


9th GEOTECHNICAL, GEOPHYSICAL AND
GEOENVIRONMENTAL TECHNOLOGY TRANSFER
(Geo³T²) CONFERENCE AND EXPO



**Modeling Traffic and Construction
Equipment Surcharges
for Geotechnical Analysis: 2-D or 3-D?**

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The 6th GEOTECHNICAL, GEOPHYSICAL AND GEOENVIRONMENTAL
TECHNOLOGY TRANSFER (Geo³T²) CONFERENCE AND EXPO

April 5 & 6, 2011
McKimmon Conference and Training Center
Raleigh, North Carolina

*Where is the Fixity below Top of
an Embedded Laterally Loaded
Pile or Drilled Shaft?*

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➤ **Make a Complicated Question Simple ...**

➤ **Make a Simple Question Complicated ...**

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Question

What is the Typical Highway Live/Traffic Surcharge Load in Geotechnical Designs?

Answer #1: 250 PSF; Typically

Answer #2: Conventionally,

- 100 psf for light traffic and parking
- 250 psf for highway traffic

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Answer #3:

According to 2014 AASHTO LRFD Article 11.19.10.2,
“Traffic loads shall be treated as uniform surcharge loads in accordance with the criteria outlined in Article 3.11.6.2. The live load surcharge pressure shall not be less than 2.0 ft of earth.”

Q: What is the unit weight of the “earth”?

FHWA NHI-10-025
MSE Walls and RSS – Vol II

E1 – 5

Example E1 – Broken Backslope & Traffic
November 2009

Traffic Load

The traffic load is on the level surface of the retained backfill. For external stability, traffic load for walls parallel to traffic have an equivalent height of soil, h_{eq} equal to 2.0 ft.

$$q = 2.0 \text{ ft} (125 \text{ pcf}) = 250 \text{ psf}$$

$$F_2 = q h K_{a0} = 250 \text{ psf} (2.0 \text{ ft}) (0.360) = 2.61 \text{ k/ft}$$

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Question

Where was the 250 psf Uniform Surcharge originated from?

Answer

- Civil Engineering Handbook (1940) refers to the Equivalent Surcharge and shows a 2 foot (scaled; not specified) fill on top of a retaining wall backfill.
- Elsewhere

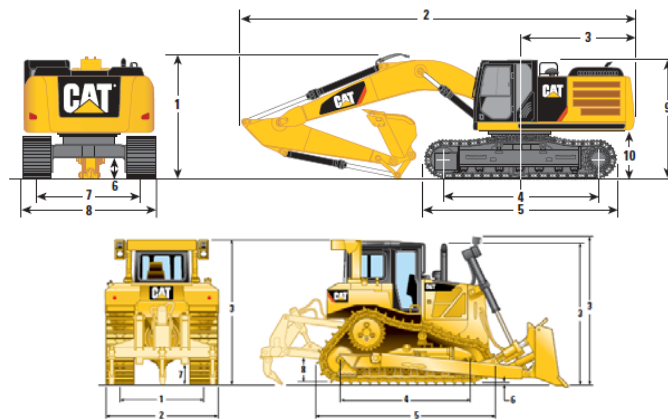
Question

Practically, in reality, there is no such a Uniform, Infinite Long Strip Load of 250 psf.



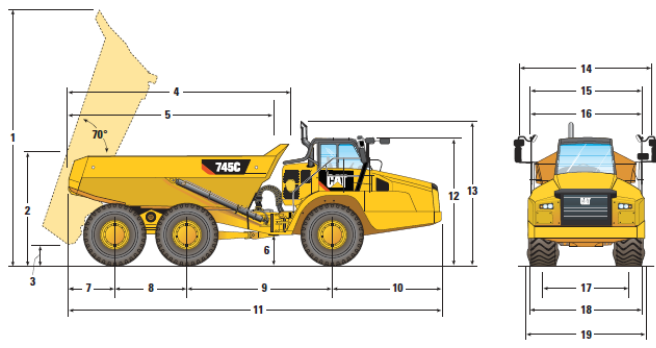
Common Cases

Conventional Construction Equipment



Common Cases

Conventional Construction Equipment



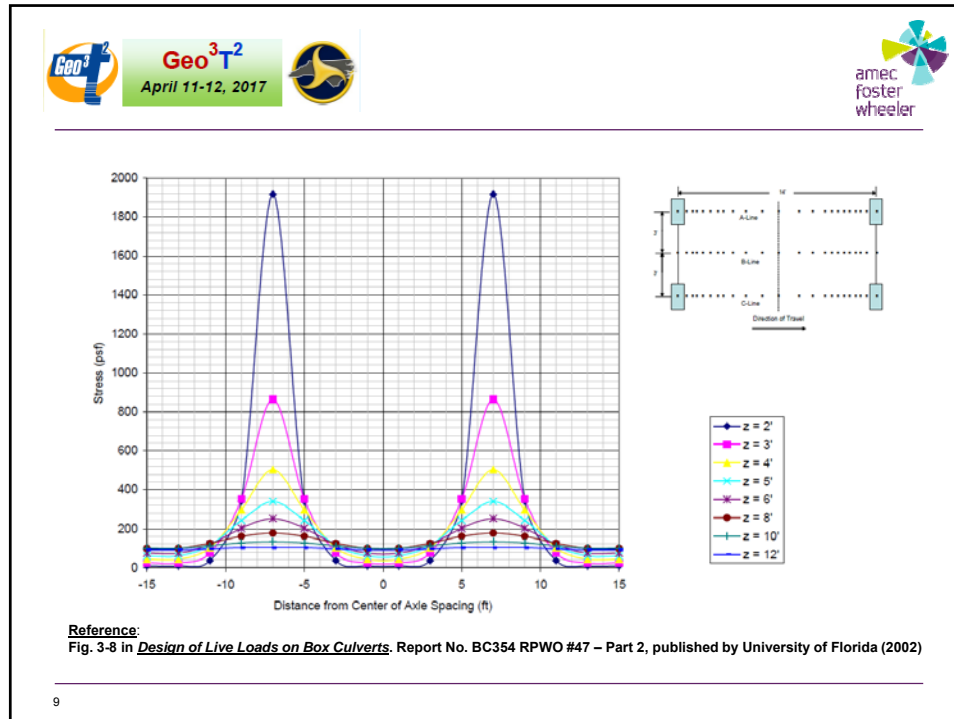
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Special Cases

Self Propelled Modular Transporter (SPTM)



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Methodology of Design Analysis


Rigorous Analytical (High-Tech) Approach

- Model Traffic Surcharge as an actual 3-Dimensional loading
- Run Roadway Embankment Global Slope Stability; using a 3-D Computer Software


Conventional Analytical Approach


- Model Traffic Surcharge as a **Uniform, Infinite Long Strip Load of 250 psf.**
- Global Slope Stability of Roadway Embankment; using a 2-D Computer Software; e.g., SLOPE/W

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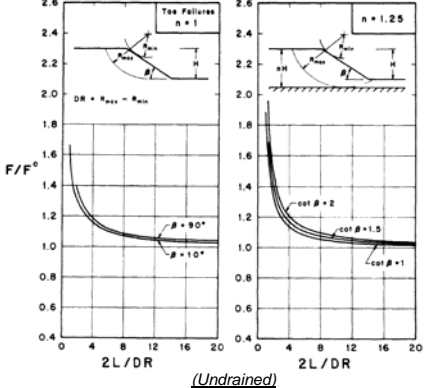
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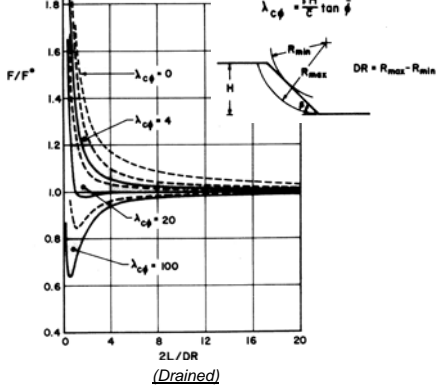


Semi- Rigorous Approach

- Correlations between results of 2-D and 3-D




(Undrained)




(Drained)


Reference: "Three-Dimensional Analysis of Slope, by Amr Sayed Azzouz (1977)

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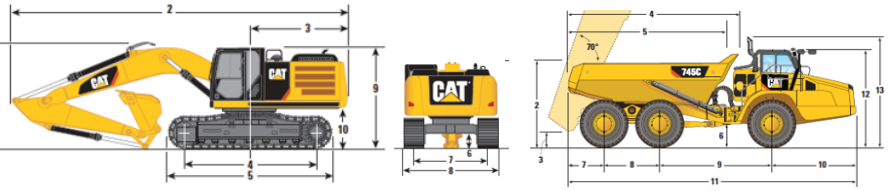
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Proposed practically "Quick" (Low-Tech) Evaluation


- Perform the Conventional 2-D Analysis, using 250 psf infinite uniform loading
- Max. operating weight (A)
- Overall width of the contact footprint (B) ... Edge-to-Edge
- Overall wheelbase distance (C) ... Front-to-Rear
- Equivalent Surcharge (DD) = (1/2)(A)/(BC) Why (1/2)?**
- IF DD ≤ 250 psf, OK**




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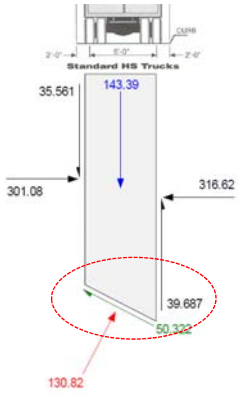


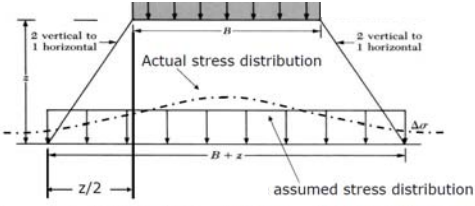
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Effects of Live / Traffic Surface Surcharge?







$\Delta\sigma_z = q_0(B \times L) / (B+z)(L+z) = P / (B+z)(L+z)$


- Mobilization of soil base resistance (Limitations of limiting equilibrium analysis)
- Local stability concerns (sloughing, ground bearing, deflections); rather than global.
- Run 3D FEM/FLAC

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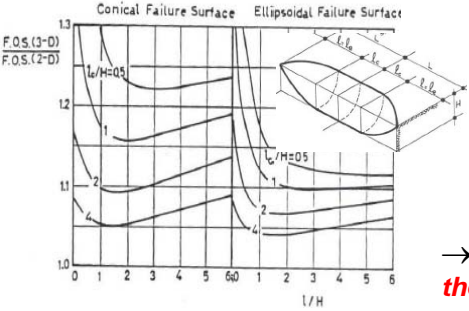


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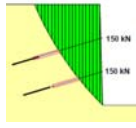




- Embankments on Homogeneous undrained clay;
 - $FS_{3D} \cong (114\%) * FS_{2D}$ (Ref.1)
 - $(l_c/H) \geq 4$; 3-D failure close to plane-strain; i.e., $FS_{3D} \cong FS_{2D}$ (Ref.2)
- 3-D effect for cohesive soils is more than for cohesionless soils (Ref.2)
 - Sand & c-φ Soils; tentatively taking $(l_c/H) = 2$ for reaching plane-strain



- Adopting similar concepts modeling the anchor load in 2-D Stability Analysis:




→ Use a factor of $(1/2)$ to distribute the 3-D Loading for a 2-D analysis


References:


1. Z. Habibnezhad (2014), "Stability Analysis of Embankments Founded on Clay - a comparison between LEM & 2D/3D FEM" A Mater Thesis; Royal Institute of Technology, Stockholm.
2. Baligh, M., and Azzouz, A. S., (1975) "End Effects on Stability of Cohesive Slopes," Journal of the Geotechnical Engineering Division, ASCE, Vol. 101, No. GT11, pp.1105-1117.

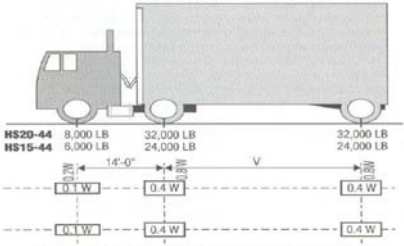
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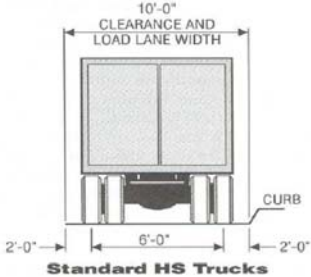






HS20-44 8,000 LB 32,000 LB 32,000 LB
HS15-44 6,000 LB 24,000 LB 24,000 LB


W = COMBINED WEIGHT ON THE FIRST TWO AXLES, WHICH IS THE SAME AS FOR THE CORRESPONDING H TRUCK.
V = VARIABLE SPACING - 14 FT TO 30 FT INCLUSIVE. SPACING TO BE USED IS THAT WHICH PRODUCES MAXIMUM STRESSES.




Standard HS Trucks

- HS20-44: Minimum AASHTO recommended design load for bridges on Interstate Highways
- Axle Loads: (1) 8-Kip & (2) 32-Kips; i.e., total 72 Kips
- Max. Overall contact projection = (6')*(14'+14')= 168 ft²
- Projection Surcharge = (72kips/ 168 ft²) = 428 psf
- **Equivalent Surcharge = (1/2)(428 psf) = 214 psf ≈ 250 psf**


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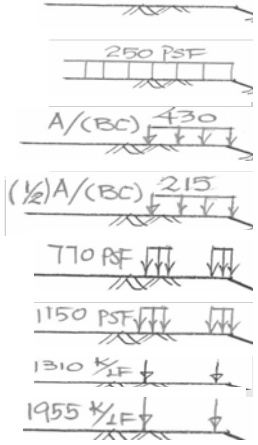
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COMPARISONS




Surcharge Conditions (Slope 20'-High, 2H:1V, $\gamma_s=120$ pcf)	FOS (Global Stability)		
	c=0 $\phi=34^\circ$	c=100 psf $\phi=30^\circ$	c=500 psf $\phi=0^\circ$
No Surcharge	1.43 (+1%)	1.82 (+4%)	1.35
Infinite Uniform	1.42	1.74	1.23
Overall Contact	1.41 (-1%)	1.69 (-3%)	1.28
(1/2) Overall Contact	1.43 (+1%)	1.75 (+1%)	1.31
Infinite Strips (Overall Contact)	1.38 (-5%)	1.70 (-2%)	1.27
Infinite Strips (Max. Contact)	1.35 (-5%)	1.64 (-6%)	1.24
Infinite Line (Overall Contact)	1.38 (-3%)	1.67 (-4%)	1.27
Infinite Line (Max. Contact)	1.35 (-5%)	1.61 (-7%)	1.27




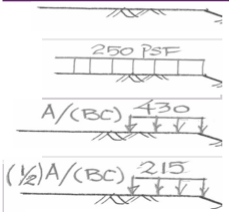
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Surcharge Conditions (Slope 20'-High, 2H:1V, $\gamma_s=120$ pcf)	FOS (Global Stability)			
	c=0 $\phi=34^\circ$	c=100 psf $\phi=30^\circ$	c=500 psf $\phi=0^\circ$	
	No Surcharge	1.43 (+1%)	1.82 (+4%)	1.35
	Infinite Uniform	1.42	1.74	1.23
	Overall Contact	1.41 (-1%)	1.69 (-3%)	1.28
	($\frac{1}{2}$)	1.43 (+1%)	1.75 (+1%)	1.31
	Overall Contact			

Conclusions


Evaluate acceptability of a specific construction equipment based on results of stability analyses with 250 psf infinite, uniform strip load -


- Reduce the Overall Contact Pressure by a factor of 2 (i.e., Considering extending the footprint 2x along the longitudinal direction of the slope), if ≤ 250 psf, OK

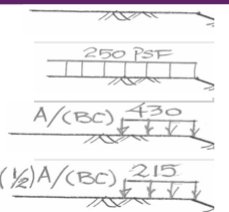
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Surcharge Conditions (Slope 20'-High, 2H:1V, $\gamma_s=120$ pcf)	FOS (Global Stability)			
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	($\frac{1}{2}$)	1.43 (+1%)	1.75 (+1%)	1.31
	Overall Contact			
	Infinite Strips	1.38 (-5%)	1.70 (-2%)	1.27

Conclusions

- In determining the equipment acceptability, the factor of 2 reduction is applicable to c=0, c- ϕ , and $\phi=0$ (conservative) Soils.

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Surcharge Conditions
(Slope 20'-High, 2H:1V, $\gamma_s=120$ pcf)

FOS (Global Stability)

	c=0 $\phi=34^\circ$	c=100 psf $\phi=30^\circ$	c=500 psf $\phi=0^\circ$
Infinite Strips (Overall Contact)	1.38 (-5%)	1.70 (-2%)	1.27
Infinite Line (Overall Contact)	1.38 (-3%)	1.67 (-4%)	1.27

Conclusions

➤ **Not to apply extending the footprint 2x along the transverse direction of the slope.**

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CONCLUSION





Modeling Hwy Traffic Surcharge

Perform a Conventional 2-D Analysis, using 250 psf infinite uniform loading to satisfy Provision requirements.

Acceptability of a Construction Equipment

- Max. operating weight (A)
- Equivalent Surcharge $(DD) = \frac{1}{2}(A)/(BC)$
- IF $DD \leq 250$ psf, OK
- IF $DD > 250$ psf, rerun 2-D analysis, assuming an infinite, uniform load of DD

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➤ *Keep a Simple Question Simple*

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Thank you!

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Q&A



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The slide features a header with the 'Geo³T²' logo and the dates 'April 11-12, 2017'. The main content area contains the text 'Q&A' in a purple font. The right side of the slide is decorated with several overlapping teal-colored geometric shapes, including triangles and circular segments, creating a dynamic, abstract design.