBANK. +%	BORROW	WASTE
-L- 10+00.00	-L− 12 + 94	176	176	0	
	SUBTOTAL	176	176	0	
-L- 13+66	-L- 16+45	90	67		23
	SUBTOTAL	90	67		23
SUMMAR	Y TOTALS	266	243		23
WASTE TO BE USED	IN LIEU OF BORROW				
SHOULDER CO	ONSTRUCTION		30	30	
PROJECT	TOTALS	266		30	
5% TO REPLACE TOP	SOIL IN BORROW PIT			30	
WASTE IN LIEU	J OF BORROW			23	
GRAND	TOTALS	266		7	
S	AY	266		7	

PAVEMENT REMOVAL SUMMARY

SURVEY LINE	STATION	STATION	LOCATION LT/RT/CL	YDŽ
4-	10+00	13+05 (EX. BRIDGE)	CL	610
4-	13 + 45 (EX. BRIDGE)	16+45	CL	600
			TOTAL:	1210
			SAY:	1210

SHOULDER BERM GUTTER SUMMARY

SURVEY LINE	STATION	STATION	LENGTH
-L-	13+66.00 LT	14+25.00 LT	59.00
		TOTAL;	59.00
		SAY:	60.00

LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

STATION	ON (LT,RT, OR CL) STRUCTURE NO.	VATION	ELEVATION	ELEVATION		DRAINAGE RCP, CSP, CAAP, H NOTE: DO NOT HDPE or			(U)	C.: NLESS NOT	i. PIPE ED OTH	HERWISE)			(UN	CIASS ILESS O	FIV R.C. PII THERWISE I	PE NOTED)				STD. 838. STD. 838. OR STD. 838 (UNIES NOTE:	OI, OI	DRAINAGE UCTURES L.F. FOR P	7 + 'A' + (1)	TD. 840.02	FRAME, GRATES AND HOOD STANDARD 840.03	STD. 840.15	840.17 OR 840.26	840.18 OR 840.27	GRATE STD. 840.22	TWO GRATES STD. 840.22 VITH GRATE STD. 840.24	VITH TWO GRATES STD. 840.24	840.32	TWO GRATES STD. 840.20		i NO. & SIZE ."8" C.Y. STD 840.72	PLUG, C.Y. STD. 840.71	C.I N. D.I G. G.	B. CATC D.I. NARRI I. DROP D.I. GRATI D.I. (N.S.) GRATI (NARR	D DROP INLET D DROP INLET DW SLOT)	
SIZE	PEQO!		INVERT	INVERT	12"	15" 18" 24" 30	0" 36" 42"	18" 12" 1:	18"	24"	30"	36"	42"	48"	12" 15"	18" 2	30" 3	6" 42"	48"		분	CU. YD	s. 3		ة اليا	ĕ		114 OR	. SI	, d	HE N	E WITH	FRAME V	31 OR	E WITH		ELBOWS	ICK PIP	A 1.0	H. MANH	ION BOX OLE C BEARING DROP INLET	,
THICKNESS OR GAUGE	FROM	<u>و</u>						990.	2 490 440	999	λ()	620.	60I.	901.						15" SIDE DRAIN 18" SIDE DRAIN	24" SIDE DRAIN	R.C.P.	C.S.P.	PER EACH (0' TH	10.0' AND ABOV	C.8. STD. 840.01	TYPE OF GRATE	D.I. STD. 840	G.D.I. TYPE	G.D.I. TYPE	G.D.I. FRAME	G.D.I. FRAME	G.D.I. (N.S.) F	1.8. STD. 840	G.D.I. FRAM		CORR. STEEL	CONC. & BR	ิ สา 🗀	3.J.B. TRAFF	C BEARING JUNCTION AARKS	
13+90.00	LT 0401	220.7	,		+							$\overline{}$							+				+	1	+	+	- 1 0				+++			1	1	+	 -	+	\dashv			\dashv
14+20.00	LT 0402	220,7	7																				1	1										1	1							
13 + 90.00	LT 0401 0	402	21B.9	218.8											24																											
14+20.00	LT 0402 0	103	21B.8	218.7		12																															4	\blacksquare				
																																					\pm	\Box				\exists
			PROJECT	TOTAL	-	12		+	+						24								\perp	2		+								2	2		+	+	-			\dashv
			†	SAY		12									24									2											2	+	+	+				\dashv

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

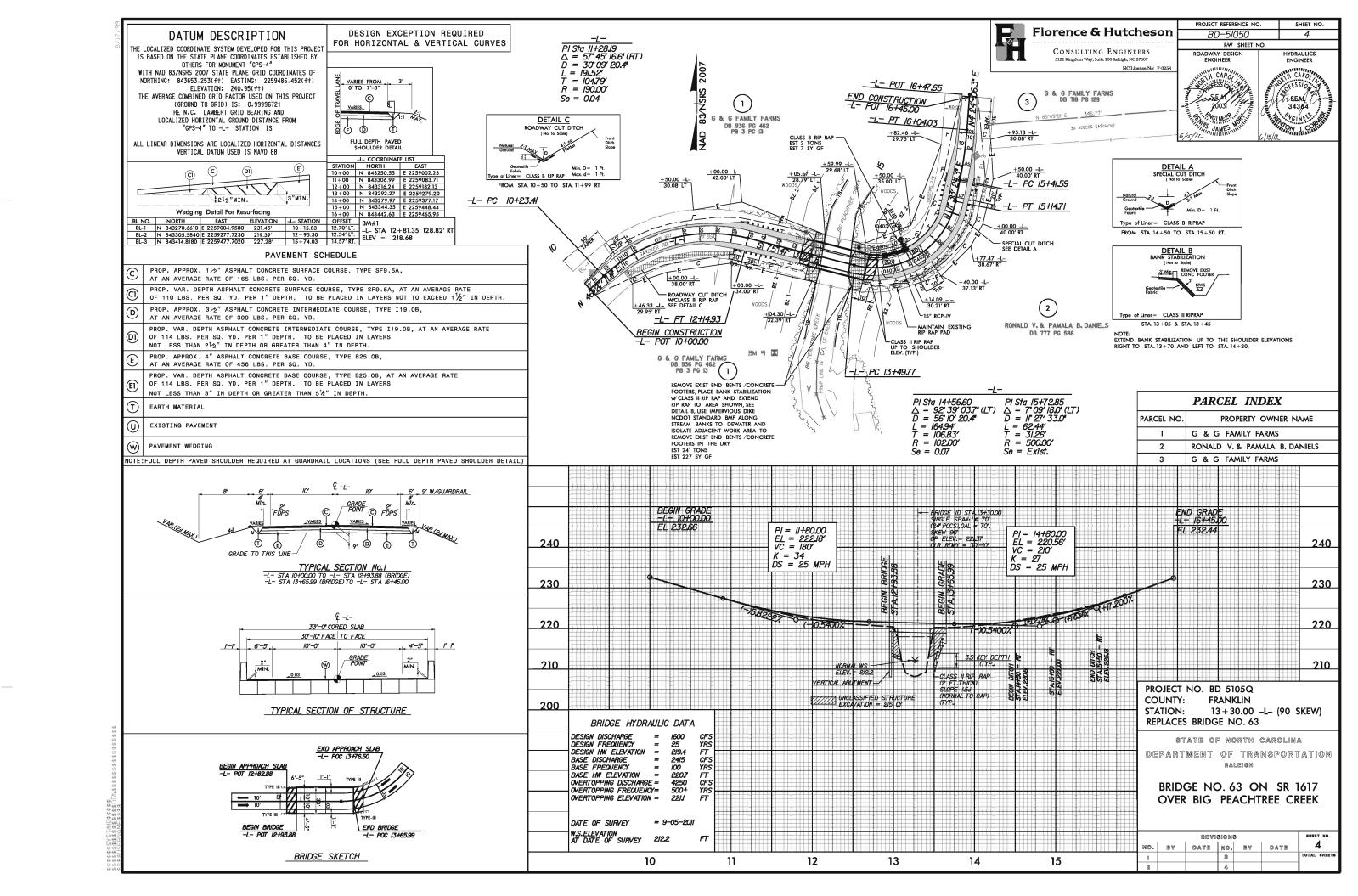
FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLE GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

G = GATING, JURGACT ATTENLISTING TYPE 350.

CHARDRAII SHMMARV

↔ ↔	G = GATIN NG = NO	NG IMPACT ATTENUA N-GATING IMPACT A	ATOR TYPE 350 ATTENUATOR TYPE 35	0								GUA	IKDK	AIL 3	CIVIIVL	AKI																						
\$ \$ \$ \$ \$ \$ \$ \$	SURVEY	DEG ET		LOCATION		LENGTH		WARRA	NT POINT	"N" DIST.	TOTAL	FLARE	LENGTH	,	*		ANCHORS			es						s						A1		CT ATOR	SINGLE FACED	REMOVE	REMOVE	E
***	LINE	BEG. STA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	SHOUL, WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	TYPE III	XI	GRAU 350	350 XII	CAT	-1 VI	віс	AT-1	EA G		FACED GUARDRAIL	GUARDRAIL	AND STOCKPILE EXISTING GUARDRAIL	REMARKS									
Ž,	4	12+06.38	12+93.88	RT	87.5			12 + 94		4.42	7.42	50		1		1		1																				
\$ # # \$ # #	-L-	12 +06.3B	12 + 93.8B	LT	87.5				12+94	6.42	9.42		50		1	1		1																				
00 00 00 00 00 00	-L-	13+78.50	15 + 41.00	RT	162.5				13 + 78.50	4.42	7.42		50		1	1		1																				
₩ ₩ ₩ ₩	-L-	13+66.00	15+41.00	LT	175.0			13+66		6.42	9.42	50		1		1		1																				
¥\$€ BBNG				SUBTOTAL	512.5											4		4		AN	CHOR DEDU	CTIONS																
000 €				LESS ANCHORS	(-) 275														1	YPE III =	4 @ 18.75	= 75.00																
00 00 00 00 00 00				TOTAL	237.5														GRAU	35D -	4 @ 50.00	- 200.00																
60 60 60				SAY	250			ADDITIONAL GUARDR	AIL POSTS = 5 EA												TOTAL	. = 275.00																



CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

LANE AND SHOULDER CLOSURE REQUIREMENTS

A) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.

TRAFFIC PATTERN ALTERATIONS

B) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

- C) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.

 PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN IN THE TRAFFIC CONTROL PLANS.
- D) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

 COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE

DETOUR WHEN THE DETOUR IS NOT IN OPERATION.

E) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

F) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKING AND MARKERS

G) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME MARKING MARKER

SR 1617 (GARDNER RD) PAINT PERMANENT RAISED

- H) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE ACCORDING TO THE ROADWAY STANDARDS.
- I) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.

MISCELLANEOUS

J) MAINTAIN ACCESS TO ALL RESIDENCES AND BUSINESSES BETWEEN THE CLOSURE POINTS AT ALL TIMES DURING CONSTRUCTION.



Florence & Hutcheson

5121 Kingdom Way, Suite 100 Raleigh, NC 27607

Consulting Engineers BD

BD-5105Q TCP-1

ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2012 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	<u>TITLE</u>
1101.03	TEMPORARY ROAD CLOSURES
1101.04	TEMPORARY SHOULDER CLOSURES
1110.02	PORTABLE WORK ZONE SIGNS
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO LANE AND MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING

PHASING

GUARDRAIL END DELINEATION

STEP 1

1262.01

USING ROADWAY STANDARD DRAWING NUMBER 1101.04, SHEET 1 OF 1, INSTALL ALL DETOUR SIGNING KEEPING SIGNS COVERED.

STEP 2

PRIOR TO CLOSING SR 1617 (GARDNER RD.), UNCOVER ALL DETOUR SIGNING AND OPEN DETOUR TO TRAFFIC.

USING ROADWAY STANDARD DRAWING NUMBER 1101.03, SHEET 1 OF 9, CLOSE SR 1617 (GARDNER RD.).

STEP 3

DISMANTLE AND REMOVE EXISTING BRIDGE.

STEP 4

COMPLETE CONSTRUCTION OF PROPOSED STRUCTURE, APPROACH ROADWAY TIE-INS, AND ASSOCIATED ITEMS.

STEP 5

PLACE FINAL PAVEMENT MARKINGS ON SR 1617 (GARDNER RD.) AND OPEN TO TRAFFIC.

STEP 6

USING ROADWAY STANDARD DRAWING NUMBER 1101.04, SHEET 1 OF 1, REMOVE ALL DETOUR SIGNING AND ALL TRAFFIC CONTROL DEVICES.

FINAL PAVEMENT MARKING SCHEDULE

DESCRIPTION

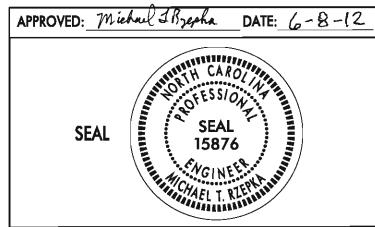
WHITE EDGELINE (2X)

DOUBLE YELLOW CENTER LINE (2X)

PAINT (4")

PAINT (4")

YELLOW/YELLOW



PERMANENT RAISED MARKER

GENERAL NOTES, PHASING, ROADWAY STANDARD DRAWINGS FINAL PAVEMENT MARKING SCHEDULE

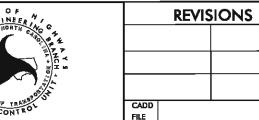
SCALE: NONE

DATE: JUNE '12

DWG. BY: GEP

DESIGN BY: MTR

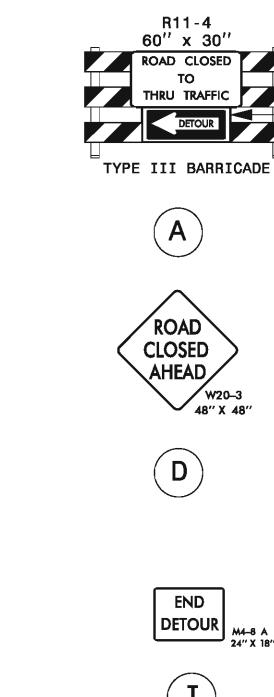
REVIEWED BY: MTR



Florence & Hutcheson

CONSULTING ENGINEERS
5121 Kingdom Way, Sulte 100 Raleigh, NC 27607

PROJ. REFERENCE NO. SHEET NO. BD-5105Q TCP-2



 (H)

#63

1322

RICH/RD

-BRIDGE

H

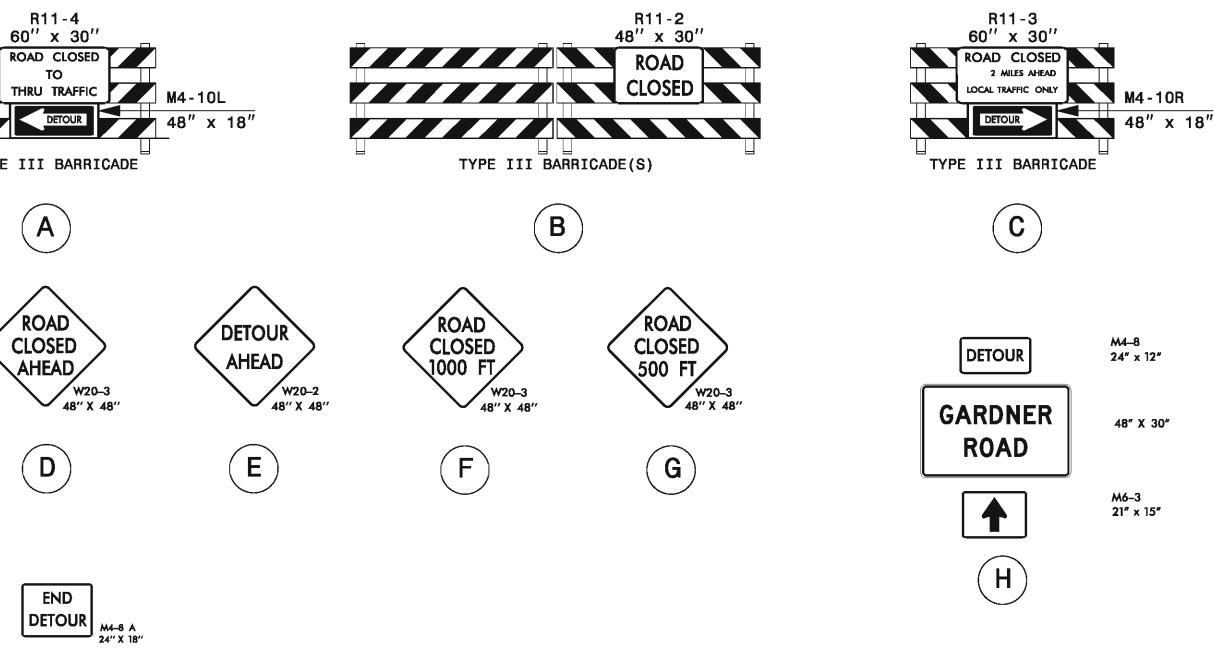
 \mathbf{G}

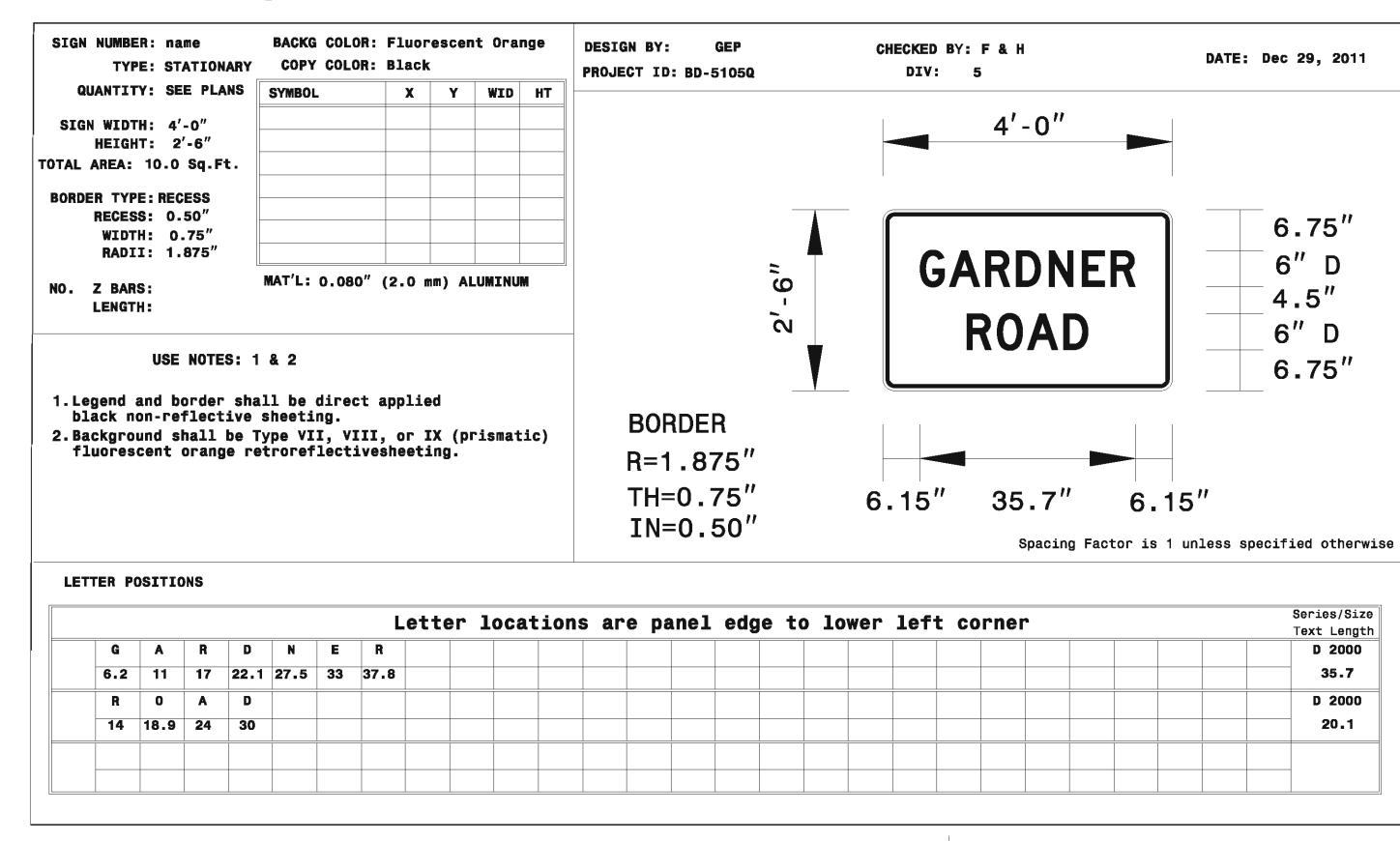
1617

MEMORIAL

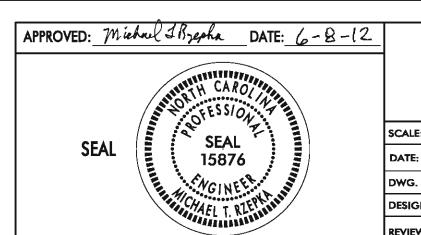
(B)

(G)



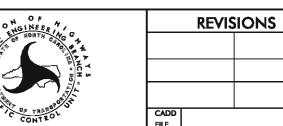






DETOUR	FOF
GARDNER	RE
CLOSU	RE

LE:	NONE	
E:	JUNE '12	
G. BY:	GEP	
IGN BY:	MTR	
EWED BY:	MTR	





NC GRID

NAD 83/NSRS 2007

PROJECT REFERENCE NO. BD-5105Q

R/W SHEET NO. ROADSIDE ENVIRONMENTAL PROJECT ENGINEER

SHEET NO.

EC-I/CONS7

LEVEL III CERTIFIED BY: ALEXANDER SNIDER, E.I. **CERTIFICATION NUMBER: 3064** ISSUED: JUNE 27, 2012

CLEARING AND GRUBBING EROSION CONTROL FOR CONSTRUCTION SHEET 4

Description Temporary Silt Fence... Special Sediment Control Fence Temporary Rock Silt Check Type-A.

1633.02 Wattle / Coir Fiber Wattle with Polyacrylamide (PAM).

-L- POT 16+47.65 -L- PT 16+04.03 -L- PC 15+41.59 -L- PC 10+23.41 - PT 15+14.71 -L- PT 12+14.93 BEGIN CONSTRUCTION -L= POT 10+00.00 -L- PC 13+49.77

> ALEXANDER SNIDER, E.I. ROADSIDE ENVIRONMENTAL ENGINEER 3064
> LEVEL III CERTIFICATION NUMBER

TRENTON J. CORMIER, P.E. ROADSIDE ENVIRONMENTAL PROJECT ENGINEER

118
LEVEL III CERTIFICATION NUMBER

ENVIRONMENTALLY SENSITIVE AREA SEE PROJECT SPECIAL PROVISIONS

ALL EROSION CONTROL DEVICES SHOWN ARE LOCATED WITHIN EXISTING RW OR EASEMENT.



Florence & Hutcheson

CONSULTING ENGINEERS 5121 Kingdom Way, Suite 100 Raleigh, NC 27607 NC License No: F-0258

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

Prepared in the Office of:

FLORENCE & HUTCHESON

5121 KINGDOM WAY, SUITE 100 RALEIGH NC 27607 NC License No: F-0258

2012 STANDARD SPECIFICATIONS

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2012 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans.

1604.01 Railroad Erosion Control Detail 1632.01 Rock Inlet Sediment Trap Type A 1605.01 Temporary Silt Fence 1632.02 Rock Inlet Sediment Trap Type B 1606.01 Special Sediment Control Fence 1632.03 Rock Inlet Sediment Trap Type C 1607.01 Gravel Construction Entrance 1633.01 Temporary Rock Silt Check Type A 1622.01 Temporary Berms and Slope Drains 1633.02 Temporary Rock Silt Check Type B 1630.01 Riser Basin 1630.02 Silt Basin Type B

1630.03 Temporary Silt Ditch 1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin 1631.01 Matting Installation

1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type B
1640.01 Coir Fiber Baffle 1645.01 Temporary Stream Crossing

FINAL EROSION CONTROL FOR CONSTRUCTION SHEET 4

ISSUED: JUNE 27, 2012

Temporary Silt Fence - || || || Special Sediment Control Fence Rock Inlet Sediment Trap: Type C.

Temporary Rock Silt Check Type-A.

Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)

Wattle / Coir Fiber Wattle with Polyacrylamide (PAM) Type of Liner = CLASS B RIPRAP

DETAIL B BANK STABILIZATION (Not to Scale) Type of Liner = CLASS II RIPRAP

STA. 13 + 05 & STA. 13 + 45

FROM STA. 14+50 TO STA. 15+50 RT.

DETAIL A

SPECIAL CUT DITCH

EXTEND BANK STABILIZATION UP TO THE SHOULDER ELEVATIONS RIGHT TO STA. 13+70 AND LEFT TO STA. 14+20.

NC GRID

NAD 83/NSRS 2007

-L- POT 16+47.65 -L- POT 16+45.00 -L- PT 16+04.03 **DETAIL C** ROADWAY CUT DITCH -L- PC 15+41.59 FROM STA. 10+50 TO STA. 11+99 RT -L- PC 10+23.4/ -L- PT 15+14.71 SPECIAL CUT DITCH SEE DETAIL A ROADWAY CUT DITCH W/CLASS B RIP RAP SEE DETAIL C ¹15" RCP−IV -L- PT 12+14.93 MAINTAIN EXISTING
RIP RAP PAD -L- POT 10+00.00 CLASS II RIP RAP UP TO SHOULDER ELEV. (TYP.) BM #I X 3 N: 843171 E: 2259229 ELEV.=218.68/ # \ \ \ -L-\ PC 13+49.77 REMOVE EXIST END BENTS /CONCRETE -FOOTERS, PLACE BANK STABILIZATION w/ CLASS II RIP RAP AND EXTEND RIP RAP TO AREA SHOWN, SEE DETAIL B, USE IMPERVIOUS DIKE Place Matting for Erosion Control NCDOT STANDARD BMP ALONG STREAM BANKS TO DEWATER AND on 2:1 Slope ISOLATE ADJACENT WORK AREA TO REMOVE EXIST END BENTS /CONCRETE FOOTERS IN THE DRY EST 241 TONS EST 227 SY GF Contractor will install impervious dike to

dewater both streambanks to allow for removal of existing concrete footers in the dry Use "NCDOT Best Management Practices for Construction and Maintenance Activities" manual for isolation and dewatering operations

NOTE: IMPERVIOUS DIKE SHALL BE CONSIDERED INCIDENTAL TO THE REMOVAL OF THE EXISTING STRUCTURE.

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011 ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER QUALITY.

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1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type B 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type B 1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type B
1635.01 Rock Pipe Inlet Sediment Trap Type A
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1640.01 Coir Fiber Baffle

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 PROJECT REFERENCE NO.
 SHEET NO.

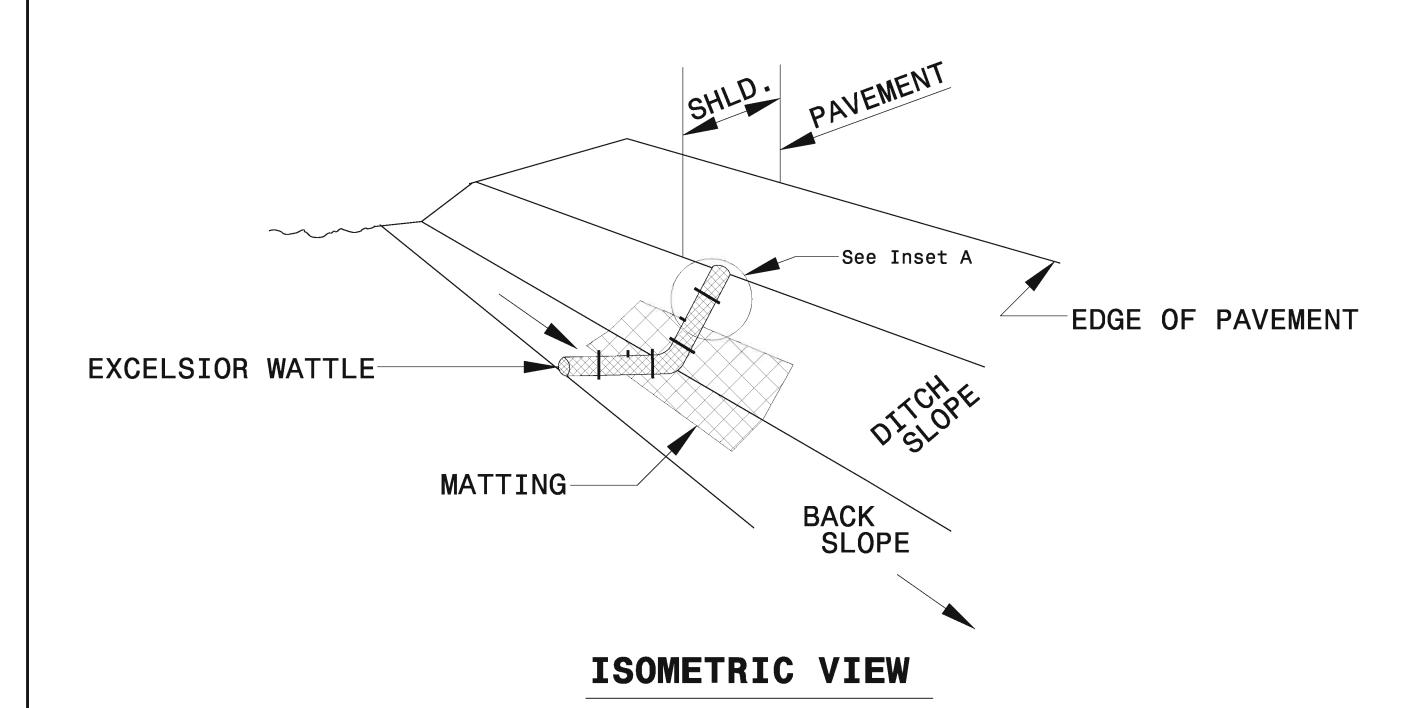
 BD-5/05Q
 FC-3

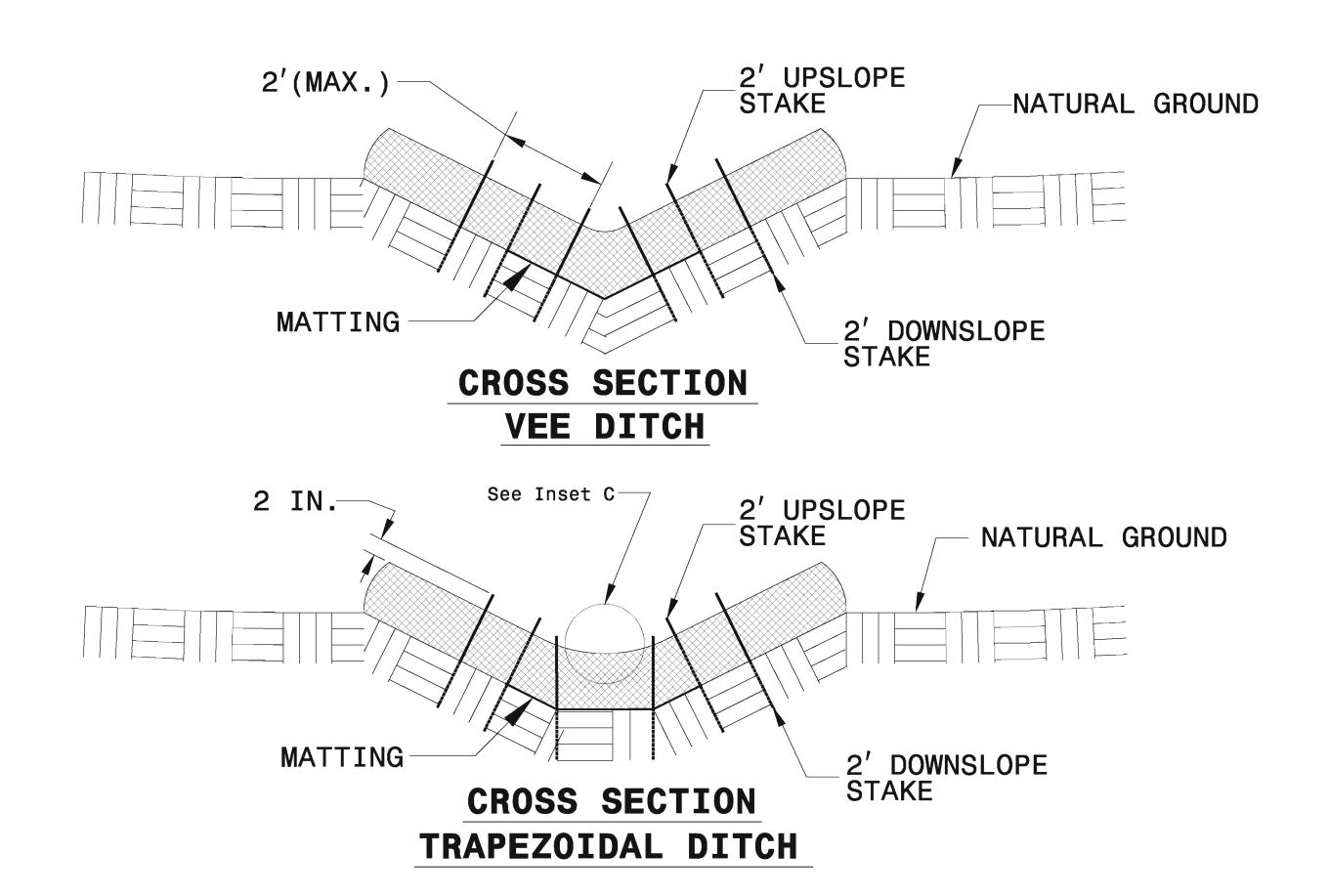
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	14 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	14 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

WATTLE WITH POLYACRYLAMIDE (PAM) DETAIL





NOTES:

USE MINIMUM 12 IN. DIAMETER EXCELSIOR WATTLE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

ONLY INSTALL WATTLE(S) TO A HEIGHT IN DITCH SO FLOW WILL NOT WASH AROUND WATTLE AND SCOUR DITCH SLOPES AND AS DIRECTED.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO BOTTOM OF DITCH.

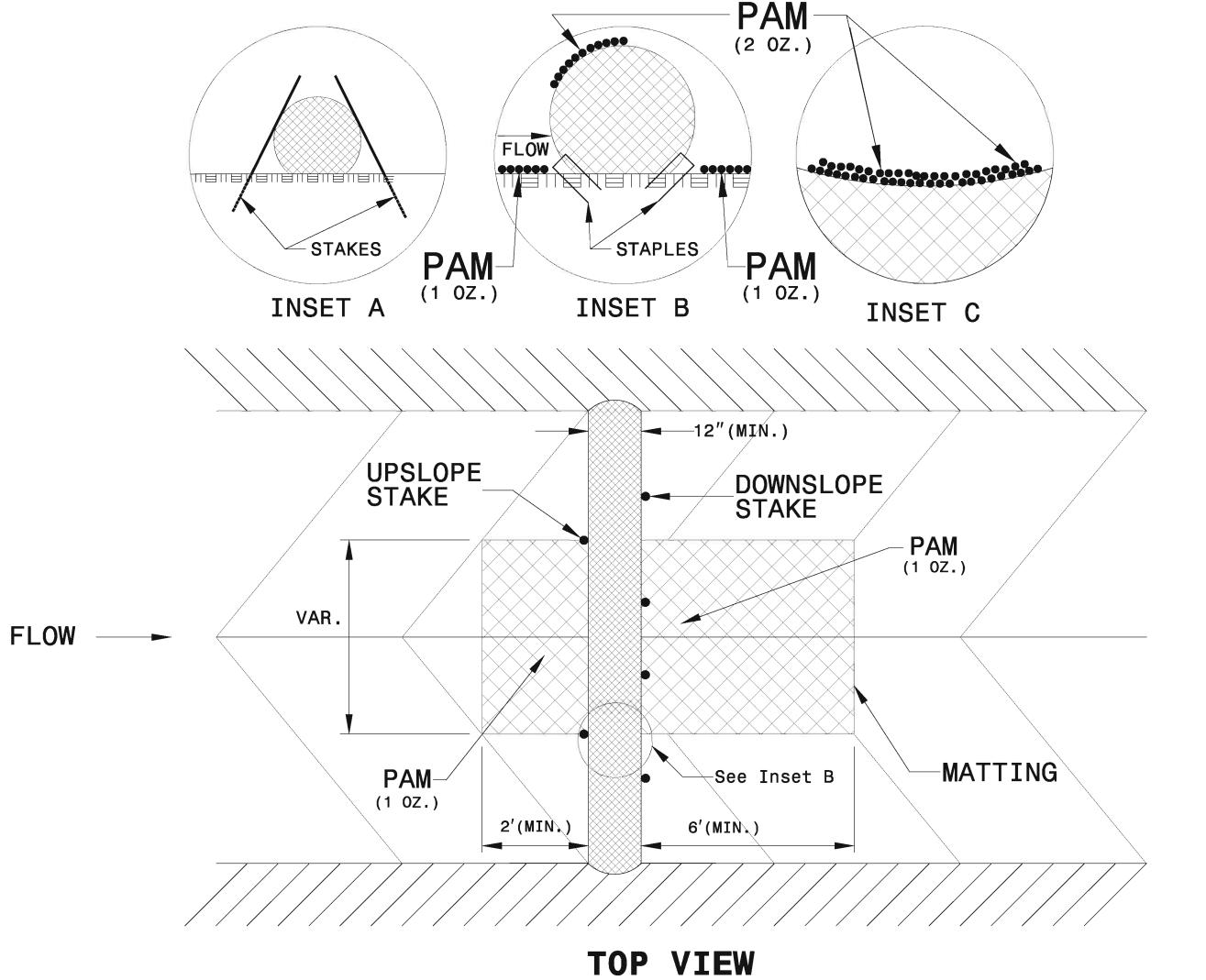
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

INSTALL MATTING IN ACCORDANCE WITH SECTION 1631 OF THE STANDARD SPECIFICATIONS.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH WATTLE.

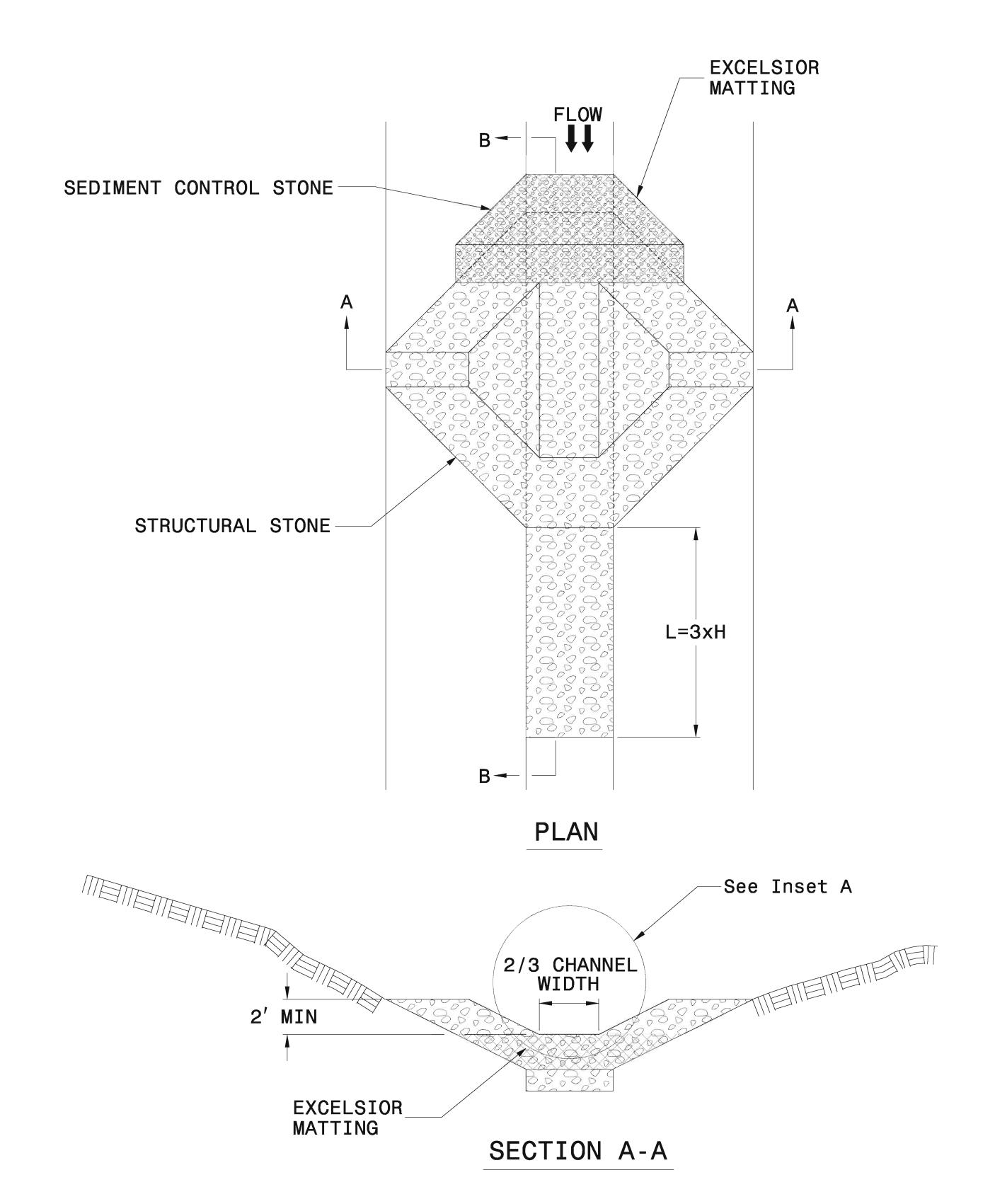
INITIALLY APPLY 2 OUNCES OF ANIONIC OR NEUTRALLY CHARGED PAM OVER WATTLE WHERE WATER WILL FLOW AND 1 OUNCE OF PAM ON MATTING ON EACH SIDE OF WATTLE. REAPPLY PAM AFTER EVERY RAINFALL EVENT THAT IS EQUAL TO OR EXCEEDS 0.50 IN.



 PROJECT REFERENCE NO.
 SHEET NO.

 BD-5/05Q
 EC-5

TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)

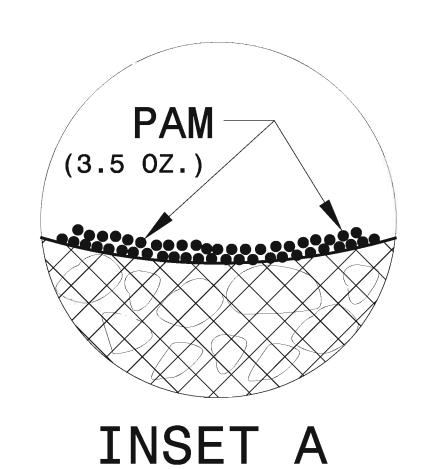


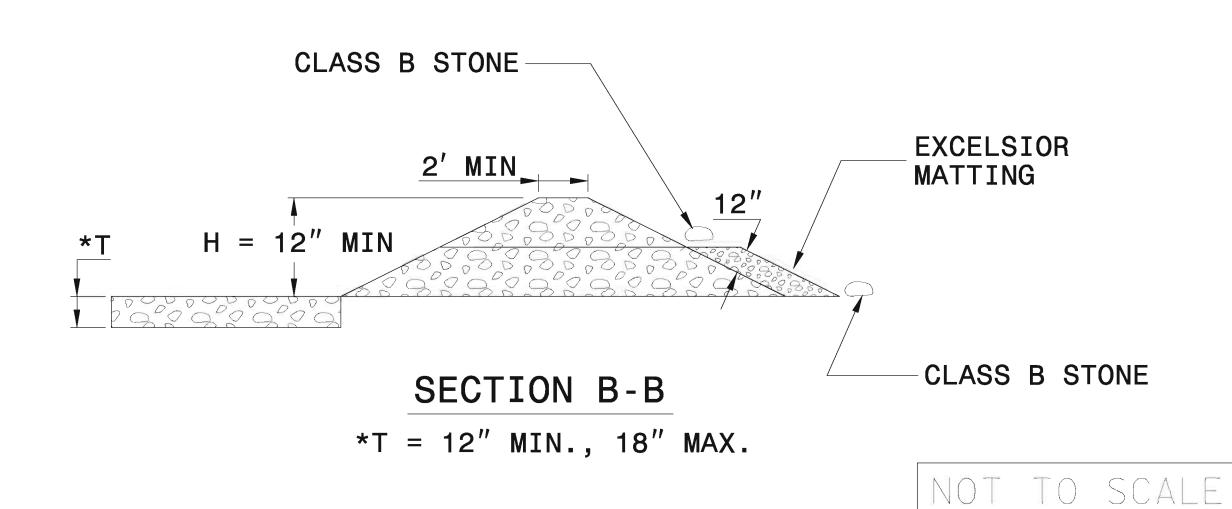
NOTES

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 3.5 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



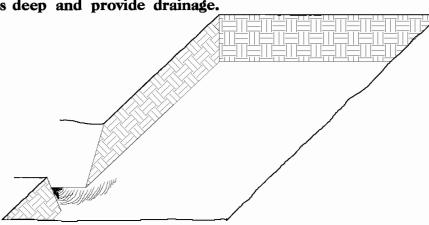


PLANTING DETAILS

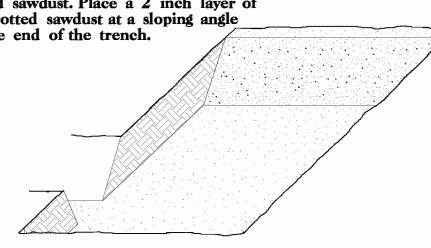
SEEDLING / LINER BAREROOT PLANTING DETAIL

HEALING IN

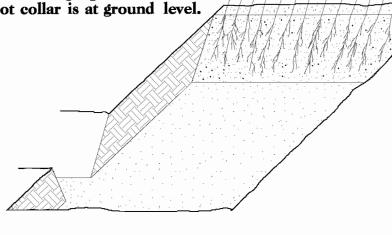
- 1. Locate a healing-in site in a shady, well protected area.
- 2. Excavate a flat bottom trench 12 inches deep and provide drainage.



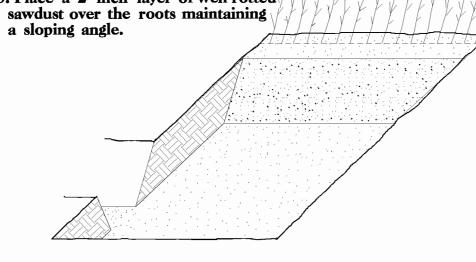
3. Backfill the trench with 2 inches well rotted sawdust. Place a 2 inch layer of well rotted sawdust at a sloping angle at one end of the trench.



4. Place a single layer of plants against the sloping end so that the root collar is at ground level.

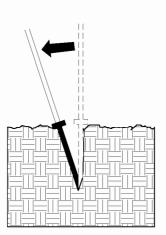


5. Place a 2 inch layer of well rotted sawdust over the roots maintaining

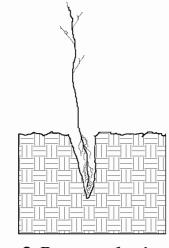


6. Repeat layers of plants and sawdust as necessary and water thoroughly.

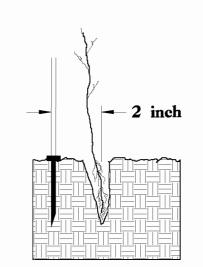
DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR



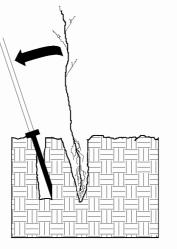
1. Insert planting bar as shown and pull handle



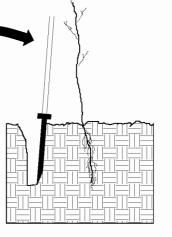
2. Remove planting bar and place seedling at correct depth.



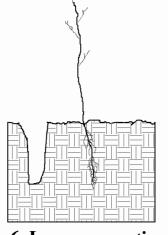
3. Insert planting bar 2 inches toward planter from seedling.



4. Pull handle of bar toward planter, firming



5. Push handle forward firming soil at top.



Leave compaction hole open. Water thoroughly.

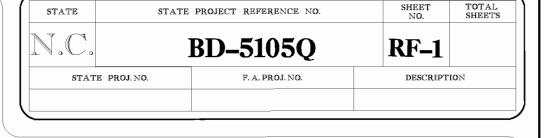
PLANTING NOTES:

PLANTING BAG
During planting, seedlings
shall be kept in a moist
canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR
Planting bar shall have a
blade with a triangular
cross section, and shall
be 12 inches long,
4 inches wide and
1 inch thick at center.

ROOT PRUNING
All seedlings shall be root pruned, if necessary, so that no roots extend more than 10 inches below the root collar.



REFORESTATION FOR BUFFERS

☐ TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

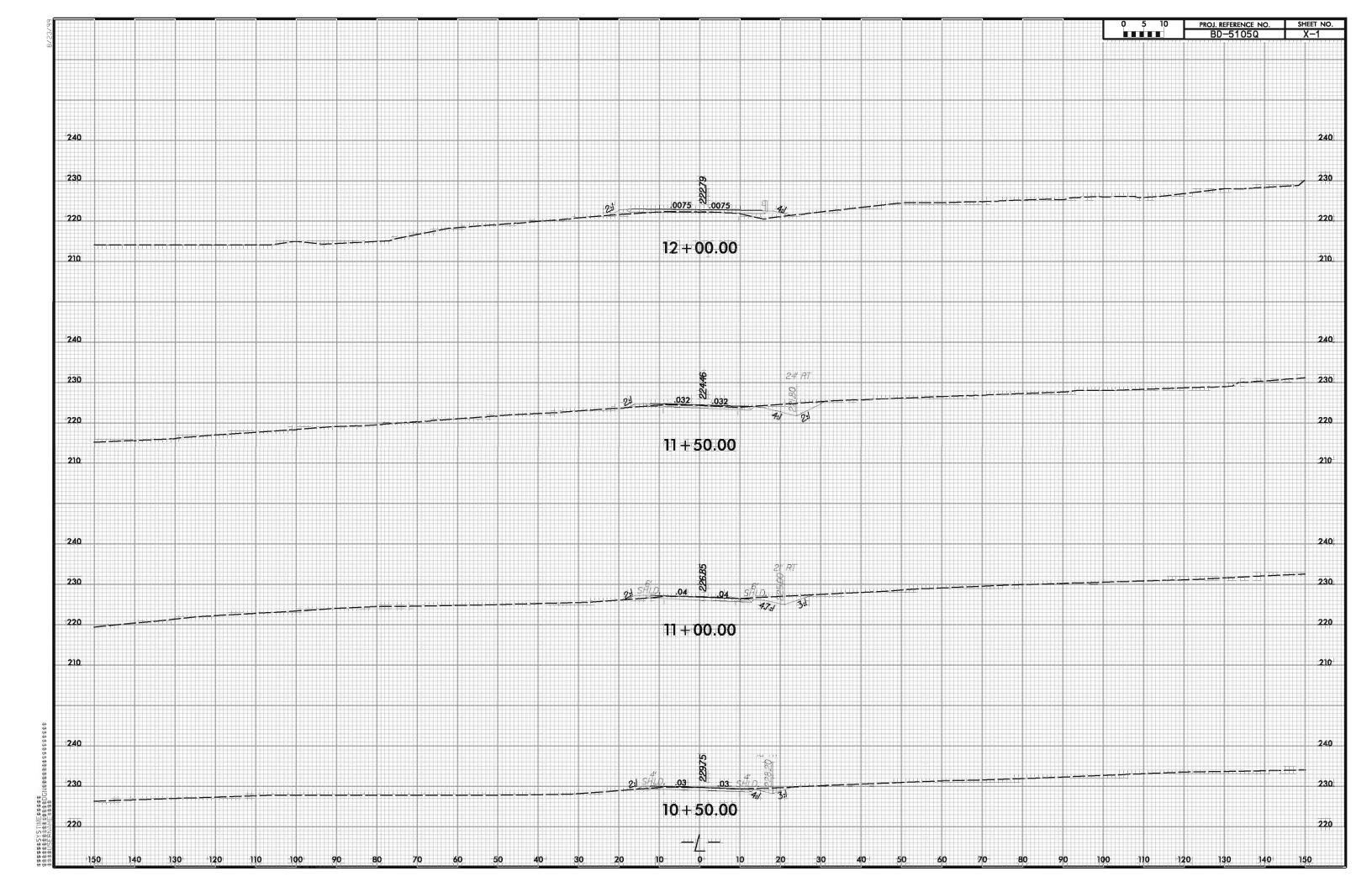
REFORESTATION

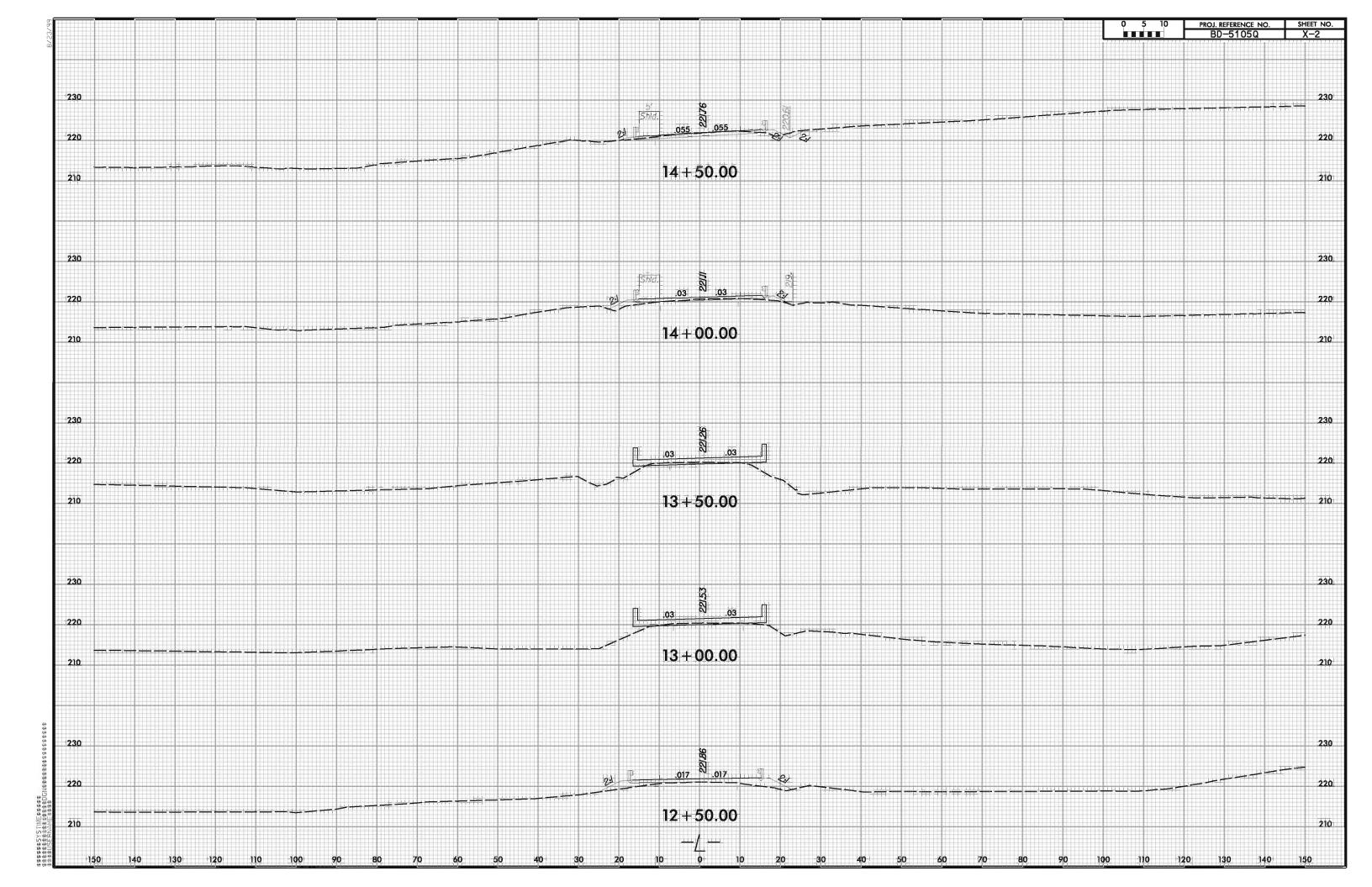
MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

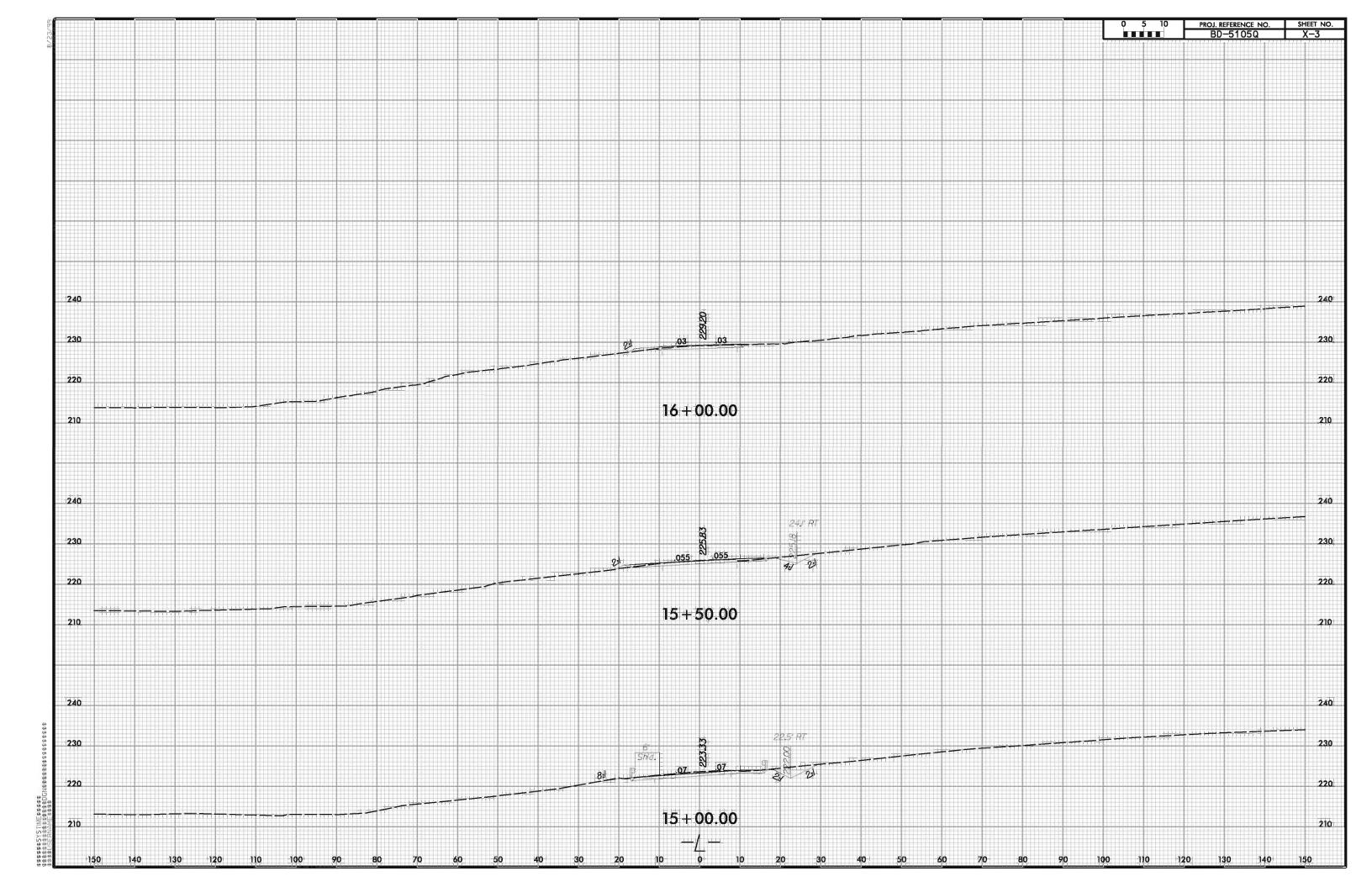
25% LIRIODENDRON TULIPIFERA TULIP POPLAR 12 in - 18 in BR **SYCAMORE** 12 in - 18 in BR 25% PLATANUS OCCIDENTALIS 25% FRAXINUS PENNSYLVANICA **GREEN ASH** 12 in - 18 in BR 25% BETULA NIGRA RIVER BIRCH 12 in - 18 in BR

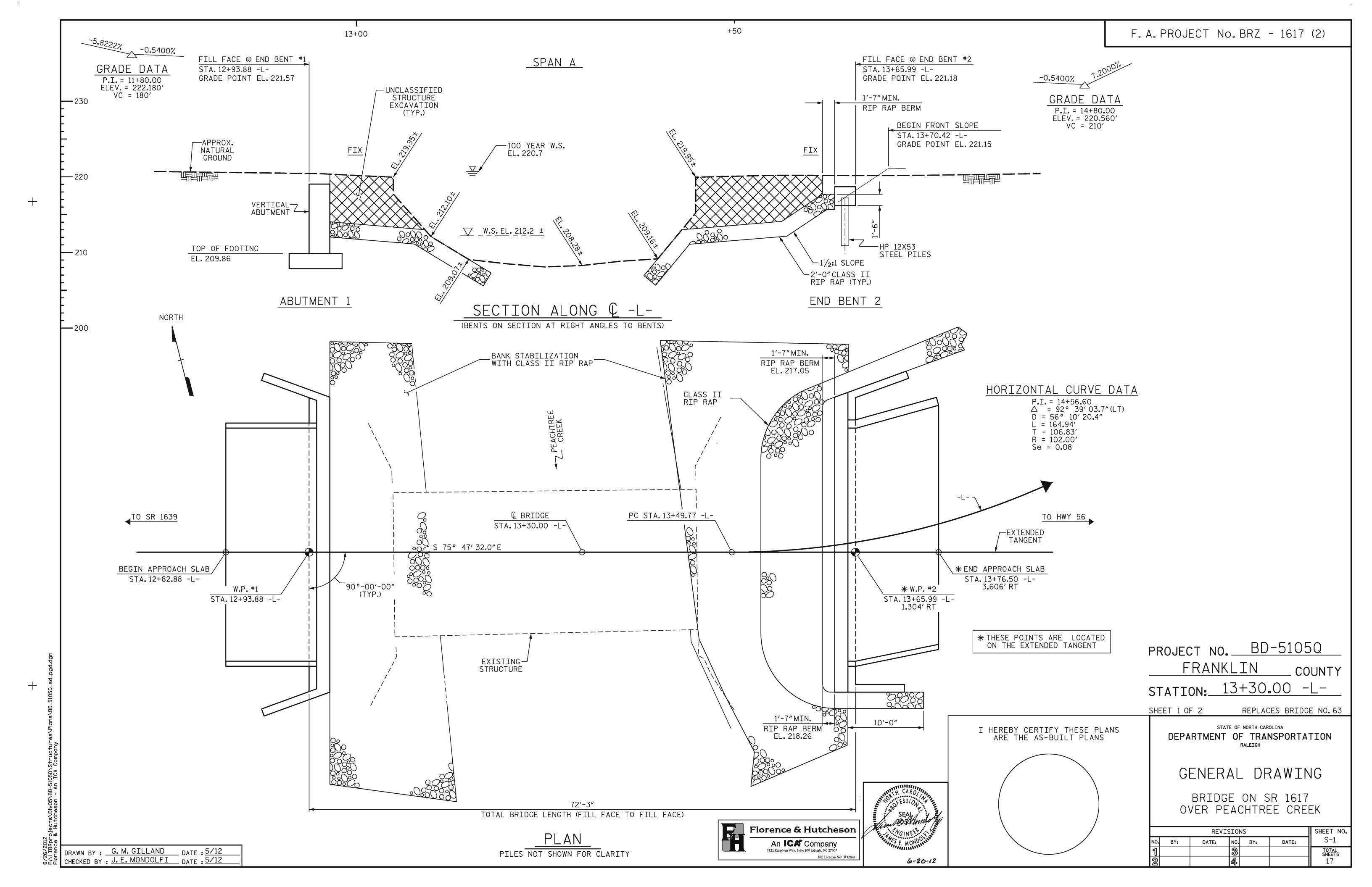
REFORESTATION DETAIL SHEET

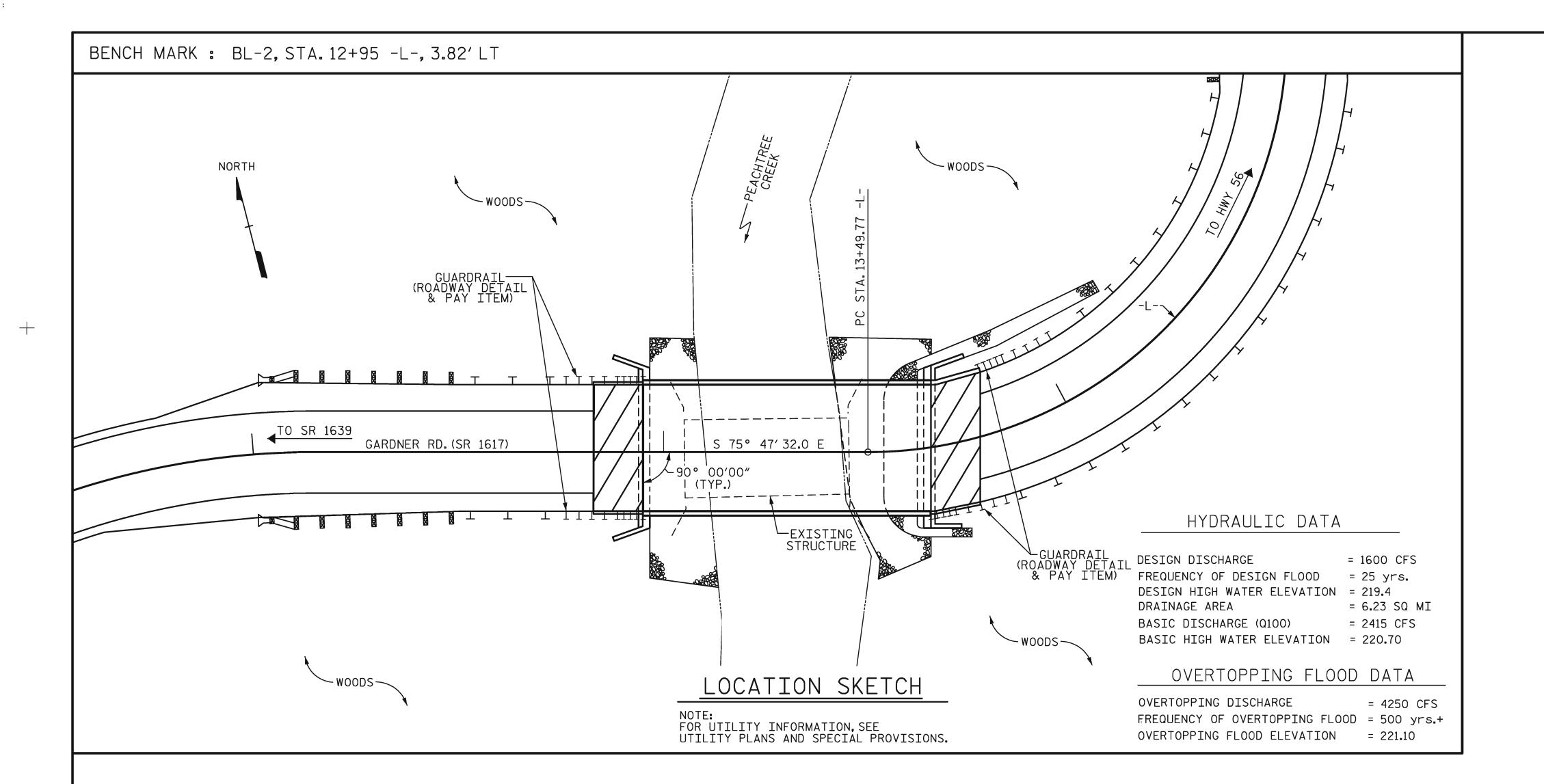
N.C.D.O.T. - ROADSIDE ENVIRONMENTAL UNIT











				$-$ TOT μ	AL BI	ILL C	F MA	\ T	ERIA	Δ \Box $$						
	EXISTING	EXCAVATION	APPROACH FILL SUBREGIONAL	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS	REINFORC- ING STEEL	HP STEI	12 X 53 EL PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRE: CON	O"X 2'-O" STRESSED CRETE ED SLAB TS
	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	CU. YDS.	LUMP SUM	LBS.	NO.	LIN. FT.	NO.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM							140.3			LUMP SUM	11	770
END BENT NO. 1					68.8		10,083									
END BENT NO. 2					14.8		2,169	7	70	7	12.53	124	140			
TOTAL	LUMP SUM	LUMP SUM	LUMP SUM	LUMP SUM	83.6	LUMP SUM	12,252	7	70	7	152.83	124	140	LUMP SUM	11	770

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

FOR OTHER DESIGN DATA AND GENERAL NOTES. SEE SHEET SN.

THE CONTRACTOR SHALL PROVIDE INDEPENDENT ASSURANCE SAMPLES OF REINFORCING STEEL AS FOLLOWS: FOR PROJECTS REQUIRING UP TO 400 TONS OF REINFORCING STEEL, ONE 30 INCH SAMPLE OF EACH SIZE BAR USED, AND FOR PROJECTS REQUIRING OVER 400 TONS OF REINFORCING STEEL, TWO 30 INCH SAMPLES OF EACH SIZE BAR USED. THE BARS FROM WHICH THE SAMPLES ARE TAKEN MUST THEM BE SPLICED WITH REPLACEMENT BARS OF THE SIZE AND LENGTH OF THE SAMPLE PLUS A MINIMUM LAP SPLICE OF THIRTY BAR DIAMETERS, PAYMENT FOR THE SAMPLES OF REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO VARIOUS PAY ITEMS.

THE MATERIAL IN THE CROSS HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FEET EACH SIDE OF CENTERLINE ROADWAY AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AS "UNCLASSIFIED STRUCTURE EXCAVATION". LUMP SUM.

THE LOWER LIMIT OF THE UNCLASSIFIED STRUCTURE EXCAVATION IS APPROXIMATE ELEVATION 212.0 IN THE CHANNEL.

THE EXISTING STRUCTURE CONSISTING OF ONE, 40.5' STEEL I-BEAM SPAN; $2\frac{1}{2}$ " ASPHALT WEARING SURFACE ON TIMBER FLOOR ON TIMBER END BENT CAPS ON TIMBER PILES LOCATED ON THE PROPOSED ALIGNMENT SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED BELOW THE LEGAL LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE FURTHER DETERIORATE THIS LOAD LIMITATION MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE SPREAD FOOTING AT END BENT NO.1 IS DESIGNED FOR A FACTORED RESISTANCE OF 10 TSF, CHECK FIELD CONDITIONS FOR THE REQUIRED RESISTANCE OR 15 TSF JUST BEFORE PLACING CONCRETE.

CARRY IN SPREAD FOOTING AT END BENT NO.1 AT LEAST 12" INTO ROCK WITH MINIMUM THICKNESS AS SHOWN ON THE PLANS.

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

PILES AT END BENT NO. 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 81 TONS PER PILE.

DRIVE PILES TO A REQUIRED DRIVING RESISTANCE OF 135 TONS PER PILE.

STEEL H PILE POINTS ARE REQUIRED FOR STEEL H PILES AT END BENT NO. 2. FOR STEEL PILE POINTS, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES". MAY. 2001.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR VERTICAL CONCRETE BARRIER RAIL, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

6-20-12

FOR BRIDGE APPROACH FILLS. SEE SPECIAL PROVISIONS.

PROJECT NO. BD-5105Q FRANKLIN _ COUNTY

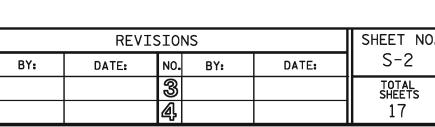
STATION: 13+30.00 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

BRIDGE ON SR 1617 OVER PEACHTREE CREEK



Florence & Hutcheson An ICA Company

DRAWN BY: G. M. GILLAND DATE: 5/12 CHECKED BY : J. E. MONDOLFI DATE : 5/12

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

- 1 DESIGN LOAD RATING (HL-93)
- 2 DESIGN LOAD RATING (HS-20)
- 3 LEGAL LOAD RATING **

** SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

- I INTERIOR GIRDER
- EL EXTERIOR LEFT GIRDER
- ER EXTERIOR RIGHT GIRDER

PROJECT NO. BD-51050

FRANKLIN COUNTY

STATION: 13+30.00 -L-



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD LRFR SUMMARY FOR 70'CORED SLAB UNIT 90° SKEW

(NON-INTERSTATE TRAFFIC)

REVI	SION	IS		SHEET NO.
DATE:	NO.	BY:	DATE:	S-3
	3			TOTAL SHEETS
	4			17

LOAD AND RESISTANCE FACTOR RATING (LRFD) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS

										STRE	ENGTH	I LIN	ΛΙΤ S	TATE				SE	RVICE	III	LIMIT	T STA	TE	
										MOMENT					SHEAR			The second secon			MOMENT			
LEVEL		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W X RF	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVELOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93(Inv)	N/A	1	1.006		1.75	0.273	1.03	70′	EL	34.5	0.507	1.32	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	
DESIGN		HL-93(0pr)	N/A		1.341		1.35	0.273	1.34	70′	EL	34.5	0.507	1.72	70′	EL	6.9	N/A		<u>-</u> _				
LOAD RATING		HS-20(Inv)	36.000	2	1.306	47.02	1.75	0.273	1.34	70′	EL	34.5	0.507	1.65	70′	EL	6.9	0.80	0.273	1.31	70′	EL	34.5	
		HS-20(0pr)	36.000		1.74	62.64	1.35	0.273	1.74	70′	EL	34.5	0.507	2.14	70′	EL	6.9	N/A						
		SNSH	13.500		2.917	39.379	1.4	0.273	3.75	70′	EL	34.5	0.507	4.87	70′	EL	6.9	0.80	0.273	2.92	70′	EL.	34.5	
		SNGARBS2	20.000		2.187	43.741	1.4	0.273	2.81	70′	EL	34.5	0.507	3.47	70′	EL	6.9	0.80	0.273	2.19	70′	EL	34.5	
		SNAGRIS2	22.000		2.077	45.69	1.4	0.273	2.67	70′	EL	34.5	0.507	3.23	70′	EL	6.9	0.80	0.273	2.08	70′	EL	34.5	
	>	SNCOTTS3	27.250		1.452	39.565	1.4	0.273	1.87	70′	EL	34.5	0.507	2.43	70′	EL	6.9	0.80	0.273	1.45	70′	EL	34.5	
	S	SNAGGRS4	34.925		1.218	42.554	1.4	0.273	1.57	70′	EL	34.5	0.507	2.03	70′	. EL	6.9	0.80	0.273	1.22	70′	EL	34.5	
		SNS5A	35.550		1.191	42.346	1.4	0.273	1.53	70′	EL	34.5	0.507	2.06	70′	EL	6.9	0.80	0.273	1.19	70′	EL	34.5	
		SNS6A	39.950		1.095	43.747	1.4	0.273	1.41	70′	EL	34.5	0.507	1.88	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
LEGAL		SNS7B	42.000		1.043	43.801	1.4	0.273	1.34	70′	EL	34.5	0.507	1.85	70′	EL	6.9	08.0	0.273	1.04	70′	EL	34.5	
LOAD RATING		TNAGRIT3	33.000		1.336	44.087	1.4	0.273	1.72	70′	EL	34.5	0.507	2.23	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
		TNT4A	33.075		1.342	44.401	1.4	0.273	1.72	70′	EL	34.5	0.507	2.17	70′	EL	6.9	0.80	0.273	1.34	70′	EL	34.5	
		TNT6A	41.600		1.1	45.746	1.4	0.273	1.41	70′	EL	34.5	0.507	1.98	70′	EL	6.9	0.80	0.273	1.10	70′	EL	34.5	
	151	TNT7A	42.000		1.106	46.462	1.4	0.273	1.42	70′	EL	34.5	0.507	1.94	70′	EL	6.9	0.80	0.273	1.11	70′	EL	34.5	
	-	TNT7B	42.000		1.147	48.18	1.4	0.273	1.47	70′	EL	34.5	0.507	1.8	70′	EL	6.9	0.80	0.273	1.15	70′	EL	34.5	
		TNAGRIT4	43.000		1.089	46.838	1.4	0.273	1.4	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.09	70′	EL	34.5	
		TNAGT5A	45.000		1.026	46.175	1.4	0.273	1.32	70′	EL	34.5	0.507	1.74	70′	EL	6.9	0.80	0.273	1.03	70′	EL	34.5	
		TNAGT5B	45.000	3	1.013	45.579	1.4	0.273	1.3	70′	EL	34.5	0.507	1.66	70′	EL	6.9	0.80	0.273	1.01	70′	EL	34.5	

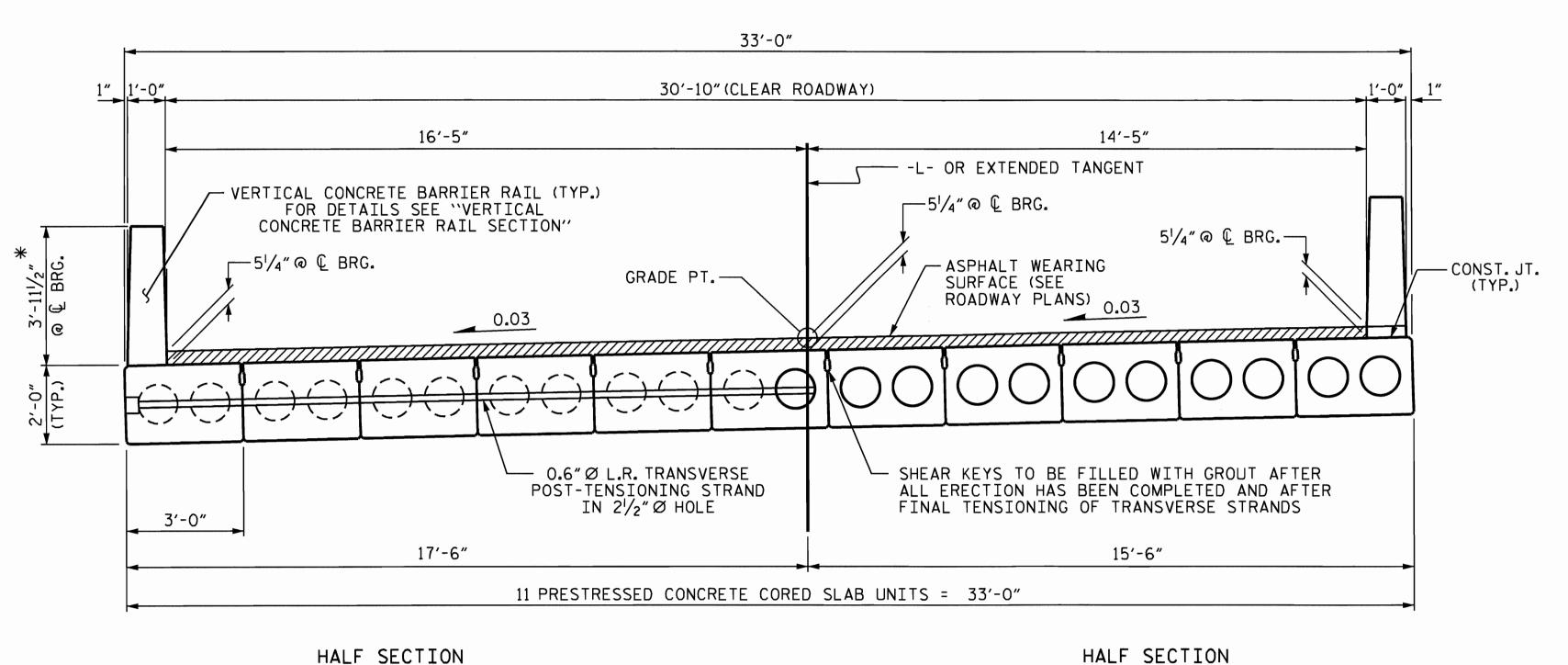
<u>1</u>
<u>2</u>
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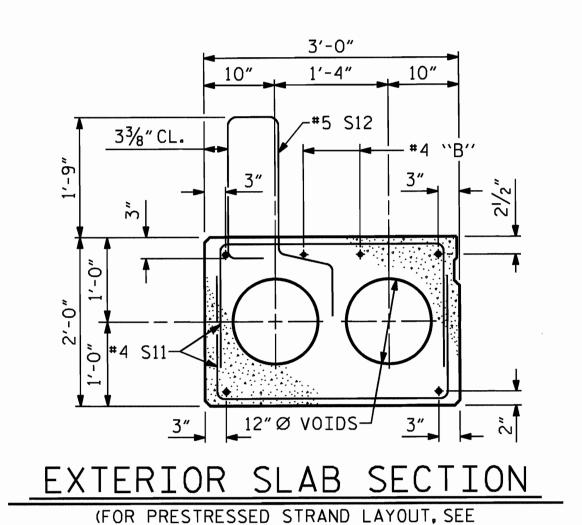
LRFR SUMMARY

FOR SPAN 'A'

ASSEMBLED BY: A.C. OUTLAW DATE: 2/21/12 CHECKED BY: K.W. ALFORD DATE: 3/12

DRAWN BY : CVC 6/10 CHECKED BY : DNS 6/10





INTERIOR SLAB SECTION.)

1'-6" 1'-6" 10" 1'-4" 11" 4" 4" 11" #4 \`B''---r12" Ø VOIDS ≦∾ -2 SPA. @ 2"CTS. 2 SPA. @ 2"CTS. @ 2"CTS. @ 2"CTS.

INTERIOR SLAB SECTION (70'UNIT) (28 STRANDS REQUIRED)

0.6" Ø LOW RELAXATION STRAND LAYOUT

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 12'-0" FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

HALF SECTION AT INTERMEDIATE DIAPHRAGMS

+

TYPICAL SECTION

THROUGH VOIDS

* - THE MAXIMUM BARRIER RAIL HEIGHT AND ASPHALT THICKNESS IS SHOWN. THE HEIGHT OF THE BARRIER RAIL AND ASPHALT THICKNESS VARIES WHILE THE TOP OF THE BARRIER RAIL FOLLOWS THE PROFILE OF THE GUTTERLINE. FOR RAIL HEIGHT DETAILS AND ASPHALT THICKNESS, SEE THE "VERTICAL CONCRETE BARRIER RAIL SECTION" DETAIL.

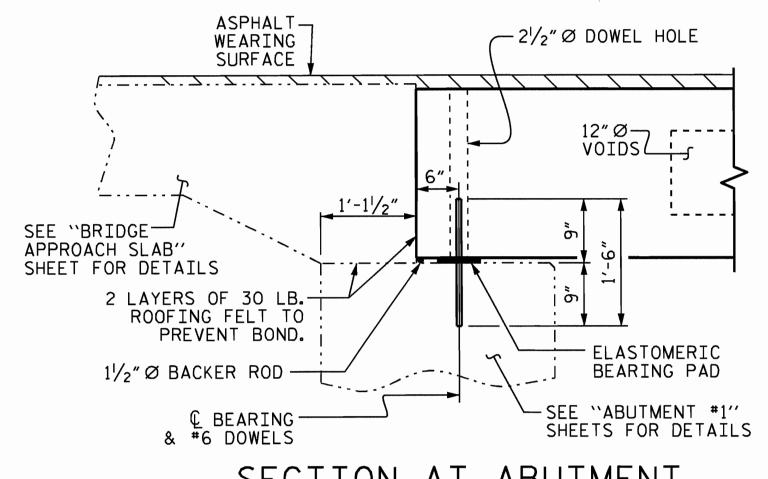
FIXED END

FIXED END

--ASPHALT

+----

—SEE "BRIDGE



ASSEMBLED BY : A.C. OUTLAW

CHECKED BY : K. W. ALFORD

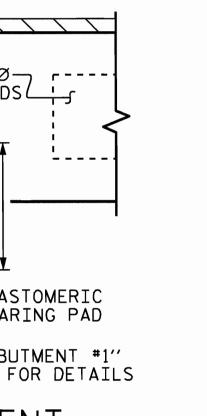
CHECKED BY : MKT 7/10

DRAWN BY : MAA 6/10 REV. 12/11

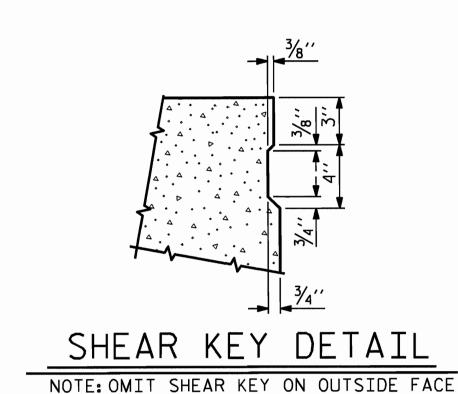
DATE : 2/21/12

MAA/AAC

DATE: 3/12



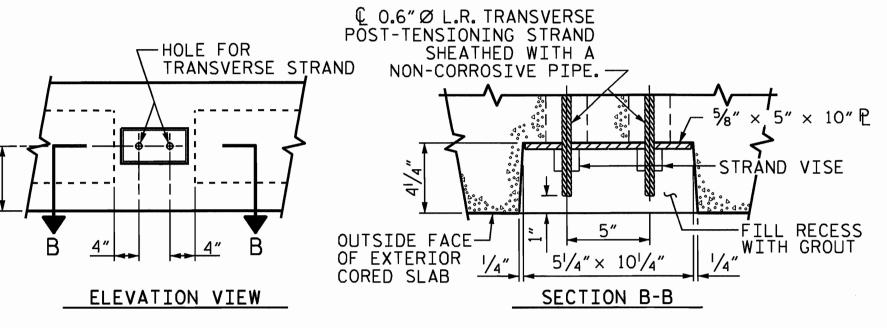
21/2" Ø DOWEL HOLE -WEARING SURFACE Ø″212 ----! VOIDS بــٰـــ - - - - - APPROACH SLAB"
SHEET FOR DETAILS 2 LAYERS OF 30 LB. ROOFING FELT TO PREVENT BOND. ELASTOMERIC 11/2" Ø BACKER ROD BEARING PAD SEE "END BENT 2" - SHEETS FOR DETAILS ♠ BEARING
& #6 DOWELS



OF EXTERIOR CORED SLABS.

SECTION AT END BENT

ABUTMENT SECTION AT



GROUTED RECESS AT END OF POST-TENSIONED STRAND · CORED SLABS

3'-0" 1'-6" 91/2" 91/2" 81/2" — (£ 2½″Ø DOWEL HOLES 1'-2" 4" 4" 1'-2" **−**#5 S15 -#4 S14 #5 S10~ └1″ CL.

END ELEVATION

SHOWING PLACEMENT OF DOUBLE STIRRUPS AND LOCATION OF DOWEL HOLES. (STRAND LAYOUT NOT SHOWN.)
INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

BD-5105Q PROJECT NO._ FRANKLIN COUNTY STATION: 13+30.00 -L-

SHEET 1 OF 3

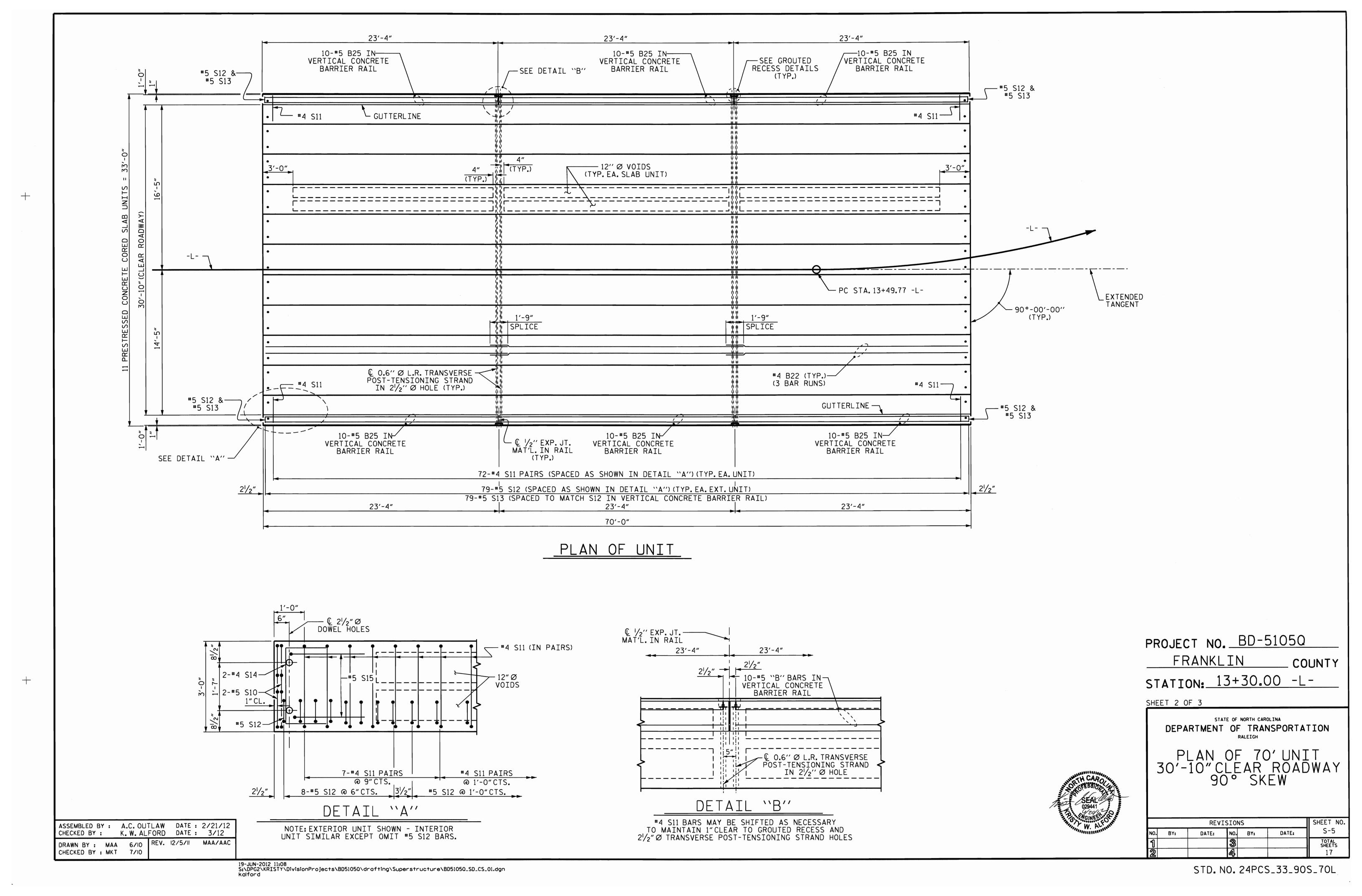
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD

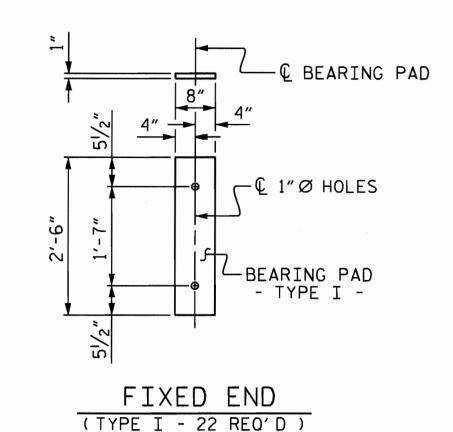
3'-0" X 2'-0" PRESTRESSED CONCRETE CORED SLAB UNIT

	REV	ISIONS			SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-4
		3			TOTAL SHEETS
		4			17

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STD. NO. 24PCS4_33_90S





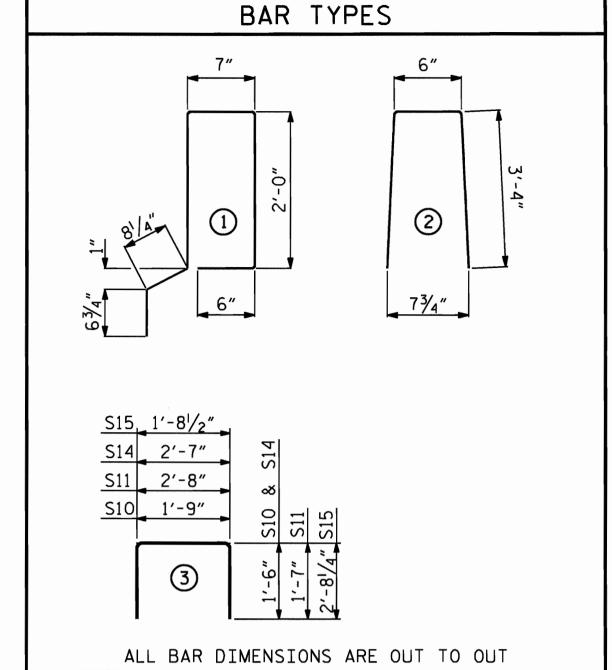
ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.

GRADE 270 S	TRANDS
	0.6"Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

CORED	SLABS	S REQ	UIRED
	NUMBER	LENGTH	TOTAL LENGTH
70' UNIT			
EXTERIOR C.S.	2	70′-0″	140'-0"
INTERIOR C.S.	9	70′-0″	630′-0″
TOTAL	11	70′-0″	770′-0″

DEAD LOAD DEFLECTION AND CAMBER							
	3'-0" × 2'-0"						
70' CORED SLAB UNIT	0.6"Ø L.R. STRAND						
CAMBER (SLAB ALONE IN PLACE)	4 ⁵ ⁄ ₁₆ ″ ∤						
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD**	13/16″ ♦						
FINAL CAMBER	31/2" 1						
** INCLUDES FUTURE WEARING SURFACE							



	BILL OF MATERIAL FOR ONE 70' CORED SLAB UNIT							
,				EXTERI	OR UNIT	INTERI	OR UNIT	
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	
B22	6	#4	STR	24'-6"	98	24'-6"	98	
S10	8	#5	3	4′-9″	40	4'-9"	40	
S11	144	#4	3	5′-10″	561	5′-10″	561	
* S12	79	#5	1	6′-4″	522			
S14	4	#4	3	5′-7″	15	5′-7"	15	
S15	4	#5	3	7'-1"	30	7'-1"	30	
REINFO	ORCING	STEEL	LBS	5.	744		744	
	Y COATE							
REIN	NFORCING	STEEL	LBS	S	522			
7000	P.S.I. CO	NCRETE	CU. YDS	ò.	11.8	11.8		
0.6"Ø	L.R. STR	ANDS	No).	28		28	

BT	LL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	TFR R	<u>Λ</u> ΤΙ
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	70' UNIT	TOTAL NO.	3122	· · · · <u>-</u>	ZZ/(O / III	WEIGHT
∗ B25	60	60	#5	STR	22'-11"	1434
* S13	158	158	#5	2	7'-2"	1181
* EPOX	Y COATED REINFORCING STEEL			LBS.		2615
CLASS	AA CONCRETE			CU.YDS.		18.9
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN. FT.		140.25

GUTTERLINE ASP	HALT THICKNESS & RAI	L HEIGHT
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	RAIL HEIGHT @ MID-SPAN
70' UNITS	13/4"	3′-8″
10 81(113	1/4	3 6

0 0 4	NCINE LE	RELEA	42F	STRENGTH
	UNIT			PSI
70	D'UNITS			5500

NOTES

270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE

REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE

THE 21/2" Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS,

LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF

TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

FOR GROUT FOR STRUCTURES. SEE SPECIAL PROVISIONS.

CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1"

STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE

EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT

TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT

SPECIFICATIONS.

BE EPOXY COATED.

10 FEET IN LENGTH.

CLEAR TO THE GROUTED RECESS.

ENDS.

PRESTRESSED CONCRETE CORED SLABS.

"CONCRETE RELEASE STRENGTH" TABLE.

TENSIONING OF THE STRANDS.

FILLED WITH NON-SHRINK GROUT.

PROJECT NO. BD-5105Q FRANKLIN _ COUNTY STATION: 13+30.00 -L-

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 2-0"
PRESTRESSED CONCRETE
CORED SLAB UNIT

	REV	ISIONS			SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-6
		3			TOTAL SHEETS
		4			17

e EBRG. O MIDSPAN 2"CL.MIN.	1"		
3'-11/2" VARIES (SEE 'GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT" TABLE) 10-#5 'B" BARS 9¾" 61/2" 61/2" 8" 10"	7" "" "" "" "" "" "" "" "" ""	SECTION S-S AT DAM IN OPEN JOINT THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED) \$\tilde{\begin{align*} \lambda_{2}'' \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \	S CHAMFER
	#5 S12 (SEE "PLAN OF UNIT" FOR SPACING)	CONST. JT.	→ s
CONST. JT.			
SECTION THRU R	AIL_	ELEVATION AT EXPANSION	<u>JOINTS</u>

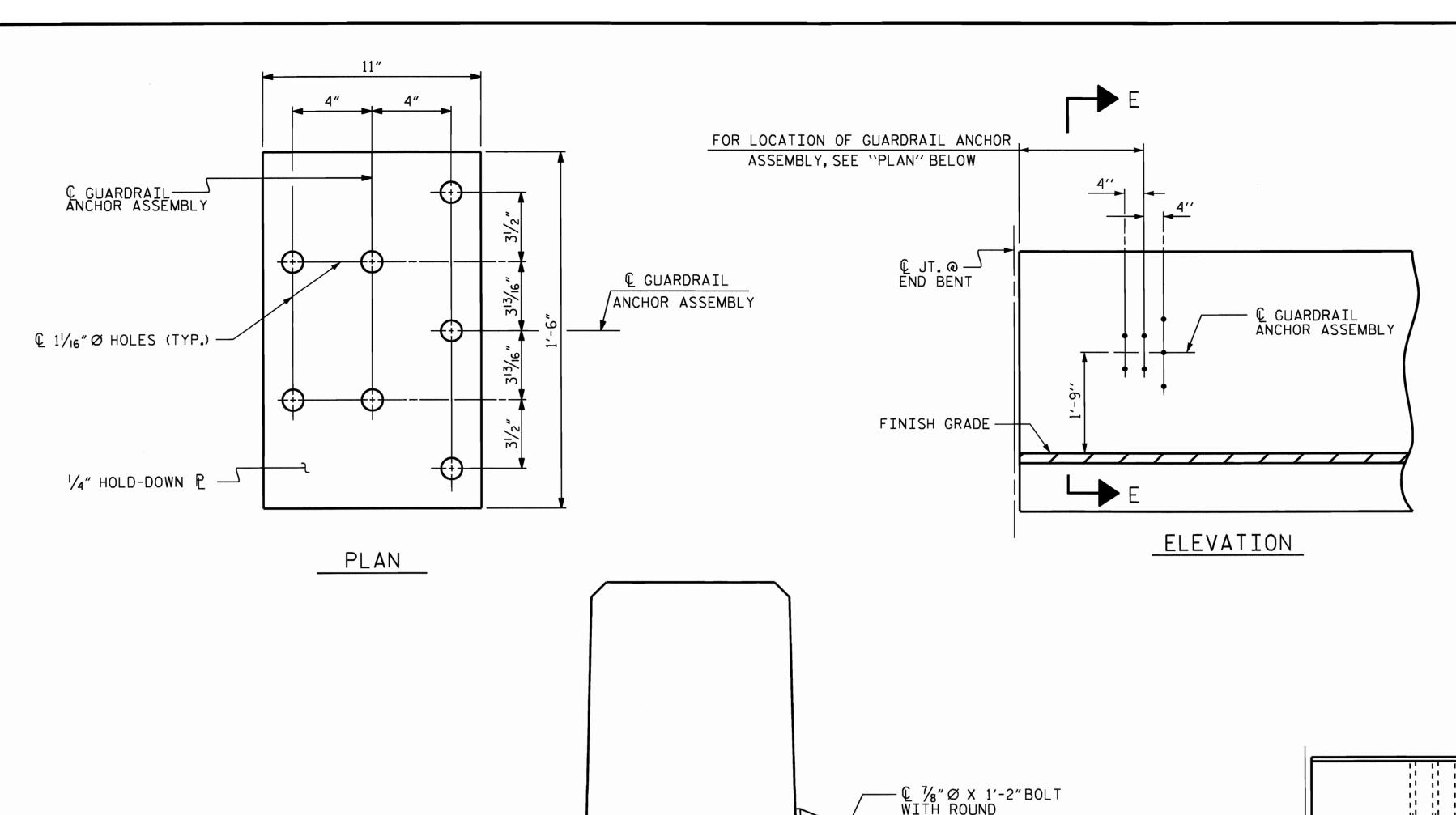
ASSEMBLED BY : A.C. OUTLAW DATE : 2/21/12 CHECKED BY: K. W. ALFORD DATE: 3/12 DRAWN BY : MAA 6/10 REV. 12/11

CHECKED BY : MKT 7/10

VERTICAL CONCRETE BARRIER RAIL DETAILS

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STD. NO. 24PCS3_33_90S



WASHERS (TYP.)

ANCHOR

 $-1^{1}/_{4}$ " Ø HOLE (TYP.)

ASSEMBLY

— C GUARDRAIL

ABUTMENT #1

1'-10"

Q GUARDRAIL
ANCHOR ASSEMBLY

A"

A"

ANCHOR ASSEMBLY

PLAN

LOCATION OF ANCHORS FOR GUARDRAIL

ABUTMENT #1 SHOWN, END BENT #2 SIMILAR.

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36.AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

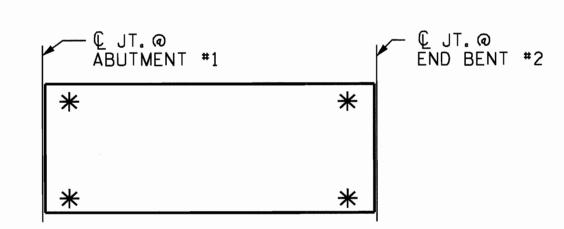
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 1/4" Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. BD-51050

FRANKLIN COUNTY

STATION: 13+30.00 -L-



DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD

GUARDRAIL ANCHORAGE

FOR VERTICAL CONCRETE

BARRIER RAIL

REVISIONS						SHEET NO.
BY:	DATE:	NO.	BY:	DATE	:	S-7
		3				TOTAL SHEETS
		4				17
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SECTION E-E

GUARDRAIL ANCHOR ASSEMBLY DETAILS

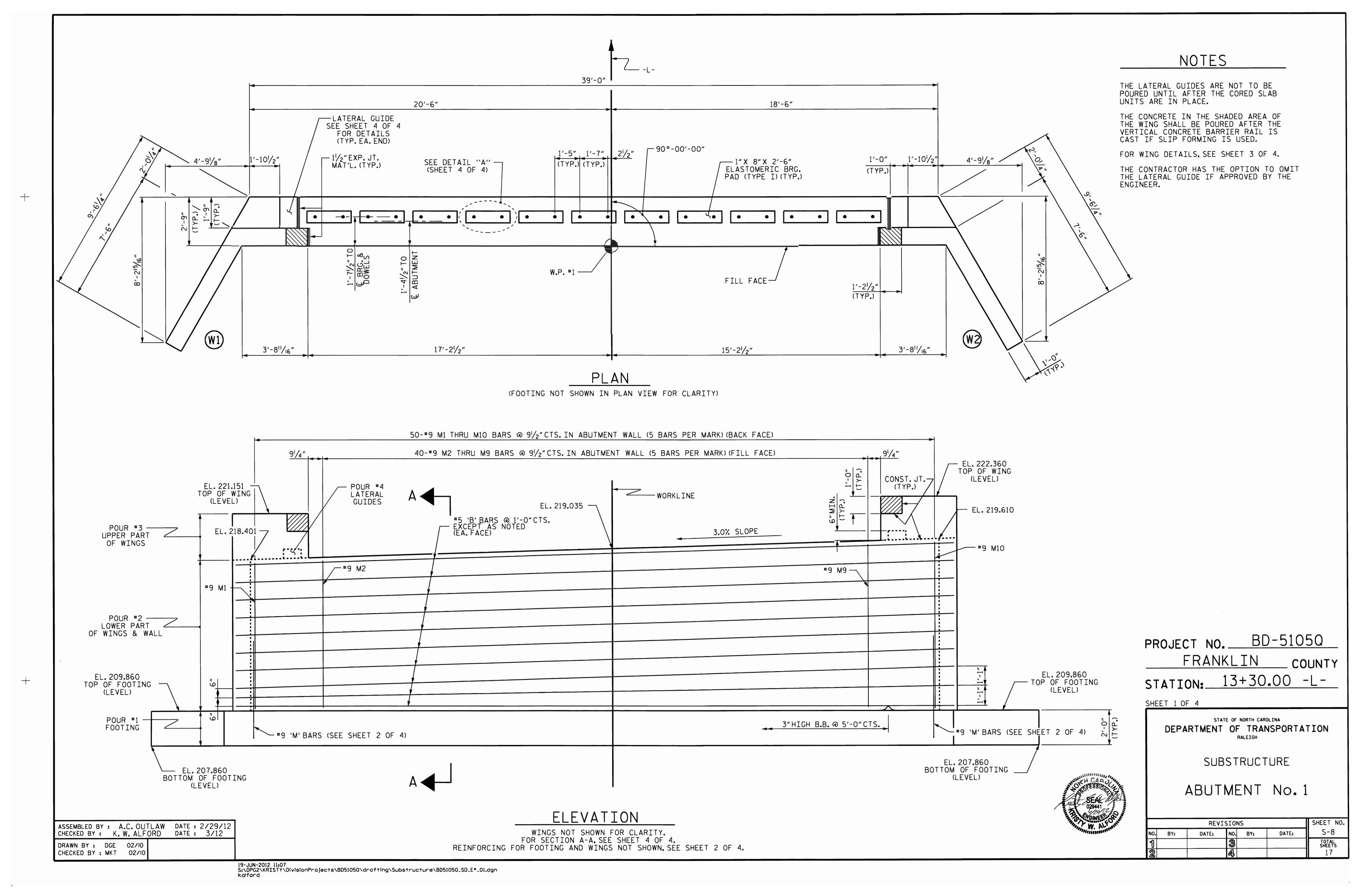
¼" HOLD-DOWN ₽—

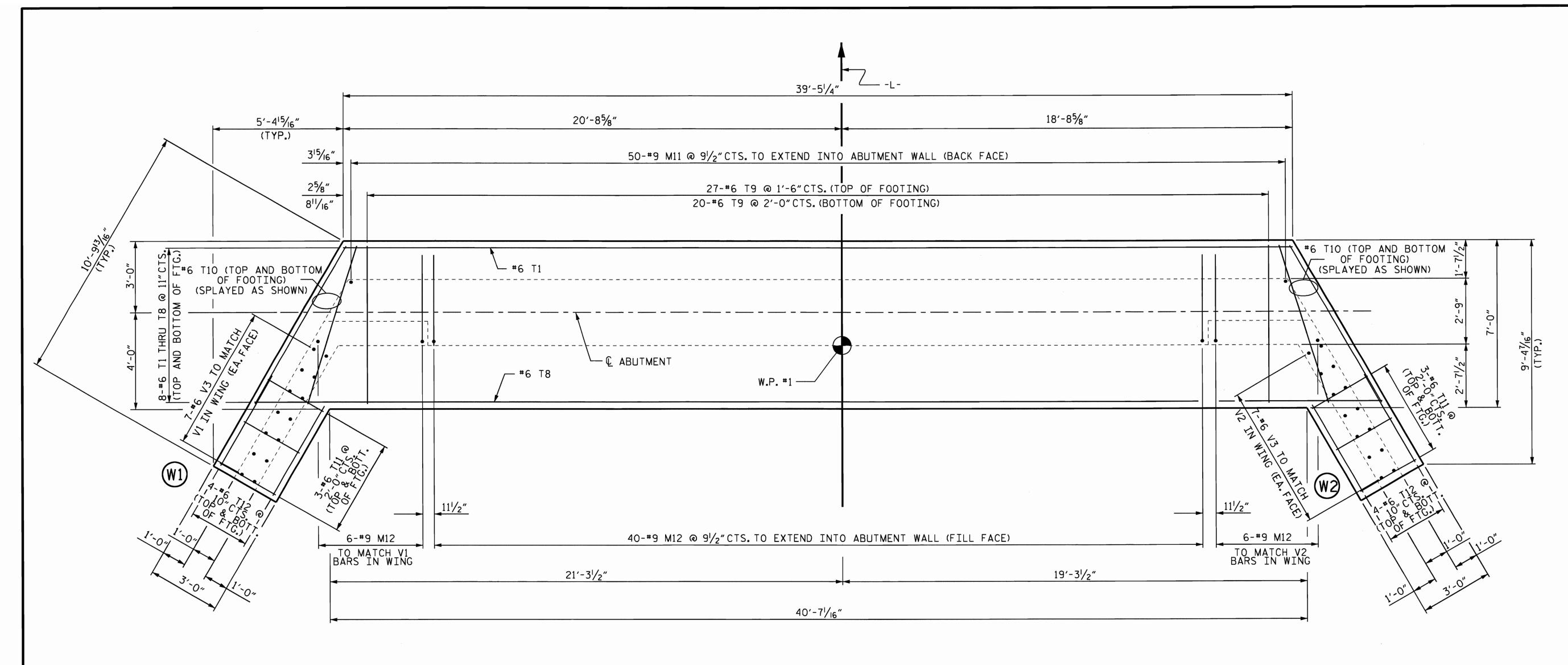
ASSEMBLED BY: A.C. OUTLAW DATE: 2/21/12 CHECKED BY: K.W. ALFORD DATE: 3/12

DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10

ADDED 5/6/IO REV. IO/I/II REV. I2/5/II

(SHT 1) STD. NO. GRA3





PLAN OF FOOTING

PROJECT NO. BD-5105Q
FRANKLIN COUNTY
STATION: 13+30.00 -L-

SHEET 2 OF 4

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

SUBSTRUCTURE

ABUTMENT No.1

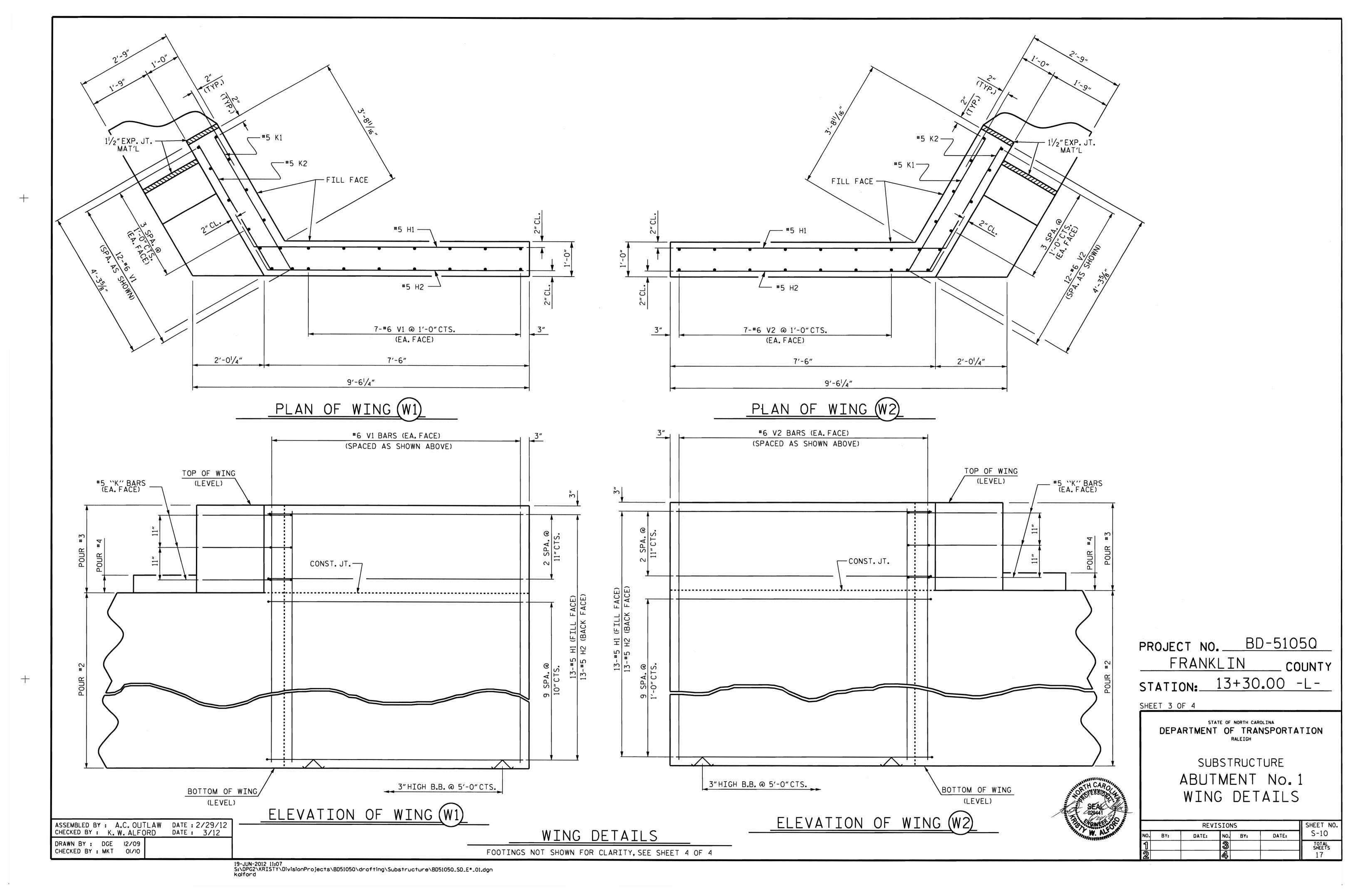
REVISIONS

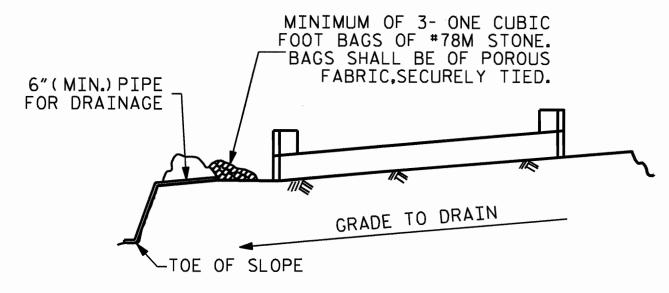
BY: DATE: NO. BY: DATE: S-9

TOTAL SHEETS
17

DRAWN BY: A.C. OUTLAW DATE: 2/29/12
CHECKED BY: K. W. ALFORD DATE: 3/12

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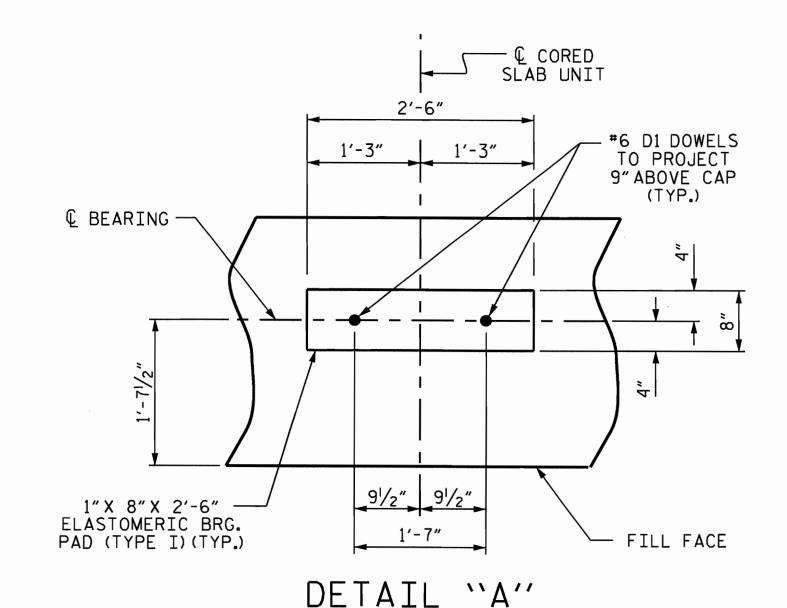


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE. CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



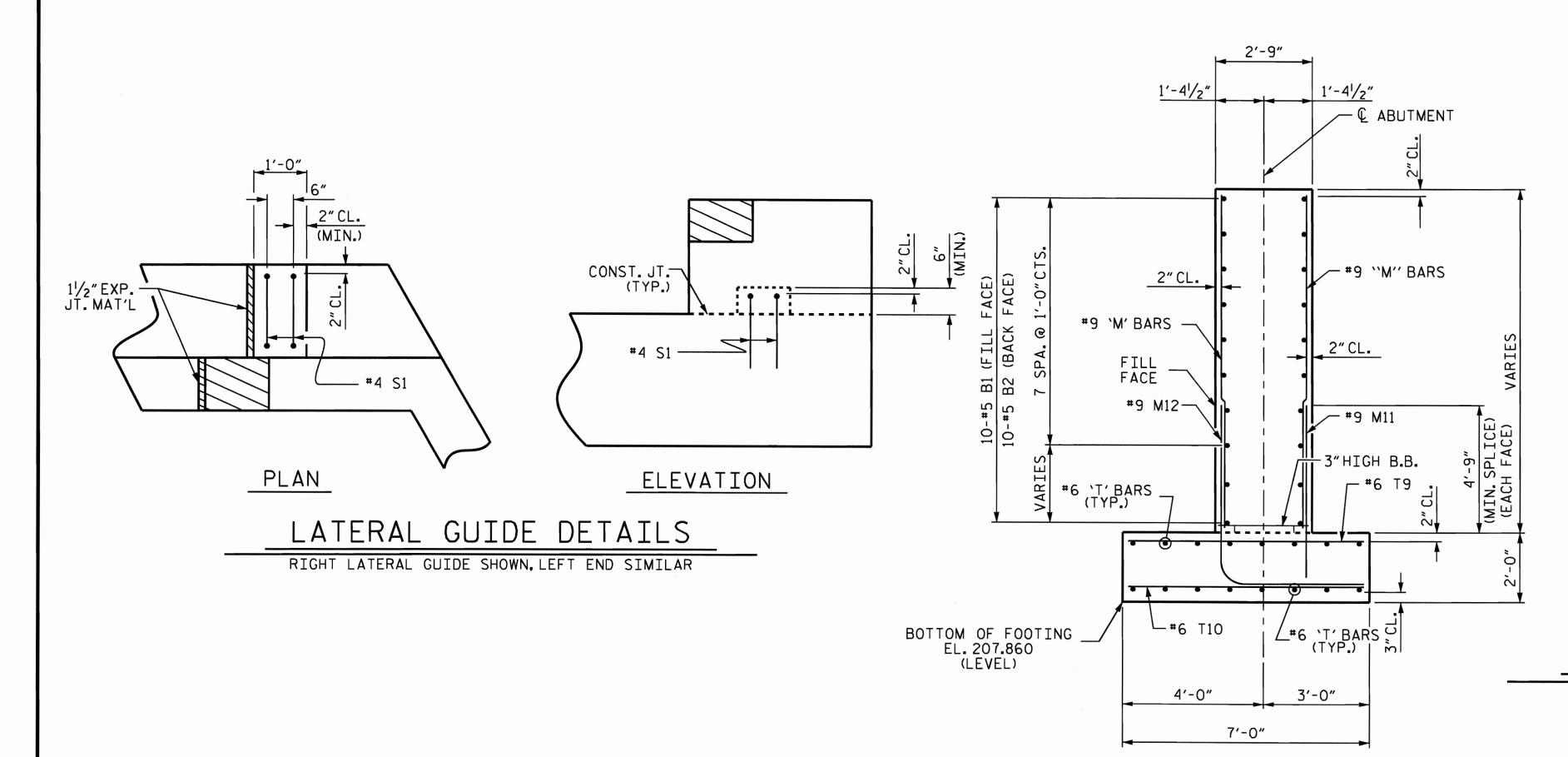
ABUTMENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION

SECTION A-A

SHOWING ABUTMENT WALL

BAR TYPES BILL OF MATERIAL FOR ABUTMENT No. 1 BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT | BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT #5 | STR | 41'-4" #5 | STR | 38'-10" STR | 40'-3" B2 | 10 405 T2 | 2 #6 #6 | STR | 41'-3" T3 | 2 D1 | 22 | #6 | STR | 1'-6" T4 50 #6 STR 42'-4" T5 STR | 43'-5" 7'-10" STR | 44'-5" H1 26 #5 8′-3″ #6 H2 | 26 | #5 | 2 | 7'-10" 212 #6 | STR | 45'-6" T8 2 #6 | STR | 46'-7" K1 6 #5 | STR | 4'-6" T9 | 47 #6 | STR | 6'-8" STR T10 #5 4'-0" STR 7'-6" #6 #6 | STR | 2'-8" T11 | 12 T12 16 #9 | STR | 8'-3" #6 | STR | 4'-4" 140 7'-7" M2 | 10 #9 | STR | 8'-4" 283 #9 | STR | 8'-5" 286 V1 | 26 | #6 | STR | 10'-11" M3 | 10 7′-2" V2 | 26 | #6 | STR | 12'-0" #9 | STR | 8'-7" M4 | 10 292 #9 | STR | 8'-8" 295 V3 | 28 | #6 | STR | 4'-0" M5 | 10 #9 STR 8'-10" M6 | 10 300 #9 | STR | 8'-11" 303 M7 | 10 M8 | 10 #9 | STR | 9'-0" 306 #9 | STR | 9'-2" M9 | 10 312 #9 | STR | 9'-4" 159 M10 | 5 M11 50 #9 | STR | 6'-3" 1063 REINFORCING STEEL M12 | 52 | #9 | 1 | 12'-2" 2151 CLASS A CONCRETE BREAKDOWN (FOR ABUTMENT No. 1) S1 | 4 #4 | 3 | 4′-5″ 12 ALL BAR DIMENSIONS ARE OUT TO OUT. POUR #1 FOOTING

*FOR SPACING OF #5 H1 & #5 H2 BARS, SEE SHEET 3 OF 4.



2" CL. CONST - #6 V1 OR V2 #6 V1 OR V2 — FILL FACE #6 V3 — #6 T12 3"HIGH B.B. BOTTOM OF FOOTING _ EL. 207.860 #6 (LEVEL) #7 └ #6 T11 (TYP.)

SECTION THRU WING AND FOOTING

PROJECT NO. BD-5105Q FRANKLIN COUNTY

POUR #2 LOWER PART OF WINGS

& WALL

POUR #3 UPPER PART OF

WINGS

POUR #4 LATERAL GUIDES

TOTAL CLASS A CONCRETE

FOUNDATION EXCAVATION

124

127

130

140

471

90

48

104

426

469

168

10083 LBS.

24.1 C.Y.

42.3 C.Y.

2.3 C.Y.

0.1 C.Y.

68.8 C.Y.

L.S.

STATION: 13+30.00 -L-

SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

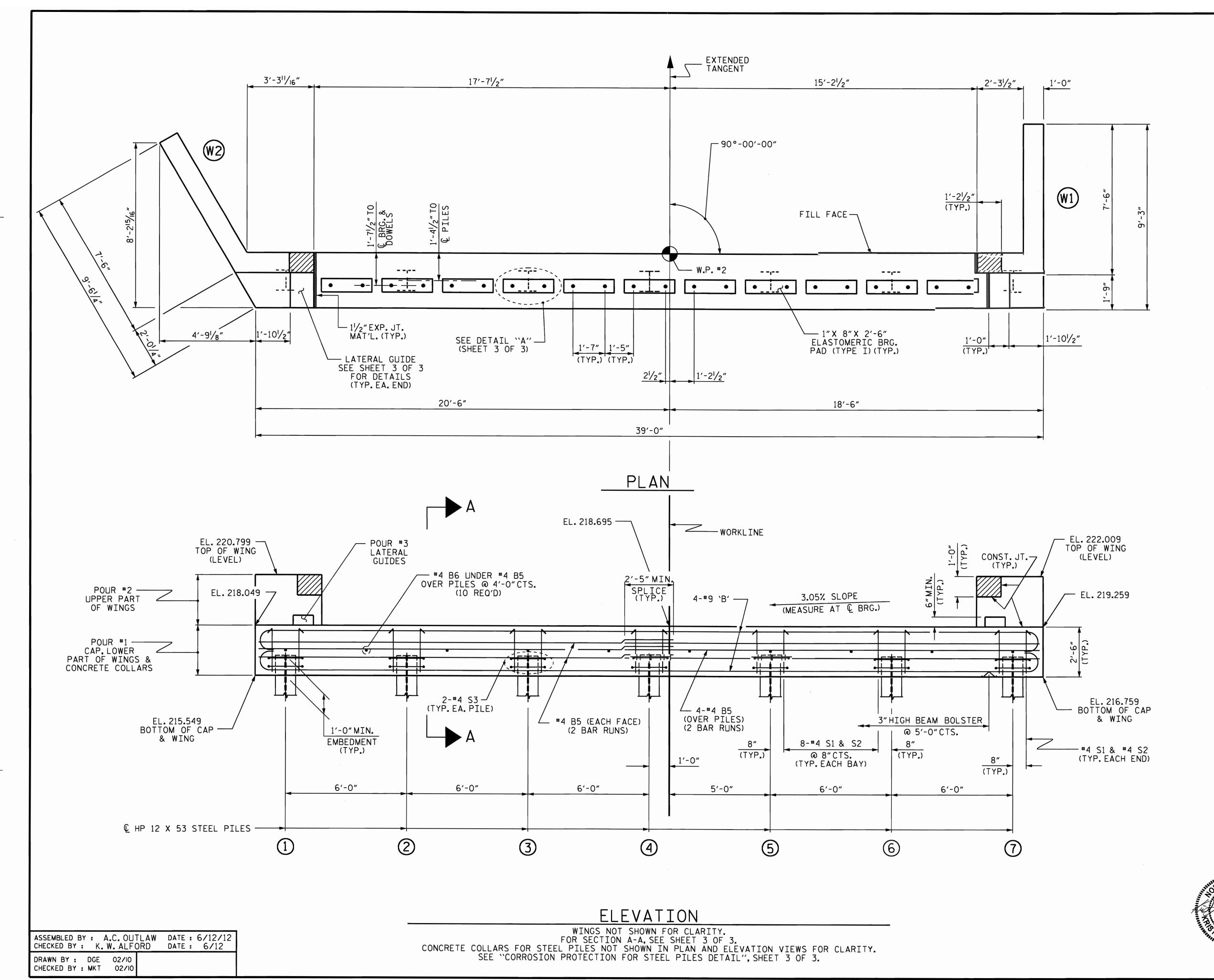
SUBSTRUCTURE

ABUTMENT No. 1 DETAILS

SHEET NO. **REVISIONS** S-11 NO. BY: DATE: TOTAL SHEETS

ASSEMBLED BY: A.C. OUTLAW DATE: 2/29/12 CHECKED BY: K.W. ALFORD DATE: 3/12 DRAWN BY: DGE 02/10 CHECKED BY: MKT 02/10

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kalford



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE LATERAL GUIDES ARE NOT TO BE POURED UNTIL AFTER THE CORED SLAB UNITS ARE IN PLACE.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 3 OF 3.

FOR WING DETAILS, SEE SHEET 2 OF 3.

THE CONTRACTOR HAS THE OPTION TO OMIT THE LATERAL GUIDE IF APPROVED BY THE ENGINEER.

TOP OF PILE ELEVATIONS							
1	216.630						
2	216.813						
3	216.996						
4	217.179						
(G)	217.362						
9	217.545						
	217.728						

PROJECT NO. BD-5105Q
FRANKLIN COUNTY

STATION: 13+30.00 -L-

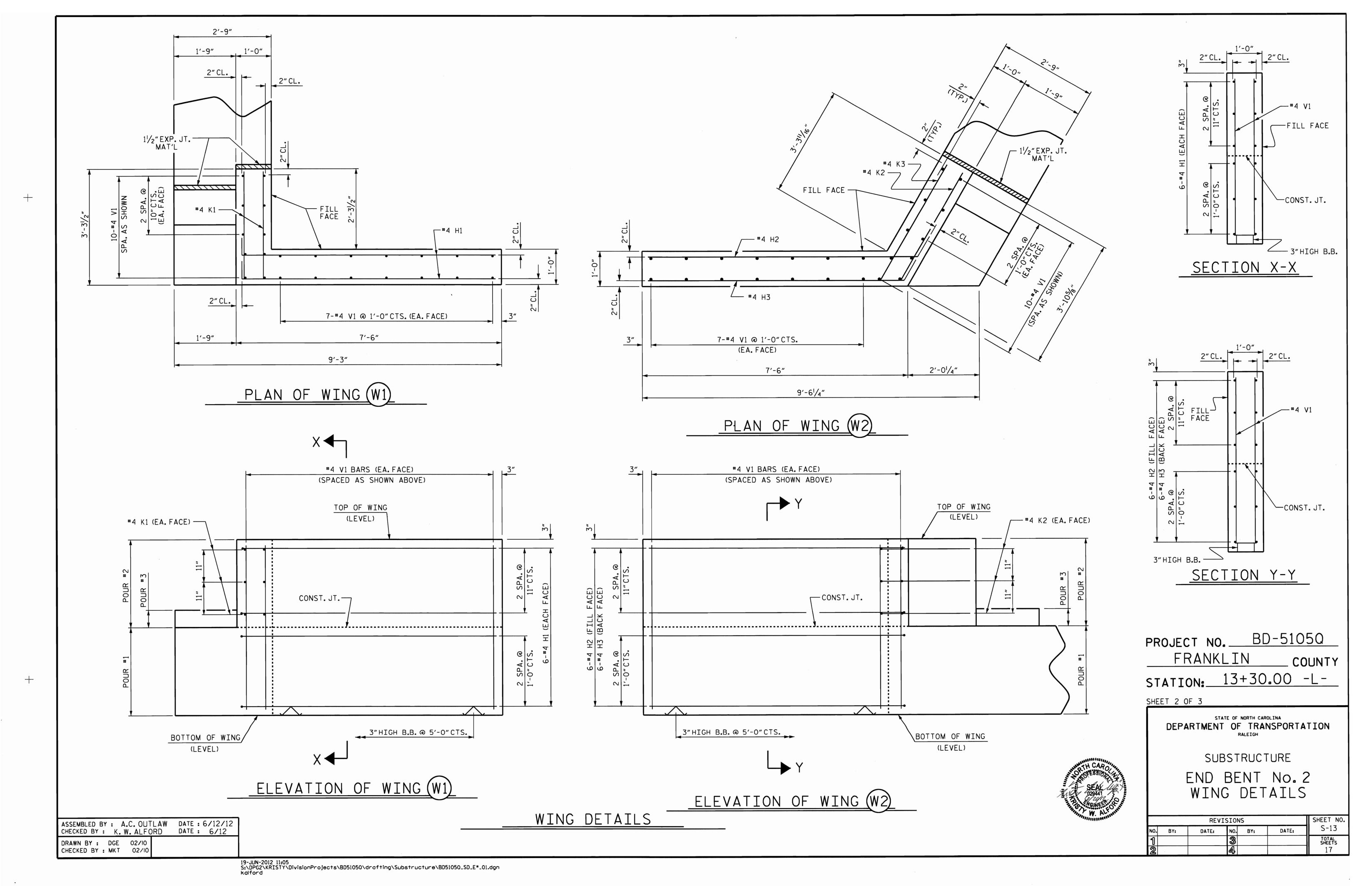
SHEET 1 OF 3

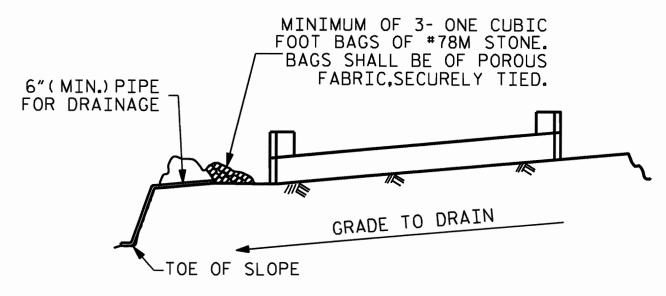
DEPARTMENT OF TRANSPORTATION
RALEIGH

SUBSTRUCTURE

END BENT No. 2

		REVIS	SIO	NS		SHEET NO.
10.	BY:	DATE:	NO.	BY:	DATE:	S-12
1			3			TOTAL SHEETS
2			4			17





BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT

2'-6"

91/2" 91/2"

1'-7"

DETAIL "A"

1'-3"

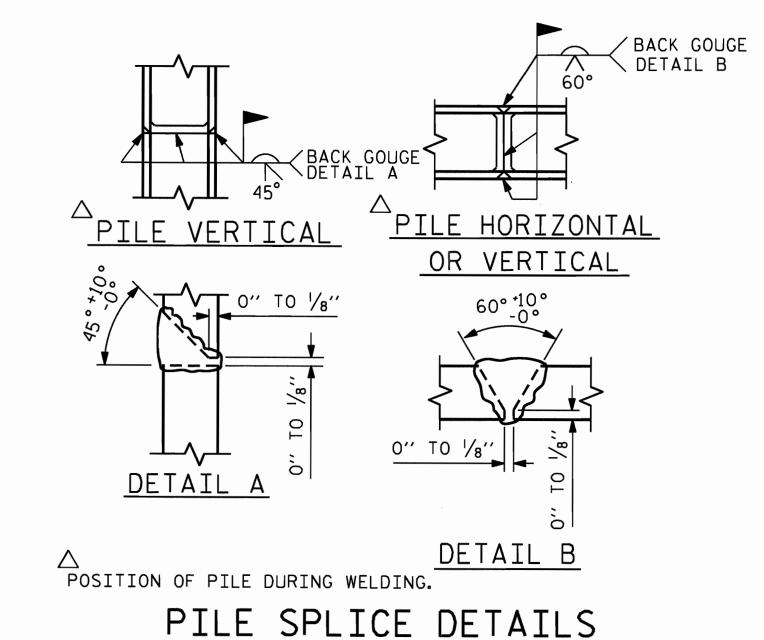
— € CORED SLAB UNIT

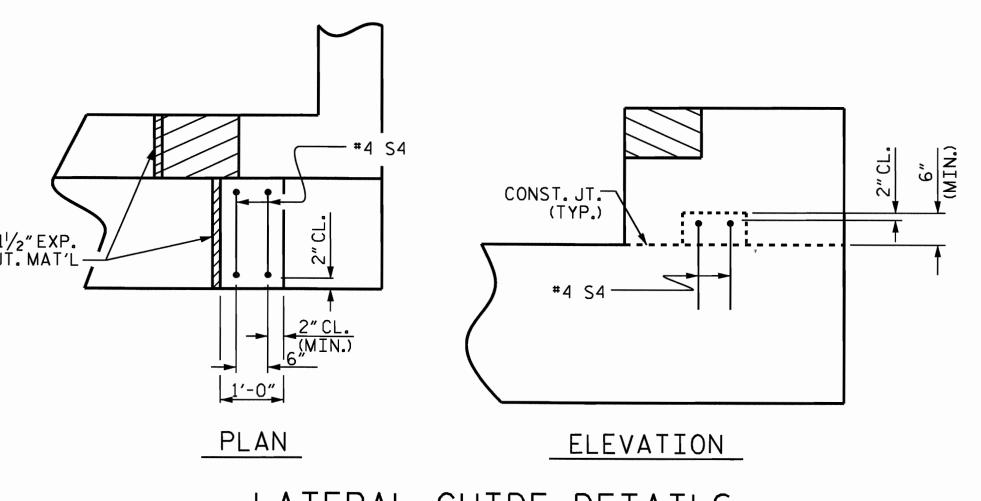
#6 D1 DOWELS

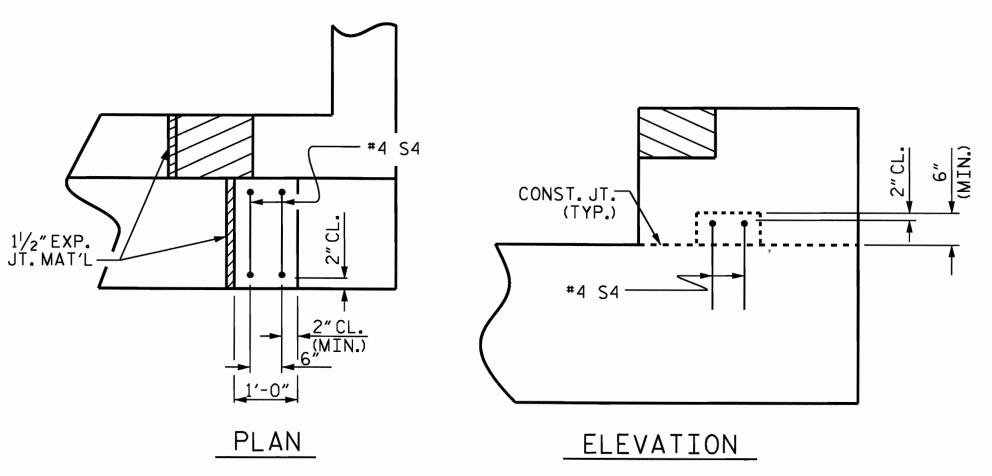
TO PROJECT 9" ABOVE CAP

(TYP.)

- FILL FACE

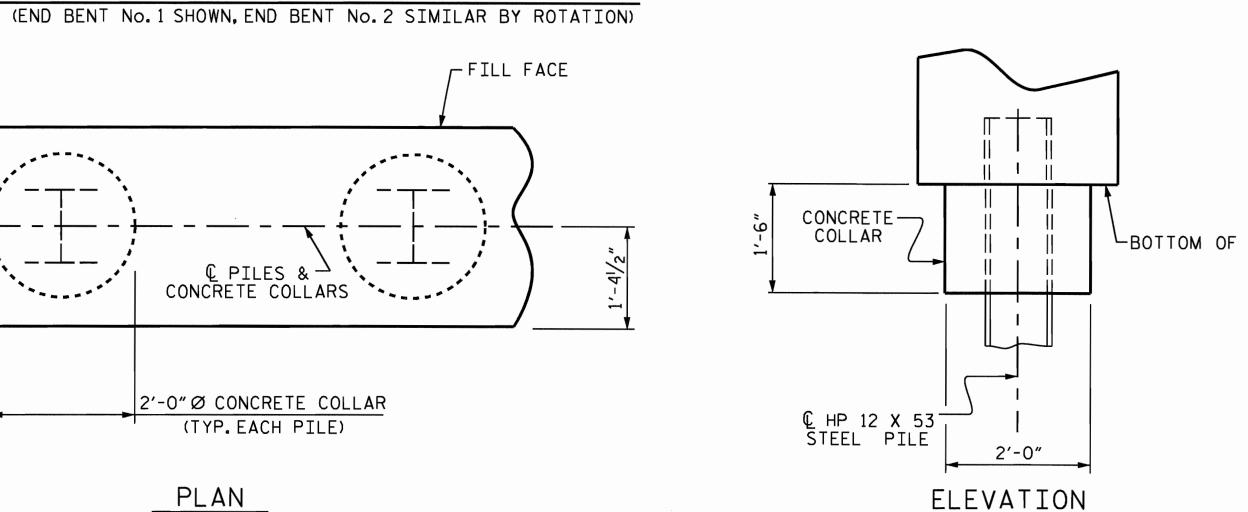






LATERAL GUIDE DETAILS

(RIGHT LATERAL GUIDE SHOWN ONLY)



CORROSION PROTECTION FOR STEEL PILES DETAIL

ASSEMBLED BY: A.C. OUTLAW DATE: 6/12/12 CHECKED BY: K. W. ALFORD DATE: 6/12 DRAWN BY: DGE 02/10 CHECKED BY: MKT 02/10

♠ BEARING —

1"X 8"X 2'-6" ——/
ELASTOMERIC BRG.
PAD (TYPE I)(TYP.)

BOTTOM OF CAP ELEVATION

> SECTION A-A CONCRETE COLLAR NOT SHOWN FOR CLARITY.
> SEE "CORROSION PROTECTION FOR STEEL PILES DETAIL."

-€ #6 D1 DOWEL 1'-71/2" FILL FACE 2" CL. −#4 S2 გ 4-#9 `B' -4-#4 B5 @ 4" CTS. OVER PILES #4 B5 (EA.FACE) #4 S1 — #4 B5 (EA. FACE) 2-#9 'B' 2" CL. (TYP.) (2-#9 'B' © HP 12 X 53 -STEEL PILE — 3" HIGH B.B. 1'-41/2" 1'-41/2" 2'-9''

	F	OR	END) BE	NT N	0.2
HK. (1) HK. $4^{1/2}$ 2'-5" $4^{1/2}$ "	BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
HK. HK.	B1	2	#9	1	41'-0"	279
11, 3" 39, 6" 11, 3" D1	B2	2	#9	1	41'-6"	282
1'-3" 38'-6" 1'-3" B1 1'-3" 39'-0" 1'-3" B2 HK. (4)	B3	2	#9	1	42'-0"	286
	B4	2	#9	1	42′-6″	289
1'-3" 39'-6" 1'-3" B3 1'-3" 40'-0" 1'-3" B4	B5	16	#4	STR	20′-7″	220
	B6	10	#4	STR	2′-5″	16
4,,				<u> </u>		
<u>4"</u> ► -	D1	22	#6	STR	1'-6"	50
7 / LAP		10			7. 10.	
	H1	12	#4	2	7′-10″	63
	H2	6	#4	7	8'-3"	33
H2 7'-7"	H3	6	#4	'	7′-10″	31
	K1	6	#4	STR	2'-11"	12
H3 7'-2" (5)	K2	3	#4	STR	3'-7"	7
	K3	3	#4	STR	3'-11"	8
	S1	50	#4	3	7′-5″	248
± 1′-8″Ø	S2	50	#4	4	3'-2"	106
7′-2″	S3 _.	14	#4	5	6′-6″	61
	S4	4	#4	6	4′-5″	12
1′-5″						
	V1	48	#4	STR	4'-11"	158
			NG STE			2161 LBS.
) i	CLASS	S A CO	DNCRET	E BREA	KDOWN	
	POUR	#1 C	AP, LOV	VER PA	.RT	12.6 C.Y.
2'-5"		0	F WINC	SS & (COLLARS	
ALL DAD DIMENSTONS ADE QUIT TO QUIT	DOLID.	#0 11	חחרה ה	ADT 0	· –	2167
ALL BAR DIMENSIONS ARE OUT TO OUT.	POUR		PPER F 'INGS	AKI U) 	2.1 C.Y.
	D0110				\	0104
HP 12 X 53 STEEL PILES STEEL PILE POINTS	POUR	#3 L	.ATERAI	L GUIL)ES	0.1 C.Y.
NO: 7 LIN. FT.= 70 NO: 7 EACH	TOTAL	_ CLAS	SS A C	ONCRE	TE	14.8 C.Y.

BAR TYPES

PROJECT NO. BD-51050 FRANKLIN _ COUNTY STATION: 13+30.00 -L-

BILL OF MATERIAL

SHEET 3 OF 3

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

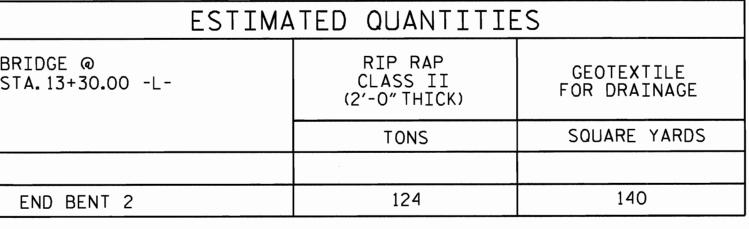
SUBSTRUCTURE

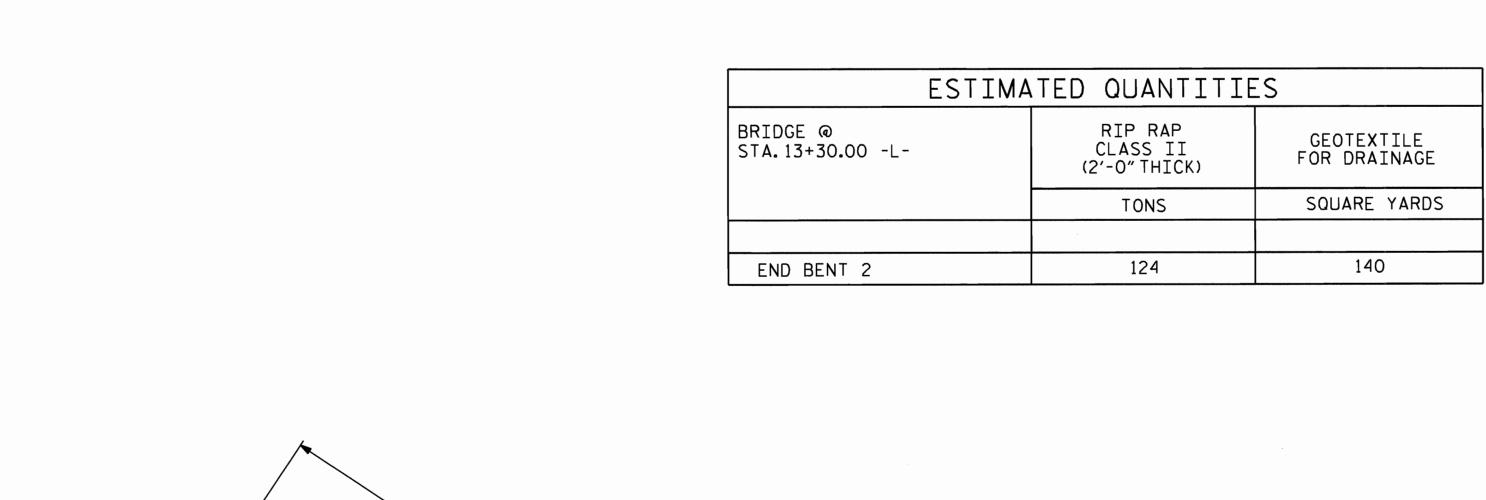
END BENT No. 2 DETAILS

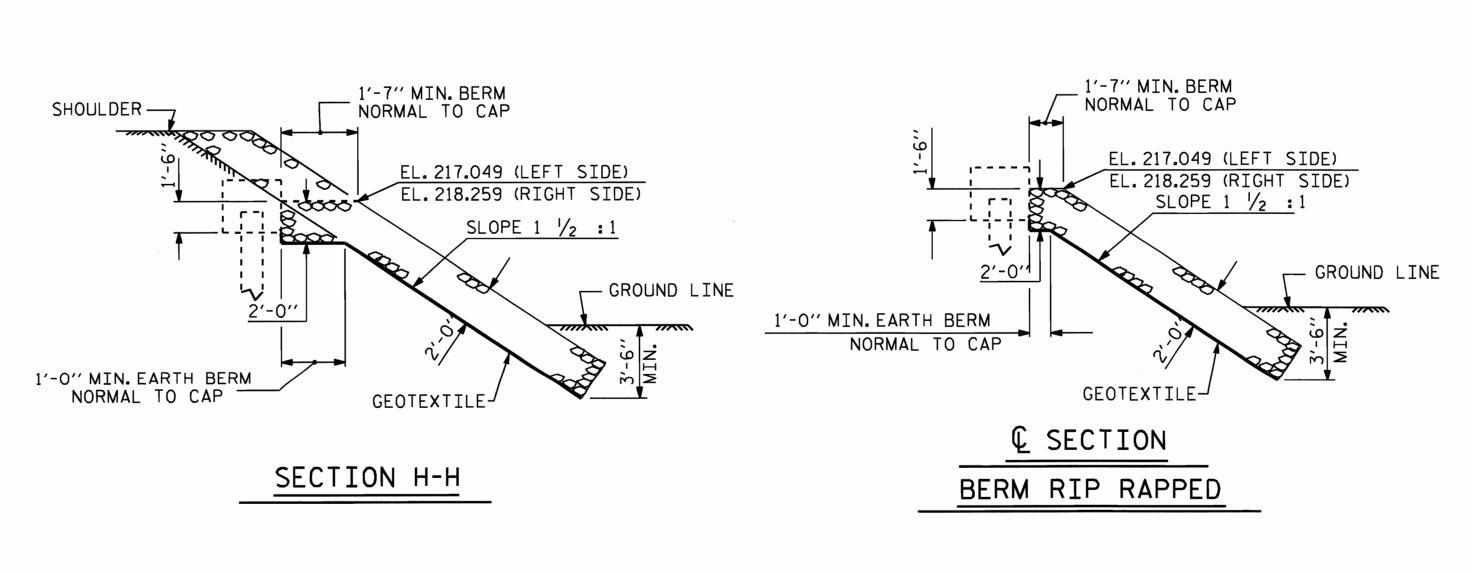
		SHEET NO.				
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2			4			17



FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.







EL. 215.049

1'-0" MIN. EARTH BERM

EL. 216.259

NORMAL TO CAP

EL. VARIES SHOULDER -SLOPE 2: 1 --- GROUND LINE GEOTEXTILE-

SECTION C-C

SHOULDER LINE

EXTENDED TANGENT

---- SHOULDER LINE

FRONT SLOPE LINE

LC

END BENT NO. 2 10'-0"

END BENT NO. 2

PROJECT NO. BD-51050 FRANKLIN COUNTY STATION: 13+30.00 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
RALEIGH

STANDARD -RIP RAP DETAILS-

SHEET NO. REVISIONS S-15 DATE: DATE: NO. BY: TOTAL SHEETS 17

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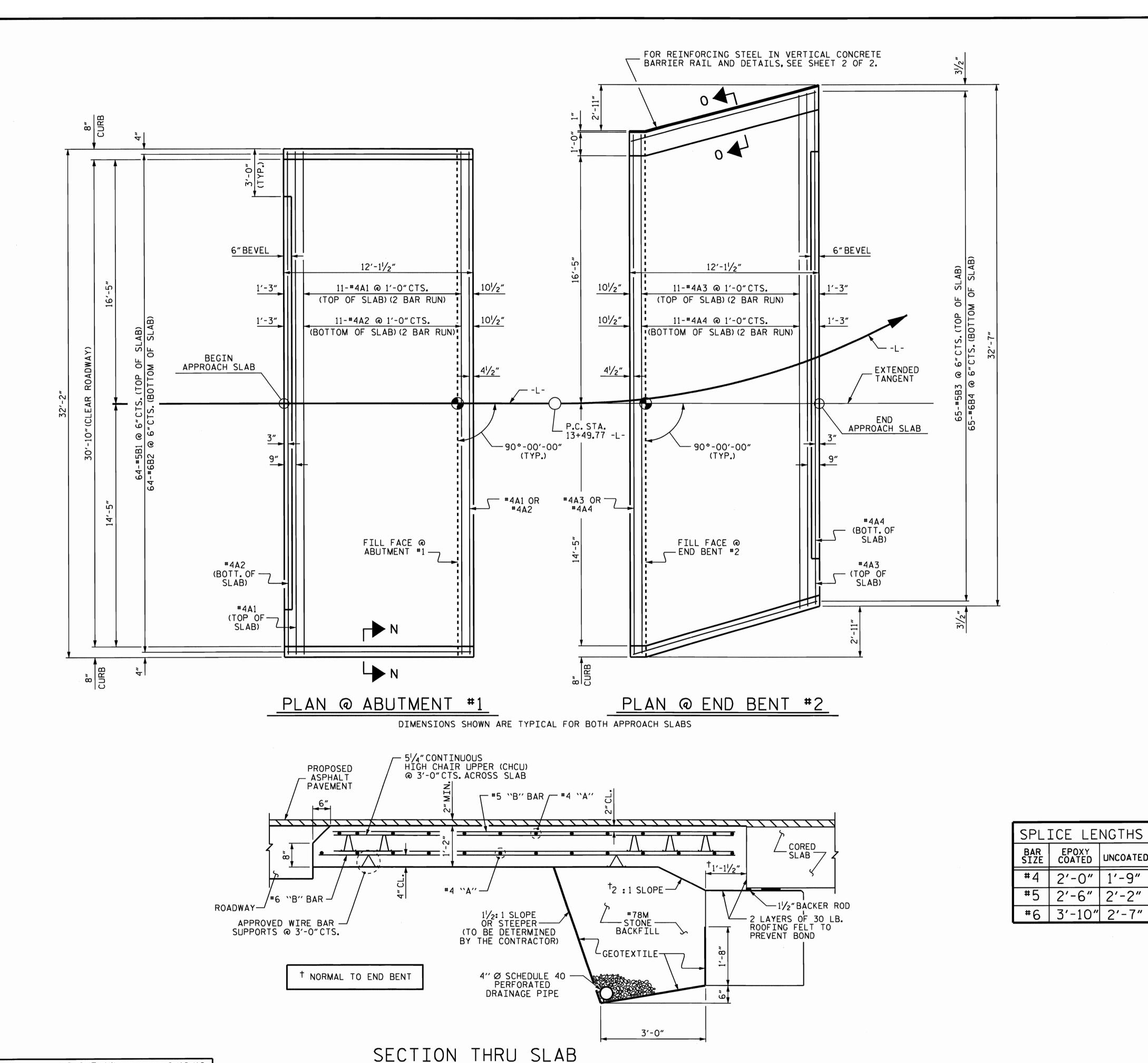
ASSEMBLED BY: A.C. OUTLAW DATE: 2/29/12 CHECKED BY: K.W. ALFORD DATE: 3/12

RWW/LES RWW/LES TLA/GM

DRAWN BY: FCJ 2/88 REV. 8/16/99 REV. 10/17/00 REV. 5/1/06R

SKEW 90°

STD. NO. RR2



BILL OF MATERIAL APPROACH SLAB AT ABUT. #1 BAR NO. SIZE TYPE LENGTH WEIGHT * A1 26 #4 STR 16'-11" 291 A2 | 26 | #4 | STR | 16'-9" *B1 | 64 | #5 | STR | 11'-2" 1121 B2 | 64 | #6 | STR | 11'-8" REINFORCING STEEL LBS. 1412 * EPOXY COATED REINFORCING STEEL 1039 LBS. CLASS AA CONCRETE C. Y. APPROACH SLAB AT EB BAR NO. SIZE TYPE LENGTH WEIGHT * A3 | 26 | #4 | STR | 17'-2" A4 26 #4 STR 17'-0" 295 *B3 | 65 | *5 | STR | 785 11'-7" B4 | 65 | #6 | STR | 12′-1″ 1180 B5 10 #5 STR 127 12'-2" 86 #5 * S1 | 13 | 6′-4″ ALL BAR DIMENSIONS ARE OUT TO OUT. * S2 | 13 | #5 | 2 | 6′-8″ 90 1602 REINFORCING STEEL LBS. * EPOXY COATED REINFORCING STEEL 1259 LBS. CLASS AA CONCRETE C.Y. 20.3 VERTICAL CONCRETE BARRIER RAIL 12.53 LIN. FT 1.6 C. Y CLASS AA CONCRETE

NOTES

FOR BRIDGE APPROACH FILL INCLUDING GEOTEXTILE, 4" Ø DRAINAGE PIPE, AND #78M STONE BACKFILL, SEE ROADWAY PLANS.

GEOTEXTILE SHALL BE TYPE 1 IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS SECTION 1056.

#78M STONE BACKFILL (CLASS V SELECT MATERIAL) SHALL BE IN ACCORDANCE WITH STANDARD SPECIFICATIONS SECTION 1016.

#78M STONE BACKFILL IS TO BE CONTINUOUS ALONG FILL FACE OF BACKWALL FROM OUTSIDE EDGE TO OUTSIDE EDGE OF APPROACH SLAB.

FOR THE 4" Ø DRAINAGE PIPE OUTLET(S), SEE ROADWAY STANDARD DRAWINGS.

AREA BETWEEN THE WINGWALL AND APPROACH SLAB SHALL BE GRADED TO DRAIN THE WATER AWAY FROM THE FILL FACE OF THE BRIDGE AND SHALL BE PAVED. SEE ROADWAY PLANS.

APPROACH SLAB GROOVING IS NOT REQUIRED.

PROJECT NO. BD-5105Q FRANKLIN _ COUNTY 13+30.00 -L-

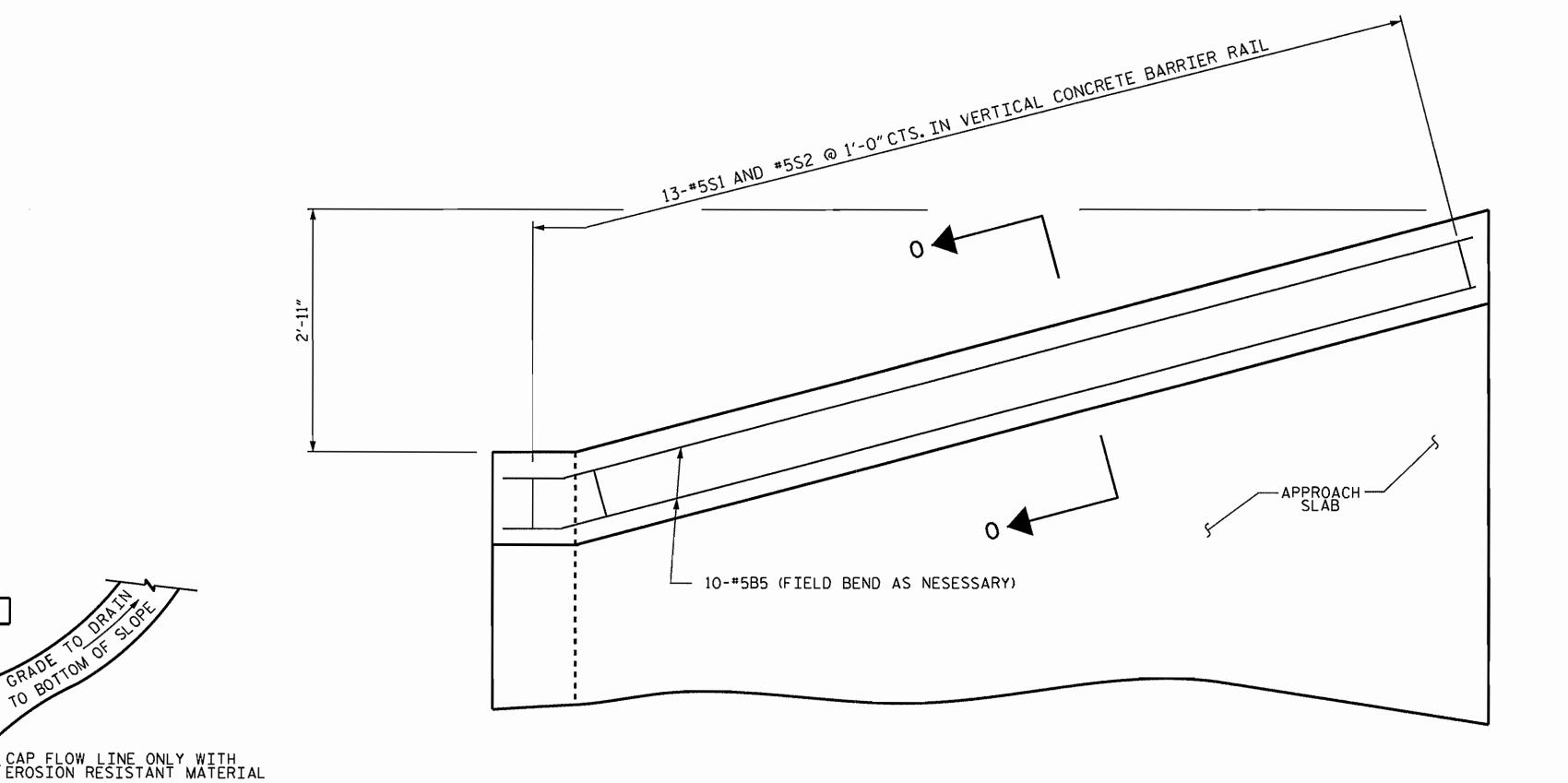
SHEET 1 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD BRIDGE APPROACH SLAB FOR PRESTRESSED CONCRETE CORED SLAB UNIT (SUB-REGIONAL TIER) 90° SKEW

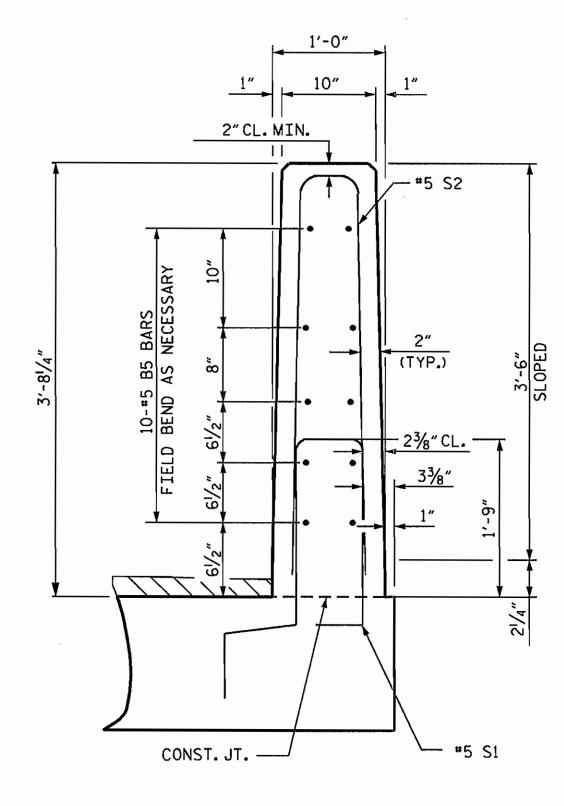
SHEET NO. REVISIONS S-16 DATE: DATE: NO. BY: BY: TOTAL SHEETS

DRAWN BY: A.C. OUTLAW DATE: 6/12/12

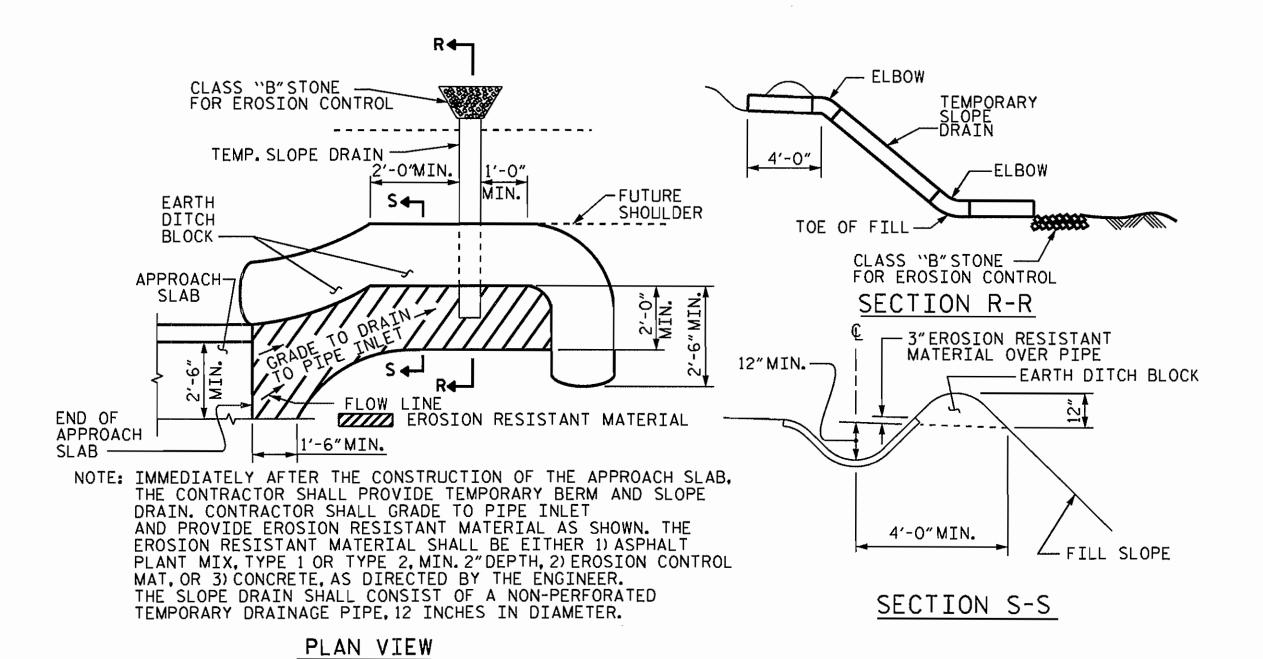
CHECKED BY : K. W. ALFORD DATE : 6/12



PARTIAL PLAN



SECTION 0-0



NOTE: IF THE APPROACH SLAB IS NOT CONSTRUCTED IMMEDIATELY AFTER THE BACKFILLING OF THE END BENT EXCAVATION, GRADE TO DRAIN TO THE BOTTOM OF THE SLOPE AND PROVIDE

TEMPORARY DRAINAGE DETAIL

EROSION RESISTANT MATERIAL, SUCH AS FIBERGLASS ROVING OR AS DIRECTED BY THE ENGINEER TO PREVENT SOIL EROSION AND TO PROTECT THE AREA ADJACENT TO THE STRUCTURE. THE CONTRACTOR WILL BE REQUIRED TO REMOVE THESE MATERIALS PRIOR TO CONSTRUCTION OF THE APPROACH SLAB.

SECTION N-N

END OF CURB WITHOUT SHOULDER BERM GUTTER

CURB DETAILS

PROJECT NO. BD-5105Q

FRANKLIN COUNTY

STATION: 13+30.00 -L-

SHEET 2 OF 2

DEPARTMENT OF TRANSPORTATION

STANDARD

BRIDGE APPROACH SLAB

FOR PRESTRESSED CONCRETE

CORED SLAB UNIT

(SUB-REGIONAL TIER)

PREVISIONS

REVISIONS

SHEET NO.

BY: DATE: NO. BY: DATE: S-17

TOTAL SHEETS

17

SEAC (1/4) 029441 029441 029441 029441 029441 029441 029441 029441 029441 029441 029441 029441

TEMPORARY BERM AND SLOPE DRAIN DETAILS

(TO BE USED WHEN SHOULDER BERM GUTTER IS REQUIRED)

DRAWN BY: A.C. OUTLAW DATE: 6/12/12
CHECKED BY: K.W. ALFORD DATE: 6/12

BRIDGE DECK

STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS ---- SEE PLANS LIVE LOAD IMPACT ALLOWANCE ---- SEE A.A.S.H.T.O. STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27.000 LBS. PER SQ. IN. REINFORCING STEEL IN TENSION GRADE 60 - - 24,000 LBS. PER SQ. IN. 1.200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION ---- SEE A.A.S.H.T.O. CONCRETE IN SHEAR STRUCTURAL TIMBER - TREATED OR ---- 1,800 LBS. PER SQ. IN. UNTREATED - EXTREME FIBER STRESS

MATERIAL AND WORKMANSHIP:

COMPRESSION PERPENDICULAR TO GRAIN

EQUIVALENT FLUID PRESSURE OF EARTH

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS. ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

OF TIMBER - - - -

375 LBS. PER SQ. IN.

(MINIMUM)

30 LBS. PER CU. FT.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, TC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN

ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER. DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE

INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS.AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE. THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16"IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE".

ELECTROSLAG WELDING WILL NOT BE PERMITTED.
WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING. GALVANIZING. OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990