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PROIECT LENGTH	Y PLANS PREPAREI	D FOR THE NCDO
),100 STV Engineers,
NGTH OF ROADWAY PROJECT WBS 17BP.9.R.81 = 0.072 MILES		NC License Number
NGTH OF STRUCTURE PROJECT WBS 17BP.9.R.81 = 0.017 MILES	2018 STANDARD SPECIFICATIONS	
TAL LENGTH OF PROJECT WBS 17BP.9.R.81 = 0.089 MILES	RIGHT OF WAY DATE.	
	SEPTEMBER 19, 2018	PROJECT EN
NCDOT CONTACT: <u>DANIEL DAGENHART</u> Division Bridge Manager	- LETTING DATE: APRIL 10, 2019	MAAMOON K. project de

INDEX OF SHEETS

SHEET NUMBER	SHEET	G
1	TITLE SHEET	
1 A	INDEX OF SHEETS, GENERAL NOTES, AND LIST OF STANDARD DRAWINGS	G
1B	CONVENTIONAL SYMBOLS	G
2A-1	TYPICAL SECTIONS SHEET	
3B-1	EARTHWORK, DRAINAGE SUMMARY, AND GUARDRAIL SUMMARY SHEET	
4	PLAN AND PROFILE SHEET	
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PMP-1	PAVEMENT MARKING PLAN	
EC-1 THRU EC-5	EROSION CONTROL PLANS	
UO-1 THRU UO-2	UTILITY PLANS	
X-1 THRU X-5	CROSS-SECTIONS	S
S-1 THRU S-15	STRUCTURE PLANS	

GENERAL NOTES

GENERAL NOTES:

2018 SPECIFICATIONS EFFECTIVE: 01-01-2018

GRADE LINE:

GRADING AND SURFACING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY CONTRACT.

STD.NO.

DIVISION	2 — EAR
200.02	Method
225.02	Guide
225.04	Method
DIVISION	4 - MAJ
422.02	Bridge
DIVISION	5 – SUB
560.01	Method
DIVISION	8 – INC
840.29	Frames
840.35	Traffi
862.01	Guardr
862.02	Guardr
876.02	Guide
DIVISION	11 - WO

101.03	Tempor
110.01	Static
145.01	Barrio



BOUNDARIES AND PROPERTY:

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State Line	
County Line	
Township Line	
City Line	
Reservation Line	
Property Line	
Existing Iron Pin	()
Computed Property Corner	×
Property Monument	 ECM
Parcel/Sequence Number	— (23)
Existing Fence Line	
Proposed Woven Wire Fence	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary	WLB
Proposed Wetland Boundary	
Existing Endangered Animal Boundary	EAB
Existing Endangered Plant Boundary	EPB
Existing Historic Property Boundary	нрв ———
Known Contamination Area: Soil	— - 🗽 — s — 🕵 -
Potential Contamination Area: Soil	— - <u>)</u> — s — <u>)</u>
Known Contamination Area: Water	X W X
Potential Contamination Area: Water	— – <u>)?</u> — w — <u>)</u> ? -
Contaminated Site: Known or Potential —	- 3. 3.
BUILDINGS AND OTHER CULT	URE:
Gas Pump Vent or U/G Tank Cap	— O
Sign	
Well	Q
Small Mine	
Equipartian	
Area Outline	
Cemetery	+ _]
Building	
School	
Church	
Stream or Body of Water	
Hydro Pool or Reservoir	
Iurisdictional Stream	
Buffer Zone 1	–JS
Buffer Zone 2	BZ 1
Flow Arrow	UL 1
Disappearing Stream	->
Spring	-0
Wetland	- ¥
Proposed Lateral, Tail, Head Ditch	$\rightarrow\rightarrow\rightarrow\rightarrow\rightarrow$
False Sump	- FLOW
I.	



Standard Ga RR Signal Mi Switch —— RR Abandon **RR** Dismantled

Secondary Primary Hor Primary Hor Exist Permar New Perme Vertical Ben **Existing** Rig **Existing** Rig New Right New Right New Right Concrete New Contr Concrete Existing Co New Contr **Existing** Eas New Temp New Tempc New Permo New Permo New Permo New Temp New Aerial

Existing Edg Existing Cu Proposed S Proposed SI Proposed C Existing Met Proposed G Existing Ca Proposed C Equality Syr Pavement R VEGETA Single Tree Single Shru

STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

CONVENTIONAL PLAN SHEET SYMBOLS Note: Not to Scale

***S.U.E. = Subsurface Utility Engineering**

auge	CSX TRANSPORTATION	Hedge	
ilepost	 MILEPOST 35	Woods Line	
		Orchard	
ied	SWIICH	Vineyard	

RIGHT OF WAY & PROJECT CONTROL:

Horiz and Vert Control Point ——	•	Bri
oriz Control Point		MIN
oriz and Vert Control Point	۲	He
inent Easment Pin and Cap ———	$\langle \cdot \rangle$	Pip
anent Easement Pin and Cap ——	$\langle \! \! \diamond \! \! \rangle$	Fo
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ght of Way Marker	\bigtriangleup	Pa
ght of Way Line		Ste
of Way Line		Ste
of Way Line with Pin and Cap—		U
of Way Line with e or Granite R/W Marker		PO Fx
rol of Access Line with e C/A Marker		Pro
ontrol of Access		Ex
rol of Access		Pro
sement Line	——E——	Ро
oorary Construction Easement –	E	Ро
oorary Drainage Easement	TDE	Ро
anent Drainage Easement	PDE	U⁄
anent Drainage / Utility Easement	DUE	H-
anent Utility Easement	PUE	U⁄
oorary Utility Easement	TUE	U⁄
I Utility Easement	AUE	U⁄

ROADS AND RELATED FEATURES:

ge of Pavement	
urb	
Slope Stakes Cut	<u>C</u>
Slope Stakes Fill	F
Curb Ramp ————	CR
etal Guardrail ————	TT
Guardrail ————	<u> </u>
ıble Guiderail ————	<u> </u>
Cable Guiderail	
mbol ———	\bullet
Removal	$\times\!\!\times\!\!\times\!\!\times\!\!\times$
TION:	
e ————	- &:
np ————————————————————————————————————	- ¢3

Hedge	 ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~			
Woods Line	 	$\therefore _ \ddots]$	ٮ_ڹٛٮ	_د```
Orchard	 순	순	භි	£
Vineyard		Viney	ard	

EXISTING STRUCTURES:

MAJOR:

Bridge Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall	- J CONC WW L
MINOR:	
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge	≻
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole	S
Storm Sewer	S

- UTILITIES:

6
- -
-6-
P
•—•
P P
P P P
P

TELEPHONE:

Existing Telephone Pole	
Proposed Telephone Pole	
Telephone Manhole T	
Telephone Pedestal 🔳	
Telephone Cell Tower	
U/G Telephone Cable Hand Hole	
U/G Telephone Cable LOS B (S.U.E.*)	
U/G Telephone Cable LOS C (S.U.E.*)	
U/G Telephone Cable LOS D (S.U.E.*)	
U/G Telephone Conduit LOS B (S.U.E.*)	
U/G Telephone Conduit LOS C (S.U.E.*)	
U/G Telephone Conduit LOS D (S.U.E.*)	
U/G Fiber Optics Cable LOS B (S.U.E.*)	
U/G Fiber Optics Cable LOS C (S.U.E.*) — τ FO	
U/G Fiber Optics Cable LOS D (S.U.E.*)	

	17BP .9. R.81	
WATER:		
Water Manhole	W	
Water Meter	— O	
Water Valve	── ⊗	
Water Hydrant		
U/G Water Line LOS B (S.U.E*)	w -	
U/G Water Line LOS C (S.U.E*)		
U/G Water Line LOS D (S.U.E*)	w-	
Above Ground Water Line	A/G Wc	iter
TV:		
IV Pedestal		
TV Tower	— 🛛	
U/G TV Cable Hand Hole	[H _H]	
U/G TV Cable LOS B (S.U.E.*)	——————————————————————————————————————	
U/G TV Cable LOS C (S.U.E.*)		
U/G TV Cable LOS D (S.U.E.*)	TV-	
U/G Fiber Optic Cable LOS B (S.U.E.*) —	——— — — — TV F)— —
U/G Fiber Optic Cable LOS C (S.U.E.*) —	TV F)— —
U/G Fiber Optic Cable LOS D (S.U.E.*)	TV F	0
GAS:		
Gas Valve	♦	
Gas Meter	♦	
U/G Gas Line LOS B (S.U.E.*)		
U/G Gas Line LOS C (S.U.E.*)		
U/G Gas Line LOS D (S.U.E.*)	GGGG	16
Above Ground Gas Line		10
SANITARY SEWER:		
Sanitary Sewer Manhole		
Sanitary Sewer Cleanout	(±)	
U/G Sanitary Sewer Line	SSSS-	
Above Ground Sanitary Sewer	A/G Sanitary	/ Sew
SS Forced Main Line LOS B (S.U.E.*) —	— — — — FSS	
SS Forced Main Line LOS C (S.U.E.*) —	—— — — FSS	
SS Forced Main Line LOS D (S.U.E.*) —	FSS	
MISCELLANEOUS:		
Utility Pole	- •	
Utility Pole with Base	·	
Utility Located Object	•	
Utility Traffic Signal Box	[S]	
Utility Unknown U/G Line LOS B (S.U.E.*)		
U/G Tank; Water, Gas, Oil		
Underground Storage Tank, Approx. Loc. —	(<u>UST</u>)
A/G Tank; Water, Gas, Oil		
Geoenvironmental Boring]
U/G Test Hole LOS A (S.U.E.*)	— (
Abandoned According to Utility Records —	— AATI	JR
End of Information	E C	

	PAVEMENT SCHEDULE
C1	PROP. APPROX. 3.0" ASPHALT CONCRETE SURFACE COURSE TYPE S9.5B, AT AN AVERAGE RATE OF 165 LBS. PER SQ. IN EACH OF TWO LAYERS.
C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE S9.5B, AT AN AVERAGE RATE OF 110 LBS. PER SQ. PER 1.0" DEPTH. TO BE PLACED IN LAYERS NOT LESS TH IN DEPTH OR GREATER THAN 2.0" IN DEPTH.
D1	PROP. APPROX. 4.0" ASPHALT CONCRETE INTERMEDIATE C TYPE I19.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ
D2	PROP. VAR. DEPTH ASPHALT CONCRETE INTERMEDIATE COU TYPE I19.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ PER 1.0" DEPTH. TO BE PLACED IN LAYERS NOT LESS TH IN DEPTH OR GREATER THAN 4.0" IN DEPTH.
E1	PROP. APPROX. 4.0" ASPHALT CONCRETE BASE COURSE, TYPE B25.0C, AT AN AVERAGE RATE OF 456 LBS. PER SQ
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYP B25.0C, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. 1.0" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3 DEPTH OR GREATER THAN 5.5" IN DEPTH.
R	CONCRETE SHOULDER BERM GUTTER
Т	EARTH MATERIAL
U	EXISTING PAVEMENT
W	PAVEMENT WEDGING

ALL PAVEMENT SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.











TYPICAL SECTION 2 -L- STA. 13+61.33 TO 14+48.67

EARTHWORK SUMMARY (IN CUBIC YARDS)

CHAIN	FROM STATION	TO STATION	SIDE	UNCL. EXCAVATION	UNDERCUT	EMBT+%	BORROW	WASTE
L	12+00.00	13+61.33	LT & RT	19		236	217	
-L-	14+48.67	16+70.00	LT & RT	13		223	210	
	-							
TOTAL				32		459	427	
LOSS DUE	E TO CLEARING	AND GRUBBING						
WASTE IN	LIEU OF BORRO	W						
PROJECT	TOTAL			32		459	427	
ESTIMATE	5% FOR TOPSO	L ON BORROW	PITS				21	
GRAND T	OTAL			32		459	448	
SAY				35			450	

NOTE: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, Breaking of Existing Pavement, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading."

STATION	LOCATION (LT,RT, OR CL)	STRUCTURE NO.	TOP ELEVATION	INVERT ELEVATION	INVERT ELEVATION	SLOPE CRITICAL	AI 12″ 15″ 18	TERNATE PIPE	6" 42" 48"	B 12″ 15″ 1	ITUMINOUS (UNLESS 8″ 24″	S COATED C S NOTED OT	.S. PIPE TYP THERWISE) 36″	PE B 42″ 4	ALU/ H	CLASS MINIZED IDPE PIPE	5 III R.C. PIPE OR O C.S. PIPE, T OR PE, TYPE S OF	YPE IR R D 42" 48"	N PIPE	N PIPE	ENDW STD. 8 STD. 8 STD. 8 (UNI NO OTHER	/ALLS 38.01, 338.11 R 338.80 LESS TED RWISE) YDS.	THRU 5.0') FOR DRAINAGE FOR DRAINAGE STRUCTURES * TOTAL L.F. FOR PAY ALANTITY SHALL RF COL	OVE B	FRAME, GRATES AND HOOD STANDARD 840.	S .03	40.14 OR STD. 840.15 AND GRATE STD. 840.16	"A" STD. 840.17 OR 840.26 "B" STD. 840.18 OR 840.27	"D" STD. 840.19 OR 840.28 EARING G.D.I. STD. 840.35	AE WITH GRATE STD. 840.20 AE WITH TWO GRATES STD. 840.22) FRAME WITH GRATE STD. 840.29) FRAME WITH TWO GRATES STD. 840.24	10.31 OR 840.32	. ELBOWS NO. & SIZE LARS CL. "B" C.Y. STD 840.72 RICK PIPE PLUG, C.Y. STD. 840.71	al Lin.ft.	ABBREVIATIONS .B. CATCH BASIN I.D.I. NARROW DROP INLET J.I. DROP INLET J.D.I. GRATED DROP INLET J.D.I. (N.S.) GRATED DROP INLET (NARROW SLOT) I.B. JUNCTION BOX M.H. MANHOLE T.B.D.I. TRAFFIC BEARING DROP II	NLET
THICKNESS OR GAUGE		MOX 01																	DE DRA		C.P.	.S.P.	CH (0, IRU 10.	ND AB	TYPE OF GRAT	E	STD. 84 FRAME	.I. TYPE	I. TYPE	.I. FRAN	.I. (N.S.	STD. 84	R. STEEL	L REMOV	.B.J.B. TRAFFIC BEARING JUNCTIC	on box
		Ŧ																	15" SIC	18″ SIC	R 8	0	PER EA 5.0' TH	10.0' A C.B. ST	E F G		D.I. D.I.	G.D G.D	G.D TRA	G.D G.D	0. 0. 0.	J.B.	CON COR	PIPE	REMARKS	
-L- STA. 13+40	RT 4	01 402	2 708.50	704.75	701.45	5 0.062	28′																1						1		1		2			
–L– STA. 15+00	RT 4	03		705.59	704.8	3 0.022	36′								36′																			22′		
TOTAL							64′								36′								1						1		1		2	22′		

* W MEASURED FROM "N" AT THE BEGINNING OF THE ANCHOR TO "N" AT THE END OF THE ANCHOR. "N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE 350

SURVEY	SURVEY BEG. STA. END S		LOCATION		LENGTH	WARRANT POINT		"N" DIST. FROM	TOTAL	FLARE	LENGTH	v	/ *				ANCHORS			
LINE	BEG. STA.	END STA.	LOCATION	STRAIGHT	TRAIGHT SHOP CURVED	APPROACH END	TRAILING END	FROM E.O.L.	SHOUL. WIDTH	APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD	B-77	GREU TL–3	GREU TL-2	TYPE III	CAT-1	VI MOD
L	13+09.04	13+65.87	RT	56.25		13+65.87		7.5′	10.5′		25		0.5				1	1		
-L-	13+01.23	13+56.89	LT	56.25			13+56.89	7.5′ – 8.6′	11.6′	25		0.5					1	1		
-L-	14 + 54.10	14+94.79	RT	25.00	25.00		14+54.10	7.3′	10.3′									1		
L_	14+43.34	14+99.02	LT	56.25		14+43.34		7.3′ – 8.3′	11.3′		25		0.5				1	1		
			TOTAL:	193.75	25.00															
		TOTAL ANC	HOR LENGTH:	156.25																
		TOTAL GUARE	RAIL LENGTH:	37.50																
			SAY:	37.50 LF	25.00 LF												3	4		

3/6/2019

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

) A I U M
THE LOCALIZED	COORDINAT
IS BASED UN	NCDOT FOR
WITH NAD 83	3/NSRS 201′
NORTHING:	685951.321
	ELEVA
THE AVERAGE	E COMBINED
	(GROUND TO
Т	HE N.C. L
LOCAL	_IZED HORIZ
<i>"</i> 79-	-0170-2″T(
	N 42°15
ALL LINEAR D	IMENSIONS
	VERTICAL

BL-2 N 683931.3213 E 1367242.3190 ELEV 708.80 BL-3 N 685602.6172 E 1567476.3500 ELEV 728.11 BM-1 N 686265.67 E 1566879.88 ELEV 705.46 BM-2 N 686037.01 E 1567302.20 ELEV 704.40	
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LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

GUARDRAIL SUMMARY

PROJECT REFERENCE NO. SHEET NO. 3B-I STV Engineers, Inc. 900 West Trade St., Suite 71 Charlotte, NC 28202 NC License Number F-0991

17BP.9.R.81

DATUM DESCRIPTION

TE SYSTEM DEVELOPED FOR THIS PROJECT PLANE COORDINATES ESTABLISHED BY MONUMENT "79-0170-2" 11 STATE PLANE GRID COORDINATES OF 15(ft) EASTING: 1567242.9190(ft) ATION: 708.803(f+) GRID FACTOR USED ON THIS PROJECT GRID) IS: 0.999864818 LAMBERT GRID BEARING AND IZONTAL GROUND DISTANCE FROM TO -L- STATION 12+00.00 IS 15'59" W 262.58(ft) ARE LOCALIZED HORIZONTAL DISTANCES DATUM USED IS NAVD 88

BIC	AT–1	IMPACT ATTENUATOR TYPE 350 EA G NG		T TOR 50 NG	SINGLE FACED GUARDRAIL	REMOVE EXISTING GUARDRAIL	REMOVE AND STOCKPILE EXISTING GUARDRAIL	REMARKS
	1							
	1							





SIGN M	NUMBER	: SP-1			BACK	G COL	.OR :	Orang	е		
	TYPE	STAT	IONAR	Y	COP	COLC) R :	Black			
QUA	NTITY:	8		S	SYMBOL	•		X	Υ	WID	
SIGN	WIDTH:	36″									
н	EIGHT	12″									
TOTAL	AREA:	3.0	Sq.F1	t.							_
BORDER	TYPE:	INSE	ET								_
R	ECESS:	0.47	7″								+
	WIDTH:	0.63	3″								+
		113									<u> </u>
NO. Z	BARS:			MA	AT'L:	0.080	(2.	O mm)	ALU	MINU	Jľ
L											
1.Lege lens 2.Back	USE NG end and s refle (ground	DTES: d bord ective d shal	1,2 ler shee ll be	nall etin NC	be d g. Grade	irect B fl	appl	ied e cent.	ncaps	sula	ιt
1. Lege lens 2. Back	USE NG end and s refle (ground ER POS	DTES: d bord ective d shal	1,2 ler shee ll be	nall etin NC	be d g. Grade	irect B fl	appl. uores	ied e cent.	ncaps	sula	. t
1. Lege lens 2. Back	USE NG end and s refle (ground ER POS	DTES: d bord ective d shal	1,2 ler shee ll be	nall etin NC	be d g. Grade	irect B fl	appl.	ied e cent. Le	ncaps ette	sula	
1. Lege lens 2. Back	USE NG end and s reflector (ground ER POS	DTES: d bord ective d shal	1,2 ler shee ll be	A	be d g. Grade	irect B fl	appl. uores	ied e cent. Le	ncaps ette	sula P r	
1. Lege lens 2. Back	USE NG end and s reflection (ground ER POS) N 5.6	DTES: d bord ective d shal ITIONS	1,2 ler shee ll be M 1.8	A	be d g. Grade I 18.1	irect B fl N 19.6	appl uores	ied e cent. Le S 25.8	ncaps ette T 28.4	sula pr	
1. Lege lens 2. Back	USE NG end and s reflection (ground ER POS N 5.6	DTES: d bord ective d shal ITIONS	1,2 ler shee ll be	A 15	be d g. Grade	irect B fl N 19.6	appl uores 21.8	ied e cent. Le S 25.8	ncaps ette T 28.4	sula pr	
1. Lege lens 2. Back	USE NG end and s reflection (ground ER POS) N 5.6	DTES: d bord ective d shal ITIONS	1,2 ler shee shee l be	A 15	be d g. Grade	irect B fl N 19.6	appl uores 21.8	ied e cent. Le S 25.8	ncaps ette T 28.4	sula pr	p
1. Lege lens 2. Back	USE NG end and s reflection (ground ER POS 5.6	DTES: d bord ective d shal ITIONS	1,2 ler shee shee 1 be	A 15	be d g. Grade	irect B fl N 19.6	appl uores 21.8	ied e cent. Le S 25.8	ncaps ette T 28.4	sula Pr	p
1. Lege lens 2. Back	USE NG end and s reflection (ground ER POS 5.6	DTES: d bord ective d shal ITIONS	1,2 ler shee shee 1 be	A 15	be d g. Grade	irect B fl N 19.6	appl. uores 21.8	ied e cent. Le S 25.8	ncaps ette T 28.4	sula pr	p
1. Lege lens 2. Back	USE NG end and s refle ground ER POS 5.6	DTES: d bord ective d shal ITIONS	1,2 ler shee ll be M 1.8	A 15	be d g. Grade	irect B fl N 19.6	appl. uores 21.8	ied e cent. Le S 25.8	ncaps ette T 28.4	sula pr	

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	gned by: Here	3/7/2019	OF HIC	
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TRAFFIC MANAGEMENT PLANS

	PROJ. RI	EFERENCE NO.	SHEET NO.
	17	RP.9.R.81	TMP-2
			/ Engineers, Inc.
		V 100 900 W Charle NC Li	Vest Trade St., Suite 715 otte, NC 28202
	L		
		BRIDGE	<i>#790/70</i>
ctober 25, 2018			
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Saries/Size			
Tavt Lanath			
C 2000 / 4			
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	DOCUM	ENT NOT CONSIDE	RED FINAL
	UNLESS	ALL SIGNATURES	COMPLETED
ала 3/7/2019 OF н,	Γ		
ATE: OF NORTH CALL			









TEMPORARY ROCK SILT CHECK TYPE 'A' WITH EXCELSIOR MATTING AND POLYACRYLAMIDE (PAM)





NOTES:

INSTALL TEMPORARY ROCK SILT CHECK TYPE A IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1633.01.

USE EXCELSIOR FOR MATTING MATERIAL AND ANCHOR MATTING SECTION AT TOP AND BOTTOM WITH CLASS B STONE.

PRIOR TO POLYACRYLAMIDE (PAM) APPLICATION, OBTAIN A SOIL SAMPLE FROM PROJECT LOCATION, AND FROM OFFSITE MATERIAL, AND ANALYZE FOR APPROPRIATE PAM FLOCCULANT TO BE APPLIED TO EACH ROCK SILT CHECK.

INITIALLY APPLY 4 OUNCES OF POLYACRYLAMIDE (PAM) TO TOP OF MATTING SECTION AND AFTER EVERY RAINFALL EVENT THAT EQUALS OR EXCEEDS 0.50 INCHES.



PROJECT REFERENCE NO.

17BP.9.R.81

R/W SHEET NO.

STV Engineers, Inc 900 West Trade St., Suite

SHEET NO





SILT FENCE COIR FIBER WATTLE BREAK DETAIL

NOTES:

LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

STANDARD SPECIFICATIONS.

INSET A





PROJECT REFERENCE NO.	SHEET NO.
17BP.9.R.81	EC-2A
R/W SHEET NO.	
STV 100 STV Jears STV Charlo NC Lic	Engineers, Inc. est Trade St., Suite 715 tte, NC 28202 ense Number F-0991

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE

SIDE VIEW

SOIL STABILIZATION SUMMARY SHEET

MATTING FOR EROSION CONTROL

CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)	CONST SHEET NO.	LINE	FROM STATION	TO STATION	SIDE	ESTIMATE (SY)
4	- L -	14+70	14+90	RT	10	4	-L- V-DITCH	15+10	16+50	RT	70
			GUE	ATOTAL	10				61)	BTOTAL	70
MISCELLANE	OUS MATTING TO BE IN	STALLED AS DIRE	CTED BY THE	ENGINEER	1000			ADDITIONAL	PSRM TO BE	NGTALLED	0
				TOTAL	1010					TOTAL	70
				SAY	1100					SAY	70

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

PERMANENT SOIL REINFORCEMENT MAT



R/W SHEET NO. DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA SOIL STABILIZATION TIMEFRAMES SITE DESCRIPTION MEFRAME EXCEPTIONS PERIMETER DIKES, SWALES, DITCHES AND SL HIGH QUALITY WATER (HQW) ZONES ARE IO' OR LESS IN LENGTH AND ARE SLOPES STEEPER THAN 3:1 EPER THAN 2:1, 14 DAYS ARE ALLOWED. OR SLOPES GREATER THAN 50' IN SLOPES 3:1 OR FLATTER ALL OTHER AREAS WITH SLOPES FLATTER CEPT FOR PERIMETERS AND HQW ZONES.

•

	STABILIZATION TIME	T/N
LOPES	7 DAYS	NONE
	7 DAYS	NONE
	7 DAYS	IF SLOPES Not steel
	14 DAYS	7 DAYS FO Length.
THAN 4:1	14 DAYS	NONE, EXCI





















		0 2.5 5	PROJ. REFERENCE N	O. SHEET NO.
			1/BP.9.R.8	<u>1 X-4</u>
40	45 50	55 6	65 7	'0 75
				720
				715
				710
				705
				7/5
				710
				/05
				700
				7/5
				710
				/10
•				
				705
				700
				695
40	45 50	55	65 7	'0 75

		0	2.5 5	PRO	J. REFERENCE BP • 9 • R •	NO.	sheet no. X-5
40	45	50	55	60	65	70	75
							725
							· •
							720
							7/5
							725
							720
							7/5
							710
							720
							7/5
							/10
40	45	50	55	60	65	70	75

+

DESIGN ENGINEER OF RECORD : LEM DATE : 9-18

GENERAL NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE "STANDARD NOTES" SHEET.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THE EXISTING STRUCTURE CONSISTING OF (1) 40'-8" SPAN WITH STEEL PLANK DECK ON 11 LINES OF STEEL I-BEAMS WITH A CLEAR ROADWAY WIDTH OF 23'-3"± AND SUPPORTED BY CONCRETE ABUTMENT AND RUBBLE MASONRY AND LOCATED AT THE PROPOSED STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY NOT POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 14+05.00 -L-".

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA (ON SHEET 1 OF 2) SHALL BE EXCAVATED FOR A DISTANCE FROM THE CENTERLINE OF ROADWAY OF 36'± (LEFT) AND 24'± (RIGHT) AT END BENT 1 TO EL. 702.0±, AND 43'± (LEFT) AND 31'± (RIGHT) AT END BENT 2 TO EL. 702.0±, AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. SINCE THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR, THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18 - EVALUATING SCOUR AT BRIDGES".

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS. FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS. ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITY ON ROADWAY PLANS. FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

FOUNDATION NOTES

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT 1 AND END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 111 TONS PER PILE. DRIVE PILES AT END BENT 1 AND END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 185 TONS PER PILE. STEEL H-PILE POINTS ARE REQUIRED FOR STEEL H-PILES AT END BENT 1 AND 2. FOR STEEL PILE POINTS, SEE SECTION

450 OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 15,000 FT-LBS TO 26,000 FT-LBS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT 1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(d)(2) OF THE STANDARD SPECIFICATIONS.

)	FMATERIAL											
IG	PILE DRIVING EQUIPMENT SETUP FOR HP12X53 STEEL PILES	HF S	9 12 X 53 STEEL PILES	STEEL PILE POINTS	VERTICAL CONCRETE BARRIER RAIL	RIP RAP CLASS II (2'-O" THICK)	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	3'-(PRE C(B0	D″X 2'-9″ STRESSED DNCRETE X BEAMS		
	EA.	NO.	LIN.FT.	EA.	LIN.FT.	TONS	SQ. YDS.	LUMP SUM	NO.	LIN.FT.		
					170.0				13	1105.0		
	7	7	70.0	7		150	165					
	7	7	105.0	7		105	120					
	14	14	175.0	14	170.0	255	285	LUMP SUM	13	1105.0		

	PROJEC	CT NO.	176	3P.9.R.8	31
		ROWA	AN	CO	UNTY
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	SHEET 2 0	F 2			
Documento EIDSE APPROPERS SEAL 0463 14 GINEE 2/13/2019 STV ENGINEERS, INC.	DEPA G FO (N C	RTMENT	TE OF NORTH CAR OF TRAI RALEIGH DGE ON N STREI CREEK I	NSPORTA NSPORTA RAWIN SR 23 ET) OVE BETWEE	TION IG 00 R N
AUU 900 West Trade St., Suite 715 ears Charlotte, NC 28202 NC License Number F-0991		IUU6	AND E.	KEKNS	51.
		REVI	STONS		SHEEL NO. S-2
ENT NOT CONSIDERED INAL UNLESS ALL IATURES COMPLETED	NO. BY: 1 2	DATE:	NO. BY:	DATE:	TOTAL SHEETS 15
				I	

		LOAD AN	D RE	SIST	FANCE	E FA(CTOR	RAT	ING	(LRF	D) SI	UMMA	ry f	OR F	PRES	TRES	SSED	CON	CRET	E GI	rdef	?S		
										STRE	ENGTH	I LIN	AIT ST	TATE				SE	SERVICE III LIMIT STATE					
										MOMENT					SHEAR						MOMENT			
LEVEL		CEHICLE VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	CONTROLLING LOAD RATING MINIMUM RATING FACTORS (RF)	(RF) TONS = W X RF	LIVE LOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE LOAD FACTORS	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	
		HL-93(Inv)	N/A	1	1.234		1.75	0.268	1.77	А	EL	41.724	0.584	1.23	А	EL	8.345	0.80	0.268	1.43	А	EL	41.724	
DESIGN		HL-93(0pr)	N⁄A		1.600		1.35	0.268	2.29	А	EL	41.724	0.584	1.60	A	EL	8.345	N/A						
LOAD RATING		HS-20(Inv)	36.000	2	1.600	57.589	1.75	0.268	2.38	А	EL	41.724	0.584	1.60	A	EL	8.345	0.80	0.268	1.92	А	EL	41.724	
	_	HS-20(0pr)	36.000		2.074	74.652	1.35	0.268	3.08	А	EL	41.724	0.584	2.07	А	EL	8.345	N/A						
		SNSH	13.500		4.442	59.964	1.4	0.268	6.87	А	EL	41.724	0.584	4.84	А	EL	8.345	0.80	0.268	4.44	А	EL	41.724	
		SNGARBS2	20.000		3.264	65.272	1.4	0.268	5.05	А	EL	41.724	0.584	3.41	А	EL	8.345	0.80	0.268	3.26	А	EL	41.724	
		SNAGRIS2	22.000		3.072	67.575	1.4	0.268	4.75	А	EL	41.724	0.584	3.16	А	EL	8.345	0.80	0.268	3.07	А	EL	41.724	
		SNCOTTS3	27.250		2.209	60.195	1.4	0.268	3.42	А	EL	41.724	0.584	2.41	А	EL	8.345	0.80	0.268	2.21	А	EL	41.724	
	Í.	SNAGGRS4	34.925		1.828	63.848	1.4	0.268	2.83	А	EL	41.724	0.584	1.98	А	EL	8.345	0.80	0.268	1.83	А	EL	41.724	
		SNS5A	35.550		1.789	63.597	1.4	0.268	2.77	А	EL	41.724	0.584	2.00	А	EL	8.345	0.80	0.268	1.79	А	EL	41.724	
		SNS6A	39.950		1.634	65.278	1.4	0.268	2.53	А	EL	41.724	0.584	1.82	А	EL	8.345	0.80	0.268	1.63	А	EL	41.724	
LEGAL		SNS7B	42.000		1.556	65.343	1.4	0.268	2.41	А	EL	41.724	0.584	1.78	А	EL	8.345	0.80	0.268	1.56	А	EL	41.724	
LOAD		TNAGRIT3	33.000		1.990	65.683	1.4	0.268	3.08	А	EL	41.724	0.584	2.17	А	EL	8.345	0.80	0.268	1.99	А	EL	41.724	
NATINO		TNT4A	33.075		1.997	66.056	1.4	0.268	3.09	А	EL	41.724	0.584	2.12	А	EL	8.345	0.80	0.268	2.00	А	EL	41.724	
		TNT6A	41.600		1.626	67.640	1.4	0.268	2.51	А	EL	41.724	0.584	1.88	А	EL	8.345	0.80	0.268	1.63	А	EL	41.724	
	ST	TNT7A	42.000		1.630	68.476	1.4	0.268	2.52	А	EL	41.724	0.584	1.84	А	EL	8.345	0.80	0.268	1.63	А	EL	41.724	
	= [TNT7B	42.000		1.678	70.459	1.4	0.268	2.59	А	EL	41.724	0.584	1.74	А	EL	8.345	0.80	0.268	1.68	А	EL	41.724	
		TNAGRIT4	43.000		1.603	68.913	1.4	0.268	2.48	А	EL	41.724	0.584	1.69	А	EL	8.345	0.80	0.268	1.60	Α	EL	41.724	
		TNAGT5A	45.000		1.514	68.142	1.4	0.268	2.34	А	EL	41.724	0.584	1.67	A	EL	8.345	0.80	0.268	1.51	Α	EL	41.724	
		TNAGT5B	45.000	3	1.499	67.445	1.4	0.268	2.32	А	EL	41.724	0.584	1.61	А	EL	8.345	0.80	0.268	1.50	A	EL	41.724	

LRFR SUMMARY

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LOAD FACTORS:

DESIGN	LIMIT STATE	γ_{DC}	$\gamma_{D\mathbf{W}}$
LOAD RATING	STRENGTH I	1.25	1.50
FACTORS	SERVICE III	1.00	1.00

NOTES:

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MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES. ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

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- 3.
- 4.

(#) CONTROLLING LOAD RATING
1 DESIGN LOAD RATING (HL-93)
2 DESIGN LOAD RATING (HS-20)
$\sqrt{3}$ Legal load rating $**$
** SEE CHART FOR VEHICLE TYPE
GIRDER LOCATION
I – INTERIOR GIRDER EL – EXTERIOR LEFT GIRDER
ER – EXTERIOR RIGHT GIRDER

	PROJEC	CT NO.	176	3P . 9.R.8	81
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		REVI	SIONS	I	SHEET NO.
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STD.NO.33LRFR1_75&105S_85L

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39′-0″

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE BOX BEAM SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE BOX BEAMS.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}^{"}$ Ø DOWEL HOLES AT FIXED ENDS OF BOX BEAM SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE BOX BEAM UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 6000 PSI.

ALL REINFORCING STEEL IN VERTICAL CONCRETE BARRIER RAILS SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE BOX BEAM UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO BOX BEAM UNIT ENDS.

VERTICAL GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A VERTICAL CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

THE LOCATION OF THE VOID DRAINS MAY BE SHIFTED SLIGHTLY WHERE NECESSARY TO CLEAR PRESTRESSING STRANDS OR TRANSVERSE REINFORCING STEEL.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK. THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

	PROJEC [.]	T NO.	176	3P.9.R.8	31
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	PRES	3'-(TRES)″X2 SSED	-9" CONCI	RETE
VGINEERS, INC. Trade St., Suite 715 otte, NC 28202 se Number 5-0991	В	ΟΧΙ	BEAM	UNI	Γ
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		STE).NO.S	TD.33PC	CBB1_39

* <u>'-91/2"</u> © BR(

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SHOWING ELEVATION VIEW OF GROUTED RECESS

DEAD LOAD DEFLECTION AN	ND CAMBER
	3'-0"× 2'-9"
85'BOX BEAM UNIT	0.6″ØL.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	2∛₄″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	3∕4″ ↓
FINAL CAMBER	2″ 🛉
** TNCLUDES FUTURE WEARTNG SURE	

** INCLUDES FUIURE WEARING SURFACE

STD.NO.33PCBB5_75S

BOX BEAM UNITS REQUIRED						
	NUMBER	LENGTH	TOTAL LENGTH			
EXTERIOR B.B.	2	85′-0″	170'-0"			
INTERIOR B.B.	11	85'-0"	935′-0″			
TOTAL	13		1105'-0″			

GUTTERLINE ASPHALT THICKNESS & RAIL HEIGHT						
	ASPHALT	COVERLAY TH	ICKNESS		RAIL HEIGHT	
GUITERLINE	@ ⊈ BRG.EB1	@ MID-SPAN	@ € BRG.EB2	@ 🕻 BRG.EB1	@ MID-SPAN	@ 🕻 BRG.EB2
LEFT	31/2″	2 ¹ /4″	35⁄8″	3′-9 /2″	3′-8 /4″	3′-9 ⁵ ⁄8″
RIGHT	31/2″	2 /8″	31/2"	3′-9 /2″	3′-8 /8″	3′-9 /2″

BII	LL OF MATERIAL FOR VERTICAL CONCRET	TE B	ARR	IER F	RAIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	SIZE	TYPE	LENGTH	WEIGHT
	85' UNIT				
* B9	144	#5	STR	15′-9″	2365
* S6	230	#5	1	7'-2″	1719
★ EPOX	Y COATED REINFORCING STEEL		LBS.		4084
CLASS	AA CONCRETE		CU.YDS.		22.0
TOTAL	VERTICAL CONCRETE BARRIER RAIL		LN.FT.		170.0

	PROJEC	CT NO.	171	3P.9.R.8	31	
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	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH					
NC. te 715 2 0991	PRES [3'-0 Stres 30X [)"X2 SED BEAM	2'-9" CONCI UNIT	RETE	
0991		REVIS	SIONS		SHEET NO.	
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STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4. FOR WING DETAILS, SEE SHEET 3 OF 4.

TOP ELEV	OF PILE /ATIONS
	704.78
2	704.60
3	704.42
4	704.24
5	704.05
6	703.87
	703.69

PROJECT NO ROW	. <u>178</u> AN	9.9.R.8	B1 UNTY
STATION:	14+05.	00 -L-	-
SHEET 1 OF 4			
DEPARTMENT SUB	STRUCTL	ISPORTA	TION
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<u>ย</u> ๑	 		SHEETS 15
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TOP ELE\	OF PILE /ATIONS
	705.61
2	705.43
3	705.25
4	705.06
5	704.88
6	704.70
	704.52

STD. NO. EB_39_75S4_33BB

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STD. NO. EB_39_75S4_33BB

BAR T	YPES		BI	LL O	F MA	ATERIA	L
	21/16"		FOF				NT
) нк. _Г		BAR		ST7F	TYPE	LENGTH	WEICHI
	T_{1}^{1}	B1	8	#9	1	48'-8"	1324
1'-3"	»/ // (۷	B2	28	#4	STR	24'-5"	457
		B3	12	#4	STR	2'-5"	19
	H1 10'-5"	D1	26	#8	STR	2'-3"	156
	H2 10'-7"					2 0	
		H1	12	#5	2	11'-1"	139
H3		H2	12	#5	2	11'-3"	141
H4	$\vdash \frown \frown$	H3	12	#5	3	11'-6"	144
		H4	12	#5	3	11'-4"	142
<i>"</i>	<u></u>	K1	12	#4	STR	3'-1"	25
		K2	12	#4	STR	24'-5″	196
1	()		58	# ⊿	4	10′-5″	404
AP		S2	58	#4	5	3'-2"	12.3
_ / \1		S3	28	#4	6	6'-6"	122
	8″	U1	40	#4	7	3'-7"	93
		V1	61	#4	STR	7'-2″	292
		V2	80	#4	STR	5'-3"	281
		REINF (FOR	ORCIN ONE E	NG STE ND BEN	EL IT)	4(059 LBS.
		CLASS	A CO	NCRETE	BREA	KDOWN	
			FOR O	NE END	BENT) ¬Ŧ	07 0 0 V
MENSIO	NS ARE OUT TO OUT.	POUR	#1 CA OF	AP,LOW F WING	ER PAI S & C	OLLARS	23.2 0.1.
	END BENT No.2	POUR	#2 B/	ACKWAL	L & U	PPER	5.9 C.Y.
LES = 70.0	HP 12 X 53 STEEL PILES NO: 7 LIN. FT.= 105.0	TOTAL	CLAS	S A CO	NCRET	E	29.1 C.Y.
I	PILE DRIVING EQUIPMENT SETUP FOR						
	HP 12 X 53 STEEL PILES						
INU: (NU: /						
NO: 7	STEEL PILE POINTS NO: 7						
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	PROJECT NO						
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	SHEET 4 C	F 4					
	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH						
	SUBSTRUCTURE						
	EN	D BE	NT N FTATI	o.1 & S	2		
C. 715 991							
		REVI	SIONS		SHEET NO.		
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	1		3		TOTAL SHEETS 15		
			STD. NO.	EB_39_75	5S4_33BB		

Docusioner by CARO Latter FF DESCARD E1D5E APROSE245C SEAL 0463 14 MGINEER XGINEER 2/13/2019	
100 STV ENGINEERS, 900 West Trade St., S Charlotte, NC 28 NC License Number	INC. Suite 7 202 F-099
JMENT NOT CONSIDERED FINAL UNLESS ALL GNATURES COMPLETED	

ESTIMATED QUANTITIES						
BRIDGE @ STA.14+05.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE				
	TONS	SQUARE YARDS				
END BENT 1	150	165				
END BENT 2	105	120				

+

		BILL OF MATERIAL				
						_ `D #1
, 4″Ø DRAINAGE PIPE, AND SELECT				JLAI		
	BAR BAR	NO.	S⊥ZE # 4		LENGIH	WEIGHÍ
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			· ·		20 0	
) SHALL BE IN ACCORDANCE WITH	* B1	76	#5	STR	11'-1"	879
	B2	76	#6	STR	11'-7"	1322
ALONG FILL FACE OF BACKWALL FROM						
).	REIN	ORCI	NG STE	EL	LBS.	1678
SHALL BE GRADED TO DRAIN THE WATFR		NFOR	DATED	TEEI	IRC	1278
LL BE PAVED. SEE ROADWAY PLANS.			, TUD D	•		1200
AY STANDARD DRAWINGS.	CLASS	S_AA	CONCRE	TE	C. Y.	20.2
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7			ST7F		- ,,, <u> </u>	<u> </u>
	* A1	26	#4	STR	20'-8"	359
	A2	26	#4	STR	20'-6"	356
DE MOT						
GRABOTIV	₩ B1	76	#5	STR	11'-1"	879
TU	B2	76	#6	STR	11'-7"	1322
				· – ı		4070
CAP FLOW LINE ONLY WITH EROSION RESISTANT MATERIAL			NG SIE	LL	LBS.	1678
		INFOR	SING S	TEEL	LBS.	1238
AND GRADE TO DRAIN						
IMMEDIATELY AFTER THE RACKETLLING	CLASS	S AA	CONCRE	TE	C. Y.	20.2
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H AS FIBERGLASS ROVING OR AS L EROSION AND TO PROTECT THE AREA						
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EROSION CONTROL MAT,OR 3)CONCRETE, PF DRAIN SHALL CONSIST OF A		···	→ ∠	- FILL	SLOPE	
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Vears Charlotte, NC 28202 NC License Number F-0991			1 J	JNE	**	
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STD.NO.BAS_BB_39_75S

DESIGN DATA:

SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50	27,000 LBS.PER SQ.IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS.PER SQ.IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS. AND APPROACH SLABS: AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ "WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - $\frac{7}{8}$ "ø studs for 4 - $\frac{3}{4}$ "ø studs, and stud spacing changes SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 1/8" Ø STUDS FOR 4 - $\frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY. AT HIS OPTION. SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{6}$ IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY VIGINCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAÍNTING, GALVANIZING, OR METALLIZING.

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

HANDRAILS AND POSTS:

