# ANCHORED RETAINING WALLS (5-16-17)

## General

Construct anchored retaining walls consisting of ground anchors connected to steel H-piles with a cast-in-place reinforced concrete face attached to front of piles. A ground anchor consists of a steel bar or multi-strand tendons grouted in a drilled hole inclined at an angle below horizontal. H-piles are typically drilled-in and timber lagging is typically used for temporary support of excavations during construction. Design and construct anchored retaining walls based on actual elevations and wall dimensions in accordance with the contract and accepted submittals. Use a prequalified Anchored Wall Contractor to construct anchored retaining walls. Define “anchored wall” as an anchored retaining wall and “Anchored Wall Contractor” as the Contractor installing ground anchors. Define “anchor” as a ground anchor, “pile” as a steel H-pile and “concrete facing” as a cast-in-place reinforced concrete face.

## Materials

### Refer to the *Standard Specifications*.

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| **Item** | **Section** |
| Flowable Fill, Excavatable | 1000-6 |
| Geosynthetics | 1056 |
| Joint Materials | 1028 |
| Masonry | 1040 |
| Grout | 1003 |
| Portland Cement Concrete | 1000 |
| Reinforcing Steel | 1070 |
| Select Material, Class VI | 1016 |
| Shoulder Drain Materials | 816-2 |
| Steel H-Piles | 1084-1 |
| Steel Plates | 1072-2 |
| Untreated Timber | 1082-2 |
| Welded Stud Shear Connectors | 1072-6 |

Provide Type 2 geotextile for separation geotextiles and Class VI select material (standard size No. 57 stone) for leveling pads and backfilling. Use Class A concrete for concrete facing and neat cement grout for Type 2 grout for anchors. Use Class A concrete that meets Article 450-2 of the *Standard Specifications* or Type 1 grout for drilled-in piles. Provide untreated timber with a thickness of at least 3" and a bending stress of at least 1,000 psi for timber lagging.

Provide anchors consisting of grouted steel bars or multi-strand tendons and anchorages. Use high-strength deformed steel bars that meet AASHTO M 275 or seven-wire strands that meet ASTM A886 or Article 1070-5 of the *Standard Specifications*. Splice bars in accordance with Article 1070-9 of the *Standard Specifications*. Do not splice strands.

Provide Class I corrosion protection (encapsulated tendon) for anchors in accordance with *FHWA Geotechnical Engineering Circular No. 4 “Ground Anchors and Anchored Systems”* (Publication No. FHWA-IF-99-015). Use grease and grout filled sheaths for unbonded lengths of anchors and encapsulation for bond lengths of anchors that meet Article 6.3.4 of the *AASHTO LRFD Bridge Construction Specifications*. Provide trumpets with seals that meet Articles 6.3.3 and 6.4.3 of the AASHTO LRFD specifications. Use bondbreakers, spacers and centralizers that meet Article 6.3.5 of the AASHTO LRFD specifications.

Provide anchorages consisting of bearing plates with washers and nuts for bars or wedge plates and wedges for strands. Use steel plates for bearing plates and steel washers, hex nuts, wedge plates and wedges recommended by the Anchor Manufacturer.

Provide Type 3 material certifications for anchor materials in accordance with Article 106-3 of the *Standard Specifications*. Store steel materials on blocking at least 12" above the ground and protect it at all times from damage; and when placing in the work make sure it is free from dirt, dust, loose mill scale, loose rust, paint, oil or other foreign materials. Load, transport, unload and store anchor wall materials so materials are kept clean and free of damage. Bent, damaged or defective materials will be rejected.

## Preconstruction Requirements

### Anchored Wall Surveys

The Retaining Wall Plans show a plan view, typical sections, details, notes and an elevation or profile view (wall envelope) for each anchored wall. Before beginning anchored wall design, survey existing ground elevations shown in the plans and other elevations in the vicinity of anchored wall locations as needed. For proposed slopes above or below anchored walls, survey existing ground elevations to at least 10 ft beyond slope stake points. Based on these elevations, finished grades and actual anchored wall dimensions and details, submit revised wall envelopes for acceptance. Use accepted wall envelopes for design.

### Anchored Wall Designs

For anchored wall designs, submit 11 copies of working drawings and 3 copies of design calculations and a PDF copy of each at least 30 days before the preconstruction meeting. Do not begin anchored wall construction until a design submittal is accepted.

Use a prequalified Anchored Wall Design Consultant to design anchored walls. Provide designs sealed by a Design Engineer approved as a Geotechnical Engineer (key person) for the Anchored Wall Design Consultant.

Design anchored walls in accordance with the plans and the *AASHTO LRFD Bridge Design Specifications* unless otherwise required. Design anchored walls for seismic if walls are located in seismic zone 2 based on Figure 2-1 of the *Structure Design Manual*. Design anchored walls for a maximum lateral movement of 2" or 0.5% of H, whichever is less, with H as shown in the plans.

Design anchors that meet the following unless otherwise approved:

* 1. Bond length of at least 15 ft in soil and 10 ft in rock where rock is as determined by the Engineer,
  2. Unbonded length of at least 15 ft and unbonded length behind critical failure surface of at least 5 ft or H/5, whichever is longer,
  3. Inclination of at least 12° below horizontal,
  4. Clearance between ends of tendons and drill holes of at least 6",
  5. Grout cover between encapsulation and drill hole walls of at least 1/2" and
  6. Diameter of 6" to 10".

Four inch diameter anchors may be approved for anchors in rock at the discretion of the Engineer. Where anchors go through piles, reinforce H-pile webs as shown in the plans or submit alternate reinforced web details. Do not extend anchors beyond right-of-way or easement limits. If existing or future obstructions such as foundations, guardrail, fence or handrail posts, pavements, pipes, inlets or utilities will interfere with anchors, maintain a clearance of at least 6" between obstructions and anchors.

When noted in the plans, design anchored walls for a live load (traffic) surcharge of 250 lb/sf in accordance with Article 11.5.6 of the AASHTO LRFD specifications. For steel beam guardrail with 8 ft posts above anchored walls, analyze walls for a horizontal load (PH1) of 300 lb/ft of wall in accordance with Figure 3.11.6.3-2(a) of the AASHTO LRFD specifications. For concrete barrier rail above anchored walls, analyze walls for a PH1 of 500 lb/ft of wall in accordance with Figure 3.11.6.3-2(a).

When a rock mass shear strength (Sm) is noted in the plans, analyze piles using the equation shown in Figure 3.11.5.6-2 of the AASHTO LRFD specifications to calculate the passive resistance of the rock (). Use a maximum H-pile spacing of 10 ft and drilled-in piles unless noted otherwise in the plans. Use concrete or grout for embedded portions of drilled-in piles. Install drilled-in piles by excavating holes with diameters that will result in at least 3" of clearance all around piles.

Provide temporary support of excavations for excavations more than 4 ft deep and timber lagging in accordance with the *AASHTO Guide Design Specifications for Bridge Temporary Works*. Backfill voids behind lagging and piles with No. 57 stone. Place separation geotextile between No. 57 stone and overlying fill or pavement sections except when concrete pavement, full depth asphalt or cement treated base is placed directly on stone.

Provide wall drainage systems consisting of geocomposite sheet drains, an aggregate shoulder drain and outlet components. Place sheet drains with a horizontal spacing of no more than 10 ft and center drains between adjacent piles. Attach sheet drains to front of timber lagging and connect drains to aggregate leveling pads. Locate a continuous aggregate shoulder drain along the base of concrete facing in front of piles and leveling pads. Provide aggregate shoulder drains and outlet components in accordance with Standard Drawing No. 816.02 of the *Roadway Standard Drawings*.

Use No. 57 stone for aggregate leveling pads. Use 6" thick leveling pads beneath concrete facing. Unless required otherwise in the plans, embed top of leveling pads at least 12" below bottom of walls shown in the plans.

Design concrete facing in accordance with the plans and Section 5 of the *AASHTO LRFD Bridge Design Specifications*. Provide reinforcing steel of sufficient density to satisfy Article 5.7.3.4 of the AASHTO LRFD specifications. Use concrete facing with the dimensions shown in the plans and attach facing to front of H-piles with welded stud shear connectors. When concrete barrier rail is required above anchored walls, use concrete barrier rail with moment slab as shown in the plans.

Submit working drawings and design calculations including unit grout/ground bond strengths and lock-off loads for acceptance in accordance with Article 105-2 of the *Standard Specifications*. Submit working drawings showing plan views, wall profiles with pile and anchor locations including known performance test anchor locations, typical sections and details of piles including reinforced web details, anchors, drainage, temporary support, leveling pads and concrete facing. If necessary, include details on working drawings for concrete barrier rail with moment slab and obstructions extending through walls or interfering with piles, anchors, barriers or moment slabs. Submit design calculations including lateral movement calculations for each wall section with different surcharge loads, geometry or material parameters. Include analysis of temporary conditions in design calculations. At least one analysis is required for each wall section with different anchor lengths. When designing anchored walls with computer software, a hand calculation is required for the wall section with the longest anchors.

### Anchored Wall Construction Plan

Submit 4 copies and a PDF copy of an anchored wall construction plan at least 30 days before the preconstruction meeting. Do not begin anchored wall construction until the construction plan submittal is accepted. Provide detailed project specific information in the anchored wall construction plan that includes the following:

* 1. Overall description and sequence of anchored wall construction;
  2. For drilled-in piles, installation details including drilling equipment and methods for stabilizing and filling holes and for driven piles, proposed pile driving methods and equipment in accordance with Subarticle 450-3(D)(2) of the *Standard Specifications*;
  3. List and sizes of excavation equipment, drill rigs and tools, tremies and grouting equipment;
  4. Procedures for excavations including temporary support, drilling and grouting, anchor and wall drainage system installation and facing construction;
  5. Plan and methods for anchor testing with calibration certificates dated within 90 days of the submittal date;
  6. Examples of construction records to be used in accordance with Sections 4.0(G) of this provision;
  7. Approved packaged grout or grout mix design with acceptable ranges for flow and density that meets Section 1003 of the *Standard Specifications*; and
  8. Other information shown in the plans or requested by the Engineer.

If alternate construction procedures are proposed or necessary, a revised anchored wall construction plan submittal may be required. If the work deviates from the accepted submittal without prior approval, the Engineer may suspend anchored wall construction until a revised plan is accepted.

### Preconstruction Meeting

Before starting anchored wall construction, hold a preconstruction meeting to discuss the construction, inspection and testing of the anchored walls. If this meeting occurs before all anchored wall submittals have been accepted, additional preconstruction meetings may be required before beginning construction of anchored walls without accepted submittals. The Resident or Bridge Maintenance Engineer, Bridge Construction Engineer, Geotechnical Operations Engineer, Contractor and Anchored Wall Contractor Superintendent will attend preconstruction meetings.

## Construction Methods

Control drainage during construction in the vicinity of anchored walls. Direct run off away from anchored walls and areas above and behind walls. Contain and maintain No. 57 stone and protect material from erosion.

Notify the Engineer before blasting in the vicinity of anchored walls. Perform blasting in accordance with the contract. Unless required otherwise in the plans, install foundations located behind anchored walls before beginning wall construction.

Install anchored walls in accordance with the accepted submittals and as directed. Do not excavate behind anchored walls. If overexcavation occurs, repair walls with an approved method and a revised anchored wall design or construction plan may be required.

### Piles

Install piles before excavating for anchored walls. Weld stud shear connectors to piles in accordance with Article 1072-6 of the *Standard Specifications*.

Install piles within 1" of horizontal and vertical alignment shown in the accepted submittals and with no negative batter (piles leaning forward). Minimize alignment variations between piles since variations can result in thicker concrete facing in some locations in order to provide the minimum required facing thickness elsewhere. Locate piles so the minimum required concrete facing thickness and roadway clearances are maintained for variable pile alignments.

Install piles to the required elevations in accordance with Subarticles 450-3(D) and 450-3(E) of the *Standard Specifications*. Piles may be installed with a vibratory hammer as approved by the Engineer. Do not splice piles. If necessary, cut off piles at elevations shown in the accepted submittals along a plane normal to the pile axis.

Use pile excavation to install drilled-in piles. If overexcavation occurs, fill to required elevations with No. 57 stone before setting piles. After filling holes with concrete or grout to the elevations shown in the accepted submittals, remove any fluids and fill remaining portions of holes with flowable fill. Cure concrete or grout at least 7 days before excavating.

Notify the Engineer if refusal is reached before pile excavation or driven piles attain the required penetration. When this occurs, a revised anchored wall design or construction plan submittal may be required. When a minimum pile penetration into rock is noted in the plans, rock is as determined by the Engineer.

### Excavation

Excavate in front of piles from the top down in accordance with the accepted submittals. Excavate in staged horizontal lifts with a maximum height of 5 ft. Use timber lagging or an alternate approved method for temporary support of excavations in accordance with the accepted submittals.

Install temporary support within 24 hours of excavating each lift unless otherwise approved. The installation may be delayed if it can be demonstrated that delays will not adversely affect excavation stability. If excavation faces will be exposed for more than 24 hours, use polyethylene sheets anchored at top and bottom of lifts to protect excavation faces from changes in moisture content.

If an excavation becomes unstable at any time, suspend anchored wall construction and temporarily stabilize the excavation by immediately placing an earth berm up against the unstable excavation face. When this occurs, repair walls with an approved method and a revised anchored wall design or construction plan may be required.

Remove flowable fill and material in between piles as necessary to install timber lagging. Position lagging with at least 3" of contact in the horizontal direction between the lagging and pile flanges. Do not excavate the next lift until temporary support for the current lift is accepted.

### Anchors

Fabricate and install anchors in accordance with the accepted submittals and Articles 6.4 and 6.5 of the *AASHTO LRFD Bridge Construction Specifications* except use anchor materials that meet Section 2.0 this provision instead of the AASHTO LRFD specifications and do not use heat-shrink sheaths for unbonded lengths of anchors. Measure grout temperature, density and flow during grouting with at least the same frequency grout cubes are made for compressive strength. Perform density and flow field tests in the presence of the Engineer in accordance with American National Standards Institute/American Petroleum Institute Recommended Practice 13B-1 (Section 4, Mud Balance) and ASTM C939 (Flow Cone), respectively.

Test anchors in accordance with the contract and as directed. Performance and proof tests are required in accordance with the accepted submittals, Article 6.5.5 of the AASHTO LRFD specifications and the following requirements.

* 1. Performance tests are required for at least 2 anchors or 5% of total anchors, whichever is greater, for each anchored wall instead of the requirements in Article 6.5.5.2 of the AASHTO LRFD specifications.
  2. Electrical resistance load cells are required for performance tests.
  3. An additional load increment equal to the alignment load (AL) is required between the maximum test and lock-off loads in Table 6.5.5.2-1 of the AASHTO LRFD specifications.
  4. Competent rock in Article 6.5.5.5 of the AASHTO LRFD specifications will be as determined by the Engineer.
  5. The lock-off load is as shown in the accepted submittals.

The Engineer will determine the number and locations of performance tests required. The approximate known performance test anchor locations are shown in the plans. Submit identification numbers and calibration records for load cells, jacks and pressure gauges with the anchored wall construction plan. Calibrate each jack and pressure gauge as a unit.

### Wall Drainage Systems

Install wall drainage systems as shown in the accepted submittals and in accordance with Section 816 of the *Standard Specifications*. Place geocomposite sheet drains with the geotextile side facing away from wall faces. Secure sheet drains so drains are in continuous contact with surfaces to which they are attached and allow for full flow the entire height of anchored walls. Discontinuous sheet drains are not allowed. If splices are needed, overlap sheet drains at least 12" so flow is not impeded. Connect sheet drains to aggregate leveling pads by embedding drain ends at least 4" into No. 57 stone.

### Leveling Pads and Concrete Facing

Construct aggregate leveling pads at elevations and with dimensions shown in the accepted submittals. Compact leveling pads with a vibratory compactor to the satisfaction of the Engineer.

Construct concrete facing in accordance with the accepted submittals and Section 420 of the *Standard Specifications*. Do not remove forms until concrete attains a compressive strength of at least 2,400 psi. Unless required otherwise in the plans, provide a Class 2 surface finish for concrete facing that meets Subarticle 420-17(F) of the *Standard Specifications*. Construct concrete facing joints at a spacing of 10 ft to 12 ft unless required otherwise in the plans. Make 1/2" thick expansion joints that meet Article 420-10 of the *Standard Specifications* for every third joint and 1/2" deep grooved contraction or sawed joints that meet Subarticle 825-10(B) or 825-10(E) respectively for the remaining joints. Stop reinforcing steel for concrete facing 2" on either side of expansion joints.

If a brick veneer is required, construct brick masonry in accordance with Section 830 of the *Standard Specifications*. Anchor brick veneers to anchored walls with approved brick to concrete type anchors in accordance with the manufacturer’s instructions. Space brick to concrete type anchors no more than 16" apart in the vertical direction and no more than 32" apart in the horizontal direction with each row of anchors staggered 16" from the row above and below.

Seal joints above and behind anchored walls between concrete facing and slope protection with silicone sealant.

### Backfill

Backfill voids behind lagging and piles with No. 57 stone as shown in the accepted submittals. Ensure all voids between piles, lagging and excavation faces are filled with No. 57 stone. Compact stone to the satisfaction of the Engineer. When separation geotextiles are required, overlap adjacent geotextiles at least 18" and hold separation geotextiles in place with wire staples or anchor pins as needed.

### Construction Records

Provide 2 copies of anchored wall construction records within 24 hours of completing each row of anchors. Include the following in construction records:

* + - 1. Names of Anchored Wall Contractor, Superintendent, Drill Rig Operator, Project Manager and Design Engineer;
      2. Wall description, county, Department’s contract, TIP and WBS element number;
      3. Wall station and number and lift location, dimensions, elevations and description;
      4. Anchor locations, dimensions and inclinations, tendon types, sizes and grades, corrosion protection and temporary casing information;
      5. Date and time drilling begins and ends, tendons are inserted into drill holes, grout is mixed and arrives on-site and grout placement begins and ends;
      6. Grout volume, temperature, flow and density records;
      7. Ground and surface water conditions and elevations if applicable;
      8. Weather conditions including air temperature at time of grout placement;
      9. Anchor testing records including load versus movement and time versus creep movement plots; and
      10. All other pertinent details related to anchored wall construction.

The Engineer will review the construction records to determine if anchors are acceptable. If the Engineer determines an anchor is unacceptable, revise the anchor design or installation methods. Submit a revised anchored wall design or construction plan for acceptance and provide an acceptable anchor with the revised design or installation methods. If necessary, provide additional anchors with the revised design or installation methods for the unacceptable anchors.

After completing each anchored wall or stage of a wall, provide a PDF copy of all corresponding construction records.

## Measurement and Payment

*Anchored Retaining Walls* will be measured and paid in square feet. Anchored walls will be measured as the square feet of wall face area with the pay height equal to the difference between top of wall and top of leveling pad elevations. Define “top of wall” as top of concrete facing.

The contract unit price for *Anchored Retaining Walls* will be full compensation for providing designs, submittals, labor, tools, equipment and anchored wall materials, installing piles and anchors, grouting, anchor testing, excavating, backfilling, hauling and removing excavated materials and supplying temporary support of excavations, wall drainage systems, leveling pads, concrete facing, No. 57 stone, geotextiles and any incidentals necessary to construct anchored walls. The contract unit price for *Anchored Retaining Walls* will also be full compensation for brick veneers, if required. No additional payment will be made and no extension of completion date or time will be allowed for repairing overexcavations or unstable excavations, unacceptable anchors or thicker concrete facing.

The contract unit price for *Anchored Retaining Walls* does not include the cost for ditches, fences, handrails, barrier or guardrail associated with anchored walls as these items will be paid for elsewhere in the contract.

Payment will be made under:

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| --- | --- | --- |
| **Pay Item** |  | **Pay Unit** |
| Anchored Retaining Walls | | Square Foot |



**PE SEAL NAME**

**PE #**