Technical Training 2008

PIPE

INSTALLATION

Safety by the numbers

- It's
- 2 ft. The distance to stay from excavation
- 4 ft. Depth requires egress
- 5 ft. Depth requires shielding
- 6 ft. Height to ground where fall protection begins
- 10 ft. Minimum clear distance to powerline
- 20 ft. Depth requires PE
- 2:1, 5 ft If 2:1 slope from bottom of excavation intercepts existing ground closer than 5'to traffic, shoring is required.

Environmental Stewardship

- Permitted footprint
- Changing elevations
- Bury pipe (permitted or not)/perched pipe
- Pump around
- No changes without permit mod.
- Scour holes
- Sluice gates

Pipe camera video



Acceptance

- Pipe Manufacturing and Storage
- Unloading and Handling
- Pipe Material Information
- Causes for Rejection
- Concrete Pipe Cracks
- Acceptance
- Material received

Section materials specialist (meet your section mat) HiCams

Brand Certification





Inspection after unloading

(why it may be stamped but unacceptable when it arrives on the project)

Wire showing – is it a form wire or reinforcing wire?

Bells and tongues broken – pipe is resting on this connection

Patching of concrete pipe is limited to the repair of minor defects. Minor defects do not include through-wall cracks of any dimension, 0.010 in (0.25 mm) cracks measuring 12 in (300 mm) or more in length or damage/defects to pipe ends where such damage would prevent making a satisfactory joint. Pipe sections that are damaged or otherwise defective shall be rejected. Rejection criteria are outlined in AASHTO M170, Section 15.

Unloading pipe

DO: use a pneumatic unloading device use cables and skids use unloading equipment that supports the length of the pipe



Unloading



Don't: drop it off the trailer let one joint strike another on the ground use a bolt through the lifting hole

PHOTOGRAPHS

Precast Concrete Products That Should be Rejected, Accepted, or Rejected Due to Unacceptable Repairs





More cracks and wire



Pipe and Drainage Structure Stakeout

- Review drawing
 - This can't afford to lay on desk or not get done
 - A chance to reduce amount of price adjustments for extra depth pipe

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Pay Adjustment (per linear foot) = [(APE-AAE)± 1 foot] (0.15 X CUP)

Where: CUP = Contract Unit Price of Pipe Culvert

AAE = Average Actual Elevation (Actual Inlet elev. + Actual Outlet elev.)

2

APE = Average Plan Elevation (Plan Inlet elev. + Plan Outlet elev.)

2

Pay Adjustment (per linear meter) = [(APE-AAE)± 0.3 m] (0.15 X CUP)

Where: CUP = Contract Unit Price of Pipe Culvert

AAE = Average Actual Elevation (Actual Inlet elev. + Actual Outlet elev.)

2

APE = Average Plan Elevation (Plan Inlet elev. + Plan Outlet elev.)

2

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2
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Foundation

Unsuitable: Undercut (as directed) and place select material + loose compressible material for O.D. / 24" (not less than 3")

Water: Make sure there's no muck and then place 57 stone and select or loose compressible material for O.D. / 24" (not less than 3")

Rock: Undercut (O.D."./ 12 but not less than 6") and backfill with unclassified, borrow or foundation conditioning (don't forget loose compressible material)

Payment for Foundation Conditioning

Did you undercut?

If yes, then did you use unclassified or borrow to backfill the undercut?

If yes, then measure the undercut by the average end area method and pay at double unclassified. Then pay for the borrow or unclassified.

Did you undercut?

If yes, then did you use material from off of the project (other than borrow which is always off of the project)?

If yes, then pay for the material as tons of Foundation Conditioning Material and do NOT pay for the undercut.

Pipe Foundation





Good amount of loose compressible material

Poor installation. No loose compressible material nor is the pipe cradled.



Lay pipe on dry foundation

Section 300-4: Maintain the pipe foundation in a dry condition

What about paying me for an impervious dike?

"The impervious dike shall be constructed of an acceptable material in the locations noted on the plans or as directed"

No payment unless impervious dike shown in plans for that particular site.

Laying and Joining Pipe

- Trenching and Shoring
- Check invert elevation
- Check laser
- Pipe handling (lift hole, strap, bar)
- Place pipe on foundation
 Standard Drawings
- Install joint compound
- Join pipe



Pipe handling







Handling

300-3: Use a lifting device that uniformly distributes the weight of the pipe long its axis or circumference.





For pipe without a lifting hole, one acceptable method of lifting involves the use of a nylon strap or sling wrapped around the pipe. Figure 5-3 shows how, for shorter pipe lengths, one strap or sling can be used around the middle of the section when lifting or placing the pipe into the trench. For longer lengths, two nylon straps should be placed at the 1/3 and 2/3 points on the pipe, as shown in Figure 5-4.



Figure 5-3

Pipe handling (bad)











Install Joint Compound

Flexible plastic joint material





Do not put it against the shoulder. Put it half way and give it a chance to engage the bell.

Joining pipe

Do keep the pipe entrance angle low to increase The chances that the joint material will engage all Around.



Do not ram with hoe





Do home the pipe fully with strap bar, block or come-along





Size and placement of flexible pipe joint material



Good homing techniques





reparing the bedding at the socket end of the pipe to be joined.







Lay pipe con't

 Check alignment and grade Tie in at drainage structures - 5 min on Drainage structures Back fill and compact - Plug lifting holes - Density - Hauling - Proof roll

Drainage Structures

The use of traffic bearing masonry drainage structures is governed by Section 5-13 of the Roadway Design Manual as follows:

- Traffic bearing drop inlets (Std. No. 840.36) shall be used within a traveling lane (detour or permanent).

- Traffic bearing drop inlets (Std. No.'s 840.35 or 840.36) shall also be used within 4'-0" of lanes except when placed in a concrete traffic island.

- Traffic bearing steel frames and flat steel grates (Std. No.840.37) are to be used where it has been determined that traffic bearing drop inlets are needed on controlled access projects in locations that pedestrian traffic is not anticipated. On controlled access projects where pedestrian traffic is anticipated, a flat, narrow slot frame and grate (Std. No. 840.29) should be used. TheTraffic Engineering and Safety Systems Branch or the Hydraulics Unit may specify other locations where these must be used due to safety considerations.

Design Services has further clarified the use of traffic bearing drainage structures as follows:

- Drainage structures that are used in 2'-6" curb and gutter are not required to be traffic bearing. However, if the pipe size is large enough such that a wall of the drainage structure will be under the travel lane instead of under the curb and gutter, the drainage structure should be traffic bearing.

- Drainage structures that are used in shoulder berm gutter or expressway gutter and are within 4'-0" of a travel lane are required to be traffic bearing.

- Precast waffle boxes are not recognized as traffic bearing structures.

- Solid wall precast boxes are considered traffic bearing structures.

Precast Detail



Good & the bad



Frame completely on box. Concrete will flow under frame. Anchors in brick.



Connect to drainage structures and fill lifting holes


















Spec. 840-3 says "assemble and grout together the precast drainage structure units". Std. 840.45 (2 of 2) (precast drainage str.) says "seal joint with flexible buty rubber base AASHTO M198 Type B"

Std. 840.46 (1 of 1) (traffic bearing precast drainage str.) says "Seal joints with an approved Sealant (see section 840 of NCDOT Std. Specs.)"



Why plug lifting holes with grout





Tunneling in Wake Forest





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Project Diary

 Record conversations, observations, spot checks made, and work done, including material used, in the diary.

Checklist (page 3-9)

- 1. Study the Specifications, plans, permits, erosion control phasing and Special Provisions.
- 2. Does pipe staking appear to be correct? If not, contact the Engineer.
- 3. Did the permits require the culvert to be buried?
- 4. Observe pipe sections after delivery to the site. Record any joint or section rejected and reason for the same. Ensure that concrete pipe has been stamped with the Department's Seal of Approval. For flexible pipe, see that the pipe, fittings, and other accessories have been provided by a supplier having met the requirements of the Department's Brand Certification Program and listed on the Department's preapproved list. Ensure the flexible pipe has been inspected by the Materials and Tests Unit. If unloading or handling is careless, notify Contractor's supervisory personnel. Mark any rejected pieces of pipe.
- 5. Verify the class of pipe and installation method against the drainage summary sheet within the plans.
- 6. See that Department Policy & Procedures for Excavation, Trenching, and Shoring are being strictly followed. Notify the Engineer if an unsafe condition exists, stop work if imminent danger exists.
- 7. Monitor the Contractor's control of the pipe grade, including pipe camber.
- 8. If unsuitable material or rock is encountered, consult with the Engineer for the method of conditioning to be used.
- 9. If local material is used to backfill undercut areas, measure undercut and record in Pay Record Book as pipe foundation undercut.
- 10. If other than local material is used, establish the method of material measurement and payment. Ensure the material has been approved for use.
- 11. Temporary water diversion is the responsibility of the Contractor. See that this is adequate to prevent foundation damage and erosion problems.
- 12. See that shaped bedding is properly constructed.

- 13. See that pipe is laid, joints properly connected, and protected in accordance with Specification requirements. Check line and grade before starting and periodically thereafter.
- 14. Plug lifting holes with either concrete or grout mixture.
- 15. See that backfill is placed in layers of 6 inches or less, unless otherwise authorized, with both sides brought up at the same time. See that heavy equipment is not operated over any pipe until it has been backfilled with a minimum of 3 feet of cover.
- 16. Run density tests to verify that the methods of compaction are satisfactory. If results are not satisfactory, require the Contractor to change methods and obtain required density. Record on the density form that it is in a pipe backfill.
- 17. Backfill to be shaped to drain when work is suspended or completed.
- 18. If select backfill material is required, be sure this has been tested and approved prior to use.
- 19. Perform periodic inspections of completed drainage facilities to assure they are maintained in accordance with Specifications including 5% deflexion for flexible pipe. See that all damage is repaired prior to placement of base and pavement.
- 20. See that all necessary erosion control devices have been properly installed. If silt basins are constructed at ends of pipe, see that these are cleaned out as needed. Monitor all devices to ensure they are functioning properly and that they are receiving proper maintenance.
- 21. If pipe is structural plate, notify the Engineer before any phase of construction is begun.
- 22. Record conversations, observations, spot checks made, and work done, including material used, in the diary.
- 23. Keep pay records as required.

Cross Pipe End Treatments inside the clear zone

Extend pipe beyond the clear zone and use headwall on pipes that are 36" dia. and greater

If the pipe cannot be extend then:

Use a cross pipe end section for pipes 30" dia. and smaller.

Use guardrail for pipe 36" and greater with endwall on inlet end and cross pipe end unit with safety bars on the outlet end or protect with guardrail.

Side Pipe End Treatment





Side Pipe End Treatments inside the clear zone

Place pipe beyond clear zone

If pipe cannot be moved beyond clear zone

Use a drop inlet on inlet end. If a drop inlet cannot be used, use a Parallel Pipe end section on inlet end for pipe 24" in dia. or less.

Note: For multilane roads with design speeds less than 50mph and all 2 lane roads no special end treatment is required.

End Treatments



REV. DATE 01/02/02

Drainage Users Manual

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Headwalls and Clear Zone

DESIGN SPEED	DESIGN ADT	[U.S. Customary Units] FORESLOPES			BACKSLOPES		
		1V:6H of flatter	1V:5H TO 1V:4H	1V:3H	1V:3H	1V:5H TO 1V:4H	1V:6H or Flatter
40 mph or less	UNDER 750	7 - 10	7 - 10	88	7 - 10	7 - 10	7 - 10
	750 - 1500	10 - 12	12 - 14	88	10 - 12	10 - 12	10 - 12
	1500 - 6000	12 - 14	14 - 16	8.8	12 - 14	12 - 14	12 - 14
	OVER 6000	14 - 16	16 - 18	8.8	14 - 16	14 - 16	14 - 16
45–50 mph	UNDER 750	10 - 12	12 - 14	88	8 - 10	8-10	10 - 12
	750 - 1500	12 - 14	16 - 20	88	10 - 12	12 - 14	14 - 16
	1500 - 6000	16 - 18	20 - 26	8.8	12 - 14	14 - 16	16 - 18
	OVER 6000	18 - 20	24 - 28	88	14 - 16	18 - 20	20 - 22
55 mph	UNDER 750	12 - 14	14 - 18	8.8	8-10	10 - 12	10 - 12
	750 - 1500	16 - 18	20 - 24	8.8	10 - 12	14 - 16	16 - 18
	1500 - 6000	20 - 22	24 - 30	88	14 - 16	16 - 18	20 - 22
	OVER 6000	22 - 24	26 - 32 *	**	16 - 18	20 - 22	22 - 24
60 mph	UNDER 750	16 - 18	20-24	8.8	10-12	12 - 14	14 - 16
	750 - 1500	20 - 24	26 - 32 *	8.8	12 - 14	16 - 18	20 - 22
	1500 - 6000	26 - 30	32 - 40 *	**	14 - 18	18 - 22	24 - 26
	OVER 6000	30 - 32 *	36 - 44 *	**	20 - 22	24 - 26	26 - 28
65–70 mph	UNDER 750	18 - 20	20-26	8.8	10 - 12	14 - 16	14 - 16
	750 - 1500	24 - 26	28-36*	88	12 - 16	18 - 20	20 - 22
	1500 - 6000	28-32*	34 - 42 *	88	16 - 20	22 - 24	26 - 28
	OVER 6000	30-34*	38 - 46 *	8.8	22-24	26 - 30	28 - 30

* Where a site specific investigation indicates a high probability of continuing crashes, or such occurrences are indicated by crash history, the designer may provide clear-zone distances greater than the clear-zone shown in Table 3.1. Clear zones may be limited to 30 ft for practicality and to provide a consistent roadway template if previous experience with similar projects or designs indicates satisfactory performance.

** Since recovery is less likely on the unshielded, traversable 1V:3H slopes, fixed objects should not be present in the vicinity of the toe of these slopes. Recovery of high-speed vehicles that encroach beyond the edge of the shoulder may be expected to occur beyond the toe of slope. Determination of the width of the recovery area at the toe of slope should take into consideration right-of-way availability, environmental concerns, economic factors, safety needs, and crash histories. Also, the distance between the edge of the through traveled lane and the beginning of the 1V:3H slope should influence the recovery area provided at the toe of slope. While the application may be limited by several factors, the foreslope parameters which may enter into determining a maximum desirable recovery area are illustrated in Figure 3.2.

Pictures of headwalls in CRRZ

