



Modeling Intersection Delay in Static Assignment

May 11, 2016

Agenda

MOTIVATION

- Why do this?

KEY ISSUES

- Things to think about

METHODS

- Ways of doing it

SUMMARY



Press Photo – Rex Larsen



Motivation

Why bother?

GOOD REASONS

- Get speeds and delay right on arterials
 - Get emissions and vehicle operating costs right
 - Get routing right (better volumes, better paths for microsim)
- Represent rough benefits of different types of intersection control
 - Stop vs. Roundabout vs. Signal vs. Interchange

MAYBE NOT

- Test signal improvements (re-timing, coordination)
 - Better to use Synchro, HCS, or microsimulation



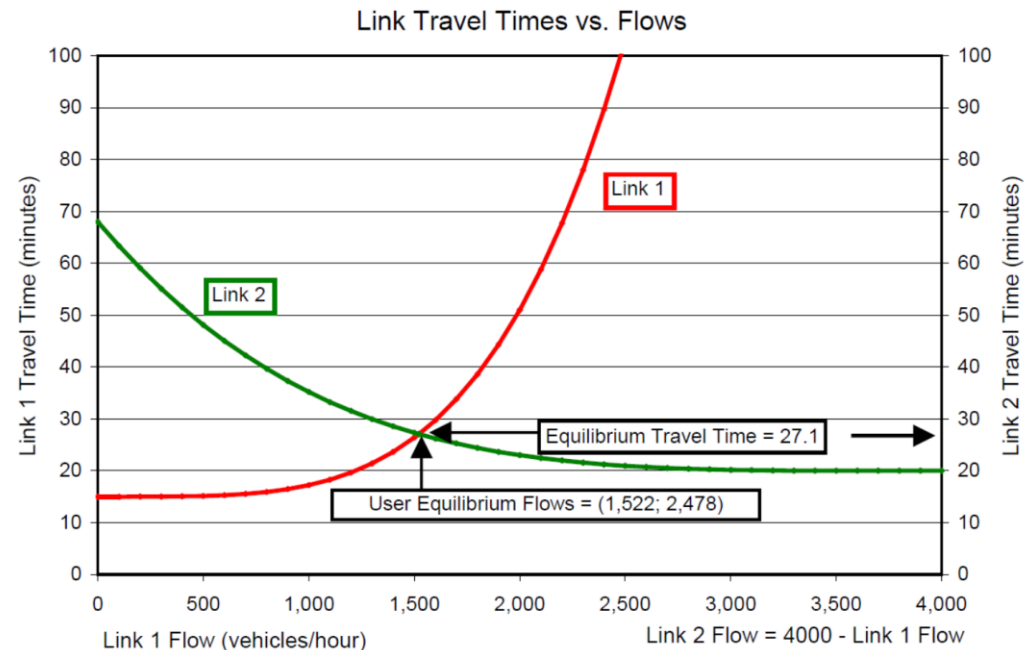
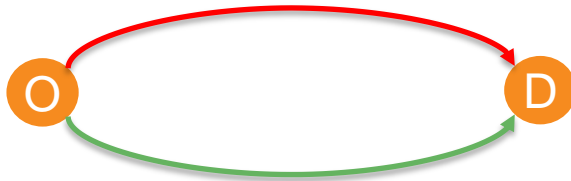


Key Issues

Solution Stability

REQUIREMENTS FOR EXISTENCE OF UNIQUE, FIXED POINT SOLUTION

- The network is strongly connected
- Demand is non-negative, finite, and either fixed or continuous and decreasing in cost
- The cost function is positive, **continuous and increasing in demand**

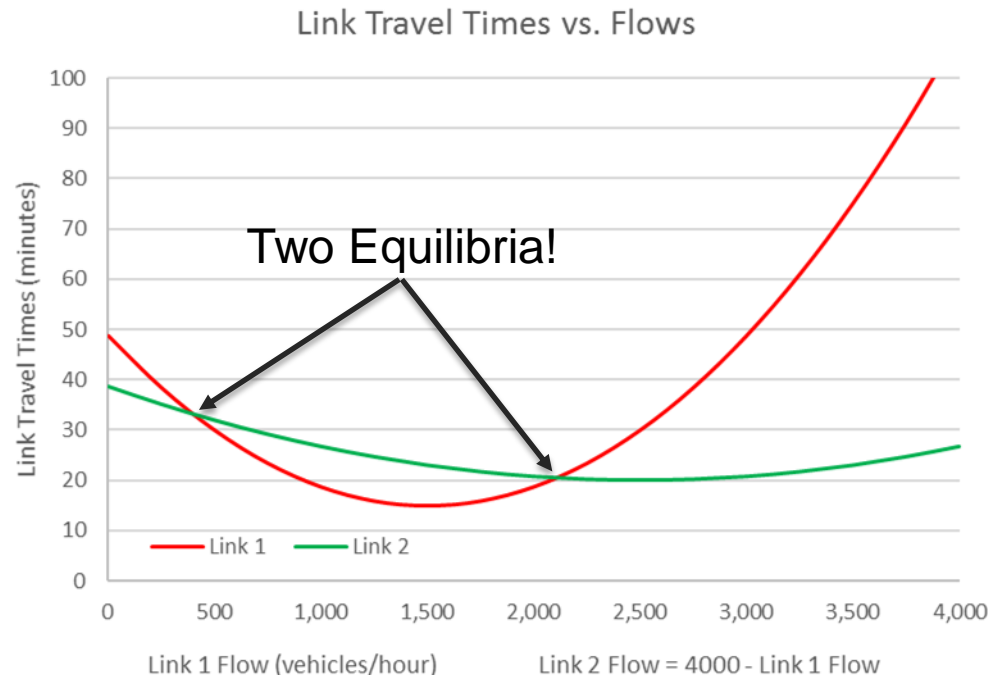
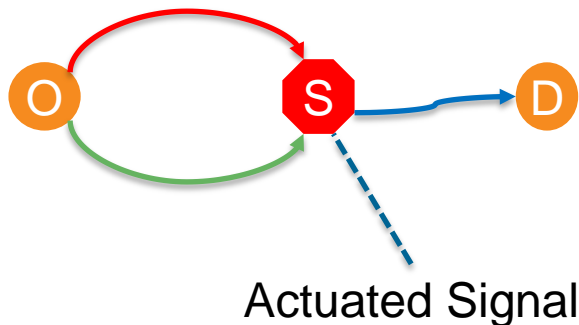


Source: D. Boyce

Trouble with Intersections

SIGNALS – ESPECIALLY ACTUATED – VIOLATE CONDITIONS

- cost function must be positive, **continuous and increasing in demand**
- cost can decrease at signals with the same or increasing demand if
 - timing changes/adapts or
 - cross street demand decreases
- multiple equilibria possible
- difference between two runs could just be this



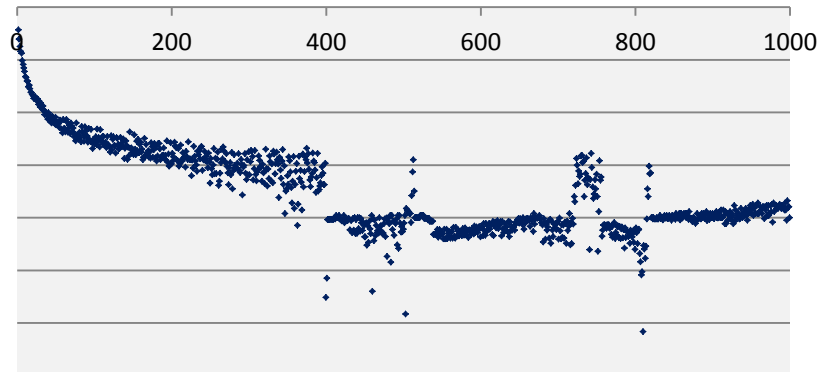
So... Good enough?

DANGER OF REALISTIC TRAFFIC DYNAMICS

- Realistic signal operations, etc., can threaten solution stability and invalidate (legally for NEPA) alternatives analysis, benefit-cost, and emissions analyses

SO, INSTEAD...

- Most methods compromise between realism and solution stability
 - Make intersection delay as realistic as possible
 - While maintaining a stable solution
- Beware methods that don't compromise!



Precision & Convergence

PRACTICAL CONSIDERATIONS

- Well converged solutions are important
 - Considerable noise in poorly converged results
 - Is 800 vph +/- 2,000 vph really helpful?
- Some methods of incorporating intersection delay can require much longer run times to achieve the same convergence – or may not be able to achieve good convergence at all

Figure 2-3 Road Traffic Changes Due to Blue Line Service Improvement: Relative Gap = 0.01

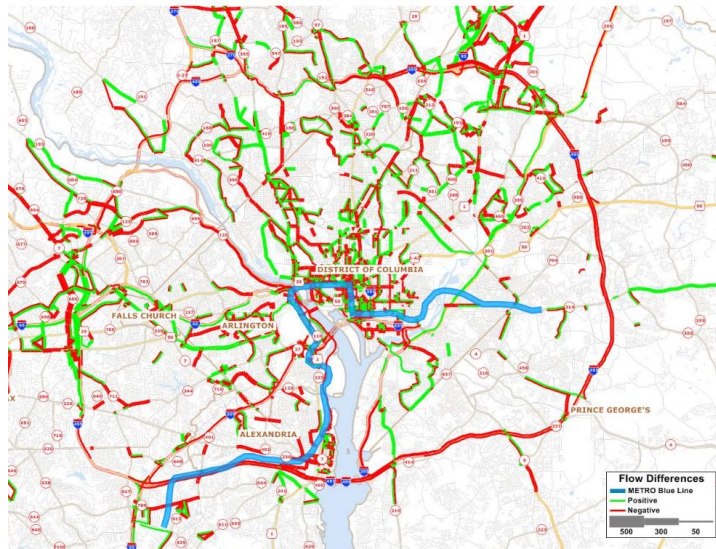
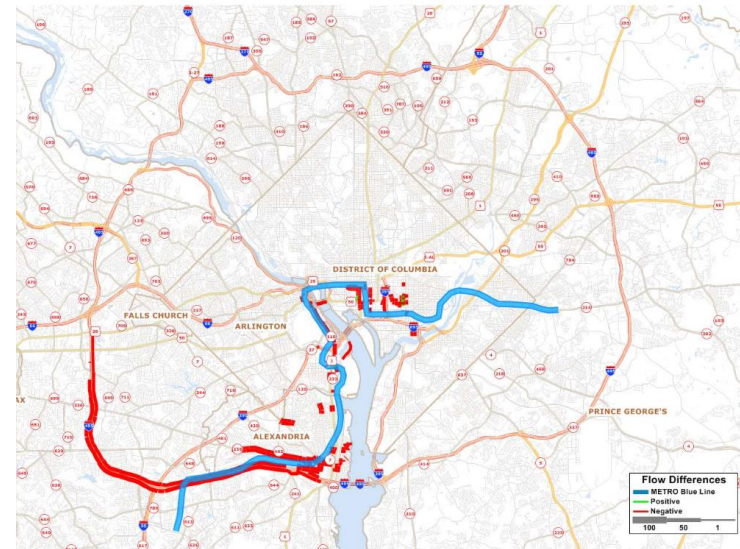


Figure 2-4 Road Traffic Changes Due to Blue Line Service Improvement: Relative Gap = 0.000001



Input Data

DATA HUNGRY?

- Different methods require different data
 - Location of signals & roundabouts (maybe stops)
 - g/C ratios, cycle length
 - Phasing, turn bay lengths, etc.

INTERSECTION: MING & REAL 2725
 Group Assignment: 4007
 Field Master Assignment: NONE
 System Reference Number: 53

N/S Street Name: Real Road
 E/W Street Name: Ming Ave

Page 1 (of 8)

Change Record

Change	By	Date	Change	By	Date
Increased	FW	7/20/04			
Yellow					
Phases 2, 4					
6, 8					

Drop Number: 4 <C/O+0+0>
 Zone Number: 1 <C/O+0+1>
 Area Number: 1 <C/O+0+2>
 Area Address: 53 <C/O+0+3>
 QuickNet Channel: COM9 (QuickNet)

Manual Plan: <C/O+A+1>
 Manual Offset: <C/O+B+1>

Manual Plan
 0 = Automatic
 1-9 = Plan 1-9
 14 = Free
 15 = Flash

Manual Offset
 0 = Automatic
 1 = Offset A
 2 = Offset B
 3 = Offset C

Flash Start: 0 <F/1+0+E>
 Red Revert: 0.0 <F/1+0+F>
 All Red Start: 0.0 <F/1+0+C>

Start / Revert Times

Exclusive Walk: 0 <F/1+0+0>
 Exclusive FDW: 0 <F/1+0+1>
 All Red Clear: 0.0 <F/1+0+2>

Exclusive Ped Phase
 (Outputs specified in Assignable Outputs at E/127+A+E & F)

Column Numbers	1	2	3	4	5	6	7	8
Phase Names	EBL	WB	SBL	NB	WBL	EB	NBL	SB
0	Ped Walk	0	7	0	4	0	7	0
1	Ped FDW	0	12	0	18	0	12	0
2	Min Green	4	10	4	4	10	4	4
3	Type 3 Disconnect	0	20	0	15	0	20	0
4	Added per Vehicle	0.0	0.0	0.0	0.0	0.0	0.0	0.0
5	Veh Extension	1.0	4.0	1.0	2.0	1.0	4.0	1.0
6	Max Gap	1.0	6.8	1.0	4.0	1.0	6.8	1.0
7	Min Gap	1.0	2.5	1.0	0.2	1.0	2.5	1.0
8	Max Limit	20	50	20	30	20	50	20
9	Max Limit 2	0	0	0	0	0	0	0
A	Adv. / Delay Walk	0	0	0	0	0	0	0
B	PE Min Ped FDW	0	12	0	18	0	12	0
C	Cond Serv Check	0	0	0	0	0	0	0
D	Reduce Every	0.0	0.5	0.0	0.5	0.0	0.5	0.0
E	Yellow Change	3.0	3.9	3.0	3.9	3.0	3.9	3.0
F	Red Clear	1.0	1.0	1.0	1.0	1.0	1.0	1.0

Phase Timing - Bank 1 <C+0+F=1>

Column Numbers	9	A	B	C	D
Phase 1	0	0	0	0	0.0
Phase 2	20	0	0	0	0.0
Phase 3	0	0	0	0	0.0
Phase 4	20	0	0	0	0.0
Phase 5	0	0	0	0	0.0
Phase 6	20	0	0	0	0.0
Phase 7	0	0	0	0	0.0
Phase 8	20	0	0	0	0.0

Max Initial
 Alternate Walk
 Alternate FDW
 Alternate Initial
 Alternate Extension

Alternate Timing <C+0+F=1>

Column Numbers	E	F
RR-1 Delay	0	0
RR-1 Clear	0	0
EV-A Delay	0	0
EV-A Clear	0	0
EV-B Delay	0	0
EV-B Clear	0	0
EV-C Delay	0	0
EV-C Clear	0	0
EV-D Delay	0	0
EV-D Clear	0	0
RR-2 Delay	0	0
RR-2 Clear	0	0
View EV Delay	---	---
View EV Clear	---	---
View RR Delay	---	---
View RR Clear	---	---

Preempt Timing

Column Numbers	12345678
Permit	12345678
Red Lock	0
Yellow Lock	0
Min Recall	2 6
Ped Recall	---
View Set Peds	---
Rest In Walk	---
Red Rest	---
Dual Entry	---
Max Recall	---
Soft Recall	---
Max 2	---
Cond. Service	---
Man Cntrl Calls	---
Yellow Start	1 5
First Phases	2 6

Phase Functions <C+0+F=1>

Dependencies on Input Data

SELF-FULFILLING PROPHECIES

- Outcomes can be dependent on input assumptions (such as g/C , phasing, etc.)
 - even if the algorithm can adjust these
 - If the initial iteration has less delay for one movement, more demand gets assigned to it, so the algorithm maintains or increases the g/C / phasing ratio in favor of the movement...repeat ad infinitum
 - Not an issue in simpler schemes without phasing or variable g/C 's

Other Self-Fulfilling Prophecies



Calibration

TO TRANSFER OR NOT TO TRANSFER

- Different methods have differing numbers and types of parameters
 - Some have observable parameters (e.g., cycle length, g/C)
 - All have calibration parameters
 - » Some have well-established defaults
 - » Others do not and may vary by location
- Ideal calibration requires having both travel time / delay and volume data for the same time periods
 - Travel time data now becoming more common – but often detailed (hourly / 15-min) counts are lacking
- Calibration can also be done by minimizing squared error versus counts in assignment



Methods

Overview of Methods

METHODS FOR INTERSECTION DELAY IN STATIC ASSIGNMENT

- **Link / Approach-based**
 - Link/Approach-based with Simple BPR form vdf
 - Link/Approach-based with “Double”/“Modified” BPR vdf
 - Others (Aashtiani *et al.*)
- **Node Delay**
 - IIT Logit Delay
 - TMODEL Node Delay
- **HCM-based Volume-Dependent Turn Delays**
 - Full HCM method
 - HCM approach method
- Others?

Link-based w/ Simple BPR form vdf

A SIMPLE METHOD

- Webster's uniform control delay for free-flow conditions added to free-flow time for each approach $d_0 = 0.5C \left(1 - \frac{g}{C}\right)^2$
- BPR form volume-delay function used to represent flow-dependent delays (uniform, random arrival / incremental & overflow / queueing delays) $t = (t_0 + d_0) \left[1 + \alpha \left(\frac{v}{c}\right)^\beta\right]$

NOTES

- Used in IN, KY, TN, AR, etc.
- Delay continuously increasing in demand
- Only requires intersection type (g/C can be assumed or input, etc.)
- Parameters (α, β) different from freeways and vary by location

Link-based w/ “Double”/“Modified” BPR vdf

A PRETTY SIMPLE METHOD

- One BPR form volume-delay function for link / mid-block delay
- Another BPR form volume-delay function used to represent flow-dependent delays (uniform, incremental / random arrival & overflow/queueing delays)

$$t_l = t_0 \left[1 + \alpha_l \left(\frac{v}{c} \right)^{\beta_l} \right]$$

$$t_n = d_0 \left[1 + \alpha_n \left(\frac{v}{c} \right)^{\beta_n} \right]$$

NOTES

- Used in AZ, CA, AK
- Delay continuously increasing in demand
- Only requires intersection type (g/C can be assumed or input, etc.)
- Need four parameters, vary by location

Node Delay – IIT Logit

$$t_n = p_1 d_0 \left[1 + \left(\frac{p_2}{1 + e^{(p_3 - p_4 \frac{v}{c})}} \right) \right]$$

A DIFFERENT METHOD

- Logit delay function (from IIT) – one for link delay, one for node delay

NOTES

- Used in NY, VT, NH
- Delay continuously increasing in demand
- Requires intersection / node capacities
 - Estimated at right from Synchro
- Requires lane configurations
- Fairly established parameters

Signalized Intersection 24-Hour Capacity Values*

	Minor Lane Groups	↕	↕↔	↕↔↔	↕↔↔↔	↕↔↔↔↔	↕↔↔↔↔↔	↕↔↔↔↔↔↔	↕↔↔↔↔↔↔↔	↕↔↔↔↔↔↔↔↔
Major Lane Group	↕	Single	Left Turn Bays	Left & Right Turn Bays	Left & 2 Through	Left, Right & 2 Through	2 Throughs	3 Throughs	Left & 3 Through	Left, Right and Three Through
↕	Single	23,000								
↕↔	Left Turn Bays	28,000	33,000							
↕↔↔	Left & Right Turn Bays	31,000	36,000	38,000						
↕↔↔↔	Left & 2 Through	40,000	44,000	49,000	53,000					
↕↔↔↔↔	Left, Right & 2 Throughs	46,000	50,000	54,000	58,000	62,000				
↕↔↔↔↔↔	2 Throughs	29,000	33,000	37,000	39,000	43,000	35,000			
↕↔↔↔↔↔↔	3 Throughs	32,000	36,000	40,000	46,000	46,000	38,000	40,000		
↕↔↔↔↔↔↔↔	Left & 3 Through	48,000	52,000	56,000	60,000	64,000	50,000	54,000	66,000	
↕↔↔↔↔↔↔↔↔	Left, Right and Three Through	50,000	54,000	58,000	62,000	66,000	52,000	56,000	68,000	72,000

HCM-based Volume-Dependent Turn Delays

A COMPLEX METHOD

- At each iteration, solve HCM for delay for each movement – update turn penalty to reflect this
- Can use full (critical movement) or simplified (approach) HCM method

NOTES

- Used in OH
- Delay NOT continuously increasing in demand with full HCM – mostly stable in practice, but...
- Significant runtime increase
- Limits convergence
- Requires lane configurations
- Fairly established parameters

Example 3-1: Critical Movement Analysis

Step 1:
Lane configuration, phasing, and volumes:

Steps 2, 3, and 4:
Check east-west critical lane volumes to determine the pair of consecutive movements that requires the most time:
EB LT + WB TH/RT = 120 + (360 + 110)/2 = 120 + 235 = 355
WB LT + EB TH/RT = 170 + (690 + 280)/2 = 170 + 485 = 655 ← critical

Check north-south critical lane volumes to determine the pair of consecutive movements that requires the most time:
NB LT + SB TH/RT = 80 + 400 = 480 ← critical
SB LT + NB TH/RT = 120 + 210 = 330

Identify critical movements: WB LT, EB TH/RT, NB LT, SB TH/RT
Determine the sum of critical movement volumes: CS = 655 + 480 = 1135

Steps 5, 6, and 7:
Determine the reference sum (capacity): $RS = 1530 \times PHF \times f_p = 1530 \times 1.00 \times 1.00 = 1530$
 PHF = Peak Hour Factor (assumed to be 1.00 in this example)
Adjustment factor for area type, $f_p = 0.90$ for Central Business District, 1.00 for other (assumed to be "other" in this example)
Identify cycle length: $C = 120$ s (assumed)
Determine total lost time: $L = 4$ critical phases $\times 4$ s per phase = 16 s

Calculate and assess intersection volume-to-capacity ratio:

$$X_{cm} = \frac{CS}{RS \left(1 - \frac{L}{C}\right)} = \frac{1135}{1530 \left(1 - \frac{16}{120}\right)} = 0.86 \rightarrow \text{Near capacity}$$



Summary

Methods and Issues

AT THE RISK OF REALLY OVERSIMPLIFYING...

	Stable	Runtime / Convergence	Input Data Required	Self-Fulfilling Prophecy	Calibration
Link / Approach-based					
Link/Approach-based with Simple BPR form vdf	😊	😊	😊	😊	😞
Link/Approach-based with “Double”/“Modified” BPR vdf	😊	😊	😊	😊	😞
Others (Aashtiani <i>et al.</i>)	😊	😊	😊	😊	😞
Node Delay					
IIT Logit Delay	😊	😊	😞	😊	?
TMODEL Node Delay	😊	😊	😊	😊	😞
HCM-based Volume-Dependent Turn Delays					
Full HCM method	😞	😞	😞	😞	😊
HCM approach method	😊	?	😞	😞	😊



Contacts

www.rsginc.com

VINCE BERNARDIN, PHD

Director

Vince.Bernardin@RSGinc.com

812-200-2351